

						S.A.P. 062	2-593-004		
				TOTAL Q	UANTITY	PARTIC	PATING		
Mn/DOT NO.	ITEM DESCRIPTION	UNIT	NOTES	ESTIMATED QUANTITY	FINAL QUANTITY	ESTIMATED QUANTITY	FINAL QUANTITY	ESTIMA	
0011.001						0.01			
2011.601		LUMPSUM		1		0.94			
2021 501				1		0.04			
2021.301		LOWF SOW		1		0.94			
2101 502		TREE		20		20			
2101.502	CLEARING (FOOTER LOG HARVESTING)	TRFE		15		15			
2101.507	GRUBBING (ROOT WAD HARVESTING)	TREE		26		26			
								-	
2104.501	REMOVE SEWER PIPE (SANITARY)	LIN FT		365		365		_	
2104.501	REMOVE PIPE CULVERTS	LIN FT		80		80			
2104.501	REMOVE OVERHEAD CABLE	LIN FT		130		130			
2104.505	REMOVE BITUMINOUS PAVEMENT	SQ Y D		2150		2150			
2104.509	REMOVE CULVERT HEADWALL	EACH		1		1			
2104.509	REMOVE MANHOLES	EACH		4		4			
2104.509	REMOVE UTILITY POLE	EACH		1		1			
2104.521	SALVAGE CHAIN LINK FENCE	LIN FT		880		880			
2104.525	ABANDON MONITORING WELL	EACH		2		2			
2104.602	CUTOFF ABANDONED MONITORING WELL	EACH		3		3			
2104.602	ABANDON PIPE SEWER	EACH		3		3			
2105.501	COMMON EXCAVATION (P)	CUYD		43210		43210			
2105.601	DEWATERING	LUMP SUM		1		1			
2105.607	CLAY FILL (P)	CUYD		60		60			
2401.601	STRUCTURE EXCAVATION (62X04 BOX CULVERT)	LUMP SUM		1		1			
2401.601	STRUCTURE EXCAVATION (BRIDGE NO. 91071)	LUMP SUM		1		1			
2411 E01				42		42			
2411.501	STRUCTURAL CONCRETE (1X43) (P)			43		43		_	
2411.501				106		106			
2411.301				0		8			
2411.341		FOUND		4140		4140			
2411.541	REINFORCEMENT BARS EPOXY COATED (P)			12128		12128			
2411 618		SO FT		740		740			
2411 618	ARCH CONCRETE TEXTURE (ASHLAR STONE) (P)	SQ FT		740		740			
2411.618	ARCH SURFACE FINISH (MULTI-COLOR) (P)	SQ FT		740		740		_	
2412.511	10 X 8 PRECAST CONCRETE BOX CULVERT	LIN FT		334		334			
2412.512	10 X 8 PRECAST CONCRETE BOX CULVERT END SECTION	EACH		4		4			
2451.511	COARSE FILTER AGGREGATE (P)	CUYD		334		334			
			h					h	
2452.601	STEEL SHEET PILING (TEMPORARY)	LUMP SUM		1		1			
2502.502	DRAINAGE SY STEM (B910) MODIFIED	LUMP SUM		1		1			
2503.602	CONSTRUCT BULKHEAD	EACH		2		2			
2503.603	30" HDPE PIPE SEWER	LIN FT		63		63			
						-			

No.	Date	Revisions	App.	DRAWING	NAME
				TCAAP_SPINERD	BP1_SEQ.dwg
				DESIGNED BY:	RJG
				DRAWN BY:	RJO
				CHECKED BY:	WCk
				DATE:	07/24/15
1	9/30/15	ADDENDUM NO. 1		PROJECT NO.	16055300>



I HEREBY CERTIFY THAT THIS PLAN, SPECIFICATION OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MINNESOTA.



RAMSEY CC

NON-PARTICIPATING							
RAMSEY	COUNTY						
ATED QUANTITY	FINAL QUANTITY						
0.06							
0.06							
0.00							
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DUNTY	CITY PROJECT	SHEET NO.	
••••	COUNTY PROJECT	2	
STRUCTION	S.A.P.	062-593-004	
NTITIES	S.A.P.		63

						S.A.P. 062	-593-004	NON-PART	ICIPATING
				TOTAL QU	JANTITY	PARTICI	PATING	RAMSEY	COUNTY
Mn/DOT NO.	ITEM DESCRIPTION	UNIT	NOTES	ESTIMATED QUANTITY	FINAL QUANTITY	ESTIMATED QUANTITY	FINAL QUANTITY	ESTIMATED QUANTITY	FINAL QUANTITY
2504.602	INSTALL MONITORING WELL	EACH		2		2			
2511,501	RANDOM RIPRAP, CLASS II (P)	CUYD	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	23	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	23~~~~	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~		~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~
2511.501	RANDOM RIPRAP, CLASS III (P)	CUYD		393		393			
~2511.501~	RANDOM RIPRAP, CLASS IV (P)	~CUYD~		494		494			
2511.513	GRANULAR FILTER MATERIAL	TON		50		50			
2511.515	GEOTEXTILE FILTER, TY PE 5 (P)	SQ YD		2593		2593			
2511.602	24" TO 36" FIELDSTONE BOULDERS (BOULDER TOE & ESCAPE LOG)	TON		120		120			
2545.523	4" NON-METALLIC CONDUIT	LIN FT		160				160	
2557.501	INSTALL CHAIN LINK FENCE	LIN FT		880		880			
2557.501	WIRE FENCE DESIGN 48-9322	LIN FT		300		300			
2563.601	TRAFFIC CONTROL - TRAIL CLOSURE	LUMP SUM		1		1			
2572.602	NON-GAME ANIMAL HABITAT - TURTLE HIBERNACULUM	EACH		2				2	
2572.602	NON-GAME ANIMAL HABITAT - SNAKE HIBERNACULUM	EACH		2				2	
2572.602	NON-GAME ANIMAL HABITAT - FOX DEN	EACH		1				1	
2572.602	NON-GAME ANIMAL HABITAT - STANDING DEAD TREE	EACH		4				4	
2573.502	SILT FENCE, TY PE MS MACHINED SLICED	LIN FT		3150		3150			
2573.505	FLOATING SILT CURTAIN, TY PE MOVING WATER	LIN FT		225		225			
2573.533	SEDIMENT CONTROL LOG TY PE STRAW	LIN FT		3600		3600			
2573.535	STABILIZED CONSTRUCTION EXIT	LUMP SUM		1		1			
2573.550	EROSION CONTROL SUPERVISOR	LUMP SUM		1		1			
2573.602	TEMPORARY HEAD WALL	EACH		1		1			
2573.602	ROOT WADS (LOG TOE & ESCAPE LOG)	EACH		26		26			
2573.602	FOOTER LOGS (LOG TOE INSTALLATION)	EACH		15		15			
2573.602	NURSERY LOGS	EACH		13		13			
2574.525	SELECT TOPSOIL BORROW	CUYD		2330		2330			
2575.501	SEEDING	ACRE		5		5			
2575.502	SEED, MIXTURE 22-111	ROUND		40	~~~~~~	40~~~~	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~		~~~~~~
2575.502	SEED, MIXTURE 34-261	POUND		10		10			
2575.502	SEED, MIXTURE 34-262	POUND		75		75			
	EROSION CONTROL BLANKET, CATEGORY.3	SOXD	h	3100					
2575.523	ROLANKA BIOD-MAT 40 OR APPROVED EQUAL	SQ Y D		3180		3180			
0575 500		POLIND		14600		14600			

No.	Date	Revisions	App.	DRAWING	NAME
`				TCAAP_SPINERD	BP1_SEQ.dwg
				DESIGNED BY:	RJG
				DRAWN BY:	RJG
				CHECKED BY:	WCK
				DATE:	07/24/15
1	9/30/15	ADDENDUM NO. 1		PROJECT NO.	16055300X



– 2550 UNIVERSITY AVENUE WEST, SUITE 238N, ST, PAUL, MN 55114 PHONE: 651–645–4197 WWW.KIMLEY-HORN.COM

I HEREBY CERTIFY THAT THIS PLAN, SPECIFICATION OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MINNESOTA. HOTMBY, June Dinnesota THOMAS J. MICOLN DATE: 07/24/15 MN LIC. NO. 21433



BID PACKAGE #1 RICE CREEK REMEANDER CONS STATEMENT OF ESTIMATED QUAI

DUNTY	CITY PROJECT	SHEET NO.		
	COUNTY PROJECT	] $3$		
STRUCTION	S.A.P.	062-593-004		
NTITIES	S.A.P.		63	

- UNSUITABLE MATERIALS ARE DEFINED AS ANY SILT, TOPSOIL, PEAT, MUCK, OTHER ORGANIC MATERIALS AND DEBRIS. 1 UNSUITABLE MATERIALS MAY NOT BE PLACED WITHIN THE GRADING LIMITS, ALL EARTH MATERIALS NOT DESIGNATED FOR SALVAGE, AND ALL DEBRIS, SHALL BECOME PROPERTY OF THE CONTRACTOR AND SHALL BE DISPOSED OF OFFSITE
- THE TOP OF BACKSLOPES AND THE TOE OF FILL SLOPES SHALL BE ROUNDED TO NATURALIZE THE CONSTRUCTION EVEN 2. THOUGH THE CROSS SECTIONS DO NOT SHOW ANY SUCH ROUNDING.
- ANY DEBRIS WHICH MAY BE ENCOUNTERED DURING GRADING SHALL BE DISPOSED OF BY THE CONTRACTOR OFF SITE IN A 3. SUITABLE DISPOSAL AREA AS APPROVED BY THE ENGINEER.
- EMBANKMENT CONSTRUCTION SHALL FOLLOW THE GUIDELINES OF SPECIFICATION 2105. BACKFILL MATERIAL FOR EMBANKMENT CONSTRUCTION SHALL BE SELECT GRADING MATERIAL UNLESS SPECIFIED OTHERWISE.
- THE CONTRACTOR IS HEREBY REMINDED OF THEIR RESPONSIBILITY UNDER STATE LAW TO CONTACT ALL UTILITIES THAT MAY 5. HAVE FACILITIES IN THE AREA. CONTACT MUST BE MADE THROUGH "GOPHER STATE ONE CALL" 1-800-252-1166.
- NO EXTRA PAYMENT WILL BE MADE FOR TEMPORARY STOCKPILING OF EXCAVATION. EMBANKMENT AND/OR BORROW MATERIAL. 6.
- STOCKPILE LOCATION WILL BE FIELD DIRECTED BY ENGINEER, AND WILL BE NO FURTHER THAN 0.5 MILES AWAY FROM THE 7. EXCAVATION AREA. STOCKPILE LOCATION WILL BE SOUTH OF THE CONSTRUCTION LIMITS AND SHALL BE LOCATED OUTSIDE OF ANY DELINEATED WETLANDS AND MN DOT RIGHT OF WAY AND SHALL NOT INTERFERE WITH ANY MONITORING WELLS. CONTRACTOR RESPONSIBLE FOR STABILIZING THE STOCKPILE WITH SEED AND HYDRAULIC MULCH AND PROVIDING SILT FENCE PERIMETER CONTROL AS REQUIRED IN THE PROJECT SWPPP.
- CONTRACTOR RESPONSIBLE TO DEVELOP AND SUBMIT DEWATERING PLAN TO ENGINEER FOR REVIEW. DEWATERING PLAN SHALL 8. BE COMPLETE WITH REQUIRED PERMITS PRIOR TO STARTING ANY CONSTRUCTION ACTIVITIES. CONTRACTOR RESPONSIBLE FOR DEVELOPING THE PLAN, SECURING ANY PERMITS, AND COMPLYING WITH THE PERMITS. DEWATERING DISCHARGE LOCATION WILL BE FIELD DIRECTED BY ENGINEER, AND WILL BE SOUTH OF THE CONSTRUCTION LIMITS AND SHALL BE LOCATED OUTSIDE OF ANY DELINFATED WETLANDS AND MN DOT RIGHT OF WAY AND SHALL NOT INTERFERE WITH ANY MONITORING WELLS.
- 9. DURING SPRING OF 2016, THE CREEK REMEANDER BETWEEN STATIONS 3+00 TO 8+00 WILL BE OCCUPIED BY BRIDGE CONSTRUCTION CONTRACTOR. ALL ACTIVITIES IN THIS AREA SHALL BE COORDINATED WITH BRIDGE CONTRACTOR.
- TREE HARVEST LOCATION FOR ROOT WADS AND FOOTER LOGS WILL BE FIELD DIRECTED AND WILL BE SOUTH OF THE 10. CONSTRUCTION LIMITS, NO FURTHER THAN 1 MILE AWAY FROM THE INSTALLATION AREA
- 11. SITE ACCESS SHALL BE FROM I-35W. THE COUNTY H BRIDGE OVER I-35W WILL BE REMOVED SPRING 2016, BUT THE ON RAMP FROM COUNTY H TO I-35W WILL REMAIN AND SITE ACCESS WILL REQUIRE STOPPING ON I-35W AND BACKING UP THE UN-USED ON RAMP
- CONTRACTOR SHALL BE RESPONSIBLE FOR TAKING AS-BUILT CROSS SECTION SURVEYS AND SUBMITTING DATA TO THE 12. ENGINEER PRIOR TO FINAL PAYMENT.
- ALL CONSTRUCTION STAKING TO BE PERFORMED BY THE CONTRACTOR. ENGINEER SHALL ESTABLISH SURVEY CONTROL POINTS 13. AT LOCATIONS AGREED UPON PRIOR TO THE START OF WORK. CONTRACTOR SHALL TAKE CARE TO PROTECT SURVEY CONTROL POINTS. ANY RE-ESTABLISHMENT OF SURVEY CONTROL REQUIRED SHALL BE AT THE CONTRACTORS EXPENSE.

## EARTHWORK SUMMARY

37,000	CU YD (P)
4,400	CU YD (P)
30	CU YD (P)
1,780	CU YD (P)
43,210	CU YD (P)
240	CU YD (EV)
1,860	CU YD (EV)
2,100	CU YD (EV)
0	CU YD (P)
2,650	CU YD (P)
1,590	CU YD (P)
810	CU YD (P)
5,050	CU YD (P)
1,000	CU YD (EV)
2,560	CU YD (LV)
560	CU YD (LV)
150	CU YD (LV)
60	CU YD (LV)
	37,000 4,400 30 1,780 43,210 240 1,860 2,100 2,650 1,590 810 5,050 

#### EARTHWORK SUMMARY NOTES

- 1 COMMON EXCAVATION INCLUDES TOPSOIL STRIPPING QUANTITY
- 3

## MN/DOT FIBER OPTIC NOTES

- 1.
- INSTALLED UNDER RICE CREEK IN THE PROJECT AREA. 2
  - BOTH SIDES OF THE CREEK FOR BOTH FO CABLES. ENGINEER.
- TO THE COMMENCEMENT OF ANY EXCAVATION IN THE AREA.
- 6

The following s	standard plates, approved
	STAN
PLATE ND.	
8000I	STANDARD BARRICADES
8337C	TEMPORARY PORTABLE PRE
9322K	CHAIN LINK FENCE

					$\label{eq:resonance} \begin{array}{ c c c } \hline \\ \hline $	NOTES INFORME ACTION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMATION INFORMAT	<complex-block></complex-block>	PULL END OF COMER 12 2000 BUT PULL END OF COMER 12 2000 BUT PULL END OF COMER 10 2000 BUT	SEC NOTE CONTRACTORY SEC NOTE TO CONTRACT	The second secon
o.	Date	Revisions	App.	DRAWING NAME			I HEREBY CERTIFY THAT THIS PLAN, SPECIFICATION OR REPORT WAS PREPARED BY ME OR UNDER MY			$\overline{co}$
				04 - CNSTRCTN NTS.dwg		EINCK	DIRECT SUPERVISION AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE			CU
				DRAWN BY: LAM		SOCIATES	LAWS OF THE STATE OF MINNESOTA.		TCAAP	
				CHECKED BY: MJS	Responsive partner, Excer	tional outcomes	20Mat-		BID PACKAG	iΕ #1
				DATE: 07/14/2015	1800 PIONEER CREEK CENTER	PHONE: 763-479-4200	ED MATTHIESEN	RICE	CREEK REMEANDER	
				PROJECT NO. 160553004	MAPLE PLAIN, MN 55359	FAX: 763-479-4242	DATE: 7/24/15 MN LIC. NO. 16800		CONSTRUCTION N	NUIES
					-					

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2. ALL EXCESS COMMON EXCAVATION MATERIAL SHALL BE STOCKPILED ONSITE. STOCKPILE SOIL STRIPPING MATERIAL SEPARATELY.

REFER TO PLAN SHEETS 31 - 63 FOR DETERMINATION OF STRUCTURAL EXCAVATION LIMITS. THIS EXCAVATION IS PAID FOR UNDER BID ITEM NO. 2401.601. BID ITEM INCLUDES TRENCH EXCAVATION AND BACKFILL OF SUITABLE MATERIAL. QUANTITY SHOWN IS EXCESS MATERIAL TO BE STOCKPILED AND IS FOR INFORMATION ONLY.

4. TOPSOIL STRIPPING QUANTITY INCLUDED IN COMMON EXCAVATION AND ESTIMATED BASED ON SOIL BORINGS.

TWO MN/DOT FIBER OPTIC CABLES ARE LOCATED EAST OF I-35W WITHIN MN/DOT RIGHT-OF-WAY. BOTH FO CABLES WERE AFTER A GOPHER ONE-CALL LOCATE IS MADE, THE CONTRACTOR SHALL VACUUM EXCAVATE FOR DEPTH AND LOCATION ON

THE CONTRACTOR SHALL FIELD SURVEY THE LOCATION AND DEPTH OF BOTH FO CABLES AND PROVED DATA TO THE

THE ENGINEER SHALL CONFIRM FO CABLE DEPTH AND DETERMINE IF THERE IS A CONFLICT EXCAVATION OR SUBCUT PRIOR THE CONTRACTOR WILL BE RESPONSIBLE FOR ANY DAMAGES TO THESE KNOWN FACILITIES.

