

Comprehensive Stormwater Management Plan (CSMP)

Prepared for the Redevelopment of the
Rice Creek Commons (formerly Twin
Cities Army Ammunition Plant) Project.



Prepared for:
Ramsey County Public Works

1425 Paul Kirkwold Drive
Arden Hills, MN 55112-3933



Responsive partner.
Exceptional outcomes.

Kimley-Horn
2550 University
Avenue West
Saint Paul, MN 55114
Phone: 651.645.4197

Prepared by:

Wenck Associates, Inc.
1800 Pioneer Creek
Center
Maple Plain, MN 55359
Phone: 763.479.4200

Approved September 23, 2015



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Executive Summary

INTRODUCTION

Ramsey County (County) and the City of Arden Hills (City) working together, under a Joint Powers Agreement, to provide this Comprehensive Stormwater Management Plan (CSMP) to Rice Creek Watershed District (RCWD) for the Rice Creek Commons (formerly Twin Cities Army Ammunition Plant) redevelopment Project (**Figure 2-2**).

Report Purpose: The goal of this CSMP is to communicate how the development, recently renamed Rice Creek Commons, will meet RCWD Rule C requirements using a regional stormwater approach. This CSMP presents the approach for meeting the Water Quality Treatment and Peak Stormwater Runoff Control requirements. The RCWD Rules adopted at the time of printing were effective on December 1, 2014.

Existing conditions were modeled, per requirements in Rule C, and compared against the proposed conditions at outfalls. **Figure 6-2** displays tributary areas by outfall in 2012. The proposed conditions outfall are shown in **Figure 6-3**. The proposed conditions are presented in two stages; completion of the public infrastructure improvements, and the fully developed conditions using landuse assumptions in the Master Plan (**Figure 3-1**) and construction of a regional stormwater system (**Figure 4-5**). The pollutant removal efficiency of the proposed regional ponding system was evaluated at outfalls for the proposed conditions plus an interim condition considered to be the worst case scenario.

Rule C.6: Some outlot purchase agreements will include the requirement of designing and constructing an infiltration device or a stormwater pond that is consistent with this CSMP (Table E-1), specifically Outlots A, J, and K. The remaining outlots in Areas 1 and 2 in **Figure 5-2** will be exempt from RCWD Rule C.6(d)(2) due to MPCA concerns relative to shallow (Unit 1) groundwater is impacted by solvent releases.

The proposed ponding system will reduce pollutant loads from stormwater before discharging to the Resource of Concern (ROC) from the proposed site conditions by more than 50%, meeting RCWD standards. Each outfall under proposed conditions was evaluated for each Resource of Concern (**Figure 6-3**) and tabulated in water quality technical memorandum (P8) provided in Appendix A. Bulleted below are the total TP % reductions summarized by scenario by ROC.

- Rice Creek TP reduced by %
 - public infrastructure improvements (interim scenario 1): 65% total
 - worst case (interim scenario 2): 55% total
 - fully developed conditions (scenario 3): 64% total
- Round Lake TP reduced by %
 - public infrastructure improvements (interim scenario 1): 53% total
 - fully developed conditions (scenario 3): 60% total

This CSMP assumes that before stormwater discharges from the land being developed to the regional system, some method of pre-treatment or grit removal must be installed and maintained by the developer (e.g., flow through device, vegetated swale, vegetated filter strip, SAFL Baffle, off-line deep sump catch basins, The Preserver™, etc.).

Rule C.7: Stormwater pond design details are included in the Stormwater Pond Data Summary Tables included in the Appendix of the HydroCAD Technical Memorandum.

The Site, under proposed fully developed conditions, has discharge rate control measures proposed to meet RCWD's peak stormwater runoff control requirements through a combination of onsite infiltration and wet detention ponds.

Rule C.8: As permits are prepared to construct stormwater ponds and outlet designs are finalized, bounce and inundation periods will be documented by applicants.

Rule C.9: Likewise, as permits are prepared, the design criteria will be followed by applicants.

Rule C.10: It is anticipated that the Site will be developed on an outlot by outlot basis. Preliminary outlots have been identified on the concept preliminary plat (**Figure 5-2**). The regional stormwater system will be preserved through easements or public ownership. Outlot C, the Natural Resources Corridor, will remain in Ramsey County ownership initially and be covered by easements for drainage/utilities, and and covenants for the wetland mitigation. Eventually the stormwater system within the Natural Resources Corridor, will be owned by the City as they will be responsible for ongoing maintenance.

A drainage and utility easement will also cover an approximate 80-ft wide area along the west edge of Outlots I, J, and K to fully meet stormwater water quality requirements.

Table E-1: Stormwater Requirements for Future Development Outlots

Outlot	Subbasin(s) (per Figure 6-1)	Rate Control (per Rule C.7)	Infiltration Required (per Rule C.6)
Spine Road	24, 25, 26	*	Per CSMP (interim conditions)
Outlot C	3, 8, 10, 11, 12, 13, 14, 16, 17		
Outlot H	4, 5, 6, 7		
Rice Creek Commons Old Hwy 8 Extension Road	27	*	✓ per table 5-1 of CSMP
Outlot A	22	*	✓ per table 5-1 of CSMP
Outlot B	16, 17, 19	* Pond P-14, per CSMP	
Outlot D	15	*	
Outlot E	1, 3, 8, 9	*	
Outlot F	1, 3	* Pond P-1, per CSMP	
Outlot G	2, 7	* Pond P-3, per CSMP	
Outlot I	18, 5	*	
Outlot J	18	*	✓ per table 5-1 of CSMP
Outlot K	18	*	✓ per table 5-1 of CSMP
Outlot L	20	*	
County Road H/I-35W Interchange	27, 28, 29, 31	✓ +	✓ +

✓ Development plans submitted in the future are required to meet Rice Creek Watershed District Rules for this stormwater parameter.

* CSMP submitted compares peak flow rates (existing compared to fully developed conditions) entering the Rice Creek Commons ROC (Rice Creek/Long Lake and Round Lake) in aggregate. The modeling completed sums flows from multiple subwatersheds which drain to the same location (e.g. Rice Creek).

+ The CSMP is based on 30% design, and these proposed ponds did not provide sufficient water quality treatment volume to meet RCWD rules, nor were infiltration BMPs incorporated in the stormwater management design, as required per this CSMP.

This CSMP will provide developers with technical details to complete site design to comply with RCWD Rule C using the regional system to be constructed by Ramsey County and maintained by the City of Arden Hills. Developers that choose to exceed the impervious limits designed to for that outlet, as summarized in Table E-2 and **Figure ES-1**, will be required to address any additional stormwater requirements within that outlet at the developers expense. Any such stormwater management facilities will also be maintained by the owner of the outlet.

Table E-2: Maximum Impervious Surface by Outlet

Outlet ID	Total Outlet Area (acres)	Maximum Impervious Area (acres)	Additional breakdowns
Outlet A	40.1	34.0	
Old Highway 8 extension Road	4.8	4.8	
Spine Road	24.0	24.0	
Outlet B (Creek)	45.9	18.1	15.2 acres in residential
Trail Dedication	1.1	0.7	0.4 acres residential, remaining in trails
Outlet C (NRC)	47.6	5.2	2.1 acres in civic, remaining in recreational and trails, realigned Creek
Outlet D (Town)	58.8	28.6	16.9 acres in residential, 9.7 acres in mixed use, 1.2 acres in commercial, remaining in recreational
Outlet E (Hill)	73.3	27.0	23.9 acres in residential
Outlet F (Flex Business East)	28.7	23.5	
Outlet G (Flex Business South)	31.7	21.6	
Outlet H (NRC)	3.5	0.4	0.4 acres in trails
Outlet I (Flex Business North)	18.2	15.4	
Outlet J (Corporate)	20.0	17.0	
Outlet K (Retail)	20.0	16.5	16.4 acres in retail, remaining in recreational
Outlet L (Creek Meander)	2.7	0.2	0.2 acres in trails, realigned creek

1.0 CSMP Goals and Intended Use

1.1 GOALS

As allowed for under RCWD Rule C.5 (f), this CSMP has been prepared as an alternative means to meet the requirements of Rule C.6 (Water Quality Treatment) and Rule C.7 (Peak Stormwater Runoff Control) for redevelopment of the Site, which will be done in various stages. Upon RCWD Board approval of this CSMP, it is agreed that the water quality treatment volumes and peak runoff rate controls standards for the 427 acre Rice Creek Commons site and the 7.1 acres of MnDOT right-of-way adjacent to the site (subwatersheds 28 & 29) will be met in aggregate for each ROC drainage area given the conditions documented herein.

The development of this CSMP is intended to streamline regulatory permit approvals saving the RCWD and developer time and money as development is proposed. This alone should make the land more attractive and valuable. The County and the City have made a considerable investment in the Site by undertaking cleanup of the site, preparing development planning documents, providing public notice and gathering public input, building public utilities (sewer, water, storm, etc.), constructing ponds and mitigation wetlands. The Site has a Master Plan so that the Site is developed in a responsible way that creates open green space, centralized stormwater management features that creates community around protection of our natural resources and provides a forum for continued education of water management and sustainable development.

By planning for development of Rice Creek Commons using a “campus” approach, the regulatory requirements can be “leveled” so that future applicants within different geographic regions of the project area aren’t faced with more challenges in meeting the requirements. Regional treatment also allows for more efficient treatment and maintenance, and helps ensure that the entire site meets RCWD requirements, from first to last development.

The JDA will use this plan to review development plans for conformance with the proposed impervious surface areas by outlot, assumptions documented in this CSMP and various land uses identified in the Master Plan. Permits will still be required from the RCWD for individual development projects to confirm compliance with this CSMP and to meet other applicable RCWD rules. More information on responsibilities are provided in Section 6.0.

1.2 DESIGN PRIORITIES

Given the overlap and complexity of these various requirements, design priorities were established to meet as many of these requirements within the Natural Resources Corridor as possible, with the following priorities in this order:

1. Accommodate all wetland replacement for public infrastructure impacts (Spine/Old Highway 8 extension Roads and utilities, stormwater ponds, associated site grading, maintenance access, County Road I/Old Highway 8 extension Road access)
2. Accommodate all required stormwater rate control and infiltration for Spine/Old Highway 8 extension Roads
3. Accommodate stormwater rate control for Rice Creek Commons development

4. Provide maintenance access to stormwater features in form of a recreational, multi-use trail
5. Provide wetland mitigation for site development to the extent feasible
6. Provide infiltration for site development (residential, retail, and commercial parcels) to the extent feasible
7. Accommodate other passive recreation opportunities where feasible

1.3 RICE CREEK WATERSHED PERMIT PHASING PLAN

Some phases of redevelopment are County projects with more predictable schedules, while other phases are dependent on developer interest on an unknown schedule. All the projects have regulatory requirements to meet.

1.3.1 RCWD Permits Issued in 2013

The redevelopment site work activities commenced after issuance of the RCWD permit 13-0023 (April 30, 2013) to Bolander for demolition and remediation activities. Activities that will be completed under RCWD permit 13-0023, include:

- ▲ the removal of railroad track, fencing, bituminous pavement, concrete sidewalk, underground storm sewer, underground sanitary sewer, underground steam, underground condensate, underground watermain, and buildings is over 95% complete. It will be completed by October 15, 2015.
- ▲ Soil remediation and cleanup activities to bring the site to within MPCA residential SRV's is approximately 75% complete. It will be completed by October 15, 2015.
- ▲ Stabilization to meet the NPDES construction stormwater permit requirements is ongoing, and is addressed as certain areas are completed. It will be 100% complete by October 15, 2015.

This work is considered a Public Infrastructure Improvement.

1.3.2 RCWD Permit Applications Planned for 2015-2016

Nearby highway and bridge improvements are needed whether Rice Creek Commons is redeveloped or not. Various periphery highway and bridge improvement projects are outlined in the Ramsey County Public Works Department Transportation Improvement Plan (such as Interstate I-35W, Highway 10, CSAH 96, County Road H and County Road I.) Ramsey County plans to apply for separate permit applications and approvals for the following public infrastructure projects. The timing is subject to change due to funding, coordination, securing permits and approvals for projects:

- ▲ Summer 2015
 - ▲ Comprehensive Stormwater Management Plan (CSMP)
 - ▲ Creek Meander Permit (Rules D, E, F, G)
 - ▲ CSAH 96/I-35W interchange (Ramsey County/SEH)
- ▲ Fall 2015
 - ▲ Rice Creek Commons Spine Road (CSAH 96 to County Road H), regional stormwater/wetland, and general site Grading for portion south of Rice Creek (Rules C, D, E, F, G)
- ▲ Spring 2016
 - ▲ County Road H/I-35W Interchange (Rules C, D, E, F)

- ▲ Undetermined
 - ▲ County Road I/I-35W roundabout (including Old Highway 8 extension Road between County Road I and northern most edge of the parcel north of Rice Creek)
 - ▲ Rice Creek Commons Old Highway 8 extension Road (within the portion north of Rice Creek parcel). The timing is dependent on when the parcels are sold/developed north of Rice Creek.

1.3.3 RCWD Permit Applications Planned for 2015 and Beyond

Dependent on developer interest, there may be permit applications for portions of the 427-acre site. The location, size and type of development applications (mixed residential, commercial, and light industrial) are unknown, but all development will be consistent with the Master Plan (**Figure 3-1**). Developers will need to prepare the necessary documents to document deviations from this CSMP as well as additional document required to obtain a RCWD permit (e.g., an erosion and sediment control plan may be required to be developed given Rule D requirements). Some development applications may request an extended stormwater management permit for phased development in accordance with Rule C.13. This CSMP will be available to developers as they prepare the necessary applications. Developers will be required to comply with RCWD Board Approved CSMP for each respective project proposed.

1.4 DEVELOPER SUBMITTAL SEQUENCING

Future developers will be responsible to first submit a proposal to the JDA. The submittal would document which outlot (or portion of an outlot) will be developed. The application must document that the land use proposed is consistent with the Master Plan used in developing this CSMP (**Figure 3-1**). If the Land Use is different, the developer must demonstrate how the proposed use will meet the impervious assumptions for that outlot in the CSMP.

If development is within CSMP requirements, the developer application must include:

- ▲ Name the outlot(s) or portion of an outlot requested (per **Figure 5-2**)
- ▲ Document the total area requested broken down by roof area, pervious and impervious areas,
- ▲ Preliminarily communicate the method of proposed grit removal (e.g., flow through device, vegetated swale, vegetated filter strip, SAFL Baffle, off-line deep sump catch basins, The Preserver™, etc.) before stormwater discharges from the land being developed to the regional system,
- ▲ Explicitly name the stormwater pond(s) the site is tributary (P-X per **Figure 4-5**), and,
- ▲ The applicant must submit existing and proposed topography and describe the infrastructure proposed to convey stormwater.

If the development exceeds the CSMP maximum impervious, the developer application must demonstrate how the additional runoff will be treated to meet RCWD rules. Developers that choose to exceed the impervious limits designed to for that outlot, as summarized in Table E-2, will be required to address any additional stormwater requirements within that outlot at the developers expense. Any such stormwater management facilities will also be maintained by the owner/developer of the outlot.

1.5 JOINT DEVELOPMENT AUTHORITY

Ramsey County (County) and the City of Arden Hills (City) formed a partnership to redevelop the 427 acres. The Joint Powers Agreement (JPA) defines what portions of the redevelopment project are the responsibility of the County, the City, jointly the County and the City, and the developer responsibilities. The JPA calls for the establishment of a Joint Development Authority (JDA) or “governing body.” The JDA Board consists of two members from the Ramsey County Board of Commissioners, two members from the City Council, and one appointed resident from Arden Hills who serves a two-year appointment as the JDA Chair. The main duties of the JDA Board are to implement the Rice Creek Commons Master Plan and oversee Rice Creek Commons redevelopment process and activities.

1.5.1 JDA Use of the CSMP

After the RCWD Board approves this CSMP, the JDA Board may need to adjust the requirements for a Development Site. During the Rice Creek Commons redevelopment process, the JDA Board may need to adjust the requirements for a Development Site. The JDA Board will follow the amendment process defined in the JPA. JDA approved requirements related to meeting RCWD’s Water Quality Treatment (Rule C.6) and Peak Stormwater Runoff Control (Rule C.7) will be incorporated into the development agreements for each Development Site.

The County and JDA Board will make the CSMP available to interested Developers to promote conformance with RCWD Board Approved CSMP for each respective project.

1.5.2 JDA Responsibilities Administering the CSMP

Three outlots (A, J, K) include the requirement of designing and constructing an infiltration device or a stormwater pond that is consistent with this CSMP (Table 1-1).

Stormwater pond design details are included in the Stormwater Pond Data Summary Tables 6-1 through 6-5, included as part of Appendix A: HydroCAD Technical Memorandum. These tables summarize for each pond, the:

- ▲ Tributary subwatershed area,
- ▲ Outlet elevation (normal stormwater pond elevation),
- ▲ Emergency overflow elevation,
- ▲ Dead pool storage (the volume that remains in the ponds “dead storage” for settling until the next storm arrives)
- ▲ HydroCAD model predicted high water level (HWL), live storage volume, and peak discharge rate for the following design rainfall events (NOAA Atlas 14)
 - 100-year 24-hour
 - 10-year 24-hour
 - 2-year 24-hour
- ▲ Stormwater control structures (e.g., orifice, weir, culvert)
- ▲ Stormwater conveyance details (e.g., overland flow channel, pipe details)

The JDA will provide developers with technical details to complete their respective site designs to comply with the assumption used to develop this CSMP.