LOCATING OF THESE TWO FO CABLES SHALL BE INCIDENTAL TO THE EXCAVATION WORK.

by the Federal Highway Administration, shall apply on this project. NDARD PLATES DESCRIPTION CAST CONCRETE BARRIER





DATE:

PROJECT NO.

160553004

UNTY	CITY PROJECT	SHEET NO.	
	COUNTY PROJECT		5
RUCTION	S.A.P.	062-593-004	
	S.A.P.		63







## CONSTRUCTION SEQUENCING:

#### 1. FALL 2015

- ESTABLISH SITE EROSION CONTROL MEASURES PRIOR TO LAND DISTURBANCE. 1.1.
- 1.2. SALVAGE CHAIN LINK, PERIMETER FENCE AND REINSTALL AS SHOWN. USE SALVAGED FENCE UNTIL OUT, REMAINING CHAIN LINK FENCE TO BE NEW AS REQUIRED TO MAINTAIN PERIMETER CONTROL OF THE PROPERTY.
- COMPLETE CLEARING AND GRUBBING AS SHOWN ON THE PLANS, HARVEST ROOT WAD, FOOTER LOG AND DEAD TREE MATERIALS FROM TREES MARKED FOR REMOVAL OR OTHER TREES ONSITE AS DIRECTED. 1.3.
- 1.4. COMPLETE REMOVALS AS SHOWN ON THE PLANS.
- STRIP TOPSOIL FROM THE PEND. STRIP TOPSOIL FROM THE PROJECT AREA AND STOCKPILE FOR REUSE. STOCKPILE LOCATION TO BE DETERMINED. COMPLETE EXCAVATION OF REMEANDER AS SHOWN ON THE PLANS. EXCAVATION SPOIL TO BE STOCKPILED FOR REUSE. 1.5.
- 1.6. STOCKPILE LOCATION TO BE DETERMINED.
- 1.7. INSTALL BRIDGE NO. 91071 EXTENSION AS SHOWN ON THE PLANS. INSTALL I-35W RAMP CULVERT AS SHOW ON THE PLANS, INCLUDING TEMPORARY WEIR ON UPSTREAM END OF THE CULVERTS AS SHOW ON THE PLANS. CONSTRUCTION SHALL BE COMPLETED TO PREVENT RICE CREEK FROM 1.8. BACKWATERING INTO THE NEW REMEANDER.
- 1.9. INSTALL REMEANDER STABILIZATION PRACTICES.
- SPREAD TOPSOIL FROM STOCKPILE OVER THE PROJECT AREA AS SHOWN ON THE PLANS, IMPORTING ADDITIONAL 1.10.
- TOPSOIL AS REQUIRED. TOPSOIL SHALL BE A MINIMUM OF 4" THICK. REVEGETATE PROJECT AND DISTURBED AREAS AND STABILIZE WITH EROSION CONTROL MEASURES AS SHOWN ON THE 1.11. PLANS. LIVE SEED INSTALLATION SHALL OCCUR OCTOBER 15 TO FIRST FROST AND SHOULD BE COMPLETE AS DETAILED IN THE SWPPP.

SPRING 2016 2.

- 2.1. MAINTAIN INSTALLED VEGETATION, RESEEDING ALL AREAS THAT DO NOT SHOW GERMINATION BY MAY 31, 2016. SALVAGE CHAIN LINK, PERIMETER FENCE AND REINSTALL AS SHOWN. USE SALVAGED FENCE UNTIL OUT, REMAINING CHAIN LINK FENCE TO BE NEW AS REQUIRED TO MAINTAIN PERIMETER CONTROL OF THE PROPERTY. 2.2.
- DIVERT RICE CREEK INTO THE NEW REMEANDER JUNE 16, 2016 THROUGH JULY 15, 2016 2.3.
- 2.3.1. MAINTAIN TEMPORARY WEIR UPSTREAM OF I-35W CULVERT EXTENSION UNTIL THE END OF DIVERSION TO CAPTURE ANY SEDIMENT FLUSH FROM THE DIVERSION.
- 2.3.2. COMPLETE THE DIVERSION PROCESS AS QUICKLY AS POSSIBLE BY STAGING EQUIPMENT AND MATERIALS AS REQUIRED TO MINIMIZE TIME AND EXPOSURE OF DISTURBED SOILS TO FLOWING WATER.
- 2.3.3. EXCAVATE THE BERM AT UPSTREAM END OF THE REMEANDER SEPARATING THE REMEANDER FROM RICE CREEK. SPOILS FROM THE EXCAVATION TO BE PLACED IN RICE CREEK AS SHOWN ON THE FINAL GRADING PLAN.
- BACKFILL ADDITIONAL COMMON BORROW IN RICE CREEK AS SHOWN ON THE FINAL GRADING PLAN TO COMPLETE 2.3.4. THE DIVERSION AND PLUG FLOW INTO THE ABANDONED RICE CREEK CHANNEL.
- INSTALL STABILIZATION PRACTICES AS SHOWN ON FINAL STABILIZATION PLAN. MODIFY TEMPORARY WEIR UPSTREAM OF I-35W CULVERT EXTENSION 2.3.5.
- 24
- MODIFICATION OF THE TEMPORARY HEADWALL (CONSTRUCTED OF JERSEY BARRIERS AND SAND BAGS SEE 2.4.1.
- DETAIL 6 ON SHEET 7) SHALL BE COMPLETED DURING A DRY, NON-PRECIPITATION PERIOD. REMOVE THE CENTER JERSEY BARRIER FROM THE TEMPORARY HEADWALL UPSTREAM OF THE I-35W RAMP 2.4.2. CULVERT. ALLOW FLOW AND WEIGHT OF THE WATER TO COLLAPSE THE SANDBAGS IN THE VOID CREATED BY THE REMOVED JERSEY BARRIER. THE INTENT IS TO CREATE A LOWER ELEVATION IN THE TEMPORARY HEADWALL TO CONTROL FLOW. ONE OR MORE ADDITIONAL JERSEY BARRIERS MAY NEED TO BE REMOVED TO BALANCE FLOW AND WILL BE DIRECTED BY THE ENGINEER IN THE FIELD.
- 2.4.3. AFTER 24 TO 48 HOURS, REMOVE SEDIMENT ACCUMULATION UPSTREAM OF THE TEMPORARY WEIR AND PLACE IN STOCKPILE LOCATION.
- REMOVE ALL REMAINING TEMPORARY WEIR COMPONENTS. 2.4.4.
- 2.4.5. STABILIZE ALL DISTURBED AREAS TO MATCH STABILIZATION PLANS.

No. Date Revisions	App.	DRAWING 1 09 - CNSTRCTN 5	NAME SQNCNG.dwg	W	<b>ENCK</b>	I HEREBY CERTIFY THAT THIS PLAN, SPECIFICATION OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY
		DESIGNED BY:	EAM		SSOCIATES	LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MINNESOTA.
		DRAWN BY:	LNJ			Salar
		CHECKED BY:	MJS	Responsive partner. Exce	eptional outcomes.	( d Mat
		DATE:	07/14/2015	1800 PIONEER CREEK CENTER	PHONE: 763-479-4200	ED MATTHIESEN
		PROJECT NO.	160553004	MAPLE PLAIN, MN 55359	FAX: 763-479-4242	DATE: 7/24/15 MN LIC. NO. 16800



BID PACKAGE #1 RICE CREEK REMEANDER CONSTR CONSTRUCTION SEQUENCING

UNTY	CITY PROJECT		SHEET NO.
	COUNTY PROJECT	9	
UCTION	S.A.P.	062-593-004	
	S.A.P.		63



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		S.	.A.P.	

S.A.P.

062-593-004 63



ALIGNMENT TABULATION											
		CURVE DATA			COORD	INATES	A 71841 IT11				
DELTA	DEGREE	RADIUS	TANGENT	LENGTH	x	Y	AZIWUTH				
					551226.432	211187.776	262° 13' 8"				
54.6967	43.2733	132.405	68.480	126.40	551158.589	211178.457	PI				
					551126.987	211117.704	207° 28' 55"				
52.1639	81.8511	70.000	34.265	63.73	551111.175	211087.305	PI				
					551125.483	211056.170	155° 19' 5"				
11.7740	19.5020	293.794	30.293	60.37	551138.354	211028.747	PI				
					551156.549	211004.527	143° 5' 2"				
24.1518	19.5020	293.794	62.855	123.84	551194.303	210954.274	PI				
					551249.313	210923.867	118° 55' 55"				
96.6009	66.9680	85.557	96.029	144.25	551332.481	210875.860	PI				
				95.13	551275.232	210798.762	216° 35' 43"				
					551217.307	210723.301	220° 20' 53"				
84.3250	65.6284	87.303	79.058	128.49	551166.123	210663.049	PI				
					551101.105	210708.025	304° 40' 23"				
86.7849	57.3036	99.986	94.527	151.45	551024.124	210762.882	PI				
					550965.035	210689.098	218° 41' 20"				
126.2499	76.3944	75.000	147.993	165.26	550872.511	210573.594	PI				
					550834.074	210716.508	342° 23' 15"				
85.5631	125.9800	45.480	42.088	67.92	550821.339	210756.623					
					550780.339	210756.623	256° 49' 28"				
6.9659	29.7917	192.321	11.705	23.38	550768.741	210745.623					
				197.35	550757.036	210745.595	267° 55' 33"				
					550559.811	210738.452					

CITY PROJECT		SHEET NO.
COUNTY PROJECT		11
S.A.P.	062-593-004	
S.A.P.		63



#### Construction Dates: September 2015 - July 2016

#### Party Responsible for Long Term Operation and Maintenance of the Site -OWNER

Ramsey County 1425 Paul Kirkwold Drive Arden Hills, MN 55112

Beth Engum, Project Engineer (651) 266-7115 Beth.Engum@co.ramsey.mn.us

Party Responsible for Implementation of the SWPPP - CONTRACTOR (TBD)

#### Surface Area Tabulation

Total Site Area	7.2 acres
Existing Impervious Area	0.0 acres
Proposed Impervious Area	0.0 acres
Net Impervious Area Increase	0.0 acres



#### CERTIFICATION

#### In accordance with Part III.A.2 of the General Permit Authorization to Discharge Stormwater Associated with Construction Activity under the NPDES, the preparer of this document was trained under the University of Minnesota Erosion and Sediment Control Certification Program. Mr. Louis Sigtermans' certification in Design of SWPPP is valid through May 31, 2017.

SSURGO Soils

UNIVERSITY OF MINNESOTA

#### Louis H Sigtermans Saint Paul, MN

Figure 1

Design of Construction SWPPP (May 31, 2017)

V\13	No.	Date	Revisions	App.		NAME
°∕c						P.awg
R					DESIGNED BT:	EAM
2					DRAWN BY:	LNJ
2					CHECKED BY:	MJS
382					DATE:	07/14/2015
1:1	1	9/30/15	ADDENDUM NO. 1		PROJECT NO.	160553004

#### **EROSION AND SEDIMENT CONTROL PRACTICES**

All exposed soil areas must have temporary erosion protection (erosion control blanket, hydromulch, seed) as soon as possible or within 7 days after the construction activity in that portion of the site has temporarily or permanently ceased.

CONTRACTOR shall implement appropriate construction phasing, vegetative buffer strips, horizontal slope grading, and other construction practices that minimize erosion when practical.

The normal wetted perimeter of any temporary or permanent drainage ditch that drains water from a construction site, or diverts water around a site, must be stabilized within 200 lineal feet from the property edge, or from the point of discharge to any surface water. Stabilization must be completed within 24 hours of connecting to a surface water. Pipe outlets must be provided with temporary or permanent energy dissipation within Hazardous Materials 24 hours of connection to a surface water.

The following measures will be taken as sediment control practices in order to minimize sediments from entering surface waters

- 1. Installation of perimeter silt fence/bioroll as shown on sheet 7 prior to site disturbance. Perimeter sediment control
- structures shall be located as shown on sheets 14 and 15. 2. Rows of straw wattle or bioroll placed parallel to the contour
- lines every 25 feet for slope stabilization control. 3. Construction phasing as shown in interim stabilization plan
- on sheet 9 and as described below:
- a. Installation of temporary head wall up to normal water level at box culvert.
- b. Stabilization of interim grading and establishment of vegetative cover within the meander channel in Winter/Spring 2016.
- c. Removal of downstream meander berm to allow creek flow to enter meander channel.
- d. Allow 24 hours for sediment to settle and remove sediment trapped at the temporary head wall.
- e. Removal of temporary head wall to complete reroute. 4. Street sweeping of tracked sediment when necessary.

#### Temporary Sediment Basin

Stormwater runoff on site will flow into the meander channel. During the interim grading phase the meander channel, head wall, and meander berm will form a basin with a live storage volume greater than the required minimum of 3,600 cubic feet per acre drained.

#### **Final Stabilization**

All areas disturbed by construction will receive seed according to the plans and specifications and within the specified vegetative time schedule. The following additional stabilization measures shall be taken:

1. Erosion control blanket installed 1 roll width above normal water level along entire length of the creek on both sides.

2. Remainder to be hydromulched with bonded fiber matrix.

Final stabilization will occur when the site has a uniform vegetative cover with a density of 70% over the entire disturbed area. All temporary synthetic erosion prevention and sediment control BMPs (such as silt fence) must be removed as part of the site final stabilization. All sediment must be cleaned out of conveyances and temporary sedimentation basins if applicable

Notice of Termination (NOT) must be submitted within 30 days of final stabilization. Before Termination, revegetation establishment and coverage must meet the permit requirements.



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#### Dewatering

Turbid or sediment-laden water must be discharged to an onsite sedimentation basin, or treated with the appropriate BMPs, such that discharge does not adversely affect the receiving water. Ensure that discharge points are adequately protected from erosion and scour. CONTRACTOR responsible to develop and submit dewatering plan to engineer, secure any required permits, and comply with permits.

#### Pollution Prevention Measures

#### Solid Waste

Solid waste, including but not limited to, collected asphalt and concrete millings, floating debris, paper, plastic, fabric, construction and demolition debris and other waste must be disposed of properly and must comply with MPCA disposal requirements.

Hazardous materials, including but not limited to oil, gasoline, paint and any hazardous substance must be properly stored including secondary containments, to prevent spills, leaks or other discharge. Restricted access to storage areas must be provided to prevent vandalism. Storage and disposal of hazardous waste must be in compliance with MCPA regulations.

#### Washing of Construction Vehicles

External washing of trucks and other construction vehicles must be limited to a defined area of the site. Runoff must be contained and waste properly disposed of. No engine degreasing is allowed on site.

#### Concrete Washout Area

The contractor shall use means to washout concrete offsite.

#### Amendments

Amend the SWPPP as necessary to address any changes in design, construction, operation, maintenance, weather or seasonal conditions that have a significant effect on discharge of pollutants to surface or underground waters; or to address concerns identified during inspections or investigations by OWNER, USEPA or MPCA.

#### Record Retention

The SWPPP, all changes to it, and inspection and maintenance records must be kept on-site during construction. The OWNER must retain a copy of the SWPPP along with the following records for three (3) years after submittal of the Notice of Termination. 1. Any other permits required for the project;

2. Records of all inspection and maintenance conducted during construction.

3. All permanent operations and maintenance agreements that have been implemented, including all right of way, contract, covenants and other binding requirements regarding perpetual maintenance; and

4. All required calculations for design of the temporary and permanent stormwater management systems

The inspection log will be completed by the CONTRACTOR for the construction site. Inspections at the site will be completed as follows: Once every seven (7) days during active construction and, Within 24 hours after a rainfall event greater than 0.5 inches in 24 hours.

The individual performing inspections must be trained as required by part IV.E of the Permit. CONTRACTOR to provide OWNER with proof of training. Inspections must include stabilized areas, erosion prevention and sediment control BMPs, and infiltration areas. Corrective actions must be identified and date of correction must be noted as identified in Section IV.E. of the Permit.



RICE CREEK REMEANDER CONSTR STORMWATER POLLUTION PREVENT

Inspections

OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE

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16800

7/24/15 MN LIC. NO.

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AWS OF THE STATE OF MINNESOTA







IMPAIRED WATERS, SPECIAL WATERS, AND WETLANDS This Project is not located within 1 mile of a special water.

This Project is located within 1 mile of an impaired waters (Figure 2 this sheet): Rice Creek impaired for F-IBI, M-IBI,

Because of the proximity of the project to an impaired water during construction: 1. All exposed soil areas must be stabilized as soon as possible to limit soil erosion but in no case later than 7 days after the construction activity in that portion of the site has temporarily or permanently ceased.

2. Temporary sediment basins are required to handle runoff for common drainage locations that serve an area with 5 or more acres disturbed at one time.

This Project will result in temporary impacts to wetlands when diverting the creek into the new channel. Adverse impacts will be mitigated onsite and a wetland permit has been obtained.

#### **EROSION CONTROL ESTIMATED QUANTITIES**

Material	Quantity
MnDOT Seed Mix 34-261	31.5 lb/acre x 0.25 acres = 7.9 LBS
MnDOT Seed Mix 34-262	14.5 lb/acre x 5 acres = 73 LBS
Erosion Control Blanket	3,100 SY
Bonded Fiber Matrix	3,000 lb/acre x 4.5 acres = 13,500 LBS
Hydraulic Mulch	750 lbs/acre x 1.3 acres = 950 LBS
Floating Silt Curtain	225 LF
Sediment Control Log, Type Straw	3,600 LF
Silt Fence	3,150 LF

UNTY	CITY PROJECT		SHEET NO.
•••••	COUNTY PROJECT		13
UCTION	S.A.P.	062-593-004	
ON PLAN	S.A.P.		63









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		0	15 30 60
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91.47	85.68 93.24 85.85 93.73 93.73		
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DUNTY	CITY PROJECT		17
	COUNTY PROJE	CT	' /
	S.A.P.	062-593-004	
	S.A.P.		