Table 1-1: Stormwater Requirements for Future Development Outlots

Outlot	Subbasin(s) (per Figure 6-1)	Rate Control (per Rule C.7)	Infiltration Required (per Rule C.6)
Spine Road	24, 25, 26	*	Per CSMP (interim conditions)
Outlot C	3, 8, 10, 11, 12, 13, 14, 16, 17		
Outlot H	4, 5, 6, 7		
Rice Creek Commons Old Hwy 8 Extension Road	27	*	✓ per table 5-1 of CSMP
Outlot A	22	*	✓ per table 5-1 of CSMP
Outlot B	16, 17, 19	P-14, per CSMP	
Outlot D	15	*	
Outlot E	1, 3, 8, 9	*	
Outlot F	1, 3	P-1, per CSMP	
Outlot G	2, 7	P-3, per CSMP	
Outlot I	18, 5	*	
Outlot J	18	*	✓ per table 5-1 of CSMP
Outlot K	18	*	✓ per table 5-1 of CSMP
Outlot L	20	*	
County Road H/I-35W Interchange	27, 28, 29, 31	✓ +	✓ +

✓ Development plans submitted in the future are required to meet Rice Creek Watershed District Rules for this stormwater parameter.

* CSMP submitted compares peak flow rates (existing compared to fully developed conditions) entering the Rice Creek Commons ROC (Rice Creek/Long Lake and Round Lake) in aggregate. The modeling completed sums flows from multiple subwatersheds which drain to the same location (e.g. Rice Creek).

+ The CSMP is based on 30% design, and these proposed ponds did not provide sufficient water quality treatment volume to meet RCWD rules, nor were infiltration BMPs incorporated in the stormwater management design, as required per this CSMP.

The HydroCAD model will be maintained by the County's consultant once per a five-year cycle until fully developed conditions is achieved to reflect the as-built conditions at Rice Creek Commons.

1.6 DEVELOPER DEVIATIONS FROM CSMP

Future developers may want to deviate from the CSMP. The responsibility is on the developer to prepare an application that documents what is different from the CSMP (e.g., X more square feet of impervious surface than documented in Table 6-2) and how that difference will be mitigated (e.g., stormwater cistern used to water landscaped areas).

Developers that choose to exceed the impervious limits designed to for that outlot, as summarized in Table E-2, will be required to address any additional stormwater requirements within that outlot at the developers expense. Any such stormwater management facilities will also be maintained by the owner/developer of the outlot.

2.0 Site Location

2.1 PROJECT LOCATION

The project site is located in Ramsey County, Minnesota predominately within the limits of the City of Arden Hills. The site is located within portions of Sections 9 and 16, Township 30 North, Range 23 West of the 5th Principal Meridian, (the Site). The Site is bounded by U.S. Interstate Highway 35W on the west, Minnesota State Aid Highway (CSAH) 96 to the south and U.S. Highway 10 to the southwest (**Figure 2-1**). The Arden Hills Army Training Site (AHATS) bounds the Site to the east. The redevelopment Site is comprised of approximately 427 acres of the western portion of the former TCAAP facility.

2.2 DRAINAGE AREAS AND RESOURCES OF CONCERN

Rice Creek divides the Site into two sections. The portion south of Rice Creek, and the portion north of Rice Creek (**Figure 2-2**). This project is located within Lower Rice Creek subwatershed area. Some of the Site drains to Rice Creek, and the remainder of the Site drains south to Round Lake.

3.0 Background Information

Portions of the Site will be developed by Ramsey County, the City of Arden Hills, and yet to be determined developers. Ramsey County hired the Port Authority to consult and help broker commercial developer interest in the Site. The following sections help put into context the behind-the-scenes planning accomplished leading up to this submittal by giving a brief overview of the Joint Development Authority (JDA), a brief status update on various planning steps underway, and the role of the Energy Resilience Advisory Board in Rice Creek Commons redevelopment. This section will also give a brief overview of the contamination and remediation activities on the Site at the time of printing. As well as the federal, state and local water resource regulations that need to be met on the Rice Creek Commons.

3.1 OTHER CONTROLLING DOCUMENTS AND GUIDELINES

A number of documents and guidelines have been or will be in place to guide the JPA decision making, including this CSMP.

3.1.1 Rice Creek Commons Master Plan & Alternative Urban Areawide Review

The City of Arden Hills prepared the Master Plan and the Alternative Urban Areawide Review (AUAR). The Arden Hills City Council approved the Master Plan Land Use Map on June 30, 2014 (**Figure 3-1**), after several months of development, work session meetings and public comments. Changes to the Master Plan over time can be expected, based on market demands and adjustments as development occurs.

The Master Plan Land Use Map (**Figure 3-1**) shows the Natural Resources Corridor. The site also has a number of delineated wetlands (**Figure 3-2**). It is a goal of the Master Plan to meet the majority of the wetland mitigation and stormwater management requirements within the Natural Resources Corridor.

An AUAR is a form of environmental documentation through Minnesota State Rules that evaluates one or more development scenarios for an entire geographical area rather than a specific project. Development scenarios are established based upon the comprehensive plan, zoning ordinances, developers' plans, and other relevant information. The City of Arden Hills approved the Final AUAR and Mitigation Plan for the Rice Creek Commons on July 28, 2014 (<https://mn-ardenhills.civicplus.com/DocumentCenter/View/971>).

3.1.2 Regulations and Policies

The Arden Hills City Council is working on the Regulations and Policies for the Rice Creek Commons. The City of Arden Hills maintains a website with background and additional information (<http://www.cityofardenhills.org/>)

3.1.3 Energy Resilience Advisory Board

An appointed five member Energy Resilience Advisory Board (ERAB) is working with consultants to develop the Rice Creek Commons Energy Integration and Resiliency Framework ("EIRF"), which will become a companion document to the Master Plan and policy actions that the JDA, the County, and the City can consider to support that energy

future. The visioning and development of guiding principles for Rice Creek Commons is underway at the time of this printing.

3.2 SITE HISTORY OVERVIEW

The Site was used for the production and storage of small arms ammunition from the 1940s to the mid-1990s. Prior to development in the 1940s, the Site was used for agricultural purposes. Many investigation and cleanup activities have been focused on the Site in the past. There are a number of maps available of the surficial geology and hydrogeology at Rice Creek Commons (**Figures 3-3, 3-4, 3-5 and 3-6**).

There were several redevelopment attempts between 2002 and 2011. One attempt, by Ryan Companies, completed a number of soil borings on the Site (**Figure 3-7**).

Ramsey County is cleaning up the surface of the Site to residential soil reference value (SRV) to allow unrestricted land use however, future property use is anticipated to be mixed residential, retail, non-retail commercial (i.e., office, light industrial) and park (i.e., recreational) (per the Master Plan.) Within the redevelopment Site (427 acres), the Army will continue to own, operate and monitor groundwater remediation systems in the surficial groundwater (Unit 1, shown in **Figure 3-8**) and in the deep groundwater (Unit 3, shown in **Figure 3-9**). Ramsey County maintains a website with background and additional information (<http://ricecreekcommons.com/>)

3.3 RAMSEY COUNTY PURCHASE AND CLEAN UP OVERVIEW

The redevelopment Site is comprised of approximately 427 acres of the western portion of the former TCAAP facility. Ramsey County has closed on the initial purchase and transfer of title for 397 acres of previously remediated property. The remaining 30 acres of property, which has residual soil contamination, is being leased from the federal government while Ramsey County remediates the soil. Ramsey County entered into a demolition and remediation contract with Carl Bolander and Sons in April 2013 to clean up the State's largest Superfund site. Once the cleanup is complete, the County will close on the final 30 acres and take title of the leased property. Ramsey County is completing additional subsurface investigations throughout the Site and implementing MPCA-approved response actions as necessary.

3.4 ARMY GROUNDWATER REMEDIATION SYSTEM OVERVIEW

The federal government retains responsibility for certain environmental liabilities under the Offer to Purchase, and federal environmental laws require the U.S. government to promise, for certain contaminated property it sells, that "any additional remedial action found to be necessary after the date of such transfer shall be conducted by the United States." Under advisement by the MPCA, redevelopment of the Site must proceed in a manner such that infiltration does not alter the groundwater flow regime in areas that are known to be affected by historical solvent releases. The following sections 3.4.1 and 3.4.2 define the remaining known areas.

3.4.1 TCAAP Hillside Sand Groundwater Plume

The TCAAP Groundwater Recovery System (TGRS) will continue to treat contaminated groundwater from Unit 3 (the Quaternary aged Hillside Sand Formation, which is one of the hydrogeologic units beneath Rice Creek Commons). The TGRS treats groundwater, from

Unit 3 (**Figures 3-5 and 3-6**), through a system of deep pumping stations and an on-site treatment facility that air strips the contaminants from the water, and then recharges the treated groundwater back into the Unit 3 groundwater table (**Figure 3-9**). This system will remain in place with oversight from the Minnesota Pollution Control Agency (MPCA) and the U.S. Environmental Protection Agency (EPA) for approximately the next 50 years. In the area of the Rice Creek Commons redevelopment project, Unit 2 is an aquitard which is located above Unit 3 and hydraulically separates the surficial groundwater (Unit 1) from deep groundwater (Unit 3).

3.4.2 Rice Creek Commons Surficial Groundwater Plumes

The Site has surficial (Unit 1) contaminated groundwater associated with the Installation Repair Program (IRP) Sites I and K and Building 102. The Site K and Building 102 plumes are located in the north end of the portion south of Rice Creek, while the Site I plume is located north of CSAH 96 in the southeast portion of the portion south of Rice Creek (**Figure 3-8**). This figure shows a snapshot of the Unit 1 groundwater plumes supported by monitoring data presented in the Fiscal Year 2012 Annual Performance Report (FY 2012 APR). There are variations in the size, shape and orientation of the plumes, as documented in subsequent APRs. The Unit 1 layer is comprised of generally heterogeneous and discontinuous materials. In the location of the Site K plume, Unit 1 geology can be described as having discontinuous stratigraphy (both vertically and horizontally). This makes predicting the groundwater flows using a model difficult because of heterogeneity. The Unit 1 layer is at the ground surface, extends south of Rice Creek (**Figure 3-4**) and tapers out until the Unit 2 (aquitard) is exposed at the surface. To the best of our knowledge a groundwater model does not exist for Unit 1.

3.4.2.1 Site I Surficial Groundwater Plume

According to the FY 2013 APR, the Site I has a shallow groundwater plume and has not achieved the ROD clean up levels to close the site. There are three selected remedies in the Record of Decision (ROD Amendment #2, 2009): groundwater monitoring, additional characterization, and land use controls. The annual sampling, from monitoring well 01U667, is required into the future until the cleanup goals are met. According to Conestoga-Rovers & Associates (CRA) report dated May 1994 (available on Ramsey Co ftp), Unit 1 is the uppermost unit at Site I comprised of discontinuous layers of fine grained sand, silt and clay (generally fill material from previous construction activities) with pockets and layers of organic deposits (peat). Unit 1 is less than 40 feet thick at Site I. Surficial groundwater is perched within Unit 1, which is underlain with Unit 2 (an aquitard, separating Unit 1 from the deeper aquifer).

3.4.2.2 Site K Surficial Groundwater Plume

According to the FY 2103 APR, the Site K shallow groundwater plume has not achieved the ROD clean up levels to close the site. Eight remedy components are being implemented at Site K. The plume is being treated using a groundwater extraction trench and air stripper, and monitored using monitoring wells, piezometers and sentinel wells. When the system operates as designed the treated water discharges to the storm sewer that, in turn, discharges to Rice Creek. This system will remain in place with oversight from the MPCA and the EPA into the future. According to CRA report dated May 1994, Unit 1 (the Fridley Formation) is the uppermost unit at Site K comprised of fine to medium grained sand with minor constituents of silt. Unit 1 is less than 2.2 to 46.8 feet thick at Site K. Surficial groundwater is perched within Unit 1, which is underlain with Unit 2 (an aquitard,

separating Unit 1 from the deeper aquifer). Groundwater at Site K flows west-northwest toward Rice Creek.

3.4.2.3 Building 102 Surficial Groundwater Plume

According to the FY 2103 APR, the Building 102 shallow groundwater plume has not achieved the ROD clean up levels to close the site. The plume is being treated using Natural Attenuation (abiotic degradation), and monitored to evaluate attainment and to verify that groundwater reaching Rice Creek does not exceed state surface water standards. This system will remain in place with oversight from the MPCA and the EPA into the future. Building 102 is close to building 103, which is a considered a part of Site K.

3.5 NO ASSOCIATION DETERMINATION

Due to the potential concern relative to surficial (Unit 1) contaminated groundwater, Ramsey County is seeking explicit MPCA approval relative to implementation of stormwater best management practices (BMPs) to ensure infiltration does not alter the groundwater flow regime in areas affected by historical solvent releases. Ramsey County is seeking protection of a No Association Determination under the Minnesota Environmental Response and Liability Act (MERLA) for the proposed actions related to plans for stormwater infiltration, as well as concurrence from the U.S. EPA and the U.S. Army with those plans.

Ramsey County submitted a Proposed Actions Letter/Request for No Association Determination request for activities under the demolition and remediation contract, described in Section 3.3, and was granted a No Association Determination on April 13, 2013. At the time of this printing, the MPCA is considering Ramsey County's additional Proposed Actions Letter/Request for No Association Determination requests for the following proposed actions:

- ▲ Redevelopment of the Site through mass grading and installation of municipal utility infrastructure, including all proposed public roadways, underground utilities (i.e., water mains, sewers, electric power, communications lines), as well as stormwater best management BMP systems, mitigated wetlands, Rice Creek channel modifications, and other infrastructure improvements.
- ▲ The proposed stormwater system will include surface water retention and surface water conveyances, though infiltration will be the preferred method for managing stormwater on-Site. Areas of the Site where increases in stormwater infiltration (above what was considered existing conditions) are not considered appropriate are defined on **Figure 5-2. Figure 3-8** shows areas of the Site where shallow (i.e., Unit 1) groundwater is currently impacted by solvent releases.
- ▲ Ramsey County met with MPCA on March 18, 2015 at Ramsey County Public Works. The MPCA requested Ramsey County document the predicted infiltration contributing to the surficial (Unit 1) aquifers in the vicinity of the Installation Repair Program (IRP) Site K before the start of the redevelopment project (circa 2012) and at the end of the redevelopment project (circa, TBD). During this meeting, MPCA discussed Ramsey County's written request made on September 16, 2014 for the MPCA to issue an assurance letter called a No Association Determination to fully develop the Site. A No Association Determination would give Ramsey County assurances that the Site could be developed, as presented in the CSMP, and Ramsey County would not be associated with known contamination for Superfund liability purposes. Ramsey County is preparing a technical document that outlines the measures prescribed in the comprehensive stormwater management planning process to limit impacts on the

remaining impacted surficial groundwater after the current demolition and remediation efforts are completed.

- ▲ Site sampling, excavation, segregation, grading, movement, stockpiling, permitting and means necessary for appropriate disposal of soils and media to accommodate the above-referenced improvements.
- ▲ Dewatering, sampling, permitting and means necessary for appropriate disposal of waters to accommodate the above-referenced improvements.

3.6 REGULATORY REQUIREMENT OVERVIEW

There are a number of federal, state and local water resource regulations that need to be met on the Rice Creek Commons. Given the complexity and potential conflicts between these regulations, we have been meeting with the respective agencies to confirm requirements. The four key agencies and their respective approvals required are listed below:

MnDOT has jurisdiction over wetland and drainage within MnDOT right-of-way, including the interchange areas of County Road H and County Road I. Wetland and drainage permits will be required from MnDOT for modifications to these interchanges that impact wetlands and surface water runoff.

MN DNR has jurisdiction over Rice Creek (within the top of banks of the Rice Creek channel) including any changes to the creek alignment, new crossings and floodplain impacts. A public waters permit and floodplain review will be required for the creek realignment, crossing and floodplain impacts adjacent to Rice Creek.

US Army Corps of Engineers (USACE) has jurisdiction over waters of the US which includes certain wetlands and streams. A Clean Water Act Section 404 permit will be required for wetland and creek impacts.

Rice Creek Watershed District (RCWD) has jurisdiction over Rice Creek Commons storm water runoff, erosion control, floodplains, wetlands, and creek crossings. RCWD approval is required in each of these areas by way of a Comprehensive Stormwater Management Plan (CSMP) and issuance of RCWD Permits. As allowed for under RCWD Rule C.5 (f), this CSMP has been prepared as an alternative means to meet the requirements of Rule C.6 (Water Quality Treatment) and Rule C.7 (Peak Stormwater Runoff Control) for redevelopment of the Site, which will be done in various stages.

4.0 Rice Creek Commons Redevelopment

4.1 PREDEVELOPMENT CONDITIONS

The predevelopment conditions for the site are agreed to be 2012, before redevelopment activities, such as demolition, commenced. The Site had open space, buildings, roads, utilities, no engineered infiltration devices, and groundwater remediation systems operating. Using LiDAR data and information available on the soils, land use, and storm sewers, the predevelopment conditions subwatersheds were delineated (**Figures 4-1** and **4-2**). The southern portion of the Site drains to Round Lake, while the remainder drains to Rice Creek. **Appendix A** includes a technical memorandum summarizing the modeling completed for the Site under predevelopment conditions. Wenck evaluated the Site stormwater runoff for 2-, 10-, and 100-year 24 hour design rainfall events (2.82, 4.22, and 7.31 inches, respectively). Precipitation depths were obtained from the National Oceanic and Atmospheric Administration (NOAA) Atlas 14 and distributed with a nested curve.

Due to the remediation at Rice Creek Commons, there is a considerable amount of data available on the groundwater levels in Unit 1. **Figure 4-3** is a historical map showing the surficial (Unit 1) groundwater levels in 1987. There are no groundwater level lines shown on the map in areas where the surficial (Unit 1) groundwater wasn't observed. Hydrogeologic factors such as heterogeneity of Unit 1, The historical map doesn't have depth to groundwater labeled. **Figure 4-4** shows the groundwater piezometric contours using 2012 groundwater conditions. The groundwater levels were evaluated using available surficial (Unit 1) groundwater monitoring wells (20 wells), and based on the water level data available between 1987 and 2013, 2012 was determined to represent the seasonably high groundwater condition.

There are 14.4 acres of wetland on the site (**Figure 3-2**). The RCWD, United States Army Corps of Engineers (USACE), and Minnesota Department of Transportation (MnDOT) have different jurisdiction and mitigation requirements for the wetlands impacted. Mitigation is needed on site to satisfy RCWD and Corps requirements for the Site infrastructure impact to wetlands (construction of the Spine and Old Highway 8 extension Roads, Natural Resources Corridor grading), as shown in **Figure 4-5**. This amount does not include the creek realignment, however that work is expected to be self-mitigating. Wetland sequencing will be defined in the County's permit applications submitted to RCWD.

4.2 CONCEPT GRADING PLAN

The current plan shows the site grading for Rice Creek Commons public infrastructure improvements and the layout of the Natural Resources Corridor (**Figure 4-5**), a blend of stormwater ponds and mitigation wetlands. **Figure 4-6** is a flow diagram showing the connections between tributary subwatersheds, stormwater ponds and wetlands. The proposed grading plan accomplishes the following priorities for fully developed conditions:

1. Provide all wetland replacement for the public infrastructure impacts
2. Accommodate all required stormwater rate control and infiltration for Spine/Old Highway 8 extension Roads
3. Accommodate stormwater rate control for all Rice Creek Commons development

4. Provide maintenance access to stormwater features in form of a recreational, multi-use trail
5. *
6. *
7. Provide some space for passive recreation opportunities along the easterly portion of the site

* Depending on further coordination with the USACE and RCWD, there is room in the Natural Resources Corridor to accommodate most of priority #5 and priority #6 (Section 1.2 of this CSMP). If a permit from the USACE cannot be obtained for wetland impacts of future development, the proposed grading plan would not provide wetland replacement (priority #5) for future site development within the Natural Resources Corridor, requiring individual developments that impact wetlands to address mitigation needs separately as a part of their planning and design activities. In that case, additional infiltration best management practices (BMPs) may be incorporated into the Natural Resources Corridor (priority #6) rather than additional wetland mitigation.

4.3 PROPOSED CONDITIONS

The proposed conditions are presented in two stages; completion of the public infrastructure improvements (interim conditions), and the fully developed conditions using landuse assumptions in the Master Plan. **Appendix A** includes technical memorandums summarizing the modeling completed for the Site.

4.3.1 Interim Conditions (Public Infrastructure Improvements) (Interim Scenario 1)

The first phase of site development includes the construction of public infrastructure improvements including Spine Road, I-35W improvements, alterations to County Road H and realignment of Rice Creek (**Figure 4-5**). During this interim construction phase, a contractor, selected by Ramsey County will construct all wetlands and ponds except P-1, P-3, and P-14. The interim conditions reflect the first stage of site development which includes the construction of public infrastructure improvements. **Appendix A** includes technical memorandums summarizing the modeling completed for the Site under interim conditions (Interim Scenario 1). The same rainfall and snowmelt events were evaluated for interim conditions, as were evaluated for existing conditions.

4.3.2 Worst Case Interim Conditions (Interim Scenario 2)

The pollutant removal efficiency of the proposed regional ponding system was evaluated for the worst case scenario. This Interim Scenario 2 assumes the public infrastructure improvements are completed, stormwater pond (P-14) is built, Rice Creek Commons develops east of Spine Road (landuse per **Figure 3-1**) and there is no development or infiltration device in subwatershed SB-18 (**Figure 4-7**). **Appendix A** includes a water quality technical memorandum summarizing the modeling completed for the Site under interim conditions (Interim Scenario 2). The same rainfall and snowmelt events were evaluated for interim conditions, as were evaluated for existing conditions.

4.3.3 Proposed Conditions (Fully Developed)

Using the grading plan and assumption that existing topography will remain relatively unchanged and information available on the soils, fully developed conditions land use, and storm sewers, the fully developed conditions subwatersheds were delineated (**Figures 3-1 and 4-7**). The comparison between existing and fully developed site drainage divide is shown on **Figure 5-1**. **Appendix A** includes technical memorandums summarizing the modeling completed for the Site under proposed conditions (fully developed). The same rainfall and snowmelt events were evaluated for fully developed conditions, as were evaluated for existing conditions.

5.0 CSMP Meets RCWD Rule C Requirements

5.1 RCWD RULE C OVERVIEW

The Rice Creek Commons to be redeveloped is located within two Resource of Concern (ROC) Drainage Areas (see **Figure 2-2**). The northern part of the site (278 acres) drains to Rice Creek which flows into Long Lake located approximately 1.3 miles downstream. The southern part of the site (208 acres) drains directly into Round Lake. General drainage patterns will be maintained under the proposed, fully developed conditions and stormwater runoff within each ROC drainage area will be managed to meet RCWD's Rule C requirements as described below (as adopted at the time of printing).

As allowed for under RCWD Rule C.5 (f), this Comprehensive Stormwater Management Plan (CSMP) has been prepared as an alternative means to meet the requirements of Rule C.6 (Water Quality Treatment) and Rule C.7 (Peak Stormwater Runoff Control) for redevelopment of the Site, which will be done in various stages. The water quality treatment volume and peak runoff rate controls standards will be met in aggregate for each ROC drainage area. As agreed to with RCWD staff, existing impervious surface area is based on 2012 conditions prior to the start of site demolition and remediation activities. Proposed impervious surface areas are based on the proposed infrastructure improvements and various land uses identified in the Master Plan.

Best management practices (BMPs) used to meet the standards will be constructed and functional prior to, or concurrent with the construction of impervious surfaces associated with each development project. Once approved by RCWD, this CSMP will apply to future development of the site. Permits will be required for individual development projects to confirm compliance with this CSMP and to meet other applicable RCWD rules.

Initial stages of redevelopment include site demolition and remediation activities that commenced in April of 2013. The next phase is construction of public infrastructure improvements including the main access road serving the development, utilities, rough site grading, regional stormwater ponds, and mitigation wetlands. The majority of the stormwater ponds and wetland mitigation areas are located within the Natural Resources Corridor that runs through the center portion of the site.

Upon completion of the public infrastructure improvement work, private development will take place over time and in various stages which will include construction of additional stormwater management facilities. Individual stormwater management plans must conform to this CSMP as well as any ordinances that may apply within the redevelopment area.

In accordance with RCWD stormwater management policy, better site design techniques shall be considered to reduce impervious surfaces and maximize water quality and flood control benefits. Two such techniques, preserving/dedicating natural areas and using a treatment train approach for runoff management, have been incorporated into this CSMP. Developers will be required to evaluate and apply better site design techniques, wherever possible, as part of the design process for future development of the site. A better site design guidance document and checklist is available on the RCWD website.

5.2 WATER QUALITY TREATMENT – RULE C.6

RCWD’s water quality treatment requirements will be met through a combination of onsite infiltration and wet detention ponds, as described below. Due to soil constraints (Type C and D soils with low permeability), ongoing groundwater treatment activities (described in Section 3.4), MPCA’s advisement to not alter the groundwater flow regime in areas that are known to be affected by historical solvent releases, and other factors such as subsurface vapor treatment and shallow Unit 1 groundwater; the use of onsite infiltration is limited and determined to not be feasible over the majority of the site. **Figure 5-1** shows locations of potential infiltration practices based on review of existing soil borings. Copies of the soil boring logs for the project site are provided in **Appendix B**. Infiltration is deemed feasible and required in the area north of Rice Creek and an area on the west side of the Spine Road alignment as shown in **Figure 5-1**. Both of these areas drain to Rice Creek. In all other areas, including areas draining south to Round Lake, water quality treatment will be provided through wet detention ponds.

Available soil borings (circa 2007) were reviewed along the Spine Road corridor, the best soils for infiltration practices are located in areas affected by historical solvent releases, thus infiltration is deemed infeasible and not required. The **Appendix A** HydroCAD Technical Memorandum Section 3.3 and Figure 3 display the conclusions of the infiltration potential analysis based on the 2007 borings. The runoff from Spine Road is collected by stormsewer and directed to stormwater ponds (P-2, P-6 and P-13). Additional soil borings were drilled to obtain geotechnical recommendations to advance the Spine Road corridor design during summer 2015. The preliminary soil borings logs available at the time of printing were reviewed and the best soils for infiltration practices were located in areas where infiltration BMPs were infeasible. **Figure 5-3** display the conclusions of the infiltration potential analysis based on the 2015 preliminary borings. Many of the borings show clay soils, which are not favorable for infiltration BMPs. The borings with feasible soils were located in the headwaters portions of the delineations where there isn’t a large enough subwatershed tributary to feed an infiltration BMP (SR-211), in areas of high groundwater (SR-214, SR-215, SR-216, SR-217) and in areas where stormwater reintroduction is not allowed due to historical solvent releases. These additional soil borings, when finalized, will be submitted with the public infrastructure improvements permit application to RCWD.

5.2.1 Stormwater BMP Strategy to meet Rules

5.2.1.1 Infiltration

Rule C.6 requires that where feasible, infiltration be provided for stormwater runoff from new and/or reconstructed impervious surfaces. Proposed impervious surface area is based on the proposed public infrastructure improvements and various land uses identified in the Master Plan.

Water quality treatment volume required for phosphorus removal is calculated as follows:

$$\begin{array}{l} \text{Required} \\ \text{Water Quality} \\ \text{Treatment} \\ \text{Volume} \\ \text{(ac-ft)} \end{array} = \begin{array}{l} \text{Area of New or} \\ \text{Reconstructed} \\ \text{Impervious} \\ \text{Surface (acres)} \end{array} \times 1.1 \text{ (in)} \div \begin{array}{l} \text{Total Phosphorus} \\ \text{Removal Factor} \end{array} \div \begin{array}{l} 12 \\ \text{(in/ft)} \end{array}$$

Where the Total Phosphorus Removal Factor = 1.0 for Infiltration BMPs

As noted above, onsite infiltration was deemed feasible and is required in the area north of Rice Creek and the northwest corner of the portion south of Rice Creek, as shown in **Figure 5-1**. The proposed new/reconstructed impervious surface area and the required water quality treatment volumes (infiltration volume) based on the above equation are provided in **Table 5-1** below:

Table 5-1: Infiltration Requirements

	Area North of Rice Creek (Figure 5-1: Area 4)	Area West of Spine Road (Figure 5-1: Area 3)
Impervious Surface (acre)	40.9	36.3
Infiltration Volume (acre-ft)	3.7	3.3

One large regional infiltration facility could be constructed in each drainage area to meet the above volume requirements. In lieu of providing large regional facilities, smaller infiltration systems could be constructed as individual parcels develop. In this case, a rate of 4,000 cubic feet of infiltration volume per acre of impervious surface area created would be used to determine infiltration requirements on a parcel-by-parcel basis.

Infiltration systems that capture, store and infiltrate stormwater runoff into the underlying permeable soils include both ground surface depressions such as infiltration basins and underground facilities such as an infiltration trench. Typical construction details for each of these practices are provided in the Minnesota Stormwater Manual (excerpts included in **Appendix C**). Drawdown must be within 48 hours after the storm event for surface systems and 72 hours for subsurface systems. A minimum of three feet of separation must be provided from the bottom of the infiltration system to the seasonally high groundwater table or bedrock. The infiltration systems will be equipped with a bypass or overflow device to redirect stormwater runoff from larger rainfall events (greater than 1.1 inches) to the downstream conveyance systems. The bypass or overflow for the infiltration device in the northwest corner of the portion south of Rice Creek will be directed to the wet detention Pond (P-13), where additional water quality benefits will be provided along with rate control.

Total suspended solids (TSS) will be removed to the maximum extent practicable from stormwater runoff from new and reconstructed impervious surfaces and from stormwater draining to infiltration areas. This will be accomplished through the use of sump catch basins and manholes, grit chambers, sand filters, or similar BMPs.

The proposed wet detention ponds described below will provide additional volume reduction and water quality benefits through infiltration, evaporation, and plant uptake between storm events creating additional storage volume in the pond as water levels drop below the outlet elevation.

5.2.1.2 Wet Detention Ponds

Stormwater ponds used for water quality treatment in meeting Rule C.6 need to be properly sized consistent with Nationwide Urban Runoff Program (NURP) criteria. This includes providing a permanent wet pool with dead storage at least equal to the runoff volume from a 2.5-inch rainfall over the pond tributary area under full development. The permanent pool volume is the most important design parameter influencing pollutant removal efficiency. It

provides storage and treatment of runoff during and between storm events. The amount of dead storage calculated for each pond is shown in the Pond Data Summary Tables provided in HydroCAD Technical Memorandum (**Appendix A**).

The proposed fully developed dead storage volume provided for each ponding system is summarized in **Table 5-2**.

Table 5-2: Dead Storage

Pond/Ponding System	Dead Storage Provided (ac-ft)
P-1, P-2, P-3, P-5, P-6	14.4
P-7, P-8, P-9, P-10, P-11, P-12, P-13	15.6
P-4	0.6
P-14	4.5
Total	35.1

Individual ponds P-4 and P-14 meet RCWD’s dead storage requirements. Although some of the individual ponds may not meet the dead storage requirements, the ponding system with multiple ponds in series provides a treatment train approach that enhances overall pollutant removal capabilities.

5.2.1.3 P8 Water Quality Modeling

To ensure that the average annual total phosphorus removal efficiency for the ponding systems in series is at least 50% (TP Removal Factor), the P8 Water Quality Model was utilized to estimate the overall phosphorus removal efficiency under fully developed conditions. For fully developed conditions, each outfall is numbered as shown in **Figure 6-3**. The tributary areas to each outfall are shaded in a similar color. A description of the modeling effort is summarized in a Technical Memorandum provided in **Appendix A**. Based on the P8 modeling results, the total phosphorus removal efficiencies for the two proposed ponding systems are as follows:

Table 5-3: TP Load Reductions to Rice Creek (Scenario 3: Fully Developed Conditions)

Discharge Point	Watershed Inflow Load (lbs./year)	Total Outflow (lbs./year)	% Reduction
Outfall #5	279	99.9	64%
Outfall #10	18.8	7.1	62%
CRH-1**	7.1	3.8	46%
CRH-3**	5.4	4.2	22%
Thumb Infiltration	13.6	0.4	97%
Total	323.9	115.4	64%

*The total TP inflow load for each discharge point was calculated by summing the TP loads from each contributing watershed (i.e. total TP inflow load for CRH-3 = TP load from CRH-2 + TP load from CRH-3).

** Ponds CRH-1 and CHR-3 are 30% designed, see text above.

Table 5-4: TP Load Reductions to Round Lake (Scenario 2: Fully Developed Conditions)

Discharge Point	Total TP Inflow Load* (lbs./year)	Total Outflow Load (lbs./year)	% Reduction
Outfall #2	168.8	66.3	61%
Outfall #1	12.4	6.1	51%
Total	181.2	72.4	60%

*The total TP inflow load for each discharge point was calculated by summing the TP loads from each contributing watershed (i.e. total TP inflow load for Outfall #1 = TP load from Pond 4 + TP load from Wetland 1)

The 2012 outfalls are numbered as shown in **Figure 6-2**. TP loads were evaluated for interim development conditions. The results from these models are presented in **Appendix A**.

The P8 modeling confirms that the wet detention ponding networks provide water quality treatment of site stormwater runoff sufficient to meet RCWD rules. Improved wet pond designs have been observed to achieve higher pollutant removal efficiencies by adding in features such as wetland benches, flow barriers that divide the pond into two or more segments, forebays, and floating wetlands and / or aerators. The BMPs modeled in P8 are simple ponds and infiltration devices without explicitly modeling the additional pollutant removals associated with these enhanced designs. The least effective BMP provides over 50% TP removal. Each ponds, shown on **Figure 4-5**, will be installed with an aquatic bench. Future private designers should not be expected to install BMPs that are more effective than required in the CSMP, unless impervious surfaces proposed are greater than assumed in this CSMP.

Pond outlets will be designed to provide skimming/retention of oils and floatable debris for at least the 1-year 24-hour design rainfall event (2.45 inches). Velocities through each skimming device will be limited to 0.5 feet per second.

The ponds will also be designed with an aquatic bench extending below the normal water level a minimum width of 10 feet and one foot deep for safety purposes, provide suitable habitat for rooted aquatic plants, and improve access for maintenance.

5.2.2 Additional BMPs for Consideration of Future Development

Additional onsite volume abstraction/reduction will be encouraged as part of future development. The following may be considered, if feasible::

- ▲ Capture and reuse of stormwater for irrigation - Operating plans for stormwater reuse including calculations and documentation will be provided by developers and submitted with individual permit applications.
- ▲ Planting of new trees - Tree planting plans will be provided by developers and submitted with individual permit applications. Tree canopy areas provide variable rainfall interception that can be estimated using a variety of methods.
- ▲ Soil amendments - Project specifications that incorporate soil amendments or conditioners, where applicable, to restore soil function and increase infiltration capacity will be consistent with RCWD Soil Amendment Guidelines.

- ▲ Native plants - Use native plants wherever possible to enhance stormwater abstraction and uptake capabilities.
- ▲ Wetland buffers - Maintain vegetative buffers along edges of wetlands to slow runoff and filter out nutrients and suspended solids.
- ▲ Grass channels - Use grass channels and swales wherever possible to convey stormwater runoff.

5.3 PEAK STORMWATER RUNOFF CONTROL – RULE C.7

With the exception of small direct tributary areas to Rice Creek and infiltration volumes described above, all stormwater runoff from developed areas of the site will be routed through stormwater ponds where both water quality and rate control will be provided.

The HydroCAD computer model was used to model existing and proposed conditions to determine peak discharges and water levels for the 2-year, 10-year, and 100-year, 24-hour design rainfall events using NOAA Atlas 14 precipitation values. Analysis of the 10-day snowmelt event was not required as all stormwater ponds have a defined outlet at an elevation below the 100-year high water level. A description of the modeling effort, calculations, technical analysis, and supporting information is summarized in a Technical Memorandum provided in **Appendix A**.