	ALIGNMENT DATA SPINE RD WATERMAIN													
SEGMENT NUMBER	BEGINNING STATION	ENDING STATION	PI STATION	NOTES	DELTA	DEGREE	RADIUS (FT)	TANGENT (FT)	LENGTH (FT)	BEGINNING COORDINATES NORTHING	BEGINNING COORDINATES EASTING	ENDING COORDINATES NORTHING	ENDING COORDINATES EASTING	AZIMUTH
L 100	272+31.87	274+64.76							232.89	210525.77	551501.68	210739.29	551408.68	336 <b>°</b> 27'43"
L 101	274+64.76	277+07.64							242.88	210739.29	551408.68	210888.97	551217.41	308'02'47"
L 102	277+07.64	279+60.67							253.03	210888.97	551217.41	210981.35	550981.84	291 <b>°</b> 24'47"

	ALIGNMENT DATA TCAAP PSAN 12 FORCE														
SEG NUN	MENT MBER	BEGINNING STATION	ENDING STATION	PI STATION	NOTES	DELTA	DEGREE	RADIUS (FT)	TANGENT (FT)	LENGTH (FT)	BEGINNING COORDINATES NORTHING	BEGINNING COORDINATES EASTING	ENDING COORDINATES NORTHING	ENDING COORDINATES EASTING	AZIMUTH
с	10	0+24.52	2+05.65	1+15.25		8 <b>•</b> 29'35"	4*41'19.28"	1222.00	90.74	181.14	210480.65	550743.09	210659.66	550769.72	12 <b>°</b> 42'39" 4 <b>°</b> 13'04"
L	110	2+05.65	3+84.66							179.00	210659.66	550769.72	210838.17	550782.89	4'13'09"



DATE: 07/24/15 MN LIC. NO. 21433

PROJECT NO.

16055300>

SIZE OF CARRIER	MJ BELL O.D.	MIN. CASING SIZE	MIN. CASING WALL THICKNESS
6	11.12	18	0.250
8	13.37	20	0.281
12	17.88	24	0.312
16	22.50	30	0.406
20	27.00	36	0.469
24	31.50	42	0.500

NINTY	CITY PROJECT		SHEET NO.
	COUNTY PROJECT		19
STRUCTION	S.A.P.	062-593-004	
l	S.A.P.		63











UNTY	CITY PROJECT		SHEET NO.
0	COUNTY PROJECT		24
UCTION	S.A.P.	062-593-004	
2	S.A.P.		30





RICE CREEK REMEANDER CONSTRU FINAL CREEK REMEANDER CROSS SECTIONS

UNTY	CITY PROJECT		SHEET NO.
0	COUNTY PROJECT		25
JCTION	S.A.P.	062-593-004	
	S.A.P.		30





DATE: 7/14/15 MN LIC. NO. 16800

PROJECT NO.

160553004

RICE CREEK REMEANDER CONSTRU FINAL CREEK REMEANDER CROSS SECTIONS

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UNTY	CITY PROJECT		SHEET NO.
<b>.</b>	COUNTY PROJECT		26
JCTION	S.A.P.	062-593-004	
	S.A.P.		30





ΤY	CITY PROJECT		SHEET NO.
• •	COUNTY PROJECT		27
	S.A.P.	062-593-004	
	S.A.P.		30

LEGEND	
	- EXISTING GROUND PROFILE
	- PROPOSED GROUND PROFILE
	<ul> <li>GROUNDWATER PROFILE (JUNE 2014)</li> </ul>



UNTY	CITY PROJECT		SHEET NO.	
	COUNTY PROJECT		28	
CTION	S.A.P.	062-593-004		
	S.A.P.		30	



ГҮ	CITY PROJECT		SHEET NO.
••	COUNTY PROJECT		29
	S.A.P.	062-593-004	
	S.A.P.		30



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2					DRAWN BY:	LNJ
10/3					CHECKED BY:	MJS
382					DATE:	07/14/2015
l: ∕1					PROJECT NO.	160553004
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I HEREBY CERTIFY THAT THIS PLAN, SPECIFICATION OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MINNESOTA.

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TCAAP BID PACKAGE #1 RICE CREEK REMEANDER CONSTRI FINAL CREEK REMEANDER CROSS SECTIONS

DATE: 7/14/15 MN LIC. NO. 16800

UNTY	CITY PROJECT	SHEET NO.	
	COUNTY PROJECT		30
RUCTION	S.A.P.	062-593-004	
२	S.A.P.		30



EL. 880 EL. 870

QUANTITY

8 (P)

1650 (P)

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S.P.

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DESIGN DATA 2012 AASHTO LRFD BRIDGE DESIGN, SEVENTH EDITION AND MNDOT BRIDGE DESIGN MANUAL

LOAD AND RESISTANCE FACTOR DESIGN METHOD HL93 LIVE LOAD

INSIDE HEIGHT = 8'-0" INSIDE WIDTH = 10'-0" BARREL LENGTH = 2 LINES OF 69'-0" HEIGHT OF WINGWALL AT END = 1'-9" DEPTH OF DROPWALL = 3'-2" SKEW ANGLE = 0 MINIMUM DESIGN FILL DEPTH = 3'-0" MAXIMUM DESIGN FILL DEPTH = 7'-0" UNIT WEIGHT FILL = 130.0 LBS./CU.FT. ANGLE INTERNAL FRICTION = 30°

MATERIAL DESIGN PROPERTIES: PRECAST CONCRETE: fy = 60 ks1 REINFORCEMENT BARS f'c = 5 KSI CONCRETE fy = 65 kst WELDED WIRE FABRIC REINFORCEMENT

	LIST OF SHEETS
NO.	DESCRIPTION
C1	GENERAL PLAN AND ELEVATION
C2	PRECAST CONCRETE BARREL DETAILS
C3	PRECAST CONCRETE END SECTION
	TYPE I-SINGLE OR DOUBLE BARREL
C4-C5	PRECAST CONCRETE END SECTION
	TYPE III-SINGLE OR DOUBLE BARREL
C6	C.I.P. TRANSITION DETAILS
C7	ALTERNATE DROPWALLS FOR BOX CULVERT
C8	EMBANKMENT PROTECTION FOR BOX CULVERTS

CONSTRUCTION NOTES

THE 2014 EDITION OF THE MINNESOTA DEPARTMENT OF TRANSPORTATION "STANDARD SPECIFICATIONS FOR CONSTRUCTION" AND THE 2014 EDITION OF THE "MATERIALS LAB SUPPLEMENTAL SPECIFICATIONS FOR CONSTRUCTION" SHALL GOVERN.

THE BAR SIZES SHOWN IN THIS PLAN ARE IN U.S. CUSTOMARY DESIGNATIONS.

ALL EXPOSED CONCRETE EDGES SHALL BE FORMED WITH A  $1\!\!\!/_2"$  OR  $1\!\!\!/_3"$  CHAMFER UNLESS OTHERWISE NOTED.

CONSTRUCTION TO BE IN ACCORDANCE WITH SPEC. 2411 AND 2412, EXCEPT AS NOTED.

FOR STRUCTURE EXCAVATION AND BACKFILL SEE SPEC. 2451.

SEE ROADWAY PLANS FOR CULVERT SUBCUT, BEDDING AND BACKFILL REQUIREMENTS.

THE SUBSURFACE UTILITY INFORMATION IN THIS PLAN IS UTILITY QUALITY LEVEL D. THIS UTILITY QUALITY LEVEL WAS DETERMINED ACCORDING TO THE GUIDELINES OF CI/ASCE 38-02, ENTITLED "STANDARD GUIDELINES FOR THE COLLECTION AND DEPICTION OF EXISTING SUBSURFACE UTILITY DATA".

> DATE ..........

## BRIDGE NO. 91071

DBL. 10' W × 8' H BOX CULVERT EXTENSION UNDER N.B. T.H. 10 TO N.B. I-35W RAMP 1.2 MILES NORTH OF THE JUNCTION OF I-694 IN ARDEN HILLS

**IDENTIFICATION NO. 113** 

31

63

C 1

138 (P) GENERAL PLAN AND ELEVATION (P) 2 SEC. 8/9 TWP. 30 N. R. 23 W. 85 CITY: ARDEN HILLS COUNTY: RAMSEY APPROVED DATE 7715 FOR STATE BRIDGE ENGINEER (P 14 289 (P) COUNTY PROJECT SHEET NO.

062-593-004



CULVERTS TO BE CONSTRUCTED AS PER SPEC. 2412 EXCEPT AS NOTED.

REFER TO THE GENERAL PLAN AND ELEVATION SHEET FOR THE DISTANCE BETWEEN BARRELS OF ADJACENT BOXES AND TO STANDARD FIGURE 5-395.115 FOR MATERIAL REQUIREMENTS FOR FILL BETWEEN ADJACENT BOXES.

THE WELDED WIRE FABRIC, SHEAR REINFORCEMENT AND REINFORCEMENT BARS SHALL CONFORM TO APPLICABLE REQUIREMENTS OF AASHTO M259.

11/2" MIN. AND 2" MAX. CONCRETE COVER ON ALL REINFORCEMENT, INCLUDING SHEAR REINFORCEMENT, EXCEPT FOR TONGUE AND GROOVE DETAIL.

ANY OF THE FOLLOWING COMBINATIONS OF STEEL REINFORCEMENT MAY BE USED: (a) 1 OR 2 LAYERS OF WELDED WIRE FABRIC OR (b) 1 LAYER OF WELDED WIRE FABRIC AND 1 LAYER OF REINFORCEMENT BARS OR (c) 1 LAYER OF REINFORCEMENT BARS.

THE REINFORCEMENT SHALL BE DEVELOPED IN ACCORDANCE WITH AASHTO "LRFD BRIDGE DESIGN SPECIFICATIONS". IF BAR REINFORCEMENT IS SUBSTITUTED FOR WELDED WIRE FABRIC, THE AREA OF REINFORCEMENT SHALL BE INCREASED BY 8%, AND CONTRACTOR SHALL SUBMIT DESIGN CALCULATIONS VERIFYING COMPLIANCE WITH AASHTO 5.7.3.4. "CONTROL OF CRACKING BY DISTRIBUTION OF REINFORCEMENT".

THE MAXIMUM SIZE OF REINFORCEMENT BARS SHALL BE NO.6. THE MAXIMUM WELDED WIRE FABRIC SIZE SHALL BE A W23 PER LAYER (MAXIMUM OF 2 LAYERS).

THE SPACING CENTER TO CENTER OF THE TRANSVERSE WIRES SHALL NOT BE LESS THAN 2" NOR MORE THAN 4".THE SPACING CENTER TO CENTER OF THE LONGITUDINAL WIRES SHALL NOT BE MORE THAN 8".

WELDING WILL NOT BE ALLOWED ON REINFORCEMENT BARS OR WELDED WIRE FABRIC, EXCEPT THAT THE ORIGINAL WELDING REQUIRED TO MANUFACTURE WIRE FABRIC IS ACCEPTABLE.

WHEN REINFORCEMENT IS CUT. ADDITIONAL REINFORCEMENT SHALL BE ADDED ON BOTH SIDES OF THE CUT MEMBER TO REPLACE OR EXCEED THE CUT STEEL.

CONCRETE SHALL BE MIX NO. 3W36 WITH NO CALCIUM CHLORIDE ALLOWED.

SHOP DRAWING APPROVAL PER SPEC. 3238.2.A IS NOT REQUIRED UNLESS OPENINGS OR ATTACHMENTS ARE PLACED ON A BARREL SEGMENT.

COMPACT THE FIRST 1.5' (LOOSE) OF FILL ABOVE THE BOX WITH LIGHT COMPACTION EQUIPMENT SUCH AS PLATE COMPACTORS OR WALK BEHIND ROLLERS.

TRANSVERSE REINFORCEMENT IS PARALLEL TO THE CULVERT SPAN. LONGITUDINAL REINFORCEMENT IS PERPENDICULAR TO THE CULVERT SPAN.

(1) CULVERT TIES ARE TO BE 1" DIAMETER RODS. SEE STANDARD PLATE NO. 3145 FOR CONNECTION DETAILS.

(2) HAUNCH SIZES ARE TO BE 12" VERTICAL, 12" HORIZONTAL ON ALL BOX SIZES.

(3) LONGITUDINAL REINFORCEMENT DENOTED AS AS5 AND AS6 MUST BE PLACED IN ALL SLABS AND WALLS AND MUST BE 0.06 S0. IN./FT. MIN.

(4) ROADWAY OR SHOULDER FILL HEAGHTS OF LESS THAN 2'-O" REQUIRE A DISTRIBUTION SLAB. USE CONCRETE MEX 3Y43 FOR THE DISTRIBUTION SLAB. CAST-M-PLACE DISTRIBUTION SLADS SHALL BE & THICK. PROVIDE 3" MININGM SELECT GRANULAR MATERIAL PER SPEC. 3149.2,8.2 BETWEEN BARREL AND DISTRIBUTION SLAB.

PRECAST DISTRIBUTION SLADS SHALL BE O" THICK AND MAY BE USED FOR FUL HEIGHTS OVER 1'-O'. PROVIDE 6" MINIMUM SELECT GRANULAR MATERIAL PER SPEC. 3149.2.B.2 BETWEEN BARBEL AND DISTRIBUTION SLAB

THE WIDTH OF THE DISTRIBUTION BLAB SHALL EXTEND BETWEEN THE OUTSIDE PAYMENT FOR THE DISTRIBUTION SLAB AND SELECT GRANULAR MATERIAL BENEATH THE SLAB SHALL BE CONSIDERED INCIDENTAL.

IF DISTRUCTION SLAB IS USED AS PAVEMENT SURFACE IT MUST BE REDESIGNED

F	REINFORCEMENT										
1	As	54	As	57	As8						
	AREA (IN.²/FT.)	LENGTH (FT.)	AREA (IN. ² /FT.)	LENGTH (FT.)	AREA (IN. ² /FT.)	LENGTH (FT.)					
	0.20	8'-6"	0.24	8'-3"	0.24	8'-3"					

	FIG. 5-395	.101(A)
RETE ILS	×.	BRIDGE NO. 91071
	COUNTY PROJECT	SHEET NO.
	S.A.P. 062-593-00	4 32 /
	S.P.	C 2
ETE S	S.P.	
	S.P.	



SEE STANDARD FIG. 5-395.101(A) AND FIG. 5-395.101(B) FOR ADDITIONAL DIMENSIONS AND CONSTRUCTION NOTES.

- ALL END SECTIONS REQUIRE CURB ON LINTEL BEAM.
- ON ALL END SECTIONS FOR WATERWAYS, USE DROPWALLS ON INLET AND OUTLET ENDS.

SEE STANDARD FIG. 5-395.115 FOR EMBANKMENT PROTECTION.

FINISH ALL EXPOSED EDGES OF CONCRETE WITH  $1\!\!/_2"$  OR  $2\!\!/_4"$  CHAMFER OR RADIUS UNLESS OTHERWISE NOTED.

- (1) WITH DOUBLE BOXES LOCATE DROPWALL JOINTS BETWEEN END SECTIONS, SEE STANDARD FIG. 5-395.111 FOR ALTERNATE DROPWALLS. LIMITS OF EXCAVATION FOR DROPWALL TO BE APPROXIMATELY THE SAME AS DROPWALL DIMENSIONS. DROPWALL TO BE CONCRETE MIX NO. 1A43 OR MIX NO. 3Y43. FURNISHING AND INSTALLATION OF DROPWALL TO BE INCLUDED IN PRICE BID FOR END SECTIONS. DROPWALL NOT REQUIRED FOR NON-WATERWAY USE.
- (2) CHECK LOCATION TO DETERMINE WHETHER A TONGUE OR A GROOVE IS USED.
- (3) FILL HOLE WITH GROUT. GROUT SHALL CONSIST OF 1 PART CEMENT AND 2 PARTS SAND. USE TYPE 1A AIR ENTRAINED PORTLAND CEMENT. GROUT MIX SHALL HAVE A MAXIMUM SLUMP OF 4".
- (4) 3'-6" MIN. TONGUE AND 3'-7" MIN. GROOVE FOR CULVERTS WITH 6'-0" SPANS. 5'-0" MIN. TONGUE AND 5'-1" MIN. GROOVE FOR CULVERTS WITH SPANS GREATER THAN 6'-0". CENTER TONGUE AND GROOVE ON € OF EACH APRON JOINT. TONGUE AND GROOVE JOINT ON ALL THREE SIDES OF APRON IS PERMISSIBLE.
- (5) AS AN ALTERNATE TO THE ONE LAYER WELDED WIRE FABRIC, PROVIDE TWO LAYERS OF REBAR OR WELDED WIRE FABRIC WITH THE STEEL AREA EQUAL TO HALF OF THE TEMPERATURE STEEL PER CODE REQUIREMENTS IN EACH FACE OF THE DROPWALL.
- (6) APRON TOP AND BOTTOM SLAB THICKNESS MAY BE 8" FOR CULVERTS WITH 6'SPANS ONLY. BOTTOM SLAB THICKNESS MAY BE INCREASED UP TO 2" MAX. PROVIDED CONCRETE COVER IS 11/2" MIN., 2" MAX.
- (7) 10" MINIMUM TOP SLAB FOR 14' AND 16' SPANS.
- (8) LONGITUDINAL REINFORCEMENT PERPENDICULAR TO THE CULVERT SPAN SHALL HAVE A MINIMUM OF 0.06 SQUARE INCHES PER PERIPHERAL FOOT ON ALL FACES OF THE BARREL. 9"

Att. Atb REINFORCEMENT								
SPAN Att Atb (FT.) (IN ² /FT.) (IN ² /FT.)								
6	0.27	0.44						
8	0.47	0.60						
10	0.62	0.74						
12	0.88	1.06						
14	1.20	1.58						
16	1.52	2.09						

#### NO. 4 BENT BAR

ADT REINFORCEMENT						
SPAN (FT.)	Abt (IN ² /FT.)					
6-10	0.20					
12	0.30					
14	0.39					
16	0.39					

ENSIONS & AN REINFORCEMENT										
h7	SECT	ION 3	<b>b4</b>	SECT	ION 4	<b>b5</b>	SECT	ION 5	<b>b6</b>	
ns	Z	Ah	114	ZZ	Ah	115	ZZZ	Ah	110	
(1'-9")										
1'-9"										
1'-9"										
-9"(3'-9")	(4')	(0.192)	(1'-9")							
3'-9"	4'	0.192	1'-9"							
4'-9"	6'	0.192	1'-9"							
5'-9"	8' (4')	0.192	1'-9" (3'-9")	(4')	(0.192)	(1'-9")				
6'-9"	6'	0.192	3'-9"	4'	0.192	1'-9"				
7'-9"	6'	0.20	4'-9"	6'	0.192	1'-9"				
8'-9"	6'	0.28	5'-9"	8' (4')	0.192	1'-9"(3'-9")	(4')	(0.192)	(1'-9")	
9'-9"	6'	0.40	6'-9"	6'	0.192	3'-9"	4'	0.192	1'-9"	

		FIG. 5-395				
D SECTION BLE BARREL 71/2*			BRIDGE NO. 91071			
	COUNTY	PROJECT	SHEET NO.			
	\$.A.P	062-593-00	4 3 3 🖊			
	S.P.		C3			
DSECTION	S.P.					
LE BARREL	S.P.		03			



SEE STANDARD FIG. 5-395.101(A) AND FIG. 5-395.101(B) FOR ADDITIONAL DIMENSIONS AND CONSTRUCTION NOTES.

ON ALL END SECTIONS FOR WATERWAYS, USE DROPWALLS ON INLET AND OUTLET ENDS.

SEE STANDARD FIG. 5-395.115 FOR EMBANKMENT PROTECTION.

FINISH ALL EXPOSED EDGES OF CONCRETE WITH 1/2" OR 3/4" CHAMFER OR RADIUS UNLESS OTHERWISE NOTED.

- (1) WITH DOUBLE BOXES LOCATE DROPWALL JOINTS BETWEEN END SECTIONS. SEE STANDARD FIG. 5-395.111 FOR ALTERNATE DROPWALLS. LIMITS OF SEE STANDARD FIG. 5-355.111 FOR ALTERNATE UNDERWALLS. LIMITS OF EXCAVATION FOR DROPWALL TO BE APPROXIMATELY THE SAME AS DROPWALL DIMENSIONS. DROPWALL TO BE CONCRETE MIX NO. 1A43 OR 3Y43. FURNISHING AND INSTALLATION OF DROPWALL TO BE INCLUDED IN PRICE BID FOR END SECTIONS. DROPWALL NOT REQUIRED FOR NON-WATERWAY USE.
- (2) CHECK LOCATION TO DETERMINE WHETHER A TONGUE OR A GROOVE IS USED.
- (3) SEE STANDARD FIG. 5-395.104(B) FOR LINTEL BEAM DETAILS.
- (4) FILL HOLE WITH GROUT. GROUT SHALL CONSIST OF 1 PART CEMENT AND 2 PARTS SAND. USE TYPE 1A AIR ENTRAINED PORTLAND CEMENT. GROUT MIX SHALL HAVE A MAXIMUM SLUMP OF 4".
- (5) 2" DIAMETER HOLE, 6" DEEP IN TOP OF THE SECTION WALL.
- (a) (b) 3'-6" MIN. TONGUE AND 3'-7" MIN. GROOVE FOR CULVERTS WITH 6'-0" SPANS. 5'-0" MIN. TONGUE AND 5'-1" MIN. GROOVE FOR CULVERTS WITH SPANS GREATER THAN 6'-0". CENTER TONGUE AND GROOVE ON € OF EACH APRON JOINT. TONGUE AND GROOVE JOINT ON ALL THREE SIDES OF APRON IS PERMISSIBLE.
  - (7) AS AN ALTERNATE TO THE ONE LAYER WELDED WIRE FABRIC, PROVIDE TWO LAYERS OF REBAR OR WELDED WIRE FABRIC WITH THE STEEL AREA EQUAL TO HALF OF THE TEMPERATURE STEEL PER CODE REQUIREMENTS IN EACH FACE OF THE DROPWALL.
  - (8) APRON BOTTOM SLAB THICKNESS MAY BE 8" FOR CULVERTS WITH 6' SPANS ONLY. BOTTOM SLAB THICKNESS MAY BE INCREASED UP TO 2" MAX. PROVIDED COVER IS 11/2" MIN., 2" MAX.
  - (9) LONGITUDINAL REINFORCEMENT PERPENDICULAR TO THE CULVERT SPAN SHALL HAVE A MINIMUM OF 0.06 SQUARE INCHES PER PERIPHERAL FOOT ON ALL FACES OF THE BARREL.

-LAYER OF WELDED WIRE FABRIC

_

LAYER OF WELDED WIRE FABRIC-

## FABRIC LAYER DETAIL

DROPWALL (CAST-IN-PLACE OR PRECAST)

WHEN MORE THAN ONE LAYER OF WELDED WIRE FABRIC IS USED TO OBTAIN THE REQUIRED REINFORCEMENT AREAS, THE WIRES OF THE WELDED WIRE FABRIC SHALL BE PLACED AS SHOWN.

NSIONS & AN REINFORCEMENT										
h3	SECT	ION 3	<b>b4</b>	SECT	ION 4	b5	SECT	ION 5	b6	
	Z	Ah	114	ZZ	Ah	115	ZZZ	Ah	110	
'-9")										
1'-9"										
1'-9"										
"(3'-9")	(4')	(0.192)	(1'-9")							
3'-9"	4'	0.192	1'-9"							
4'-9"	6'	0.192	1'-9"							
5'-9"	8' (4')	0.192	1'-9" (3'-9")	(4')	(0.192)	(1'-9")				
6'-9"	6'	0.192	3'-9"	4'	0.192	1'-9"				
7'-9"	6'	0.20	4'-9"	6'	0.192	1'-9"				
8'-9"	6'	0.28	5'-9"	8' (4')	0.192	1'-9" (3'-9")	(4')	(0.192)	(1'-9")	
9'-9"	6'	0.40	6'-9"	6'	0.192	3'-9"	4'	0.192	1'-9"	

NO. 1
67
03



LINTEL BEAM BOTTOM REINFORCEMENT								
SPAN (FT.)	A1	^A 2						
6	NO. 4 @ 1'-2"	NO. 4 @ 91/2"						
8	NO. 4 @ 8"	NO. 5 @ 8"						
10	NO. 5 @ 8"	NO. 6 @ 71/2"						
12	NO. 5 @ 6"	NO. 6 @ 6"						
14	NO. 6 @ 6"	NO. 7 @ 6"						
16	NO. 6 @ 6"	NO. 7 @ 6"						

REDUCE AS NECESSARY





Ì	REVISION: 11-	06-2013								FIG. 5-395.1	04(B)	
	APPROVED: MA	RCH 24, 2011 Jackenberger BRIDGE ENGINEER						TYPE III - SINGLE OR DOUBLE BARREL FOR SKEWS UP TO 71/2*			BRIDG 91	E NO. 071
Default	No. Date	Revisions	Арр 	DRAWING N DESIGNED BY: DRAWN BY: CHECKED BY: DATE: PROJECT NO.	NAME MAW MAW MHD 129594	J SEH	I HEREBY CERTIFY THAT THIS PLAN, SPECIFICATION OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MINNESOTA.	RAMSEY COUNTY TCAAP BP1 PRECAST CONCRETE END SECTION TYPE III - SINGLE OR DOUBLE BARREL	COUNTY S.A.F S.P. S.P. S.P.	Y PROJECT P. 062-593-004	SHEET NO	63

1/7/2015

\R\Ramse\129594\5-final-dsgn\51-const-dwgs-CAD\20-Struct\culver+\dgn\Hfig51 i+

#### CONSTRUCTION NOTES

- SEE STANDARD FIG. 5-395.101(A) AND FIG. 5-395.101(B) FOR ADDITIONAL DIMENSIONS AND CONSTRUCTION NOTES.
- ALL END SECTIONS REQUIRE CURB ON LINTEL BEAM.

GROUT SHALL CONSIST OF 1 PART CEMENT AND 2 PARTS SAND. USE TYPE 1A AIR ENTRAINED PORTLAND CEMENT. GROUT MIX SHALL HAVE A MAXIMUM SLUMP OF 4".

- (1) 3" DIA. HOLE THROUGH LINTEL BEAM AND 2" DIA. HOLE IN TOP OF WALL SECTION. PLACE NO. 8 DOWEL, 1'-O" LONG, IN HOLE AND FILL HOLE WITH GROUT.
- (2) CHECK THE LOCATION TO DETERMINE WHETHER A TONGUE OR A GROOVE IS USED. TONGUE AND GROOVE TO TERMINATE AT HAUNCH.
- 3 For spans under 10'-0" use no.8 bars. For spans of 10'-0" to 12'-0" use no.9 bars. For 14'-0" and 16'-0" span, use no.10 bars.
- ( ALTERNATE BAR BEND MAY BE USED FOR B401.



PRECAST CULVERT	(1) FED. PROJ. NO.
	SMALL RADIUS OR 4"  OUTSIDE FACE
- OUTSIDE FACE	
ES AL	
THIC	
E	1/2" MIN
<u>[÷↓</u> Ĕ	<u> </u>
-	TONGUE AND GROOVE JOINT
N INTO TRANSITIO	MARE DIMENSION OF TORGUE OR GROOVE • ON ADJACENT PRECAST BARREL SECTIONS SO INSTRE FACES ARE FLUES
IS INTO	SU INSIDE FACES ARE FLUSH.
	TRANSITION QUANTITIES 29
	STRUCTURE CONC. (3Y46) 7 CY REINFORCEMENT BARS 1650 POUND
CAST CHI VERT	REINFORCEMENT FOR TRANSITION (9)
	BAR NO. LENGTH SHAPE LOCATION
	C302 32 1'-4" STR INSIDE CORNERS
_  ↑	C504 56 14'-10" BENT OUTSIDE CORNERS
	C505 16 1'-9" BENT INTERIOR WALL HORIZ.
	C507 12 4'-O" STR OUTSIDE TOP AND BOTTOM
	NUIES: CONSTRUCTION SHALL START FROM THE EXISTING CULVERT
	AND PROCEED OUTWARD IN EACH DIRECTION.
®	TAPERED IN THE TRANSITION AREA TO MATCH PRECAST CULVERT DIMENSIONS.
	ALL JOINTS BETWEEN BARREL LENGTHS TO BE SEALED WITH
	MASTIC. SEE SPECIAL PROVISIONS.
I ¥	LIMITS HAVE BEEN OUTLINED BY THE CONTRACTOR AND APPROVED BY THE ENGINEER. REMOVAL AND RECONSTRUCTION SHALL CONFORM TO SPEC. 2433, EXCEPT AS NOTED.
RANSITION.	PRECAST CULVERTS TO BE CONSTRUCTED PER SPEC. 2411.
	ANY ADDITIONAL REINFORCEMENT SHALL BE PER SPEC. 3301, GRADE 60 OR SPEC. 3303.
1	END SECTIONS TO BE EITHER TYPE I, II, OR III, DEPENDING ON CULVERT SKEW. SEE STANDARD PLAN SHEETS
3	5-395.110M
3	NOT USED.
(4)	REMOVE INPLACE CULVERT WINGS AS NECESSARY TO PROVIDE
0	CLEARANCE FOR CONSTRUCTION OF SIDE WALL TRANSITION AS DIRECTED BY THE ENGINEER.
5	CULVERT TIES ARE TO BE 1" DIA RODS. SEE STANDARD PLATE NO M3145 FOR CONNECTION DETAILS. MODIFY AS
	REQUIRED AT TRANSITION. ALTERNATE CULVERT TIES MAY BE USED IF APPROVED BY THE ENGINEER. ALL HOLES IN
	THE BARRELS ARE TO BE APPROVED BY THE ENGINEER.
6	SURFACES IN CONTACT WITH NEW CONCRETE.
$\bigcirc$	FILL 6" VOID BETWEEN CULVERT WALLS WITH LEAN MIX BACKFILL (SPEC. 2520).
8	CUT BARS AS NECESSARY IN FIELD.
9	QUANTITIES ARE FOR ONE DBL BARREL TRANSITION USING ALTERNATE 1 DESIGN.
5 ⁽⁰⁾	SEE "PLAN DETAIL A" SHEET 1 FOR ADDITIONAL INFORMATION.
ION DETAILS	
CAST-IN-PLACE EXTENSION	
	COUNTY PROJECT SHEET NO.
	S.A.P. 062-593-004 36
	S.P. C 6
	S.P. 67
DETAILO	S.P. 0 5



GALVANIZE ALL FASTENERS AS PER SPEC. 3392.

BEFORE CULVERT PLANS ARE PREPARED, SAMPLES SHALL BE TAKEN FROM THE DRAINAGE AREA FOR PH DETERMINATION. THE SOIL AND WATER SHOULD HAVE A PH OF 6.5 OR MORE IF SHEET STEEL IS USED.

- (1) 21/2" × 1/2" OR 27/2" × 1/2" CORRUGATED (12 GAGE) OR HEAVIER GALVANIZED STEEL SHEETS.
- NEAREST VALLEY).
- (3) FASTEN THE 8"× 4"× ¾" OR 6"× 4"× ¾" ANGLE WITH ¾" DIAMETER 4" LONG BOLTS, 1" O.D. WASHER AND AN APPROVED ANCHORAGE (2'-O" SPACING).
- (4) FASTEN THE STEEL SHEETS TO THE FRONT EDGE OF THE APRON WITH %" DIAMETER 5" LONG BOLTS, NUT AND LOCK WASHER (10" ± CENTER TO CENTER, TO THE NEAREST VALLEY).
- (5) (12 GAGE) GALVANIZED CORRUGATED STEEL SHEET PILING, INTERLOCKING TYPE A.
- ⑥ ¾" DIA. × 1" LONG BOLT WITH NUT, TO LAP STEEL SHEETS.
- (7) STEEL SHEET PILING, SECTION NO. MP-112 OR EQUAL.
- $(\ensuremath{\mathbb{B}})$  fill the voids as shown, with concrete or concrete grout, as approved by the engineer.

	FIG. 5-395	.111
WALLS ERTS		BRIDGE NO. 91071
UNTY	COUNTY PROJECT S.A.P. 062-593-004	SHEET NO.
	S.P.	C7 /
ALLS	S.P.	67
10	S.P.	/ 03



THIS PLAN SHEET IS FOR CULVERT EMBANKMENT PROTECTION ONLY. REFER TO THE GRADING PLANS FOR ADDITIONAL RIPRAP OR OTHER SCOUR PROTECTION MEASURES.

RIPRAP SHALL COMPLY WITH SPECS. 2511 AND 3601.

- (1) FOR TYPE OF GEOTEXTILE FILTER MATERIAL REQUIRED, SEE SPEC. 3733. GEOTEXTILE STRIPS SHOULD BE CONTINUOUS WITHOUT OVERLAPS, EXCEPT FOR THE TOP STRIP, WHICH SHOULD SHINGLE VERTICAL STRIPS. THE TOP EDGE SHOULD BE BURIED TO PREVENT UNDERMINING.
- (2) IF THE DISTANCE BETWEEN DOUBLE BARRELS IS LESS THAN 2'-O" USE EITHER PEA ROCK OR LEAN MIX BACKFILL (SPEC, 2520) BETWEEN THE CULVERTS AS APPROVED BY THE THE CULVENTS AS APPROVED BY THE ENGINEER, IF DEA ROCK IS USED PROVIDE APPROVED GROUT SEEPAGE CUTOFF CORE, MINIMUM 12" THICK BETWEEN THE CULVERT'S-TWO ENDS AND PROVIDE CLASS I GROUTED -RIPRAP IN LIEU OF CLASS III RIPRAP.
- ③ REFER TO THE GENERAL PLAN AND ELEVATION SHEET FOR THE DISTANCE BETWEEN BARRELS OF ADJACENT BOXES.