5.3.1 Allowable Peak Discharge Rates

Since the project site is located within the Flood Management Zone of the lower Rice Creek Watershed, proposed discharge rates will be limited to 80% of existing peak discharge rates. Based on modeling of existing conditions, the allowable peak rates for the proposed redevelopment (80% of existing/Flood Management Zone) is as follows:

Table 5-5: Allowable Peak Rates

Drainage Area	2-Year	10-Year	100-Year
Rice Creek (cfs)	252	483	975
Round Lake (cfs)	127	254	525

5.3.2 Proposed Peak Discharge Rates

Stormwater runoff from redeveloped areas within both drainage areas will be routed through onsite regional ponds to meet the above peak stormwater runoff control requirements for the 2-year, 10-year and 100-year 24-hour design rainfall events (80% of existing).

Based on modeling of proposed conditions, the following peak rates have been determined:

Table 5-6: Proposed Peak Rates

	2-Year	10-Year	100-Year
Interim Rice Creek (cfs)	104	232	607
fully developed conditions Rice Creek (cfs)	237	414	853
Interim Round Lake (cfs)	51	124	349
fully developed conditions Round Lake (cfs)	54	117	326

Overall, peak rates will be reduced and there should be no adverse downstream impacts. Peak discharge rates, water elevations, and storage volumes for each of the proposed ponding areas are summarized in the Pond Data Summary Table provided in HydroCAD Technical Memorandum (**Appendix A**).

Wet detention ponds that are not constructed initially as part of the public infrastructure improvement construction phase (Ponds P-1, P-3 and P-14) will be constructed later as part of future development phases. These ponding areas will be dedicated and space reserved for future use by way of outlots and/or easements and located in the general area shown on the site plan. Ponds will be constructed to provide the dead storage volume and flood storage consistent with what was modeled and included as part of the overall stormwater analysis presented in this CSMP.

5.3.3 Outlet Control Structures

Each pond will be designed with a multi-stage outlet control structure to manage stormwater discharges for each of the design events. The outlet structure will also be designed to provide skimming/retention of oils and floatable debris for at least the 1-year 24-hour design rainfall event (2.45 inches).

Each pond will be equipped with a stabilized emergency overflow spillway to convey flows greater than the 100-year 24-hour design rainfall event (7.31 inches). Emergency overflow routes shall also be provided for added protection against flooding and local erosion.

Stormwater conveyance systems such as storm sewers and drainage channels will be designed and constructed by commercial developers to route stormwater runoff to various ponding areas serving the development.

5.3.4 Low Floor and Low Entry Freeboard Requirements

The storage volumes and discharge rates established for each pond must be maintained to prevent flooding of property and meet peak flows. The lowest floor elevation of future buildings and structures adjacent to ponds and connecting drainage channels must be at or above the 100-year high water level and the lowest opening elevation must be at least two feet above the 100-year high water level. These building elevations should be indicated on site grading plans to ensure adequate freeboard is provided. Pond emergency overflow elevations must be at least one foot below the lowest opening elevation.

6.0 Stormwater BMPs

6.1 CONSTRUCTION OF PONDS

The stormwater ponds (P-2, P-4, P-5, P-6, P-7, P-8, P-9, P-10, P-11, P-12, P-13, CRH-1, CRH-2, and CRH-3) will be constructed by the contractor(s) selected by the County (**Figure 4-5**). The County's contractor will construct storm sewer conveyance from P-5 to P-3, along with a temporary connection to the outfall to provide conveyance until P-3 is constructed. Stormwater ponds P-1, P-3 and P-14 will be constructed by respective developers. The engineering design for stormwater ponds located in MnDOT right-of-way (CRH-1, CRH-2, and CRH-3) was preliminary (30%). The proposed ponds submitted did not provide sufficient water quality treatment volume to meet RCWD rules, nor were infiltration BMPs incorporated in the stormwater management design, as required per this CSMP.

The planned phasing is that all but stormwater ponds (P-1, P-3 and P-14) will be constructed as part of the public infrastructure improvements (Interim Scenario 1) staged outlets and general site grading to create wetland hydrology.

6.2 VOLUME REDUCTION PRACTICES

The volume reduction BMPs (e.g., infiltration practices) on the portion area and the northwest corner of the portion south of Rice Creek will be designed and constructed by the respective developer following the Minnesota Stormwater Manual guidance (**Appendix C**) and the RCWD Permit requirements. In Areas 3 and 4 (**Figure 5-1**) soil infiltration rates will be evaluated to determine if the native soils at the bottom of the proposed infiltration device infiltrate more than 8.3 inches per hour. The soils shall be amended to slow the infiltration rate below 8.3 inches per hour or as allowed by a local unit of government with a current MS4 permit. The respective developer will be the entity responsible for long-term operations and maintenance of volume reduction practices. Respective developers shall submit an infiltration management plan as part of the RCWD permit application.

6.3 RESPONSIBLE PARTY FOR STORMWATER MANAGEMENT FEATURES

Ramsey County is the entity responsible for short-term (through 5-year establishment and approval period) operations and maintenance of the wetland mitigation (Wi, W-1, W-2, W-3, W-4, and W-5).

MnDOT is the entity responsible for long-term operations and maintenance of the stormwater management and drainage facilities receiving stormwater from MnDOT right-of-way. Preliminarily these consist of stormwater ponds; P-15, CRH-1, CRH-2, and CRH-3.

The City of Arden Hills is the entity responsible for long-term operations and maintenance of the stormwater management and drainage facilities receiving stormwater from public and private spaces within the Site. Most of these stormwater ponds are located within the Natural Resources Corridor (P-4, P-5, P-6, P-7, P-8, P-9, P-10, P-11, P-12, and P-13). Regional ponds owned by public entities that are used to meet rate control requirements do not need a maintenance agreement with the RCWD.

Some ponds (P-1, P-3 and P-14) will be constructed by the developer but will be maintained by the city once constructed and stabilized to the City's satisfaction. The individual Developers are not known at this time.

The infiltration BMPs in Areas 3 and 4 will be responsibility of the Developer. The Developer will need to record a Stormwater Operation and Maintenance Agreement (see RCWD website for template) with RCWD. Submittals must include an exhibit and/or legal description that identify the BMPs. The Agreement must be submitted to the RCWD for review and approval prior to submitting it to the County Recorder's office.

Table 6.1 summarizes responsible entity for the long-term operations and maintenance of the stormwater management and drainage facilities. Pretreatment features to provide grit removal will be constructed by developers and maintained by developers per each outlot.

Table 6-1: Pond Responsibilities

Feature	Constructed by	Long-Term O&M Responsibility
P-1	Developer	City of Arden Hills
P-2	Ramsey County's Contractor	City of Arden Hills
P-3	Developer	City of Arden Hills
P-4	Ramsey County's Contractor	City of Arden Hills
P-5		
P-6		
P-7		
P-8		
P-9		
P-10		
P-11		
P-12		
P-13		
P-14	Developer	City of Arden Hills
P-15	Ramsey County's Contractor	MnDOT
Infiltration BMP outlot A	Developer	Developer
CRH-1	Ramsey County's Contractor	MnDOT
CRH-2	Ramsey County's Contractor	MnDOT
CRH-3	Ramsey County's Contractor	MnDOT
Infiltration BMP outlot J	Developer	Developer
Infiltration BMP outlot K	Developer	Developer
Pretreatment BMP, grit removal	Developer	Developer

6.4 MAINTENANCE GUIDELINES FOR STORMWATER FEATURES

6.4.1 Ponds

Stormwater management and drainage facilities shall be maintained as necessary to ensure the stormwater system functions as it was originally designed, including:

- ▲ Removing accumulated sediment from low areas
- ▲ Regularly inspecting pipes, structures, and embankments for structural integrity
- ▲ Removing trash, debris, or other obstructions from site
- ▲ Clearing tributary areas of invasive or nuisance/undesirable vegetation
- ▲ Addressing any erosion issues, including restoring slope protection and riprap

Site access routes shall be well maintained and clear of obstruction. Detailed guidelines for maintenance is provided in the Minnesota Stormwater Manual. Stormwater ponds will not function properly unless a pre-treatment device is properly sized, installed and maintained as necessary to ensure the device functions as it was originally designed.

6.4.2 Infiltration BMPs

The Minnesota Stormwater Manual presents a multitude of options of infiltration BMPs for stormwater runoff. These devices will be selected, designed and installed after the CSMP is adopted. The Minnesota Stormwater Manual presents a variety of design guidelines, as well as operation and maintenance considerations for BMPs. Appendix C contains an example of the information available. Some infiltration BMPs include:

- ▲ Bioinfiltration/rain garden
- ▲ Infiltration basin
- ▲ Infiltration trench
- ▲ Permeable pavement
- ▲ Tree trench/tree box
- ▲ Underground infiltration

Some BMPs are easier to maintain than others. Infiltration BMPs will not function properly unless a pre-treatment device is properly sized, installed and maintained as necessary to ensure the device functions as it was originally designed. Infiltration BMPs must be maintained as necessary to ensure the device functions as it was originally designed.

6.4.3 Pre-Treatment (Grit Removal)

The Minnesota Stormwater Manual presents a multitude of options to pre-treat stormwater runoff prior to discharging it into stormwater BMP. Some pre-treatment systems include:

- ▲ Flow through device
- ▲ Swirl Chambers
- ▲ Trench Forebays
- ▲ Level Spreaders
- ▲ Vegetated filter strips
- ▲ Forebays
- ▲ Vegetated swales
- ▲ SAFL baffle

- ▲ off-line deep sump catch basins
- ▲ Proprietary Products

Detailed guidelines for maintenance is provided in the Minnesota Stormwater Manual.

6.5 EASEMENTS FOR PONDS

Preliminary outlots have been identified on the concept preliminary plat (**Figure 5-2**). At this time the drainage and utility easements are not defined, due to the concept nature of the stage the project is in. It is one of the objectives of the plat development process to define the drainage and utility easement corridors. It is anticipated, as the platting documents for submission are produced, these corridors will be addressed and illustrated as well as other outlots or easements that may need to account for ponds, water features, stormwater retention/detention features as well as sites for significant utility structures such as substations, lift stations, etc. that may be dedicated to the public or privately owned based how it will be managed and maintained.

Some of the areas designated for MnDOT right-of-way may be right of way dedications in the plat and may not be considered tracts or outlots in the final version. Right-of-way or easements may also be needed in the portion north of Rice Creek area's westerly road if this is to become public or have public utilities.

The public infrastructure improvement grading shows the location of trails. All ponds located in the Natural Resources Corridor (NRC) are accessible by trails for long-term operations and maintenance. **Table 6.1** summarizes who is responsible for construction and long-term operations and maintenance for each stormwater pond as drainage and utility easement corridors are identified.

If a development project submits a RCWD permit application that has an impervious acreage less than or equal to the values by outlot in **Table 6.2**, the requirements of Rule C.6 (Water Quality Treatment) and Rule C.7 (Peak Stormwater Runoff Control) will be considered satisfied.

Table 6-2: Impervious Summary by Outlot

Outlot ID	Area (acres)	Impervious Area (acres)	Additional breakdowns
Outlot A	40.1	34.0	
Old Highway 8 extension Road	4.8	4.8	
Spine Road	24.0	24.0	
Outlot B (Creek)	45.9	18.1	15.2 acres in residential
Trail Dedication	1.1	0.7	0.4 acres residential, remaining in trails
Outlot C (NRC)	47.6	5.2	2.1 acres in civic, remaining in recreational and trails, realigned Creek
Outlot D (Town)	58.8	28.6	16.9 acres in residential, 9.7 acres in mixed use, 1.2 acres in commercial, remaining in recreational
Outlot E (Hill)	73.3	27.0	23.9 acres in residential
Outlot F (Flex Business East)	28.7	23.5	
Outlot G (Flex Business South)	31.7	21.6	
Outlot H (NRC)	3.5	0.4	0.4 acres in trails
Outlot I (Flex Business North)	18.2	15.4	
Outlot J (Corporate)	20.0	17.0	
Outlot K (Retail)	20.0	16.5	16.4 acres in retail, remaining in recreational
Outlot L (Creek Meander)	2.7	0.2	0.2 acres in trails, realigned creek

The surface area at normal water elevation for ponds/wetlands is not considered part of the impervious areas by outlot in **Table 6.2**.

6.6 RECORD DRAWINGS FOR PONDS

As required by the City of Arden Hills, upon completion of construction, the responsible party shall retain a duly licensed Professional Engineer in the State of Minnesota to certify that the as-built plans are consistent with design.

The County will require the Contractor to submit as-built drawings of the stormwater ponds, outlet control structures and stormwater system. The as-built drawings are submitted to the City of Arden Hills and the RCWD. At the end of the correction period/warranty period in the construction contract, the County will transfer responsibility of operation and maintenance of the stormwater ponds, outlet control structures and stormwater system to the City of Arden Hills.

Figures

ES-1. Management Summary By Parcel

- 2- 1. Site Location Map
- 2- 2. Site Detail Map

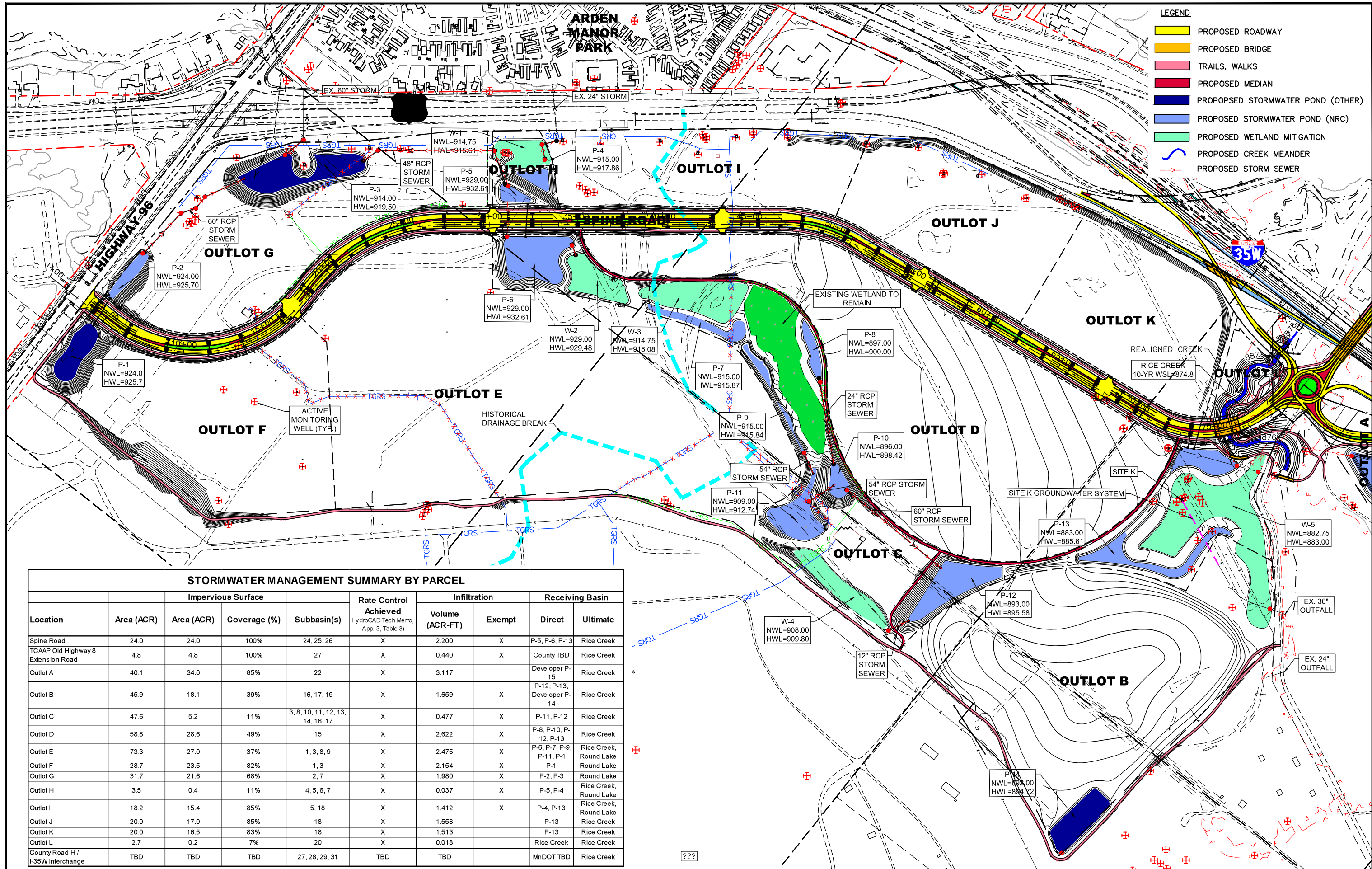
- 3- 1. (FINAL AUAR) Fully Developed Conditions: Landuse Map
- 3- 2. Pre-development (2012): Wetlands Map (Kimley-Horn)
- 3- 3. SURGO Soils Map
- 3- 4. Surficial Geology Map
- 3- 5. TCAAP Four Hydrogeologic Units
- 3- 6. TCAAP Concept Site Geology
- 3- 7. Soil Borings Map (Oct. 3, 2007)
- 3- 8. Ongoing Army Systems (Unit 1)
- 3- 9. Ongoing Army Systems (Unit 3)

- 4- 1. Pre-development (2012): Subwatershed Map
- 4- 2. Pre-development (2012): Subwatershed Map with Curve Number
- 4- 3. Pre-development (1987): Groundwater Gradient Map (Unit 1)
- 4- 4. Pre-development (2012): Groundwater Elevation Map (Unit 1)
- 4- 5. Public Infrastructure Improvements Phase 1:
Site Grading and Storm Drainage Plan (Kimley-Horn)
- 4- 6. Stormwater Pond and Wetland Flow Diagram
- 4- 7. Fully Developed Conditions: Subwatershed Map with Storm Sewers

- 5- 1. Regional BMP Map
- 5- 2. Preliminary Plat with Regional BMP Map
- 5- 3. Preliminary (June 2015) Soil Borings along Spine Road

- 6- 1. Preliminary Plat (Outlots) with Fully Developed Conditions Subwatersheds
- 6- 2. Pre-development (2012): Drainage Areas by Outfalls
- 6- 3. Fully Developed Conditions: Drainage Areas by Outfalls

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- LEGEND**
- PROPOSED ROADWAY
 - PROPOSED BRIDGE
 - TRAILS, WALKS
 - PROPOSED MEDIAN
 - PROPOSED STORMWATER POND (OTHER)
 - PROPOSED STORMWATER POND (NRC)
 - PROPOSED WETLAND MITIGATION
 - PROPOSED CREEK MEANDER
 - PROPOSED STORM SEWER

STORMWATER MANAGEMENT SUMMARY BY PARCEL



Location	Impervious Surface			Subbasin(s)	Rate Control Achieved <small>HydroCAD Tech Memo, App. 3, Table 3)</small>	Infiltration		Receiving Basin	
	Area (ACR)	Area (ACR)	Coverage (%)			Volume (ACR-FT)	Exempt	Direct	Ultimate
Spine Road	24.0	24.0	100%	24, 25, 26	X	2.200	X	P-5, P-6, P-13	Rice Creek
TCAAP Old Highway 8 Extension Road	4.8	4.8	100%	27	X	0.440	X	County TBD	Rice Creek
Outlot A	40.1	34.0	85%	22	X	3.117		Developer P-15	Rice Creek
Outlot B	45.9	18.1	39%	16, 17, 19	X	1.659	X	P-12, P-13, Developer P-14	Rice Creek
Outlot C	47.6	5.2	11%	3, 8, 10, 11, 12, 13, 14, 16, 17	X	0.477	X	P-11, P-12	Rice Creek
Outlot D	58.8	28.6	49%	15	X	2.622	X	P-8, P-10, P-12, P-13	Rice Creek
Outlot E	73.3	27.0	37%	1, 3, 8, 9	X	2.475	X	P-6, P-7, P-9, P-11, P-1	Rice Creek, Round Lake
Outlot F	28.7	23.5	82%	1, 3	X	2.154	X	P-1	Round Lake
Outlot G	31.7	21.6	68%	2, 7	X	1.980	X	P-2, P-3	Round Lake
Outlot H	3.5	0.4	11%	4, 5, 6, 7	X	0.037	X	P-5, P-4	Rice Creek, Round Lake
Outlot I	18.2	15.4	85%	5, 18	X	1.412	X	P-4, P-13	Rice Creek, Round Lake
Outlot J	20.0	17.0	85%	18	X	1.558		P-13	Rice Creek
Outlot K	20.0	16.5	83%	18	X	1.513		P-13	Rice Creek
Outlot L	2.7	0.2	7%	20	X	0.018			Rice Creek
County Road H / I-35W Interchange	TBD	TBD	TBD	27, 28, 29, 31	TBD	TBD		MnDOT TBD	Rice Creek



COMPREHENSIVE STORMWATER MANAGEMENT PLAN

MANAGEMENT SUMMARY □ □ **PARCEL**
FIGURE ES-□

Legend

-  427-Acre Boundary
-  2012 Conditions Drainage Break



2012 Aerial Photograph (Source: ESRI)

1,500 750 0 1,500 Feet

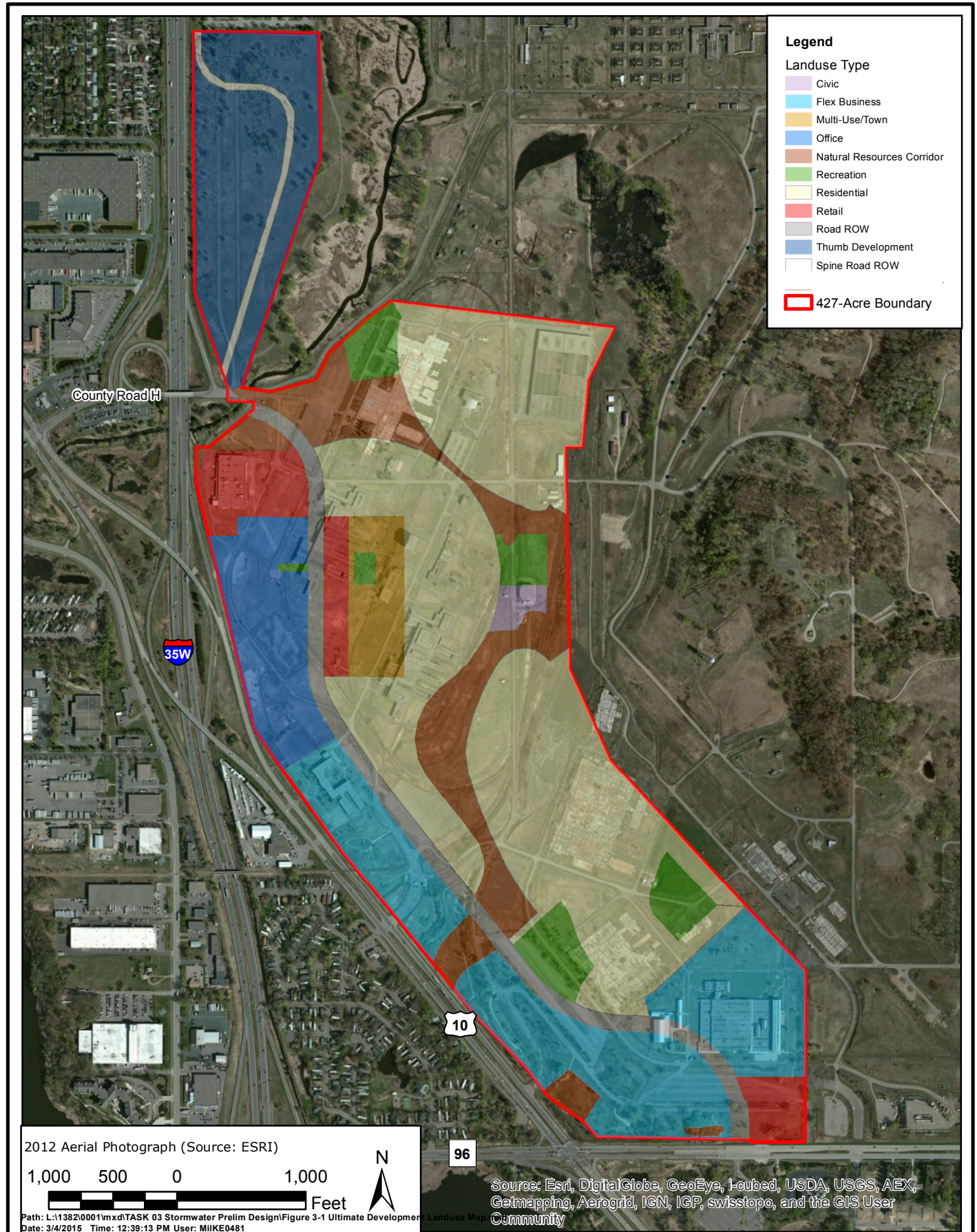
Path: L:\1382\0001\mxd\TASK 03 Stormwater Prelim Design\Figure 2-2 Site Detail Map.mxd
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Source: Esri, DigitalGlobe, GeoEye, i-cubed, USDA, USGS, AEX, Getmapping, Aerogrid, IGN, IGP, swisstopo, and the GIS User Community

RAMSEY COUNTY
Site Detail Map



JUNE 2015
Figure 2-2



Legend

Landuse Type

- Civic
- Flex Business
- Multi-Use/Town
- Office
- Natural Resources Corridor
- Recreation
- Residential
- Retail
- Road ROW
- Thumb Development
- Spine Road ROW

427-Acre Boundary

2012 Aerial Photograph (Source: ESRI)

1,000 500 0 1,000 Feet

Path: L:\1382\0001\mxd\TASK 03 Stormwater Prelim Design\Figure 3-1 Ultimate Development Landuse Map.aprx
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Source: Esri, DigitalGlobe, GeoEye, i-cubed, USDA, USGS, AEX, Getmapping, Aerogrid, IGN, IGP, swisstopo, and the GIS User Community

RAMSEY COUNTY

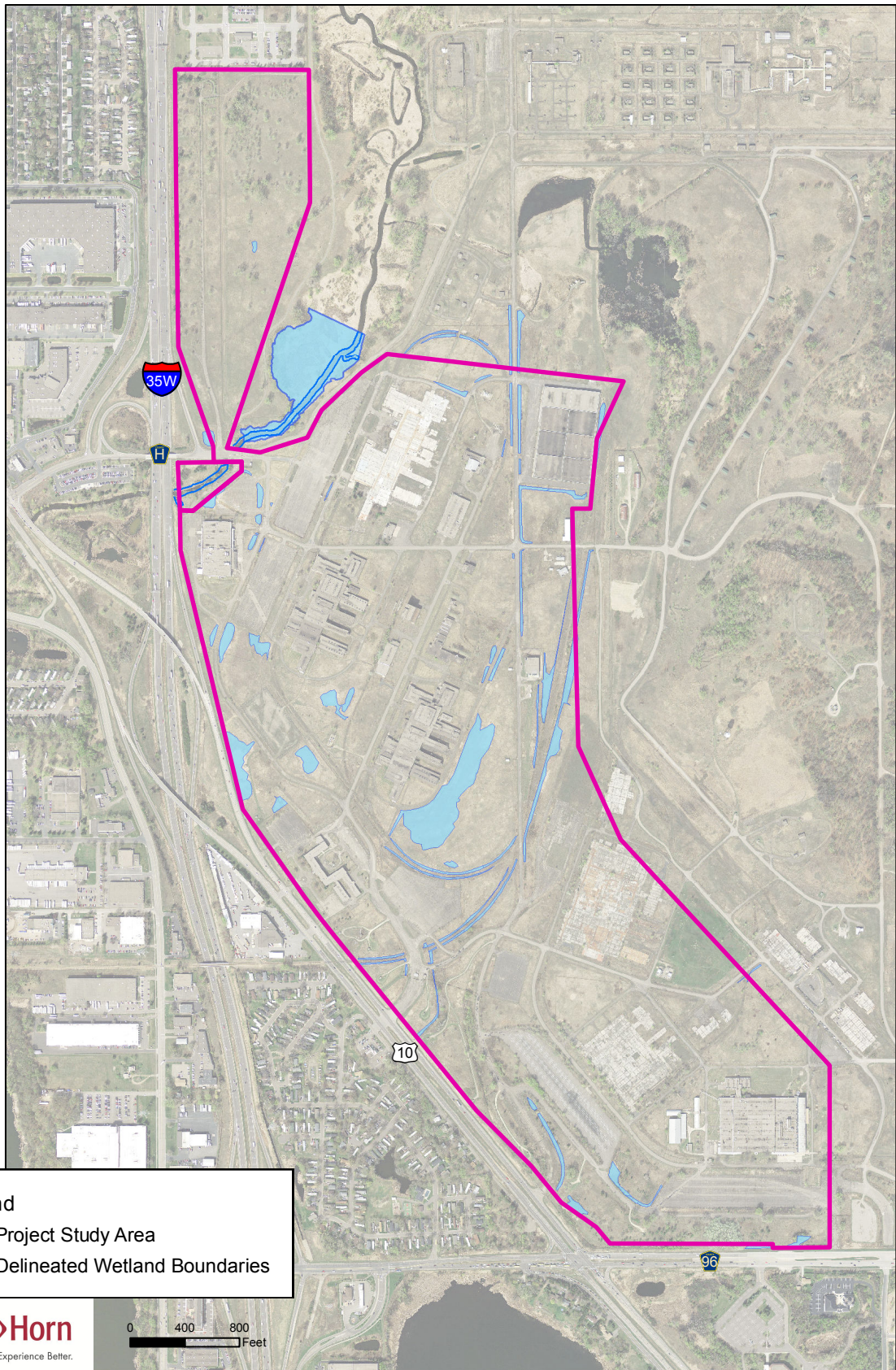
(FINAL AUAR) Fully Developed Conditions : Landuse Map



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Figure 3-1



Legend

- Project Study Area
- Delineated Wetland Boundaries

Kimley»Horn
 Expect More. Experience Better.

0 400 800
 Feet

Map Source: Kimley Horn

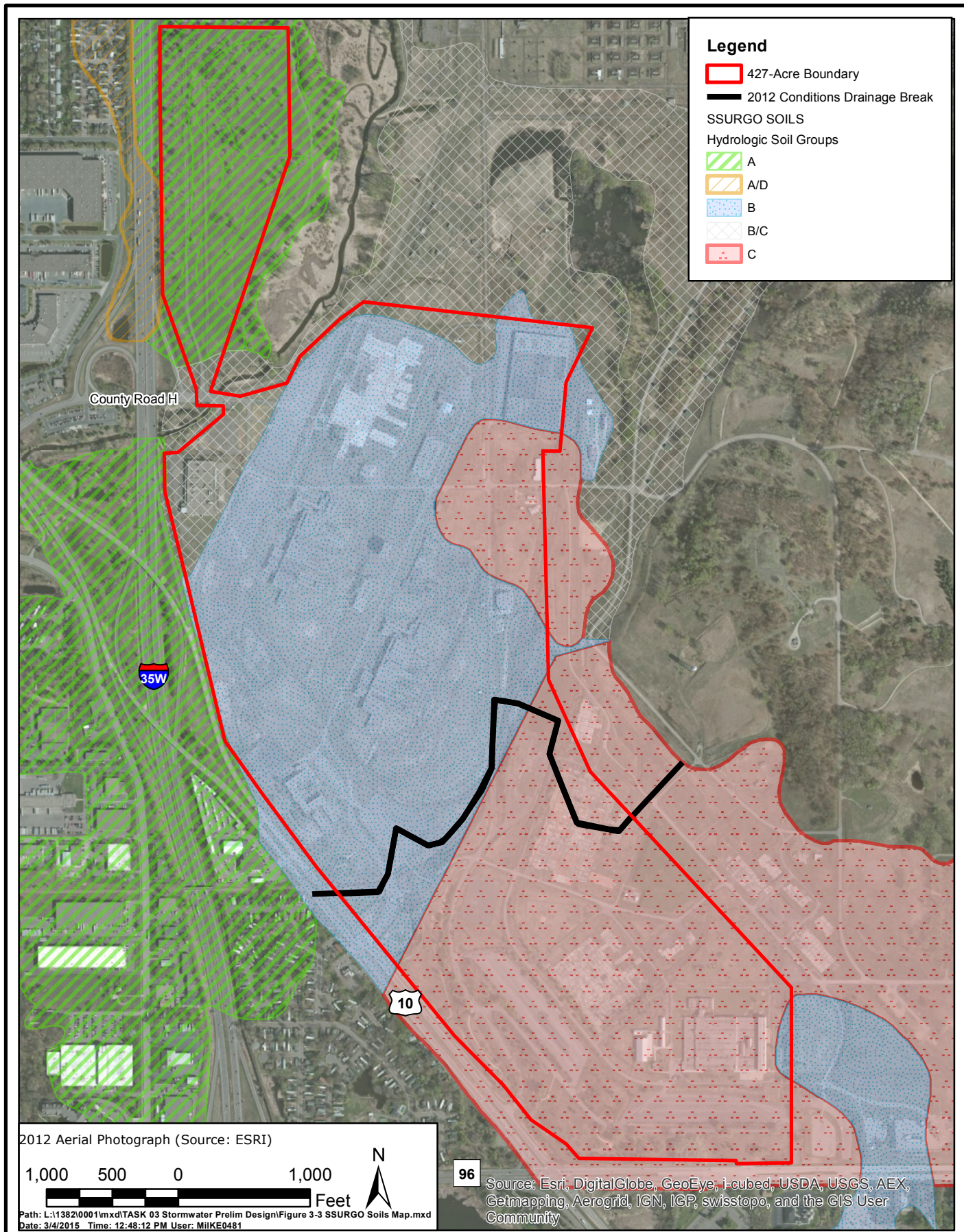
RAMSEY COUNTY

Pre-Development (2012): Wetlands Map (Kimley-Horn)

WENCK
 ASSOCIATES
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Figure 3-2



Legend

- 427-Acre Boundary
- 2012 Conditions Drainage Break

SSURGO SOILS

Hydrologic Soil Groups

- A
- A/D
- B
- B/C
- C

2012 Aerial Photograph (Source: ESRI)

1,000 500 0 1,000 Feet

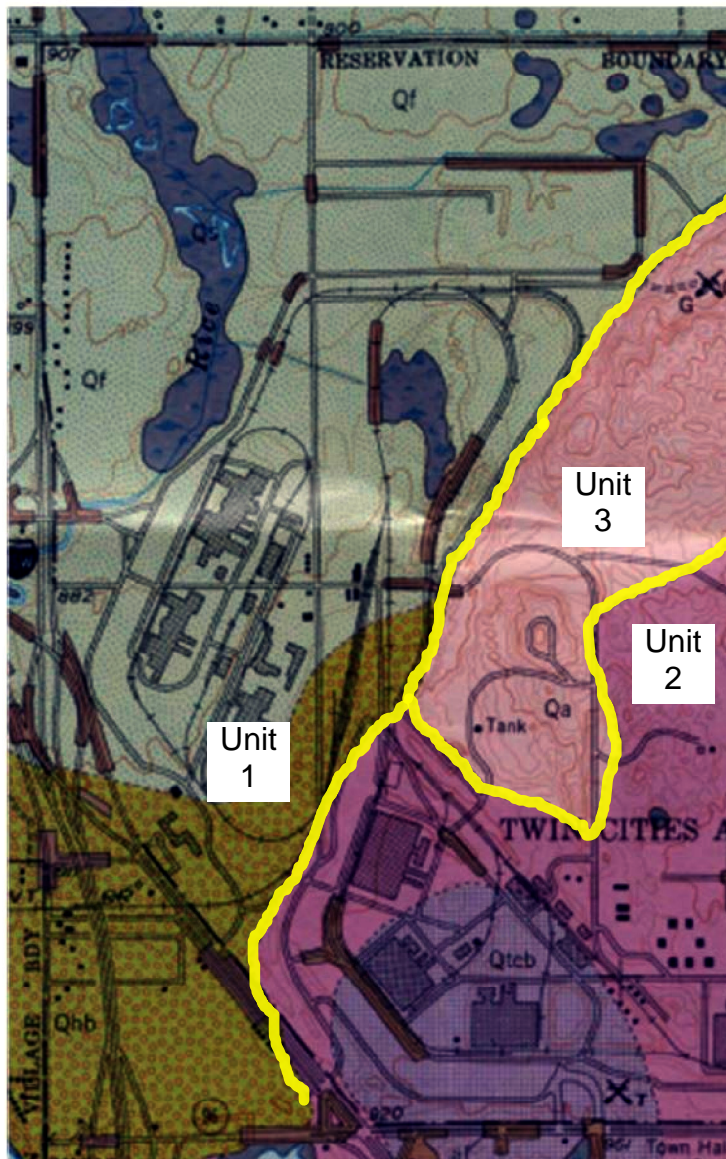
Path: L:\1382\0001\mxd\TASK 03 Stormwater Prelim Design\Figure 3-3 SSURGO Soils Map.mxd
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96 Source: Esri, DigitalGlobe, GeoEye, i-cubed, USDA, USGS, AEX, Getmapping, Aerogrid, IGN, IGP, swisstopo, and the GIS User Community

RAMSEY COUNTY
SSURGO Soils Map



JUNE 2015
 Figure 3-3



Qs
Swamp and Marsh Deposits

af
Artificial Fill

Qf
Fridley Formation

Unit 1: Water Bearing
(Low Yield)

Qa
Arsenal Sand

Unit 3: Water Bearing
(High Yield)

Qhb
New Brighton Formation

Unit 1: Water Bearing
(Low Yield)

Qtca Qtcb
Twin Cities Formation

Unit 2 (Aquitard)

Source: Stone, John E., "Geologic Map Series 2. Surficial Geology of the New Brighton Quadrangle, MN GM-2 (1966.)"