#### RIPRAP CLASS

RIPRAP CLASS	RIPRAP CLASS	т	w
$\boxtimes$	ш	1'-6"	3'-0"
	IV	2'-0"	4'-0"

		FIG. 5-395.	115
TECTION			BRIDGE NO. 91071
	COUNTY PRO	JECT	SHEET NO.
	S.A.P.	062-593-004	38 /
	S.P.		C8
CTION	S.P.		67
(15	S.P.		03

![](_page_38_Figure_0.jpeg)

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		DESIGN DAT	A
2.842	ſ	2012 AASHTO LRFD BRIDGE DESIG AND MNDOT BRIDGE DESIGN MANU	N, SEVENTH EDITION AL
939		LOAD AND RESISTANCE FACTOR D	ESIGN METHOD
	C	HL93 LIVE LUAU	$\sim$
IR S	5	INSIDE WIDTH = 10'-0" BARREL LENGTH = 2 LINES OF 98	·-0"
HE	~ ~	HEIGHT OF WINGWALL AT WEST EN DEPTH OF DROPWALL AT WEST EN	ND = 1'-9"
	RADIUS POINT	SKEW ANGLE = 83° 47'39.5" MINIMUM DESIGN FILL DEPTH = 15	5'-0"
X=550830 Y=210748	.160	MAXIMUM DESIGN FILL DEPTH = 2 UNIT WEIGHT FILL = 130.0 LBS./(	5'-0"
$\mathbf{n}$	J.	ANGLE INTERNAL FRICTION = 30°	
0		MATERIAL DESIGN PROPERTIES:	
I <b>.</b>		fy = 60 ksi REINFORCEMENT BAR	s
Y WORK, FUTURE		fy = 65 ksi WELDED WIRE FABRIC	REINFORCEMENT
	L		
-		LIST OF SHEETS	
	NO.		
395.115	D2 BRIDGE	QUANTITIES	LASE SECTIONS
ECESSED TIES	D3-D5 CAST-1 D6 CULVEF	N-PLACE CONCRETE END TRANSITION	DETAILS AND QUANTITIES
NDERED CREEK	DT-D12 BARREL	DETAILS	
	RW1 RETAIN	ING WALL GENERAL NOTES AND SUMM	MARY OF QUANTITIES
R	N2-RW12 RETAIN	ING WALL DETAILS	
CO	NSTRUCTIO	ON NOTES	
THE	2014 EDITION	OF THE MINNESOTA DEPARTMENT	OF
TRA AND	NSPORTATION " THE 2014 EDI	STANDARD SPECIFICATIONS FOR ( TION OF THE "MATERIALS LAB SU	CONSTRUCTION"
SPE	CIFICATIONS FO	OR CONSTRUCTION" SHALL GOVERN OWN IN THIS PLAN ARE IN U.S.	J.
CUS BAR	TOMARY DESIGN	IATIONS. I THE SUFFIX "F" SHALL BE EPO)	xy
COA	TED IN ACCORD	ANCE WITH SPEC. 3301.	
DRA	WINGS ARE NOT	TO BE SCALED.	
JOII IN	NTS BETWEEN S ACCORDANCE WI	ECTIONS TO BE WRAPPED TH SPEC 2412.	
ALL WIT NOT	EXPOSED CONC H A 1/2 " OR ED.	RETE EDGES SHALL BE FORMED 3/4 " CHAMFER UNLESS OTHER	WISE
CON AND	STRUCTION TO 2412, EXCEPT	BE IN ACCORDANCE WITH SPEC. 24 AS NOTED.	411
F OR SPE	STRUCTURE EX C.2451.	CAVATION AND BACKFILL SEE	
r JRB			
		APPROVED	ENGINEER
		DATE	
		BRIDGE NO 6	2X04
<u>/1\</u> {		RAMP FROM 1-35W N	N.B. TO
		1.2 MILES NORTH OF THE	JUNCTION OF
<del>آ</del>		IDENTIFICATION NO	0. 113
		GENERAL PLAN AND	
GREGATE		CITY OF ARDEN HILLS	RAMSEY CO.
		APPROVED	
		STATE BRI	DGE ENGINEER
	COUNTY	PROJECT S	SHEET NO.
	S.A.P.	062-593-004	39
	S.P.		D1
ND TRANSVERSE	S.P.		
	S.P.		/ 63

![](_page_39_Figure_0.jpeg)

## NOTES:

THE MATERIALS AND PLACEMENT OF THE CORK AND DOWEL JOINT/ CONSTRUCTION JOINT (DOWEL BAR ASSEMBLIES, NO. 16 REINFORCING BARS, JOINT FILLER, AND JOINT WATERPROOFING) ARE INCIDENTAL FOR WHICH NO DIRECT PAYMENT WILL BE MADE.

THE CONTRACTOR SHALL ASSIGN TO THE REINFORCING BAR SUPPLIER THE RESPONSIBILITY OF SUPPLYING THE NECESSARY MATERIALS ASSOCIATED WITH THE DETAILS SHOWN ON THIS SHEET.

- (3) JOINT FILLER SHALL BE CORK (SPEC. 2401.3E3).
- (4) GREASE SHALL BE AN APPROVED HIGH PRESSURE TYPE THAT IS EFFECTIVE OVER THE FULL RANGE OF EXPECTED TEMPERATURES AND RESISTANT TO CHEMICAL ACTION.
- (5) dowel bar assembly must be installed perpendicular to joint and parallel to the wall face, and to each other.
- 6 SEE REINFORCING DETAILS.

No.	Date	Revisions	Арр,	DRAWING NAME		
				DESIGNED BY: MA	w	
L				DRAWN BY: MA	w	
				CHECKED BY: MH	D	
				DATE:		
$\triangle$	9-30-15	ADDENDUM NO. 1		PROJECT NO. 1295	94	

JEFFREY A JOHNSON 7-7-15 ____MN_LIC. NO. ____ DATE:

SEH

4

17280

![](_page_39_Picture_10.jpeg)

		62X04 QUANTITIES FOR BOX CULVE	RT AND F	RETAINING	WALLS	
	SPEC NO	ІТЕМ	UNIT	BOX	RETAINING	TOTAL
1	2401.601	STRUCTURE EXCAVATION	LUMP SUM	1		i '
	2411.501	STRUCTURAL CONCRETE (1A43)	CUYD	\$	43	43 (P)
	2411.501	STRUCTURAL CONCRETE (3Y43)	CU YD >	25	81	106 (P)
[	2411.541	REINFORCEMENT BARS			4146	4146 (P)
[	2411.541	REINFORCEMENT BARS EPOXY COATED	POUND	<u>/1</u> 4790	7338	<u>1</u> 12128(P)
	2411.618	ANTI-GRAFFITI COATING	SQ.FT. (	/1 91	649	/ <u>1</u> 740 (P)
1	2411.618	ARCH CONCRETE TEXTURE (ASHLAR STONE)	SQ.FT.	1 91	649	/ <u>1</u> 740 (P)
Same -	2411.618	ARCH SURFACE FINISH (MULTI-COLOR)	SQ.FT. 🛇	1 91	649	/ <u>1</u> 740 (P)
{12	2412.511	10 × 8 PRECAST CONCRETE BOX CULVERT	LIN.FT. >	196		196 (P)
$\{\overline{\Lambda},$	2412.512	10 × 8 PRECAST CONCRETE BOX CULVERT END SECTION	EACH	2		2 (P)
~~ <u>3</u>	2451.511	COARSE FILTER AGGREGATE (CV)	CU. YD. >	/1\115	59	<u>1</u> 174 (P)
Ŭ	2502.502	DRAINAGE SYSTEM TYPE (WALL)	LUMP SUM	1		1
	2511.501	RANDOM RIPRAP CLASS III	CU YD (	1 15		<u>1</u> 15 (P)
1	2511.515	GEOTEXTILE FILTER TYPE V	SQ YD	490	213	703 (P)
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ľ						00000
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	and the second sec					
t t						
- F						
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#### FORM LINER.

- PAYMENT FOR ALL PRECAST COMPONENTS INCLUDING DESIGN, SUBMITTALS, ALL CONNECTIONS, WATERPROOFING, FABRICATION, HAULING AND COMPLETE INSTALLATION SHALL BE INCLUDED IN THE PRICE BID FOR "10x8 PRECAST CONCRETE BOX CULVERT".
- ③ COARSE FILTER AGGREGATE SHALL BE AN AGGREGATE WITH 100% PASSING THE 2" SIEVE, 0-10% PASSING THE ■4 SIEVE, 0-3% PASSING THE ■200 SIEVE. THE COARSE FILTER AGGREGATE SHALL CONSIST OF CRUSHED QUARRY ROCK.

TCAAP BP1 BRIDGE QUANTITIES

S:\PT\R Default

FOR	BOX	CULVERT	AND	RETAINING	WALLS	

	COUNTY PROJECT		SHEET NO.
	S.A.P.	062-593-004	40 /
	S.P.		D2
3	S.P.		67
	S.P.		63

![](_page_40_Figure_0.jpeg)

S:VI

C.J.	=	CONSTRUCTION JOINT
OPT. C.J.	=	OPTIONAL CONSTRUCTION JOINT
J <b>.W.</b>	=	JOINT WATERPROOFING
(1) MODIF	IE	D PORTION OF BOX SECTION.

	COUNTY PROJECT		SHEET NO.
UNTI	S.A.P.	062-593-004	41 /
	S.P.		D3
SITION DETAILS	S.P.		67
	S.P.		/ 63

![](_page_41_Figure_0.jpeg)

No.	Date	Revisions	App.	DRAWING NAME		I HEREBY CERTIFY THAT THIS PLAN, SPECIFICATION OR REPORT WAS PREPARED BY ME OR UNDER MY	RAMSEY CO
				DESIGNED BY MHD		LICENSED PROFESSION AND THAT I AM A DULT	
		-		DRAWN BY: MAW		LAWS OF THE STATE OF MINNESUTA.	TCAAP BP1
				CHECKED BY: MHD		JAG	CULVERT
				DATE: .	SFH SFH	JEFFREY A JOHNSON	CAST-IN-PLACE CONCRETE TRAN
$\triangle$	9-30-15	ADDENDUM NO.1		PROJECT NO. 129594		DATE: 7-7-15 MN LIC. NO. 17280	2 x

	COUNTY PROJECT		SHEET NO.
	S.A.P.	062-593-004	42 /
	S.P.		D4
SITION DETAILS	S.P.		
SHION DETAILS	S.P.		63

![](_page_42_Figure_0.jpeg)

5.PT\R\Ramse\129594\5-final-dsgn\51-const-dwgs-CAD\20-Struct\Br62x04\dgn\cbr62x04_ENDS Dafault

FNDS 3 don

![](_page_43_Figure_0.jpeg)

62X04_ 62×04 ģ S:VI

#### QUANTITIES FOR CULVERT & C.I.P. CONC. END TRANSITIONS QUANTITY UNIT ITEM STRUCTURE CONCRETE (3Y43) CU YD 25 (3) REINFORCEMENT BARS EPOXY COATED 4790 POUND STRUCTURE EXCAVATION LUMP SUM I IN FT 196 10 × 8 PRECAST CONCRETE BOX CULVERT 10 × 8 PRECAST CONCRETE BOX CULVERT END SECTION EACH 1 (2) DOWEL BAR ASSEMBLY

() CONTRACTOR MAY PROVIDE LONGER BOX SECTION LENGTHS, BUT NEEDS TO MAINTAIN OVERALL LENGTH, THE BOX SECTIONS ON EAST END ARE MODIFIED TO INCORPORATE THE CAST-IN-PLACE CONCRETE END TRANSITION. PAYMENT FOR ALL PRECAST COMPONENTS INCLUDING DESIGN, SUBMITTALS, ALL CONNECTIONS, WATERPROOFING, FABRICATION, HAULING AND COMPLETE

INSTALLATION SHALL BE INCLUDED IN THE PRICE BID FOR "10×8 PRECAST CONCRETE BOX CULVERT".

(2) SEE DETAILS ON SHEET C2. (INCIDENTAL)

3 INCLUDES CULVERT END SECTION TRANSITION AND HEADWALL REINFORCEMENT.

	COUNTY PROJECT		SHEET NO.
	S.A.P.	062-593-004	44
	S.P.		D6
ENT	S.P.		67
	S.P.		63

![](_page_44_Figure_0.jpeg)

S:\PT\R\Ramse\129594\5-final-dsgn\51-const-dwgs-CAD\20-Struct\Br62x04\dgn\cbr62x04.fig5101A.

## CONSTRUCTION NOTES

CULVERTS TO BE CONSTRUCTED AS PER SPEC. 2412 EXCEPT AS NOTED.

REFER TO THE GENERAL PLAN AND ELEVATION SHEET FOR THE DISTANCE BETWEEN BARRELS OF ADJACENT BOXES AND TO STANDARD FIGURE 5-395.115 FOR MATERIAL REQUIREMENTS FOR FILL BETWEEN ADJACENT BOXES.

THE WELDED WIRE FABRIC, SHEAR REINFORCEMENT AND REINFORCEMENT BARS SHALL CONFORM TO APPLICABLE REQUIREMENTS OF AASHTO M259.

 $1\prime\!/_2"$  min.and 2" max.concrete cover on all reinforcement, including shear reinforcement, except for tongue and groove detail.

ANY OF THE FOLLOWING COMBINATIONS OF STEEL REINFORCEMENT MAY BE USED: (a) 1 OR 2 LAYERS OF WELDED WIRE FABRIC OR (b) 1 LAYER OF WELDED WIRE FABRIC AND 1 LAYER OF REINFORCEMENT BARS OR

(c) I LAYER OF REINFORCEMENT BARS. THE REINFORCEMENT SHALL BE DEVELOPED IN ACCORDANCE WITH AASHTO "LRFD BRIDGE DESIGN SPECIFICATIONS". IF BAR REINFORCEMENT IS SUBSTITUTED FOR WELDED WIRE FABRIC, THE AREA OF REINFORCEMENT SHALL BE INCREASED BY 8%, AND CONTRACTOR SHALL SUBMIT DESIGN CALCULATIONS VERIFYING COMPLIANCE WITH AASHTO 5.7.3.4. "CONTROL OF CRACKING BY DISTRIBUTION OF REINFORCEMENT".

THE MAXIMUM SIZE OF REINFORCEMENT BARS SHALL BE NO.6. THE MAXIMUM WELDED WIRE FABRIC SIZE SHALL BE A W23 PER LAYER (MAXIMUM OF 2 LAYERS).

THE SPACING CENTER TO CENTER OF THE TRANSVERSE WIRES SHALL NOT BE LESS THAN 2" NOR MORE THAN 4".THE SPACING CENTER TO CENTER OF THE LONGITUDINAL WIRES SHALL NOT BE MORE THAN 8".

WELDING WILL NOT BE ALLOWED ON REINFORCEMENT BARS OR WELDED WIRE FABRIC. EXCEPT THAT THE ORIGINAL WELDING REQUIRED TO MANUFACTURE WIRE FABRIC IS ACCEPTABLE.

WHEN REINFORCEMENT IS CUT, ADDITIONAL REINFORCEMENT SHALL BE ADDED ON BOTH SIDES OF THE CUT MEMBER TO REPLACE OR EXCEED THE CUT STEEL.

CONCRETE SHALL BE MIX NO. 3W36 WITH NO CALCIUM CHLORIDE ALLOWED.

SHOP DRAWING APPROVAL PER SPEC. 3238.2.A IS NOT REQUIRED UNLESS OPENINGS OR ATTACHMENTS ARE PLACED ON A BARREL SEGMENT.

COMPACT THE FIRST 1.5' (LOOSE) OF FILL ABOVE THE BOX WITH LIGHT COMPACTION EQUIPMENT SUCH AS PLATE COMPACTORS OR WALK BEHIND ROLLERS.

TRANSVERSE REINFORCEMENT IS PARALLEL TO THE CULVERT SPAN. LONGITUDINAL REINFORCEMENT IS PERPENDICULAR TO THE CULVERT SPAN.

1 culvert ties are to be 1" diameter rods. See standard plate NO. 3145 for connection details.

(2) HAUNCH SIZES ARE TO BE 12" VERTICAL, 12" HORIZONTAL ON ALL BOX SIZES.

(3) LONGITUDINAL REINFORCEMENT DENOTED AS AS5 AND AS6 MUST BE PLACED IN ALL SLABS AND WALLS AND MUST BE 0.06 SQ. IN./FT. MIN.

(4) ROADWAY OR SHOULDER FILL HEIGHTS OF LESS THAN 2'-O" REQUIRE A DISTRIBUTION SCAB. USE CONCRETE MIX 3Y47 FOR THE DISTRIBUTION SLAB.

CAST-W-PLACE DISTRIBUTION SLAPS SHALL BE GA THICK. PROVIDE 3" MINIMUM SELECT GRANULAR MATERIAL PER SPEC. 5149.2, B.2 BETWEEN BARREL AND DISTRIBUTION SLAB.

PRECAST DISTRIBUTION SLAGS SHALL BE 6" THICK AND MAY BE USED FOR FILL HEIGHTS OVER 1-0". PROVIDE 6" MINIMUM SELECT GRANULAR MATERIAL PER SPEC. 3149.2.B.2 BETWEEN BARBEL AND DISTRIBUTION SLAB

THE WOTH OF THE DISTRIBUTION SLAB SHALL EXTEND DETWEEN THE OUTSIDE EDGES OF THE SHOULDERS UNLESS DIRECTED BY THE ENGINEER. PAYMENT FOR THE DISTRIBUTION SLAB AND SELECT GRANULAR MATERIAL BENEATH THE SLAB SHALL BE CONSIDERED MCIDENTAL.

IF DISTRIBUTION SLAB AS USED AS PAVEMENT SURFACE IT MUST BE REDESIGNED

EINFORCEMEN	Т					
A	54	As	57	As8		
AREA (IN. ² /FT.)	LENGTH (FT,)	AREA (IN. ² /FT.)	LENGTH (FT.)	AREA (IN. ² /FT.)	LENGTH (FT.)	
0.20	8'-6"	0.24	8'-5"	0.24	8'-5"	
				~~~~	$\dots$	

			FIG. 5-3	95.1	01(A)	
ETE LS					BRIDO 62	SE NO. X04
	COUNTY F	PROJECT			SHEET N	D.
	S.A.P.		062-593	3-004	45	
	S.P.				D7	
E	S.P.					67
	S.P.					63

SEE STANDARD FIG. 5-395.101(A) AND FIG. 5-395.101(B) FOR ADDITIONAL DIMENSIONS AND CONSTRUCTION NOTES.

ALL END SECTIONS REQUIRE CURB ON LINTEL BEAM.

ON ALL END SECTIONS FOR WATERWAYS, USE DROPWALLS ON INLET AND OUTLET ENDS.

SEE STANDARD FIG. 5-395.115 FOR EMBANKMENT PROTECTION.

FINISH ALL EXPOSED EDGES OF CONCRETE WITH $1/_2^{\prime\prime}$ OR $\frac{3}{2}_4^{\prime\prime}$ CHAMFER OR RADIUS UNLESS OTHERWISE NOTED.