RAMSEY COUNTY
Surficial Geology Map

 **WENCK**
ASSOCIATES
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Figure 3-4

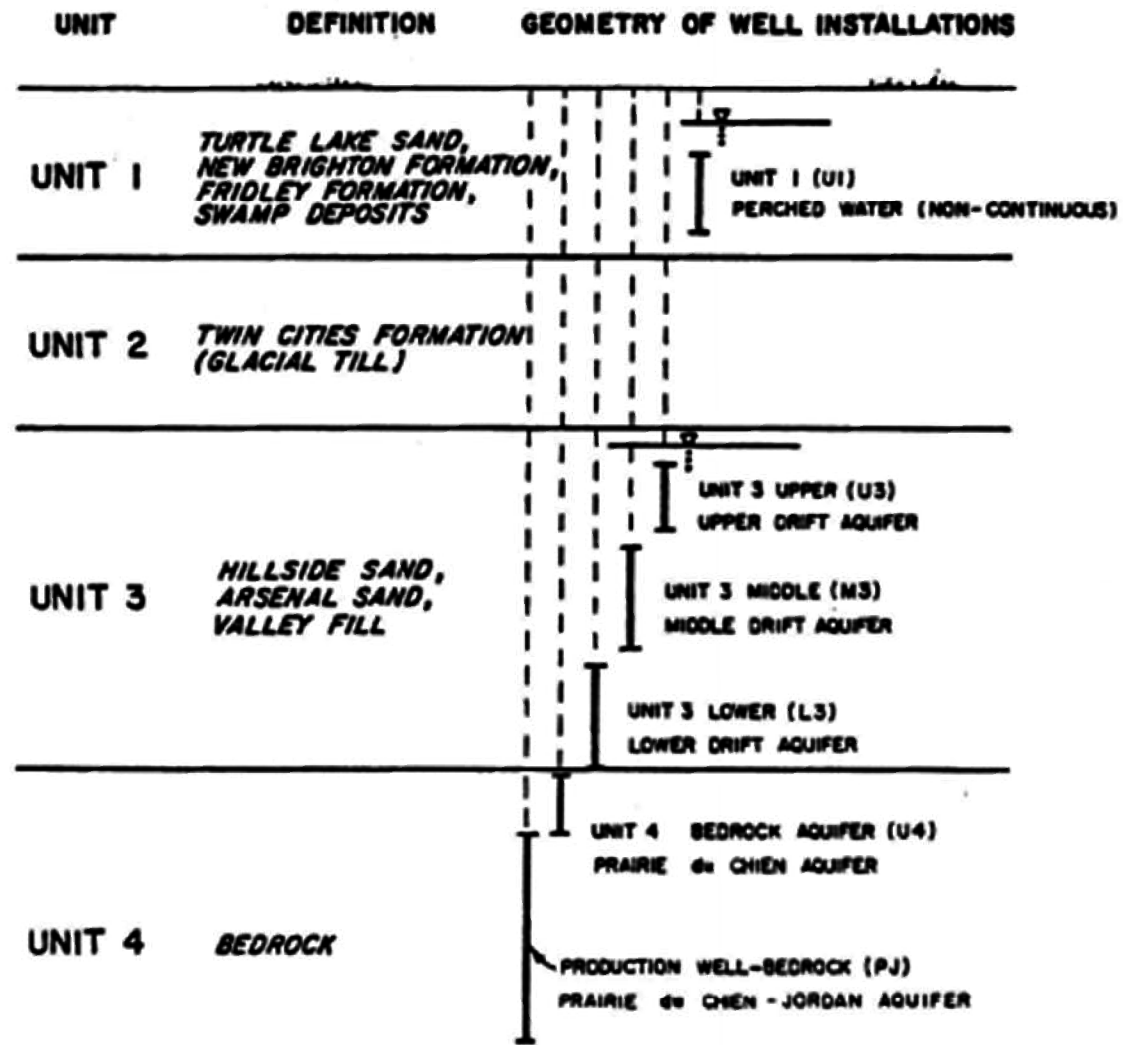


FIGURE 2.15 Illustration Showing Four Hydrologic Units at TCAAP
(Source: Ref. 5207)

Source: Wenck, "Five-year review report of the final remedy for the New Brighton/Arden Hills Superfund Site" Figure 2-15, August 2009

RAMSEY COUNTY

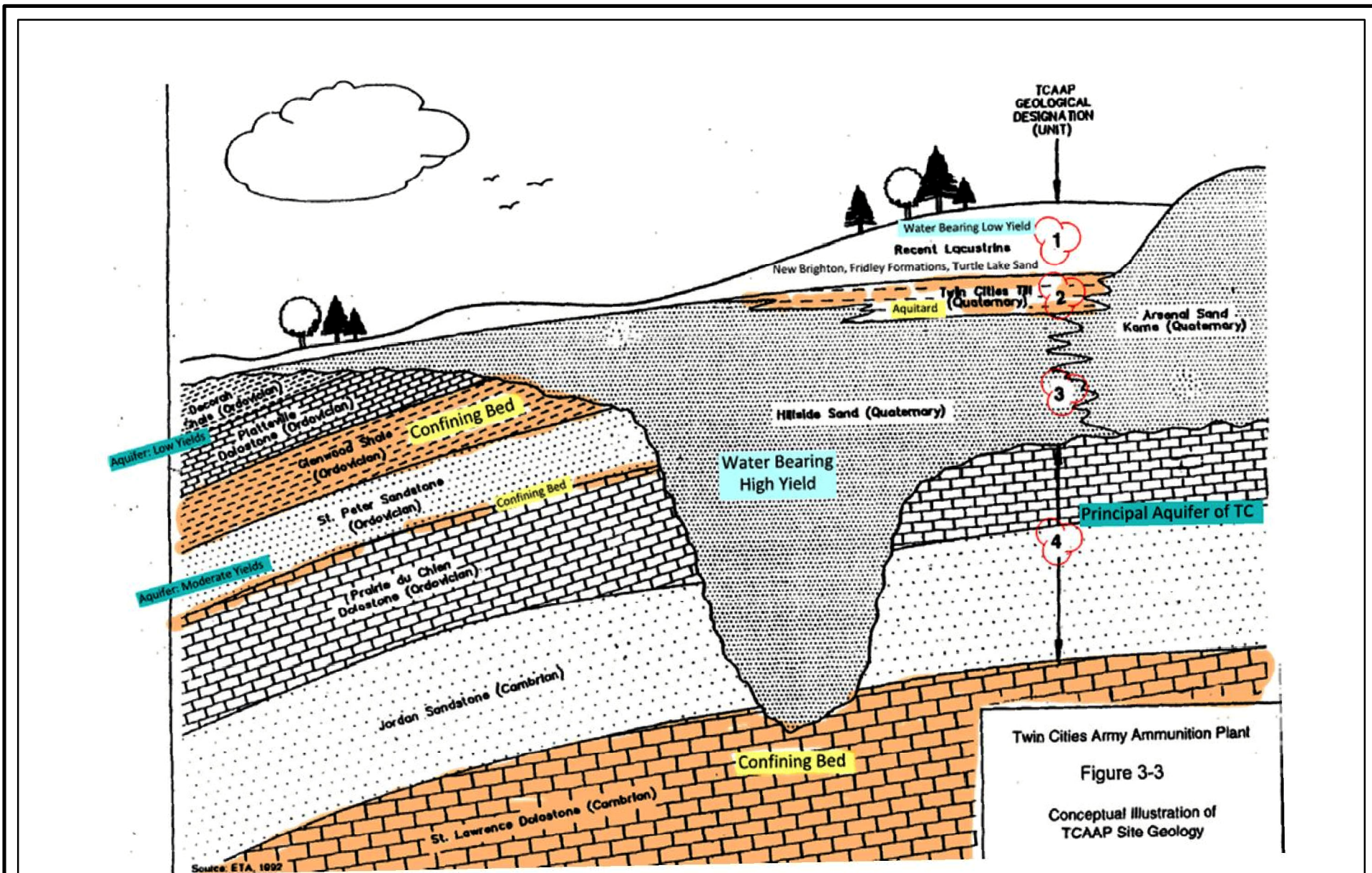
TCAAP Four Hydrologic Units



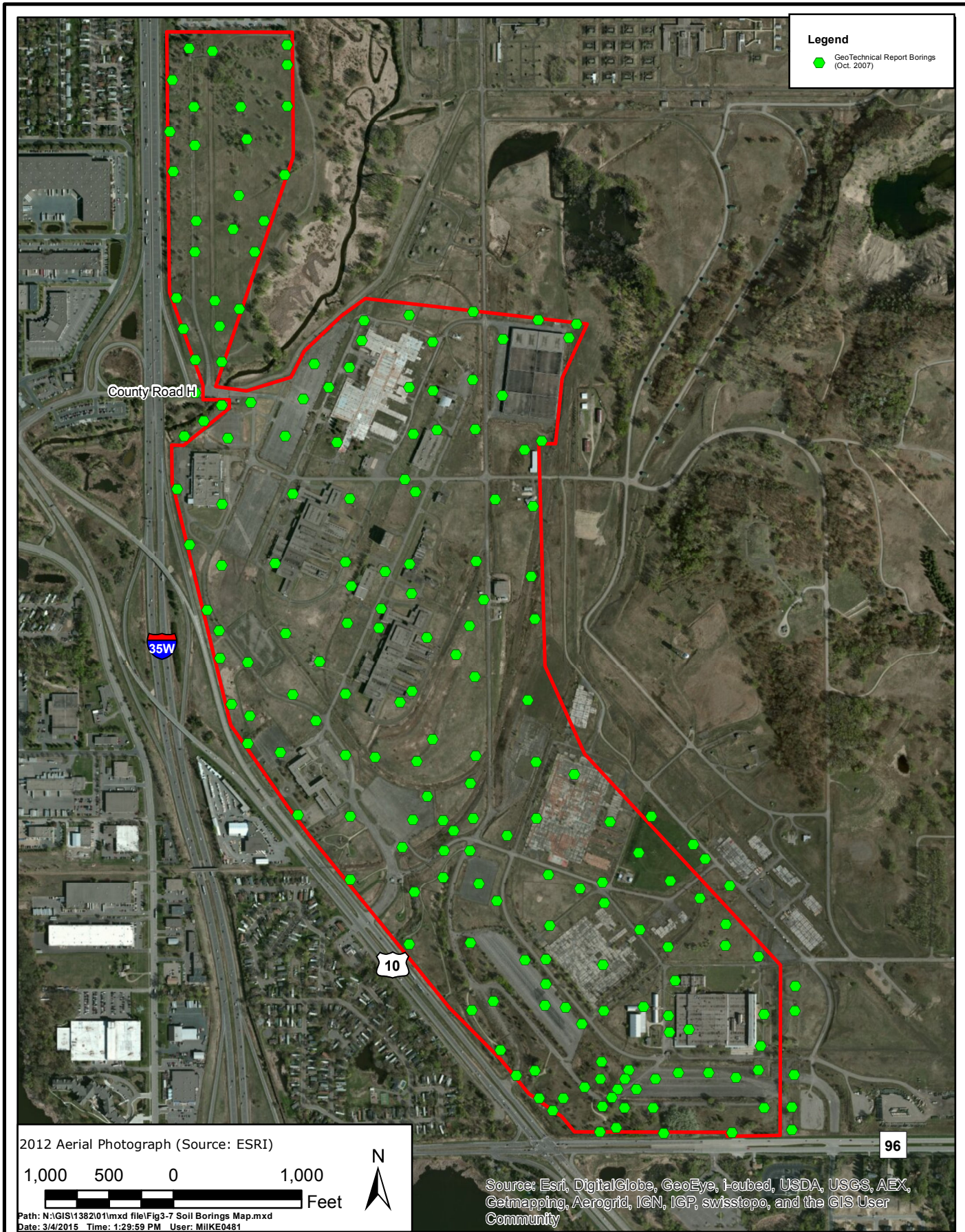
Responsive partner. Exceptional outcomes.

JUNE 2015

Figure 3-5



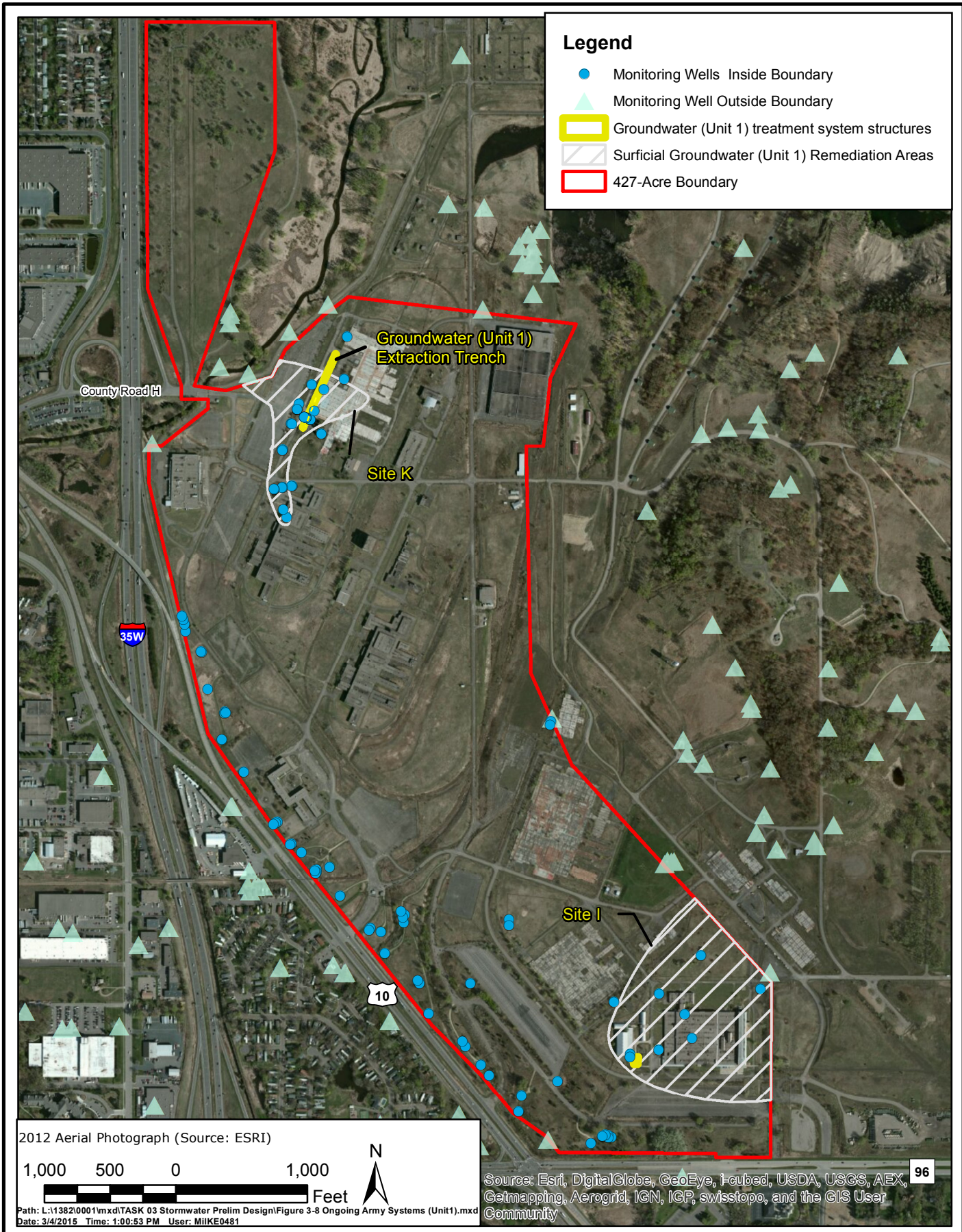
Source: Wenck, "Five-year review report of the final remedy for the New Brighton/Arden Hills Superfund Site" Figure 3-3, August 2009

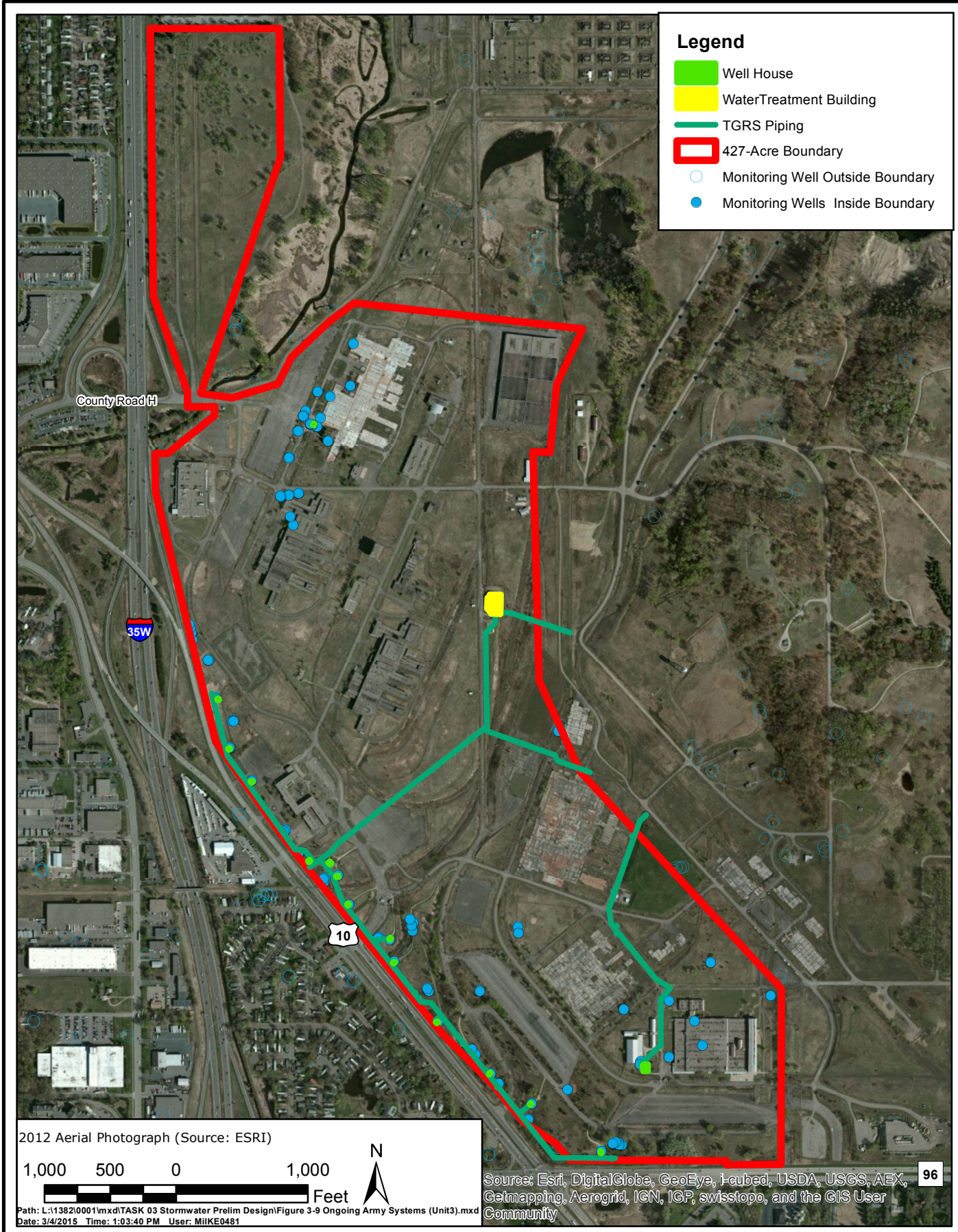


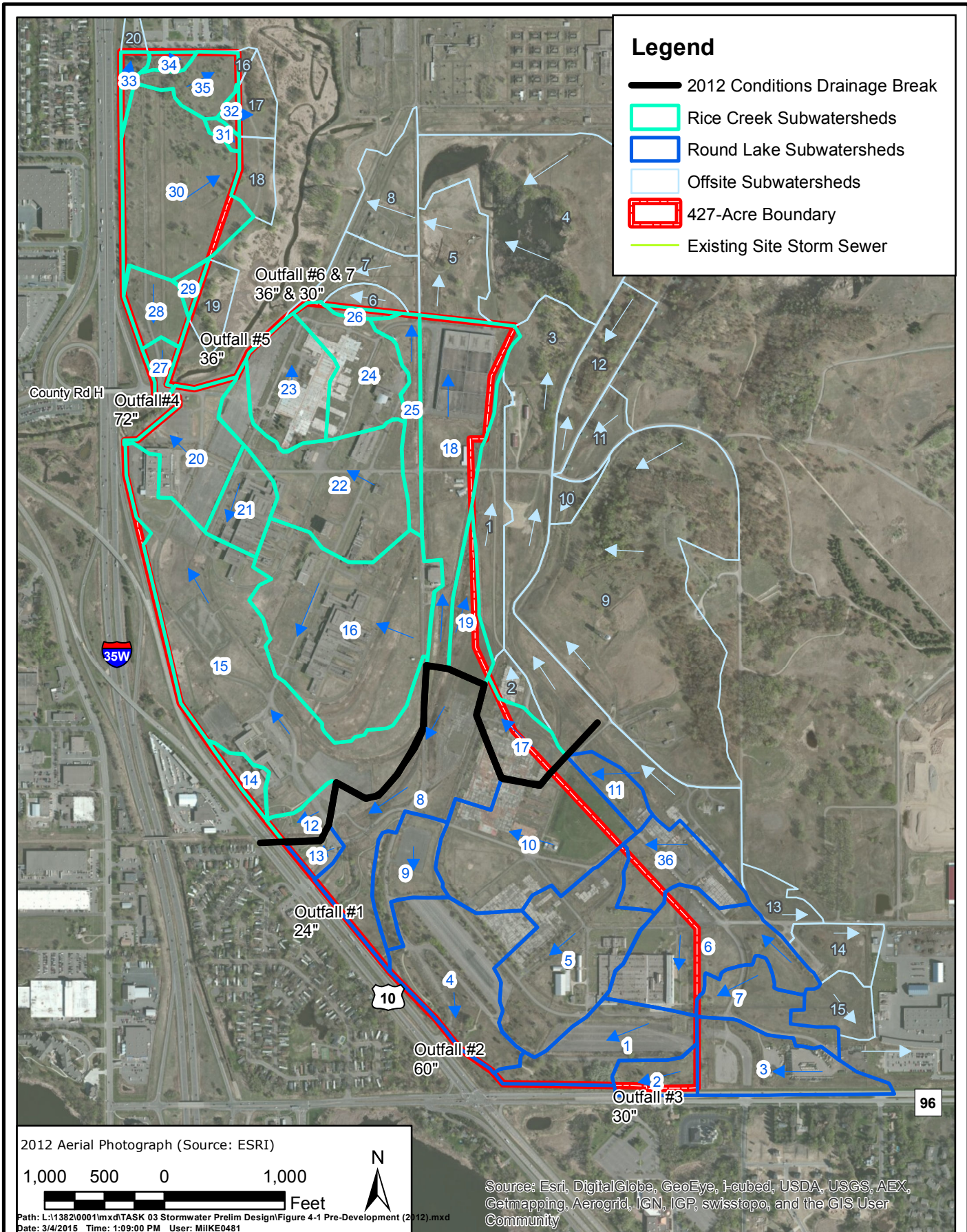
RAMSEY COUNTY
Soil Borings Map (Oct. 3, 2007)

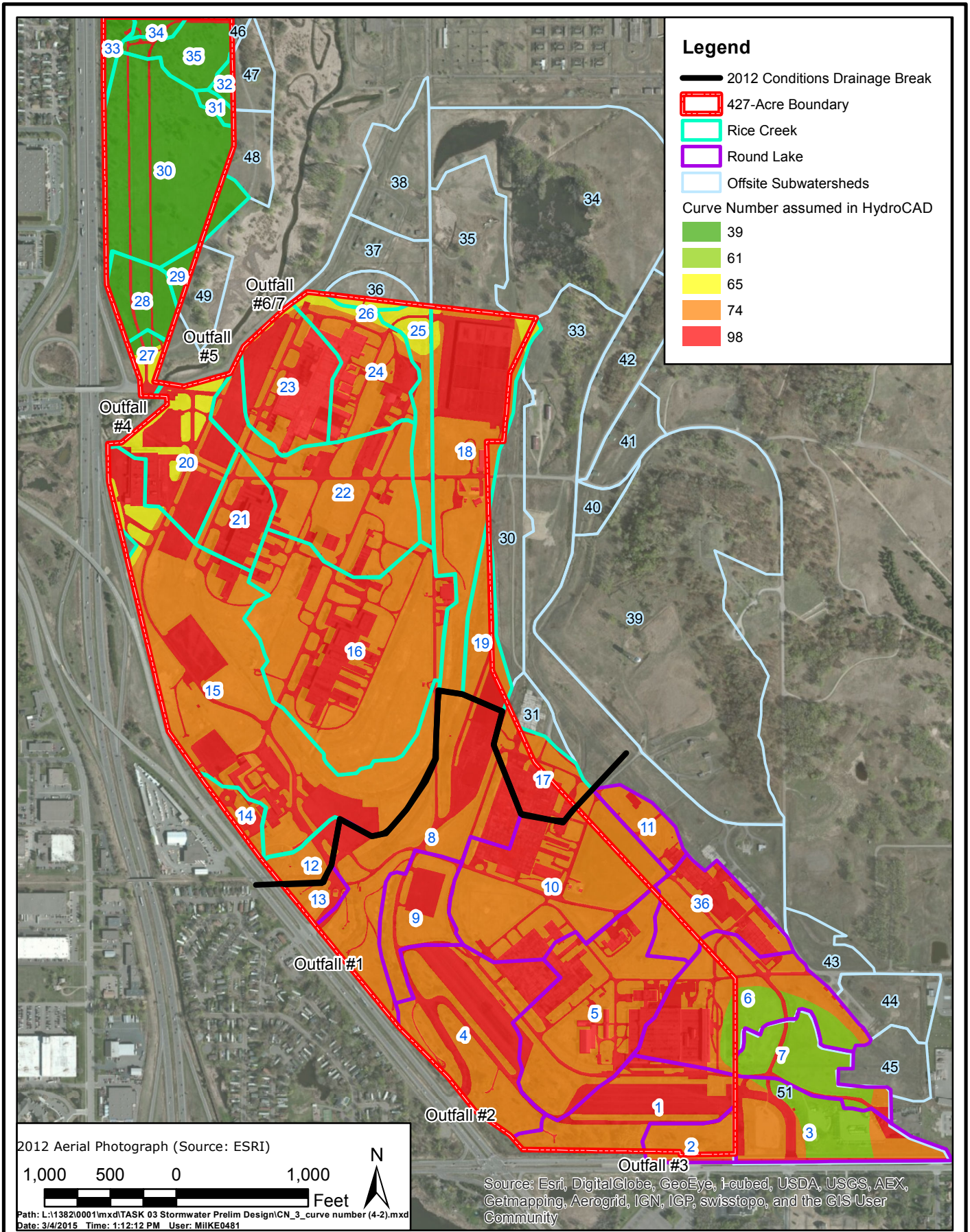


JUNE 2015
Figure 3-7









RAMSEY COUNTY

Pre-Development (2012): Subwatershed Map with Curve Number



JUNE 2015

Figure 4-2

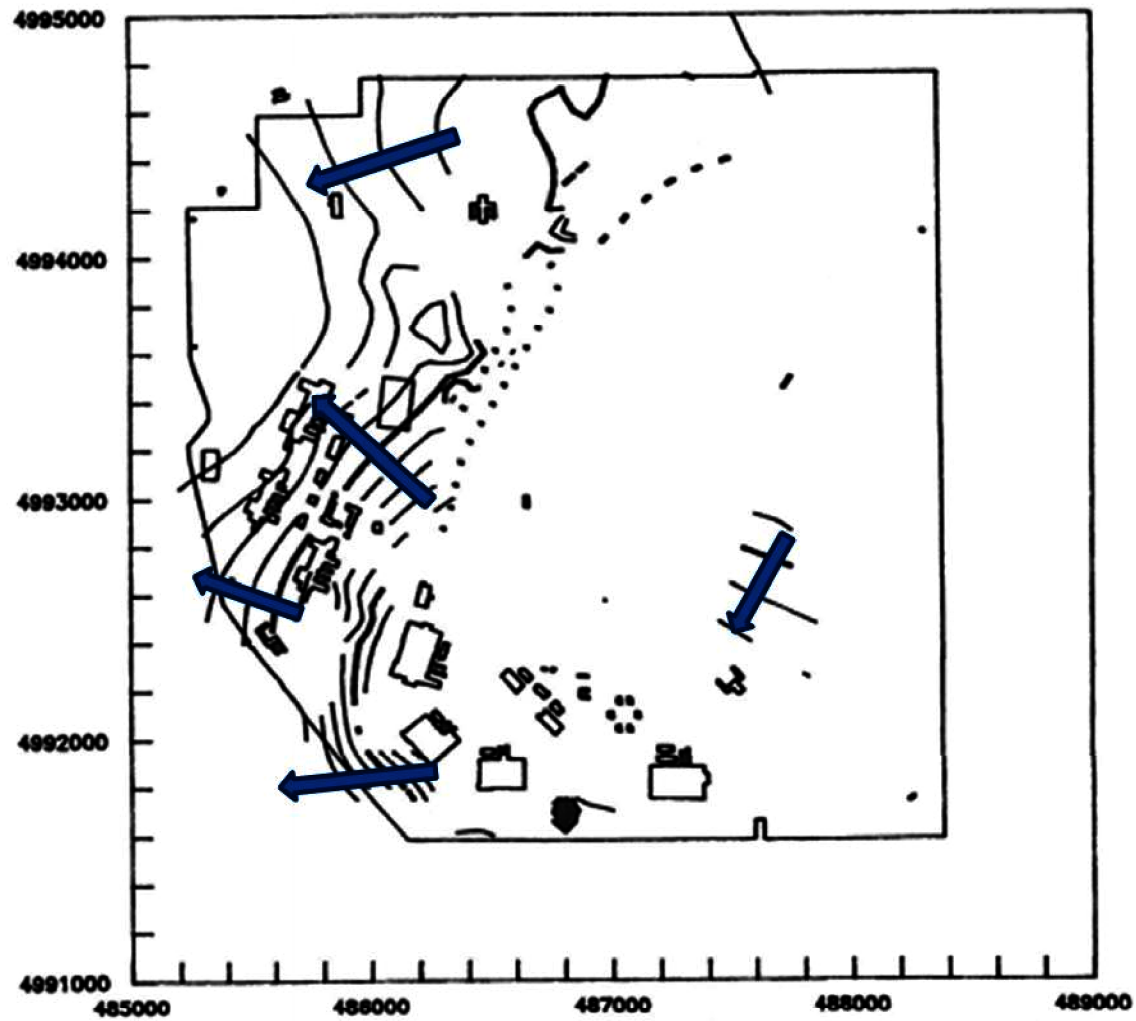
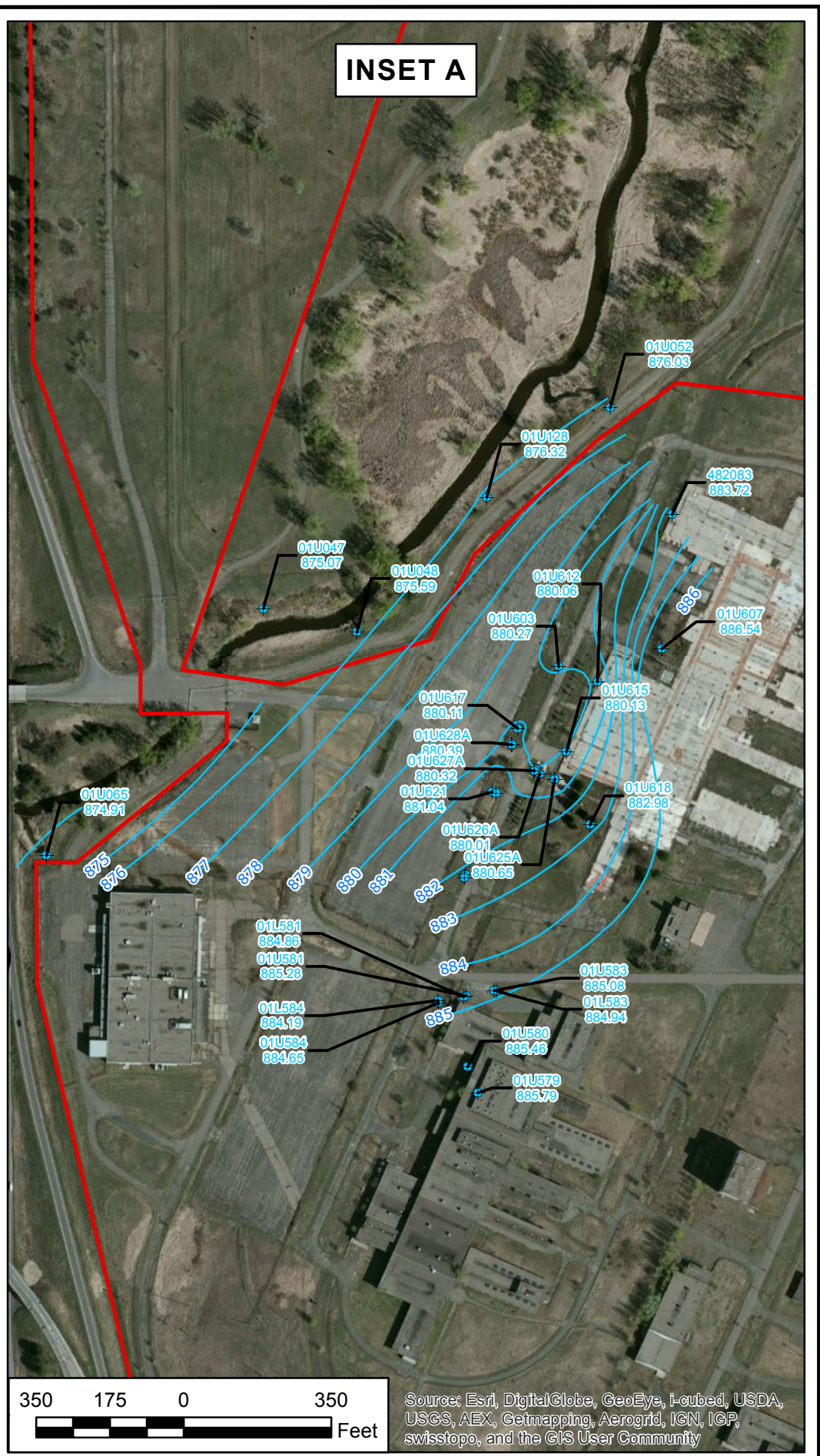