- (1) WITH DOUBLE BOXES LOCATE DROPWALL JOINTS BETWEEN END SECTIONS. SEE STANDARD FIG. 5-395.111 FOR ALTERNATE DROPWALLS. LIMITS OF EXCAVATION FOR DROPWALL TO BE APPROXIMATELY THE SAME AS DROPWALL DIMENSIONS. DROPWALL TO BE CONCRETE MIX NO. 1A43 OR MIX NO. 3Y43. FURNISHING AND INSTALLATION OF DROPWALL TO BE INCLUDED IN PRICE BID FOR END SECTIONS. DROPWALL NOT REQUIRED FOR NON-WATERWAY USE.
- (2) CHECK LOCATION TO DETERMINE WHETHER A TONGUE OR A GROOVE IS USED.
- (3) FILL HOLE WITH GROUT. GROUT SHALL CONSIST OF 1 PART CEMENT AND 2 PARTS SAND. USE TYPE 1A AIR ENTRAINED PORTLAND CEMENT. GROUT MIX SHALL HAVE A MAXIMUM SLUMP OF 4".
- ④ 3'-6" MIN. TONGUE AND 3'-7" MIN. GROOVE FOR CULVERTS WITH 6'-0" SPANS, 5'-0" MIN. TONGUE AND 5'-1" MIN. GROOVE FOR CULVERTS WITH SPANS GREATER THAN 6'-0". CENTER TONGUE AND GROOVE ON € OF EACH APRON JOINT. TONGUE AND GROOVE JOINT ON ALL THREE SIDES OF APRON IS PERMISSIBLE.
- (5) AS AN ALTERNATE TO THE ONE LAYER WELDED WIRE FABRIC. PROVIDE TWO LAYERS OF REBAR OR WELDED WIRE FABRIC WITH THE STEEL AREA EQUAL TO HALF OF THE TEMPERATURE STEEL PER CODE REQUIREMENTS IN EACH FACE OF THE DROPWALL.
- (6) APRON TOP AND BOTTOM SLAB THICKNESS MAY BE 8" FOR CULVERTS WITH 6'SPANS ONLY. BOTTOM SLAB THICKNESS MAY BE INCREASED UP TO 2" MAX. PROVIDED CONCRETE COVER IS 11/2" MIN., 2" MAX.
- (7) 10" MINIMUM TOP SLAB FOR 14' AND 16' SPANS.
- (8) LONGITUDINAL REINFORCEMENT PERPENDICULAR TO THE CULVERT SPAN SHALL HAVE A MINIMUM OF 0.06 SQUARE INCHES PER PERIPHERAL FOOT ON ALL FACES OF THE BARREL. 9"

D	

Att, Atb REINFORCEMENT							
SPAN (FT.)	Att (IN ² /FT.)	A†b (IN²/FT.)					
6	0.27	0.44					
8	0.47	0.60					
10	0.62	0.74					
12	0.88	1.06					
14	1.20	1.58					
16	1.52	2.09					

NO.	4	BENT	BAR
1101	_		0011

ADT REINFORCEMENT					
SPAN (FT.)	AD† (IN²/FT.)				
6-10	0.20				
12	0.30				
14	0.39				
16	0.39				

ENSION	IS &	Ah	REINFOR	CEM	ENT				
b 7	SECT	ION 3	ь4	SECT	ION 4	55	SECT	ION 5	50
113	Z	Ah	114	ZZ	Ah	15	ZZZ	Ah	116
1'-9")									
1'-9"									
1'-9"									
9"(3'-9")	(4')	(0.192)	(1'-9")						
3'-9"	4'	0,192	1'-9"						
4'-9"	6'	0,192	1'-9"						
5'-9"	8' (4')	0.192	1'-9"(3'-9")	(4')	(0.192)	(1'-9")			
6'-9"	6'	0.192	3'-9"	4'	0.192	1'-9"			
7'-9"	6'	0.20	4'-9"	6'	0.192	1'-9"			
8'-9"	6'	0.28	5'-9"	8' (4')	0.192	1'-9" (3'-9")	(4')	(0.192)	(1'-9")
9'-9"	6'	0.40	6'-9"	6'	0.192	3'-9"	4'	0.192	1'-9"

		FIG. 5-395.	102
SECTION LE BARREL /2*			BRIDGE NO. 62X04
	COUNTY PROJECT		SHEET NO.
	S.A.P.	062-593-004	46 /
RECTION	S.P.		D8
BARREL	S.P.		67
	S.P.		/ 63

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CONSTRUCTION NOTES

SEE STANDARD FIG. 5-395.101(A) AND FIG. 5-395.101(B) FOR ADDITIONAL DIMENSIONS AND CONSTRUCTION NOTES.

ON ALL END SECTIONS FOR WATERWAYS, USE DROPWALLS ON INLET AND OUTLET ENDS.

SEE STANDARD FIG. 5-395.115 FOR EMBANKMENT PROTECTION.

FINISH ALL EXPOSED EDGES OF CONCRETE WITH ${\rm V_{2''}}$ OR ${\rm V_{4''}}$ chamfer or radius unless otherwise noted.

(1) WITH DOUBLE BOXES LOCATE DROPWALL JOINTS BETWEEN END SECTIONS. SEE STANDARD FIG. 5-395.111 FOR ALTERNATE DROPWALLS. LIMITS OF EXCAVATION FOR DROPWALL TO BE APPROXIMATELY THE SAME AS DROPWALL DIMENSIONS, DROPWALL TO BE CONCRETE MIX NO. 1A43 OR 3Y43. FURNISHING AND INSTALLATION OF DROPWALL TO BE INCLUDED IN PRICE BID FOR END SECTIONS. DROPWALL NOT REQUIRED FOR NON-WATERWAY USE

(2) CHECK LOCATION TO DETERMINE WHETHER A TONGUE OR A GROOVE IS USED.

(3) SEE STANDARD FIG. 5-395.104(B) FOR LINTEL BEAM DETAILS.

(4) FILL HOLE WITH GROUT. GROUT SHALL CONSIST OF 1 PART CEMENT AND 2 PARTS SAND. USE TYPE 1A AIR ENTRAINED PORTLAND CEMENT. GROUT MIX SHALL HAVE A MAXIMUM SLUMP OF 4".

(5) 2" DIAMETER HOLE, 6" DEEP IN TOP OF THE SECTION WALL.

6 3'-6" MIN. TONGUE AND 3'-7" MIN. GROOVE FOR CULVERTS WITH 6'-0" SPANS. 5'-0" MIN. TONGUE AND 5'-1" MIN. GROOVE FOR CULVERTS WITH SPANS GREATER THAN 6'-0", CENTER TONGUE AND GROOVE ON € OF EACH APRON JOINT. TONGUE AND GROOVE JOINT ON ALL THREE SIDES OF APRON IS PERMISSIBLE.

AS AN ALTERNATE TO THE ONE LAYER WELDED WIRE FABRIC, PROVIDE TWO LAYERS OF REBAR OR WELDED WIRE FABRIC WITH THE STEEL AREA EQUAL TO HALF OF THE TEMPERATURE STEEL PER CODE REQUIREMENTS IN EACH FACE OF THE DROPWALL.

(8) APRON BOTTOM SLAB THICKNESS MAY BE 8" FOR CULVERTS WITH 6'SPANS ONLY. BOTTOM SLAB THICKNESS MAY BE INCREASED UP TO 2" MAX. PROVIDED COVER IS 11/2" MIN., 2" MAX.

(9) LONGITUDINAL REINFORCEMENT PERPENDICULAR TO THE CULVERT SPAN SHALL HAVE A MINIMUM OF 0.06 SQUARE INCHES PER PERIPHERAL FOOT ON ALL FACES OF THE BARREL.

LAYER OF WELDED WIRE FABRIC

LAYER OF WELDED WIRE FABRIC

FABRIC LAYER DETAIL

|DROPWALL (CAST-IN-PLACE |OR PRECAST)

WHEN MORE THAN ONE LAYER OF WELDED WIRE FABRIC IS USED TO OBTAIN THE REQUIRED REINFORCEMENT AREAS, THE WIRES OF THE WELDED WIRE FABRIC SHALL BE PLACED AS SHOWN.

ISIONS & AN REINFORCEMENT									
7	SECT	ION 3	ь4	SECT	ION 4	ьE	SECT	'ION 5	ÞC
15	Z	Ah	114	ZZ	Ah	115	ZZZ	Ah	00
-9")									
-9"									
-9"									
(3'-9")	(4')	(0.192)	(1'-9")						
-9"	4'	0.192	1'-9"						
-9"	6'	0.192	1'-9"						
-9"	8' (4')	0.192	1'-9"(3'-9")	(4')	(0.192)	(1'-9")			
-9"	6'	0.192	3'-9"	4'	0.192	1'-9"			
-9"	6'	0.20	4'-9"	6'	0.192	1'-9"			
-9"	6'	0.28	5'-9"	8' (4')	0.192	1'-9"(3'-9")	(4')	(0.192)	(1'-9")
-9"	6'	0.40	6'-9"	6'	0.192	3'-9"	4'	0.192	1'-9"

FIG. 5-395.104(A)

BRIDGE NO. 62X04 COUNTY PROJECT SHEET NO. SAP 062-593-004 47 D9 SP S.P. 63 S.P.

LINTEL BEAM BOTTOM REINFORCEMENT							
SPAN (FT.)	A 1	^A 2					
6	NO. 4 @ 1'-2"	NO. 4 @ 91/2"					
8	NO.4 @ 8"	NO.5 @ 8"					
10	NO.5 @ 8"	NO. 6 @ 71/2"					
12	NO.5 @ 6"	NO.6 @ 6"					
14	NO.6 @ 6"	NO.7 @ 6"					
16	NO.6 @ 6"	NO.7 @ 6"					
NOTE: MAXI	NOTE: MAXIMUM BAR SPACING GIVEN.						
REDUCE AS	NECESSARY						

<u>-</u>ب

2 REQUIRED

REVISION: 11-06-2013						Г	FIG. 5-395.1	04(B)
APPROVED: MARCH 24, 2011 Nances Joubenberg Slate Bridge Engineer	er		· ·		TITLE PRECAST CONCRETE END SECTION TYPE III - SINGLE OR DOUBLE BARREL FOR SKEWS UP TO 71/2*			BRIDGE NO. 62X04
No. Date Revisions	Арр. 	DRAWING NAME DESIGNED BY: MAW DRAWN BY: MAW CHECKED BY: MHD DATE: . PROJECT NO. 129594	J. SEH	I HEREBY CERTIFY THAT THIS PLAN, SPECIFICATION OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY LICENSED/FROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MINNESOTA. JEFFREY A JOHNSON DATE:7-7-15MN LIC. NO17280	RAMSEY COUNTY TCAAP BP1 PRECAST CONCRETE END SECTION TYPE III - SINGLE OR DOUBLE BARREL	COUNTY PF S.A.P. S.P. S.P. S.P.	ROJECT 062-593-004	SHEET NO. 48 D10 63

S:NPT/RNRamse/129594/5-final-dsgn/51-const-dwgs-CAD/20-Struct/Br62x04/dgn/cbr62x04.fig510. Default

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CONSTRUCTION NOTES

SEE STANDARD FIG. 5-395.101(A) AND FIG. 5-395.101(B) FOR ADDITIONAL DIMENSIONS AND CONSTRUCTION NOTES.

ALL END SECTIONS REQUIRE CURB ON LINTEL BEAM.

- GROUT SHALL CONSIST OF 1 PART CEMENT AND 2 PARTS SAND. USE TYPE 1A AIR ENTRAINED PORTLAND CEMENT. GROUT MIX SHALL HAVE A MAXIMUM SLUMP OF 4".
- (1) 3" DIA. HOLE THROUGH LINTEL BEAM AND 2" DIA. HOLE IN TOP OF WALL SECTION. PLACE NO. 8 DOWEL, 1'-O" LONG, IN HOLE AND FILL HOLE WITH GROUT.
- (2) CHECK THE LOCATION TO DETERMINE WHETHER A TONGUE OR A GROOVE IS USED. TONGUE AND GROOVE TO TERMINATE AT HAUNCH.
- (3) FOR SPANS UNDER 10'-0" USE NO.8 BARS. FOR SPANS OF 10'-0" TO 12'-0" USE NO.9 BARS. FOR 14'-0" AND 16'-0" SPAN. USE NO.10 BARS.
 (4) ALTERNATE BAR BEND MAY BE USED FOR B401.

Ś CAD' 20

CONSTRUCTION NOTES

GALVANIZE ALL FASTENERS AS PER SPEC. 3392.

BEFORE CULVERT PLANS ARE PREPARED, SAMPLES SHALL BE TAKEN FROM THE DRAINAGE AREA FOR PH DETERMINATION. THE SOIL AND WATER SHOULD HAVE A PH OF 6.5 OR MORE IF SHEET STEEL IS USED.

- (1) 2¹/₂" × ¹/₂" OR 2²/₃" × ¹/₂" CORRUGATED (12 GAGE) OR HEAVIER GALVANIZED STEEL SHEETS.
- (2) FASTEN THE STEEL SHEETS TO THE FRONT EDGE OF THE APRON WITH 3%" DIAMETER BY 4" LONG BOLTS AND APPROVED ANCHORAGES (10" ± CENTER TO CENTER, TO THE NEAREST VALLEY).
- (3) FASTEN THE 8"× 4"× ¾" OR 6"× 4"× ⅛" ANGLE WITH ¾" DIAMETER 4" LONG BOLTS, 1" 0.D. WASHER AND AN APPROVED ANCHORAGE (2'-O" SPACING).
- (4) FASTEN THE STEEL SHEETS TO THE FRONT EDGE OF THE APRON WITH %" DIAMETER 5" LONG BOLTS, NUT AND LOCK WASHER (10" ± CENTER TO CENTER, TO THE NEAREST VALLEY).
- (5) (12 GAGE) GALVANIZED CORRUGATED STEEL SHEET PILING, INTERLOCKING TYPE A.
- ⑥ ¾" DIA.× 1" LONG BOLT WITH NUT, TO LAP STEEL SHEETS.
- (7) STEEL SHEET PILING, SECTION NO. MP-112 OR EQUAL.
- (B) fill the voids as shown, with concrete or concrete grout, as approved by the engineer.

	·	FIG. 5-395	.111
ALLS RTS			BRIDGE NO. 62X04
	COUNTY PROJECT		SHEET NO.
	S.A.P.	062-593-004	49 /
	S.P.		D11
LS S	S.P.		67
	S.P.		63

30

CONSTRUCTION NOTES

THIS PLAN SHEET IS FOR CULVERT EMBANKMENT PROTECTION ONLY.REFER TO THE GRADING PLANS FOR ADDITIONAL RIPRAP OR OTHER SCOUR PROTECTION MEASURES.

RIPRAP SHALL COMPLY WITH SPECS. 2511 AND 3601.

- (1) FOR TYPE OF GEOTEXTILE FILTER MATERIAL REQUIRED, SEE SPEC. 3733. GEOTEXTILE STRIPS SHOULD BE CONTINUOUS WITHOUT OVERLAPS, EXCEPT FOR THE TOP STRIP. WHICH SHOULD SHINGLE VERTICAL STRIPS. THE FOR FORE SHOULD BE DUPLED TO THE TOP EDGE SHOULD BE BURIED TO PREVENT UNDERMINING.
- (2) IF THE DISTANCE BETWEEN DOUBLE BARRELS IS LESS THAN 2'-O" USE EITHER PEA ROCK OR LEAN MIX BACKFILL (SPEC. 2520) BETWEEN THE CULVERTS AS APPROVED BY THE ENGINEER. IF PEA ROCK IS USED PROVIDE APPROVED GROUT SEEPAGE CUTOFF CORE, THOLD IS" THICK BETWEEN THE CULVERT'S-TWO ENDS AND PROVIDE CLASS I GROUTED-RIPRAP IN LIEU OF CLASS III RIPRAP.
- ③ REFER TO THE GENERAL PLAN AND ELEVATION SHEET FOR THE DISTANCE BETWEEN BARRELS OF ADJACENT BOXES.

RIPRAP CLASS

RIPRAP CLASS	RIPRAP CLASS	т	W	
Ø	111	1'-6"	3'-0"	
	I٨	2'-0"	4'-0"	

		FIG. 5-395.	.115
ECTION RTS			BRIDGE NO. 62X04
	COUNTY PROJECT		SHEET NO.
	S.A.P.	062-593-004	50 /
	S.P.		D12
TION S	S.P.		67
	S.P.		63

[\R\Ramse\]29594\5-final-dson\51-const-dwas-f40\20-5+rust\Br62x0.

UTILITIES: EXISTING AND PROPOSED UTILITIES ARE SHOWN IN THE GRADING PLANS. PRIOR TO EXCAVATIION VERIFY THE LOCATION OF EXISTING FACILITIES AND EXERCISE CARE IN ADJACENT CONSTRUCTION.

EXCAVATION AND EARTHWORK: ALL EXCAVATION AND EMBANKMENT WORK SHALL CONFORM TO SPEC. 2451.

CONCRETE: ALL CONCRETE SHALL CONFORM TO SPEC. 2461.

TRANSVERSE CONSTRUCTION JOINTS IN FOOTING ARE PERMISSIBLE. KEYWAYS AND CONTINUOUS REINFORCEMENT ARE REQUIRED THROUGH THESE JOINTS.

THE THICKNESS OF THE ARCHITECTURAL CONCRETE TEXTURE VARIES WITH THE TEXTURE RELIEF PATTERN. THE STRUCTURAL CONCRETE 3Y43 QUANTITIES DO NOT INCLUDE THE MATERIAL WITHIN THE ARCHITECTURAL CONCRETE TEXTURE. CONCRETE NEEDED FOR THE TEXTURING IS INCIDENTAL.

POURING SEQUENCE: THE POURING SEQUENCE SHALL BE AT THE CONTRACTOR'S OPTION. SUBMIT SEQUENCE (WITHIN 7 CALENDAR DAYS) TO THE ENGINEER FOR APPROVAL PRIOR TO BEGINNING THE FIRST POUR.

CONSTRUCTION: CONSTRUCT IN ACCORDANCE WITH SPEC. 2411, EXCEPT AS NOTED. REFER TO STANDARD PLAN 5-297.624 (2 OF 6) FOR WALL CORNER DETAILS AND STEPPED FOOTING DETAILS.

REFER TO STANDARD PLAN 5-297.625 FOR WALL SHEAR LUG DETAILS. APPLY MEMBRANE WATERPROOFING SYSTEM PER SPEC.2481 TO BACK SIDE OF WALL TO COVER ALL THRU-BOLT FORM HOLES.

REINFORCING STEEL: USE REINFORCEMENT BARS CONFORMING TO SPEC. 3301, GRADE 60.

BARS MARKED WITH THE SUFFIX "E" TO BE EPOXY COATED. ALL BARS WHICH EXTEND OUT OF THE FOOTING AND ALL BARS WHICH ARE ABOVE THE FOOTING TO BE EPOXY COATED.

ALL BENT BAR DIMENSIONS ARE GIVEN OUT-TO-OUT.

REVISION:

Date

APPROVED: AUGUST 27, 2014

Nancy Daubenberger No.

Revisions

TEXTURE OR RUSTICATION.

DESIGN CRITERIA

BAR LAP

2'-5"

Z'-11"

3'-7"

4'-9"

6'-0"

7'-7"

9'-4"

8

10

11

EPOXY

2'-1"

3'-1'

3'-10"

5'-1"

6'-5"

8'-2"

10'-0'

THE CONTRACTOR HAS THE OPTION OF SUBSTITUTING 60'-O" LONG BARS FOR THE LONGITUDINAL FOOTING STEEL SHOWN. CHANGES IN THE BILL OF REINFORCEMENT ARE THE RESPONSIBILITY OF THE CONTRACTOR. PAYMENT WILL BE BASED ON QUANTITIES SHOWN.

THE CONSTRUCTION JOINT FOR CONCRETE PARAPET OR BARRIER MAY BE LOCATED AT TOP OR BOTTOM OF COPING, AT THE CONTRACTOR'S OPTION. PAYMENT WILL BE BASED ON QUANTITIES SHOWN, WHICH IS BASED ON CONSTRUCTION JOINT ABOVE COPING.

FOR VARIABLE STEM HEIGHTS, VARY THE LAP LENGTH OF THE VERTICAL REINFORCEMENT. MINIMUM LAP LENGTHS ARE GIVEN IN THE TABLE ON THIS SHEET. SMALLER BAR GOVERNS LAP LENGTH.

DOWEL BAR ASSEMBLIES: DOWELED JOINTS/CONSTRUCTION JOINTS ARE SHOWN ON STANDARD PLAN 5-297.624 (3 OF 6). THESE JOINTS ARE INCIDENTAL.

AT THE CONTRACTOR'S OPTION, CONSTRUCTION JOINTS MAY BE SUBSTITUTED IN LIEU OF CORK AND DOWEL JOINTS. REINFORCEMENT QUANTITIES WERE COMPUTED ASSUMING A CORK AND DOWEL JOINT BETWEEN EVERY PANEL. CHANGES IN THE BILL OF REINFORCEMENT ARE THE RESPONSIBILITY OF THE CONTRACTOR, AND NO ADDITIONAL PAYMENT WILL BE MADE. AT A MINIMUM, PLACE CORK AND DOWEL JOINTS EVERY 91'-6". PLACE A CORK AND DOWEL JOINT AT ALL VERTICAL FOOTING STEPS.

GEOMETRICS AND GRADES: DATA FOR BASELINE GEOMETRY IS TABULATED FOR WALL ALIGNMENT, SEE LAYOUT SHEETS. WALL ALIGNMENT REFERENCE IS ALONG FRONT FACE OF WALL. (8)

ON UP TO 10% SLOPES, THE CONTRACTOR HAS THE OPTION OF POURING FOOTINGS SLOPED OR STEPPED. ADDITIONAL CONCRETE VOLUMES AND CHANGES TO THE BILL OF REINFORCEMENT WHICH MAY RESULT FROM CONTRACTOR REQUESTED OPTIONS ARE THE RESPONSIBILITY OF THE CONTRACTOR. NO ADDITIONAL PAYMENT WILL BE MADE.

QUANTITIES ARE BASED ON ASSUMED TOP OF ROCK ELEVATION. ACTUAL TOP OF ROCK TO BE DETERMINED BY ENGINEER. SEE SHEET 5-297.624 (4 OF 6) FOR PAY LIMITS.

PILE LOADS:

THE PILE LOADS SHOWN IN THE PLANS AND THE CORRESPONDING NOMINAL PILE BEARING RESISTANCES (RO) WERE COMPUTED USING LRFD METHODOLOGY.

				SU	IMMAR	Y OF	QUAN	ITITIE	S FO	R RE	TAINI	NG WAL	LS		
		STRUC	TURAL RETE	REINFOR	CEMENT RS	9 STRU	CTURE /ATION	URAL ILL	CATE FILL 06	E9	SONC.	ы К. Ш.		KTILE R V	
•	PANEI	FOOTING	STEM	PLAIN	EPOXY	CLASS	CLASS	BACKF BACKF (CV)	GGREI BACKI ICV)	ANTI SRAFFI COATIN	FRACT IN)	ARCH. SURF AC INISH SINGLI		EOTE) FILTE	
				FOLIND	POUND		CIL YD			SQ FT	SQ FT	SQ FT		SQ YD	
	A1	4.6	7.9	417.0	746.0		00 10	00.0	5.5	46	46	46		29	
	A2	16.7	32.6	1656.0	2923.0				24	279	279	279		77.5	
	A3	16.7	32.6	1656.0	2923.0				24	279	279	279		77.5	
	A4	4.6	7.9	417.0	746.0		1		5.5	46	46	46		29	
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ТО	TAL	43	81	4146	7338				59	649	649	649		213	

NOTES

* I STRUCTURAL BACKFILL SPEC. 3149.2D2. SEE EARTHWORK SUMMARY AND TABULATIONS.

COMPACT TO 100% DENSITY IN ACCORDANCE WITH SPEC. 2105.3F1 UNLESS RECOMMENDED OTHERWISE BY THE SOILS ENGINEER.

(3) LIMITING CRITERIA.

- (CURVED FORMS MAY BE USED FOR ANY WALL WITH A RADIUS, BUT MUST BE USED ON WALLS WITH RADIUS LESS THAN 23 FEET.
- (5) DOES NOT INCLUDE DOWELED JOINT/CONSTRUCTION JOINT QUANTITIES, WHICH ARE INCIDENTAL. DOWELED JOINT/CONSTRUCTION JOINT DETAILS ARE SHOWN ON STANDARD PLAN 5-297.624 (3 OF 6).
- (6) QUANTITIES FOR THE FOUNDATION WITH AGGREGATE BACKFILL OPTION ONLY.
- () DOES NOT INCLUDE ADDITIONAL REINFORCING BARS AND STRUCTURAL CONCRETE (1A43) REQUIRED FOR STEPPED FOOTINGS, WHICH IS INCIDENTAL.
- (8) FOR RETAINING WALLS THAT ABUT A BRIDGE OR BRIDGE WING WALL, NOTE THAT THE DESIGNATION OF "FRONT FACE" MAY VARY FROM THE BRIDGE PLANS TO THE RETAINING WALL PLANS.

④ QUANTITY IN EARTHWORK QUANTITIES. ★ SEE EARTHWORK SUMMARY AND TABULATIONS.

$\overline{\ }$	SHEET INDEX
NO.	TITLE
	GENERAL NOTES & SUMMARY OF QUANTITIES
	GENERAL LAXOUT
	GENERAL PLAN & ELEVATION
	WALL REINFORCEMENT
	PANEL TABULATIONS
	MISCELLANEOUS DETAILS
	MODIFIED

App.	DRAWING N	AME		
_	DESIGNED BY:	MAW		
_	DRAWN BY:	MAW		
	CHECKED BY:	MHD	8 7 1 1	
	DATE:		SEH	
	PROJECT NO.	129594	JLII	

CURVED RETAINING WALLS ④ ALLOWABLE CHORD LENGTH								
MAXIMUM DEGREE OF CURVE	RADIUS	ALLOWABLE CHORD LENGTH	DEVIATION FROM TRUE RADIUS	MAXIMUM DEFLECTION ANGLE △				
4*-00'	14321	30'-6"	+ 1/4" (3)	1*-15'				
8*-00'	716'	21'-10"	± 1/2" (3)	1*-45'				
16°-30'	347'	15'-3"	± 1/2" (3)	2*-30'				
23°-00'	249'	12'-11"	± 1/2" (3)	2*-57'				
65°-30'	87'	7'-71/2"	± 1/2"	5*-00'(3)				
114°-30'	50'	4'-45/16"	± 1/4"	5*-00'(3)				
250°-00'	23'	2'-0"	± 1⁄8"	5*-00'(3)				

* DENOTES MODIFICATIONS FROM STANDARD PLAN

RETAINING WALL GENERAL NOTES AND SUMMARY OF QUANTITIES

				_
MSEV COLINITY	COUNTY PROJEC	Т	SHEET NO.	1
	S.A.P.	062-593-004	52	
TCAAP BP1	S.P.		RW1	
RETAINING WALL GENERAL NOTES AND	S.P.		67	
SUMMART OF QUANTITIES	S.P.		03	

JOINT NO	WALL DIMENSION (IN STATIONS)	x	Y	GROUND ELEVATION	TOP OF WALL	WALL HEIGHT (FT)	TOP OF FOOTING HEEL	BOTTOM OF FOOTING	PANEL LENGTH	PANE
RETAIN	NING WALLS									
1	1 + 20.00	550778.627	210694.474	882.92	882.92			070.00		
2	1 + 34.50	550769.276	210705.526	879.45	883.34	9.00	874.34	872.92	14.5	Al
3	1 + 65.00	550757.428	210733.403	871.85	883.83	16.00	867.83	866.41	30.5	A2
4	1 + 89.50	550756.580	210757.777	871.85	883.83		867.83	866.41	24.5	
5	2 + 20.00	550766.462	210786,410	879.45	883.34	16.00	867.83	866.41	30.5	A3
6	2 + 34.50	550775.023	210798.085	882.92	882.92	9.00	874.34	872.92	14.5	A4

NOTES

(E) DENOTES WALL EXPANSION JOINT LOCATIONS (CORK AND DOWEL JOINTS). (1) CAST-IN-PLACE CONCRETE END TRANSITION AND HEADWALL.

(2) 4" DIA PERF DRAINAGE SYSTEM TYPE II. SEE RETAINING WALL DETAILS, B910.

3 ARCH CONC TEXTURE (FRACTURED FIN). TYP. ALL PANELS.

(4) ARCH SURFACE FINISH (SINGLE COLOR). TYP. ALL PANELS.

			RW2
	COUNTY PROJECT	SHEET NO.	
	S.A.P. 062-593-0	04 53	
TCAAP BP1	S.P.	RW2	2
	S.P.		67
WORETE RETAINING WALL DETAILS	S.P.		03

38

UTILITY BLOCKOUT DETAIL

NOTES:

ARCHITECTURAL TREATMENT OPTION ON FRONT FACE OF RETAINING WALL. INCLUDING COPING OR HORIZONTAL REVEL OPTION TO BE DETERMINED BY MODOT.

① REFER TO PARAPET OR BARRIER SHEETS FOR ADDITIONAL INFORMATION INCLUDING Q BAR PLACEMENT DETAILS, AND PAYMENT.

FIELD CUT/ADJUST VERTICAL AND HORIZONTAL REINFORCEMENT AS NECESSARY TO CLEAR BLOCKOUT. PLACE REINFORCEMENT AS SHOWN.

3 MODIFY AS NEEDED FOR INTERRUPTION.

(THE THICKNESS OF THE ARCHITECTURAL CONCRETE TEXTURE VARIES WITH THE TEXTURE RELIEF. THE STRUCTURAL CONCRETE 3Y43 QUANTITIES DO NOT INCLUDE THE MATERIAL WITHIN THE ARCHITECTURAL CONCRETE TEXTURE. MATERIAL NEEDED FOR THE TEXTURING SHALL BE INCIDENTAL. SEE SPECIAL PROVISIONS 2411. TEXTURE RELIEF TO ADHERE TO FHWA CRASH BARRIER GUIDANCE WHENEVER THE WALL FACE IS INSIDE OR NEAR THE CLEAR ZONE.

(5) FOR RETAINING WALLS THAT ABUT A BRIDGE OR BRIDGE WING WALL, NOTE THAT THE DESIGNATION OF "FRONT FACE" MAY VARY FROM THE BRIDGE PLANS TO THE RETAINING WALL PLANS.

* DENOTES MODIFICATIONS FROM STANDARD PLAN

RETAINING WALL MISCELLANEOUS DETAILS

COUNTY PROJEC	SHEET NO.	
Ş.A.P.	56	
S.P.		RW5
S.P.		
S.P.		63

		T	SHEET NO
	COUNTY PROJEC	1	SHEET NO.
	S.A.P.	062-593-004	57
	S.P.		RW6
FOUS DETAILS	S.P.		
	S.P.		63

r\R\Ramse\129594\5-final-dsgn\51-const-dwgs-CAD\20-Struct\Br62x04\R\\stds\cbr62X04.s624_.3_.

NOTES:

THE MATERIALS AND PLACEMENT OF THE CORK AND DOWEL JOINT/ CONSTRUCTION JOINT (DOWEL BAR ASSEMBLIES, NO. 5 REINFORCING BARS, JOINT FILLER, AND JOINT WATERPROOFING) ARE INCIDENTAL.

THE CONTRACTOR SHALL ASSIGN TO THE REINFORCING BAR SUPPLIER THE RESPONSIBILITY OF SUPPLYING THE NECESSARY MATERIALS ASSOCIATED WITH THE DETAILS SHOWN ON THIS SHEET.

1) JOINT FILLER SHALL BE CORK SPEC. 2401.3E3.

- (2) AT THE CONTRACTOR'S OPTION, CONSTRUCTION JOINTS MAY BE SUBSTITUTED IN LIEU OF CORK AND DOWEL JOINTS. REINFORCEMENT QUANTITIES WERE COMPUTED ASSUMING A CORK AND DOWEL JOINT BETWEEN EVERY PANEL. CHANGES IN THE BILL OF REINFORCEMENT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR, AND NO ADDITIONAL PAYMENT WILL BE MADE. AT A MINIMUM, PLACE CORK AND DOWEL JOINTS EVERY 91'-6". A CORK AND DOWEL JOINT IS REQUIRED AT ALL VERTICAL FOOTING STEPS.
- ③ GREASE SHALL BE AN APPROVED HIGH PRESSURE TYPE THAT IS EFFECTIVE OVER THE FULL RANGE OF EXPECTED TEMPERATURES AND RESISTANT TO CHEMICAL ACTION.
- $\textcircled{\begin{tabular}{ll} \hline 0}$ dowel bar assembly must be placed perpendicular to joint and parallel to the wall face, and to each other.
- (5) SEE PANEL SHEETS FOR REINFORCING DETAILS.

RETAINING WALL MISCELLANEOUS DETAILS

	COUNTY PROJEC	Т	SHEET NO.
	S.A.P.	062-593-004	58
	S.P.		RW7
EOUS DETAILS	S.P.		
	S.P.		63

EOUS	DETAILS
AILS)	

COUNTY PROJ	SHEET NO.	
S.A.P.	062-593-004	60 /
S.P.		RW9
S.P.		
S.P.		63

	COUNTY PROJEC	SHEET NO.	
	S.A.P.	062-593-004	61
	S.P.		RW10
EOUS DETAILS	S.P.		
	S. A. P. 0 6 2 - 5 9 3 - 0 0 4 SHEET NO. INS DETAILS S) S.P. 6 1 RW10	63	

BA	R MAF		O. LEN	NGTH	A	LOCATION	WT.	DIMENSIONS & QUANTITIES	В/	BAR	MARK N	O. LENGTH	A	LOCATION	WT.	DIMENSIONS & QUANTITIES	BAR	MAR	K NO	LENGTH	A	LOCATION	WT.	DIMENSIO	IS & ES	BAR	MARK	NO. LENGTH	A	LOCATION	WT.	DIM	ENSIONS & JANTITIES	
h	= 9'	PA	ANELS:	THE ST.	A1		- N	L = 14'-6"	h	h = 1	6' P	ANELS:	A2			L = 30'-6"	h =	16'	PAI	NELS:	A3			L	= 30'-6*	h =	9,	PANELS:	A4				L =	14'-6"
AT	LOW END	D:	8'-	-5"		AT HIGH END:	9'-(D"	AT	TLOV	V END:	15'-6"		AT HIGH END:	16'-)" "	AT LO	OW END:	È	15'-6"		AT HIGH END:	16'-	-0"		AT L	OW END:	8'-5"		AT HIGH END	: 9'-0	•		
5		SP	READ FO	OTING R	EINFORCE	EMENT		DIMENSIONS			SF	READ FOOTING	REINFORC	EMENT		DIMENSIONS			SPRE	AD FOOTING	REINFOR	RCEMENT		DIMENSIO	INS		SF	PREAD FOOTIN	G REINFOR	CEMENT		DI	MENSIONS	
a /	F 501	1 1	4 16'-	11"	STR.	LONG T & B	247	SPREAD FOOTING		A	F 501	20 32' - 11"	STR.	LONG T & B	687	SPREAD FOOTING	A	F 501	20	32' - 11"	STR.	LONG T & B	687	SPREAD FO	DTING	A	F 501	14 16' - 11	STR.	LONG T & B	247	SPRE	AD FOOTING	G
91 E	F 502	2 1	5 5'-	5"	STR.	TRANS BOT	85	b 1'-5" e		в	F 502	8' - 5"	STR.	TRANS BOT	272	b 2'-9" e 1'-4"	В	F 502	31	8' - 5"	STR.	TRANS BOT	272	b 2'-9" e	1' - 4"	В	F 502	15 5' - 5"	STR.	TRANS BOT	85	b 1'-5"	е	
32:	F 503	3 1	5 5'-1	5"	STR.	TRANS TOP	85	c 1'-5" f		C	F 803	8' - 5"	STR.	TRANSTOP	697	c 1'-5" f 4'-21/2	С	F 803	31	8' - 5"	STR.	TRANS TOP	697	c 1'-5" f	4' - 2 1/2	С	F 503	15 5' - 5"	STR.	TRANS TOP	85	c 1'-5"	f	
11			- 22					d 5'-11" g 1'-71/4								d 8'-11" g 2'-111/4								d 8'-11" g	2' - 11 1/4						_	d 5' - 11"	g 1	' - 7 1/4
								STEM								STEM								STEM					_			1 11 10 10	STEM	
								a 1'-101/2 x 2'-1"								a 2'-2" x 2'-1"								a 2'-2" x	2' - 1"						-	a 1'- 10 1/2	X 2	2 - 1"
		1						j 1'-57/8 z								j 1'-93/8 z 5'-3"			_	-				j 1'-93/8 z	5' - 3"		-				-	J 1'- 57/8	Z	
		_	-					TOW THICKNESS 1' - 6"		_		_				TOW THICKNESS 1' - 6"	-	-	_			-		TOW THICKNESS	1'- 6"		-				-	DUSTICATION	ESS EE	- 6
								RUSTICATION THICK FF								RUSTICATION THICK FF	_						_	RUSTICATION THICK	FF	_	5007		OTTNO	FOROFILENT	_	RUSTICATION	IANTITIES	_
	-	FOOTIN	IG DOWE	ELS & STE	EM REINFO	DRCEMENT	-	QUANTITIES			FOOTI	NG DOWELS & S	TEM REINF	ORCEMENT	-	QUANTITIES		FO	OTING	DOWELS & S	STEM REI	NFORCEMENT	-	QUANTI	IES		FOOT	NG DOWELS 8	STEMREN	PORCEMENT	1	01	JANTITIES	
	F 504	4E 1	15 3'-1	0"	STR.	DOWEL FF	47	STRUCTURE CONCRETE (1A43)		D	F 504E	31 3'- 0"	STR.	DOWEL FF	97	STRUCTURE CONCRETE (1A43)	D	F 504	E 31	3' - 0"	STR.	DOWEL FF	97	STRUCTURE CON	CRETE (1A43)	D	F 504E	15 3'- 0"	STR.	DOWEL FF	47	STRUCTUR	ECONCRET	E (1A43)
E	F 505	5E 1	5 4'-	- 4"	0'- 10"	DOWEL BF	68	(FOOTING)		E	F 505E	31 10' - 6"	4' - 6"	DOWEL BF	339	(FOOTING)	E	F 505	E 31	10' - 6"	4' - 6"	DOWEL BF	339	(FOOTIN	G)	E	F 505E	15 4'-4"	0 - 10	DOWEL BF	68	000540	-OOTING)	L CY
F	F 506	BE	-			DOWEL BF	-	SPREAD 4.6 C	Y	F	F 706E	30 9'- 0"	7' - 7"	DOWEL BF	552	SPREAD 16.7 CY	F	F 706	E 30	9' - 0"	7' - 7"	DOWEL BF	552	SPREAD 16.7	CY	F	F 506E	-	OTO	DOWEL BF	-	SPREAD	4.0	
	S 401	1E 1	5 6'-:	2"	STR.	VERT FF	62			G	S 401E	31 13' - 2"	STR.	VERT FF	273		G	S 401	E 31	13' - 2"	STR.	VERTEF	2/3		DETE (OV(A))	G	5 401E	15 6-2	OTR.	VERTER	02	STRUCTUR	CONCRET	E (3V/3)
-	S 502	2E 1	5 6'-1	2"	STR.	VERT BF	96	STRUCTURE CONCRETE (3Y43)		н	S 502E	31 13' - 2"	STR.	VERT EF	426	STRUCTURE CONCRETE (3Y43)	н	S 502	E 31	13' - 2"	STR.	VERT BF	426	STRUCTURE CON	RETE (3143)	-	S 502E	15 0-2	STR.	VERTBE	90	STRUCTUR	(STEM)	E (3143)
- H	S 503	3E	- 0 - 1	0"	STR.	VERTBE	-	(STEM)		J	S 503E	-	SIR.	VERTER	-	(STEM)	J	5 503		401 01	51R.	VERIBE	245	(STEN		V	S 503E	15 10'- 8"	A' - 0"	TIE	167	70	(01111)	CY
	S 504	HE 1	5 10 - 1	0"	4-9	IIE	10/		· Y	K	S 504E	31 10 - 6	4 - 9	HODIZ SE	345		-	8 405	E 31	10 - 0	4-3 CTD	HOPIZ EE	641	DEINEORCEME	T (PLAIN)	1	S 405E	18 14' - 0"	STR	HORIZ EF	168	REINFOR	CEMENT (P	LAIN)
1 3	0 400		0 14 -	4"	11. A"	EVD IT THE	100	SPREAD 417 1		M	S 405E	20 7' 4"	1'. A"	EVDITTIE	153		M	S 506	E 20	7' - 4"	1" - 4"	EXP.IT TIE	153	SPREAD 1656	IB	M	S 506E	18 7' - 4"	1' - 4"	EXP JT TIE	138	SPREAD	417	LB
CH:	0 500			. 4	1 - 4	EXP JT TIE	130	SPREAD 417 L		NI NI	0.5075	10 7 0	41 01	EXPUTTIE	155	SFREAD 1050 LB	N	C 507	E 11	7' 0"	11 01	EXP IT TIE	07			N	S 507E	7'- 9"	1'- 9"	EXP JT TIE	-			
	0 0 500	/E	- /-	2"	1-9	EXP JT TIE		PEINEORCEMENT (EPOXY)		D	S 50/E	12 7 - 9 8' 2"	7-9	EXP JT TIE	97	REINEORCEMENT (EPOXY)	P	\$ 508		8'-2"	2'- 2"	EXP JT TIE	0/	REINFORCEMEN	IT (EPOXY)	P	S 508E	- 8'-2"	2' - 2"	EXP JT TIE		REINFOR	CEMENT (EI	POXY)
H	S 500	DE	6'-	11"	2 - 2	LERAIL DOWEL	-	746	B	0	S 500E	- 6'- 11"	3'-1"	LRAIL DOWEL	-	2923 IB	0	\$ 509	F .	6' - 11"	3'- 1"	J-RAIL DOWEL		2923	LB	a	S 509E	- 6'- 11	" 3' - 1"	J-RAIL DOWE	L -	74	6	LB
Ē	2 0 500		- 0-		0-1	U-IVAL DOWLL		140 2		~	0.0002	0.11	0.1	UTURE DOTTEE		2020		0.000	-				-											
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		_				1	-		\neg	-		_							-															

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NOTES: L = DENOTES PANEL LENGTH. FF = DENOTES FRONT FACE. BF = DENOTES BACK FACE. EF = DENOTES EACH FACE. DWL = DENOTES DOWEL. BARS MARKED WITH THE SUFFIX "E" ARE EPOXY COATED. x = PROJECTION OF BAR E INTO STEM. z = PROJECTION OF BAR F INTO STEM.

RETAINING WALL PANEL TABULATIONS (IV:2H SLOPED FILL) STANDARD PLAN 5-297.627 1 OF 3 COUNTY PROJECT SHEET NO. S.A.P. 0 6 2 - 5 9 3 - 0 0 4 S.A.P. 0 6 2 PANEL TABULATIONS S.P. SHEET NO. S.P. 6 3

SPREAD FOOTING DIMENSIONS AND SOIL STRESSES 1(V) : 2(H) SLOPED FILL

	WALL	GEOMETRI	CS AND DAT	A - SPREAD F	OOTING		QUANTITI	ES PER FOOT	- SPREAD F	OOTING		EQUIV	ALENT UNIFOR	RM BEARING S	TRESS	EQUIVALEN (TRAPEZOIDAL)	T VARIABLE BEARING STRESS
							STRUCTURA	L CONCRETE	REINFOR	RCEMENT		SER	VICE	STREM	NGTH 1	STRE	NGTH
STEM HEIGHT DIM. h	STEM WIDTH DIM. a	TOE WIDTH DIM. b	FOOTING THICKNESS DIM. C	FOOTING WIDTH DIM. d	SHEAR KEY SIZE DIM. e	SHEAR KEY LOCATION DIM. f	1A43 FOOTING (CU. YD.)	3Y43 STEM (CU. YD.)	PLAIN (POUND)	EPOXY (POUND)	WALL DETAILING SCHEME	EFFECTIVE WIDTH B'	EFFECTIVE STRESS KSF	EFFECTIVE WIDTH B'	EFFECTIVE STRESS KSF	STRESS AT TOE KSF	STRESS AT HEEL KSF
5	1'-8 ¹ /2"	9"	1'-5"	3'-3"	N.A.	N.A.	0.18	0.30	14.9	31.1	SHORT	2'-45%"	1.33	1'-10¾"	1.85	2.46	0.10
6	1'-9"	11"	1'-5"	3'-8"	N.A.	N.A.	0.20	0.36	15.7	34.7	SHORT	2'-75%"	1.53	2'-01/2"	2.14	2.86	0.03
7	1'-91/2"	1'-1"	1'-5"	4'-5"	N.A.	N.A.	0.24	0.43	19.6	39.0	SHORT	3'-31/2"	1.63	2'-71/2"	2.28	3.03	0.19
8	1'-10"	1'-3"	1'-5"	5'-2"	N.A.	N.A.	0.28	0.49	23.4	42.5	SHORT	3'-111/2"	1.75	3'-25/8"	2.43	3.21	0.34
9	1'-101/2"	1'-5"	1'-5"	5'-11"	N.A.	N.A.	0.32	0.56	27.2	46.1	SHORT	4'-73/4"	1.86	3'-9%"	2.59	3.40	0.49
10	1'-11"	1'-7"	1'-5"	6'-8"	N.A.	N.A.	0.36	0.63	28.8	52.2	MEDIUM	5'-3%"	1.99	4'-51/8"	2.76	3.59	0.64
11	1'-111/2"	1'-9"	1'-5"	7'-5"	N.A.	N.A.	0.40	0.70	35.9	61.5	MEDIUM	6'-01/8"	2.11	5'-01/2"	2.93	3.79	0.78
12	2'-0"	1'-11"	1'-5"	8'-2"	N.A.	N.A.	0.44	0.78	44.3	65.2	MEDIUM	6'-8 ¹ /2"	2.24	5'-7%"	3.11	4.00	0.92
13	2'-01/2"	2'-1"	1'-5"	8'-11"	N.A.	N.A.	0.48	0.85	54.3	69.5	MEDIUM	7'-4%"	2.36	6'-31/4"	3.29	4.20	1.07
14	2'-1"	2'-3"	1'-5"	8'-11"	1'-4"	3'-71/2"	0.55	0.93	54.3	75.7	MEDIUM	7'-17/8"	2.61	5'-101/2"	3.65	4.76	0.86
15	2'-11/2"	2'-6"	1'-5"	8'-11"	1'-4"	3'-11"	0.55	1.01	54.3	85.7	MEDIUM	6'-105%"	2.81	5'-51/4"	3.98	5.26	0.58
16	2'-2"	2'-9"	1'-5"	8'-11"	1'-4"	4'-21/2"	0.55	1.09	54.3	95.7	MEDIUM	6'-71/8"	3.05	4'-113/8"	4.36	5.81	0.24
17	2'-21/2"	3'-0"	1'-7"	9'-2"	1'-6"	4'-6"	0.64	1.17	61.6	108.0	MEDIUM	6'-61/4"	3.30	4'-8%	4.83	6.44	0.00
18	2'-3"	3'-6"	1'-7"	9'-6"	2'-0"	5'-01/2"	0.72	1.25	67.4	118.5	MEDIUM	6'-10¾"	3.35	4'-101/2"	4.98	6.65	0.00
19	2'-31/2"	3'-9"	1'-9"	10'-1"	2'-2"	5'-4"	0.85	1.33	68.0	136.3	TALL	7'-35%"	3.51	5'-21/4"	5.24	6.99	0.00
20	2'-4"	4'-0"	1'-9"	10'-7"	2'-6"	5'-71/2"	0.96	1.42	75.0	151.7	TALL	7'-8 ∛ 4"	3.63	5'-6"	5.42	7.23	0.00
21	2'-41/2"	4'-4"	1'-9"	11'-1"	2'-6"	6'-0"	1.00	1.50	79.7	160.8	TALL	8'-11/2"	3.71	5'-9"	5.57	7.43	0.00
22	2'-5"	4'-8"	1'-11"	11'-8"	2'-6"	6'-4 ¹ /2"	1.11	1.59	82.7	180.0	TALL	8'-6¾"	3.84	6'-0 /2"	5.78	7.71	0.00
23	2'-51/2"	5'-0"	2'-0"	12'-4"	2'-6"	6'-9"	1.20	1.68	99.1	210.3	TALL	9'-2"	3.91	6'-6'/4"	5.85	7.80	0.00
24	2'-6"	5'-4"	2'-2"	12'-10"	2'-6"	7'-11/2"	1.32	1.77	111.0	233.9	TALL	9'-5%"	4.06	6'-8"	6.14	8.19	0.00
25	2'-61/2"	5'-8"	2'-3"	13'-4"	2'-6"	7'-6"	1.41	1.87	114.3	266.3	TALL	9'-9¾"	4.17	6'-103/8"	6.37	8.50	0.00
26	2'-7"	6'-0"	2'-5"	13'-11"	2'-6"	7'-101/2"	1.55	1.96	120.3	302.3	TALL	10'-3"	4.30	7'-1¾"	6.59	8.79	0.00
27	2'-71/2"	6'-4"	2'-6"	14'-6"	2'-6"	8'-3"	1.65	2.06	109.9	371.1	TALL	10'-8¾"	4.39	7'-5%"	6.74	8.98	0.00

N.A. = NOT APPLICABLE

NOTE: EPOXY REINFORCEMENT QUANTITY ASSUMES A CORK AND DOWEL JOINT IS USED ON BOTH PANEL ENDS. THE QUANTITY MUST BE ADJUSTED WHEN CONSTRUCTION JOINTS ARE USED.

					F	REINFORCEMENT - SPE	READ FOOTING						
				CTEN.	STEM		FOOTING						
				HEIGHT	DOWEL SIZE AND	D TOE (BOTTOM TRANSVERSE)	HEEL (TOP TRANSVERSE)	LONGITUDINAL (TOP AND BOT.)					
				h	SPACINO	BAR SIZE & SPA.	BAR SIZE & SPA.	BAR SIZE & SPA.					
				5	5 e 12"	5 e 12"	5 e 12"	5 e 12"					
				6	5 e 12"	5 e 12"	5 e 12"	5 c 12"					
				7	5 @ 12"	5 e 12"	5 c 12"	5 @ 12"					
				8	5 @ 12"	5 c 12"	5 e 12"	5 e 12"					
				9	5 c 12"	5 e 12"	5 e 12"	5 c 12"					
				10	5 c 12"	5 0 12"	5 @ 12"	5 @ 12"					
				11	5 e 12"	5 e 12"	6 @ 12"	5 e 12"					
				12	5 @ 12"	5 0 12"	7 @ 12"	5 e 12"					
				13	5 @ 12"	5 @ 12"	8 @ 12"	5 e 12"					
				14	5 0 12"	5 0 12"	8 @ 12"	5 e 12"					
				15	5 @ 12"	5 e 12"	8 @ 12"	5 e 12"					
				16	5 c 12"	5 @ 12"	8 @ 12"	5 @ 12"					
				17	6012"	5 @ 12"	9 0 12"	5012"					
				18	6 0 12"	6 c 12"	9 0 12"	5 0 12"					
				19	7012"	6 c 12"	9 0 12"	5 6 12"					
				20	7 c 12"	6 @ 12"	9 e 12"	5 @ 12"					
				21	7 0 12"	6 @ 12"	9 @ 12"	5012"					
				22	8 @ 12"	6 @ 12"	9 0 12"	5012"					
				23	8012"	6012"	10 0 12"	5012"					
				24	8012"	7 0 12"	10 0 12"	5 6 12"					
				25	10 0 12"	7 0 12"	10 0 12"	5 0 12"				RET	VISED
_				20	11 0 12"	5 0 12"	10 0 12"	5 0 12"					
F	EVISIO	N:		21	11 6 12."	5012"	10 6 12."	5012"		20 33	ΔΛ .		
	DDDDOV		T 27 2014								1 1	0	
ľ	1/		21,2014								Lutur	Ky APP	PROVED
L	Nai	reef Ja	ebenbergen							The second	O totte tip	1 1	8-27-20
		STATE BRIDG								OF TRAN	STATE DESIGN EN	GINEER	
	No. C	ote Re	visions		App.	DRAWING NAME				I HEREBY CERTIFY THAT TH OR REPORT WAS PREPARE	IS PLAN, SPECIFICATION		DANAG
F	_									DIRECT SUPERVISION AND	HAT I AM A DULY	The second s	
_					D	ESIGNED BY:	WAW		1	LICENSED PROFESSIONAL E	NGINEER UNDER THE		
						RAWN BY	WAW		I	LANS OF THE STATE OF M	Incourt.		
Г									►				
⊢	\rightarrow				C	HECKED BY:	MHD			414			RETAIN
					D	ATE:				JEFFREY A	JOHNSON		SPREAD
					P	ROJECT NO. 12	9594			DATE: 7-7-15 MN I	IC. NO. 17280		
						12 12 110.	JUJT		-				

PM 1:33:57

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