FIGURE 2.16 Water-Level Elevations for Hydrologic Unit 1 at TCAAP, February 17, 1987 (Source: USATHAMA database)

Source: Wenck, "Five-year review report of the final remedy for the New Brighton/Arden Hills Superfund Site" Figure 2-16, August 2009
 Path: L:\1382\0001\mxd\TASK 03 Stormwater Prelim Design\Fig4-3 Pre-Development (1987) Groundwater Gradient Map.mxd
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Source: Esri, DigitalGlobe, GeoEye, I-cubed, USDA, USGS, AEX, Getmapping, Aerogrid, IGN, IGP, swisstopo, and the GIS User Community

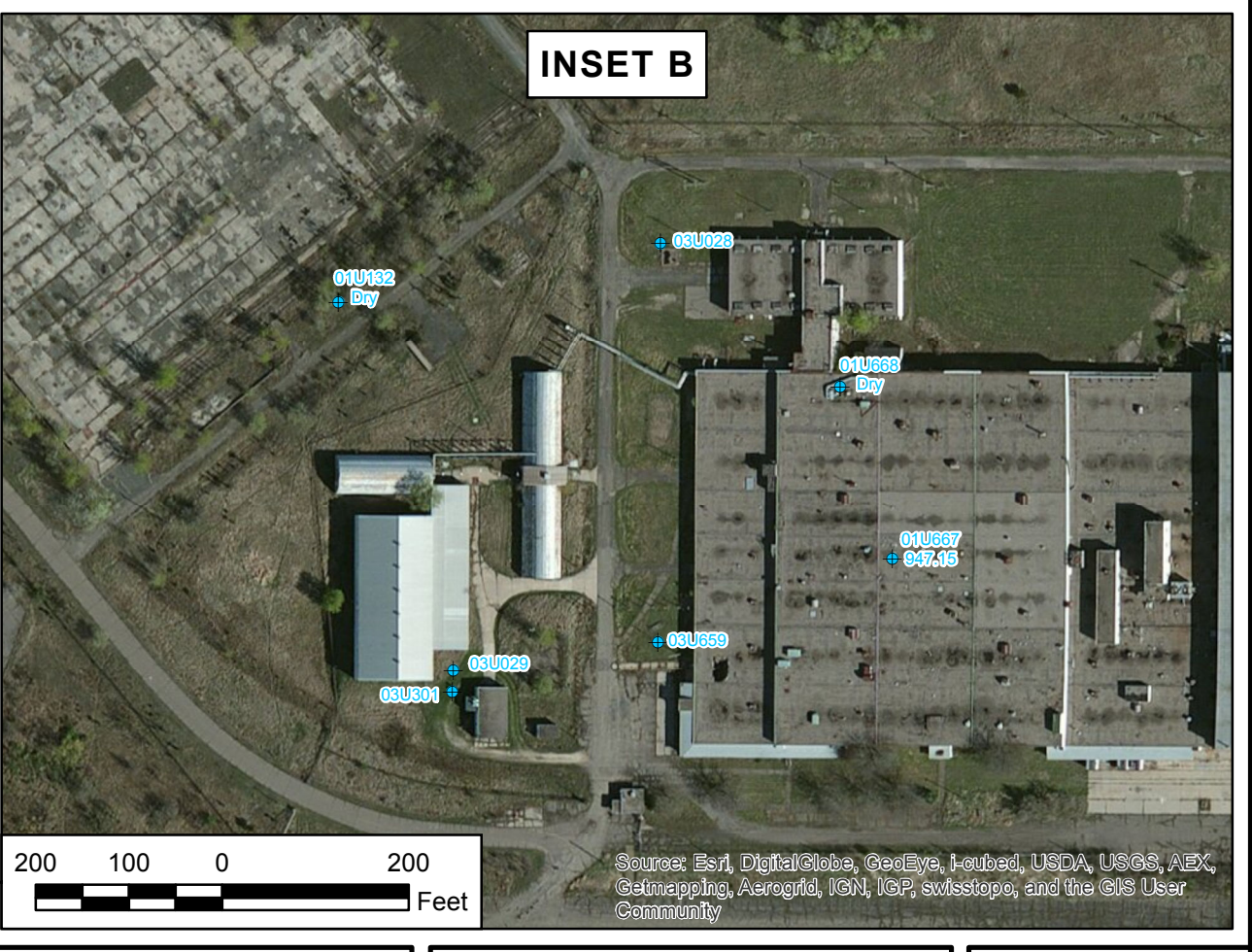
Legend

- + Unit 1 Wells
- 2012 APR Groundwater Elevation Contours (ft)
- 427-Acre Boundary

Note: Various Unit 1 groundwater wells have been sealed, since 2012. This figure only shows symbolized well locations for those wells that have not been abandoned and sealed. The unique well identifications and the water level observations are provided in this figure as label leaders. The source of this information is the 2012 TCAAP Annual Performance Report (APR).

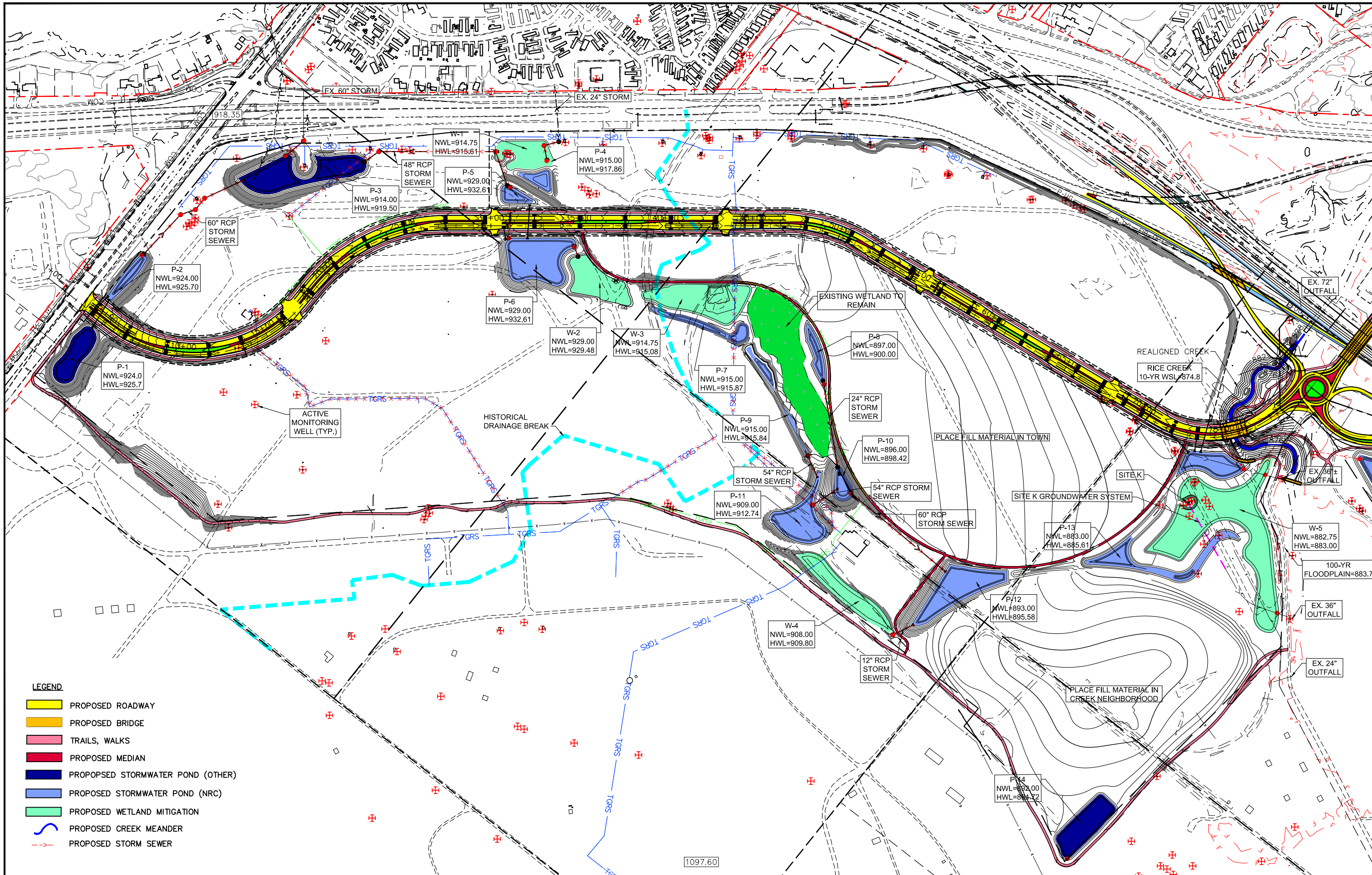
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Source: Esri, DigitalGlobe, GeoEye, I-cubed, USDA, USGS, AEX, Getmapping, Aerogrid, IGN, IGP, swisstopo, and the GIS User Community

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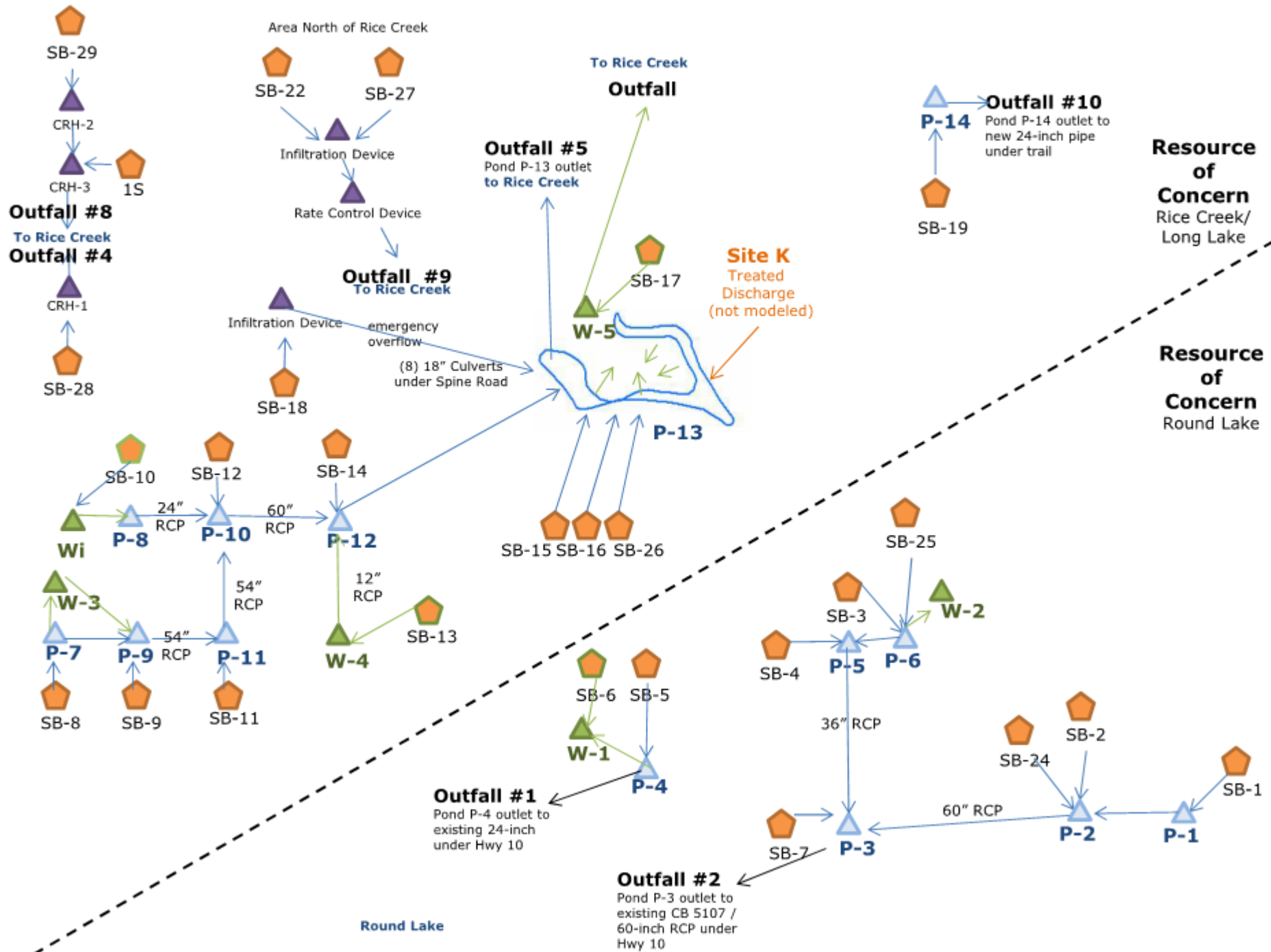


- LEGEND**
- PROPOSED ROADWAY
 - PROPOSED BRIDGE
 - TRAILS, WALKS
 - PROPOSED MEDIAN
 - PROPOSED STORMWATER POND (OTHER)
 - PROPOSED STORMWATER POND (NRC)
 - PROPOSED WETLAND MITIGATION
 - PROPOSED CREEK MEANDER
 - PROPOSED STORM SEWER



COMPREHENSIVE STORMWATER MANAGEMENT PLAN

SITE GRADING AND STORM DRAINAGE IMPROVEMENTS
FIGURE 4-5



T:\1382 KimleyHorn\01 TCAAP\TASK 03 Stormwater Prelim Design\CSMP_Final\Figures\Figure 4-6 Stormwater Pond and Flow Diagram_31 JUL 2015.ppt

RAMSEY COUNTY

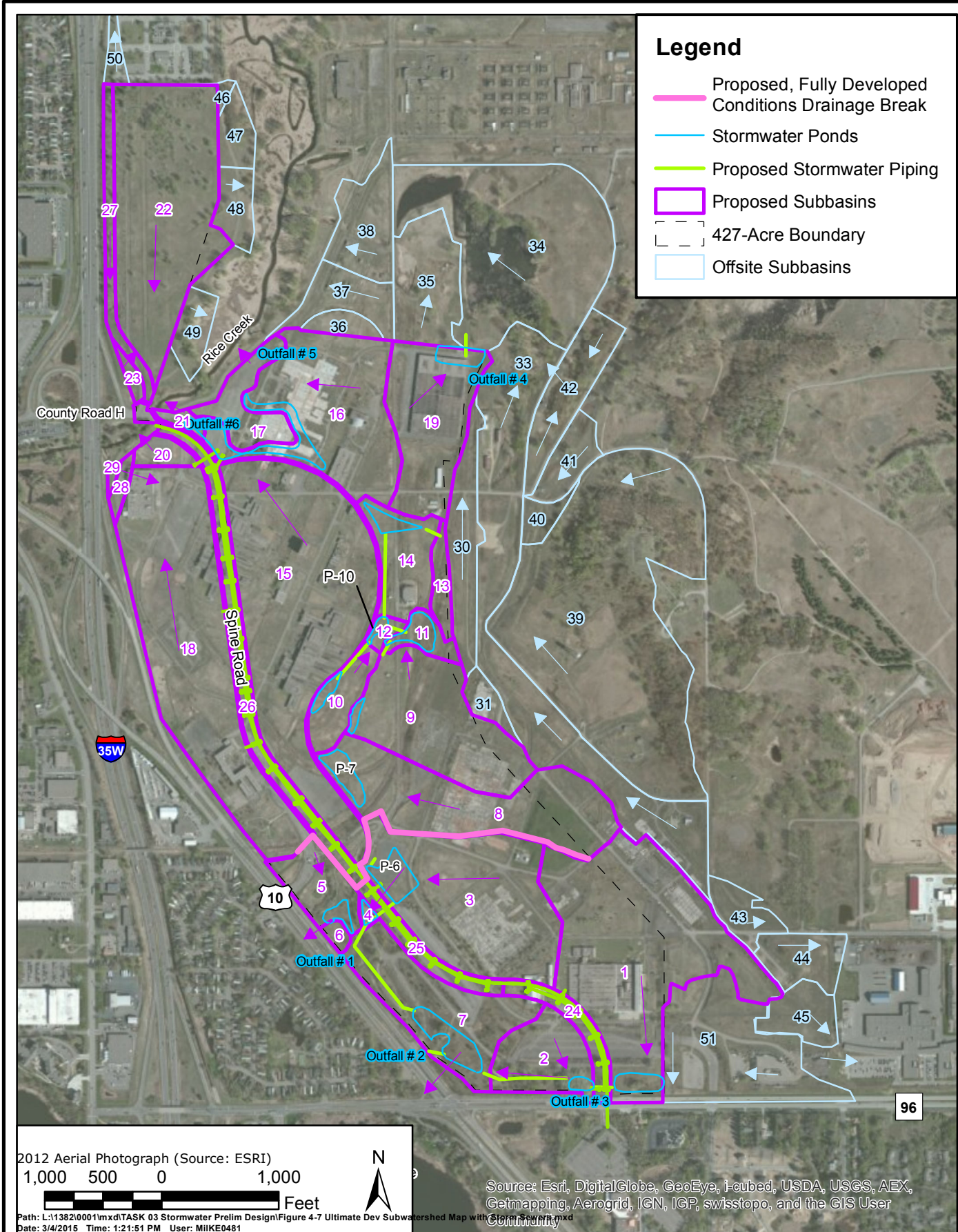
Stormwater Pond and Wetland Flow Diagram



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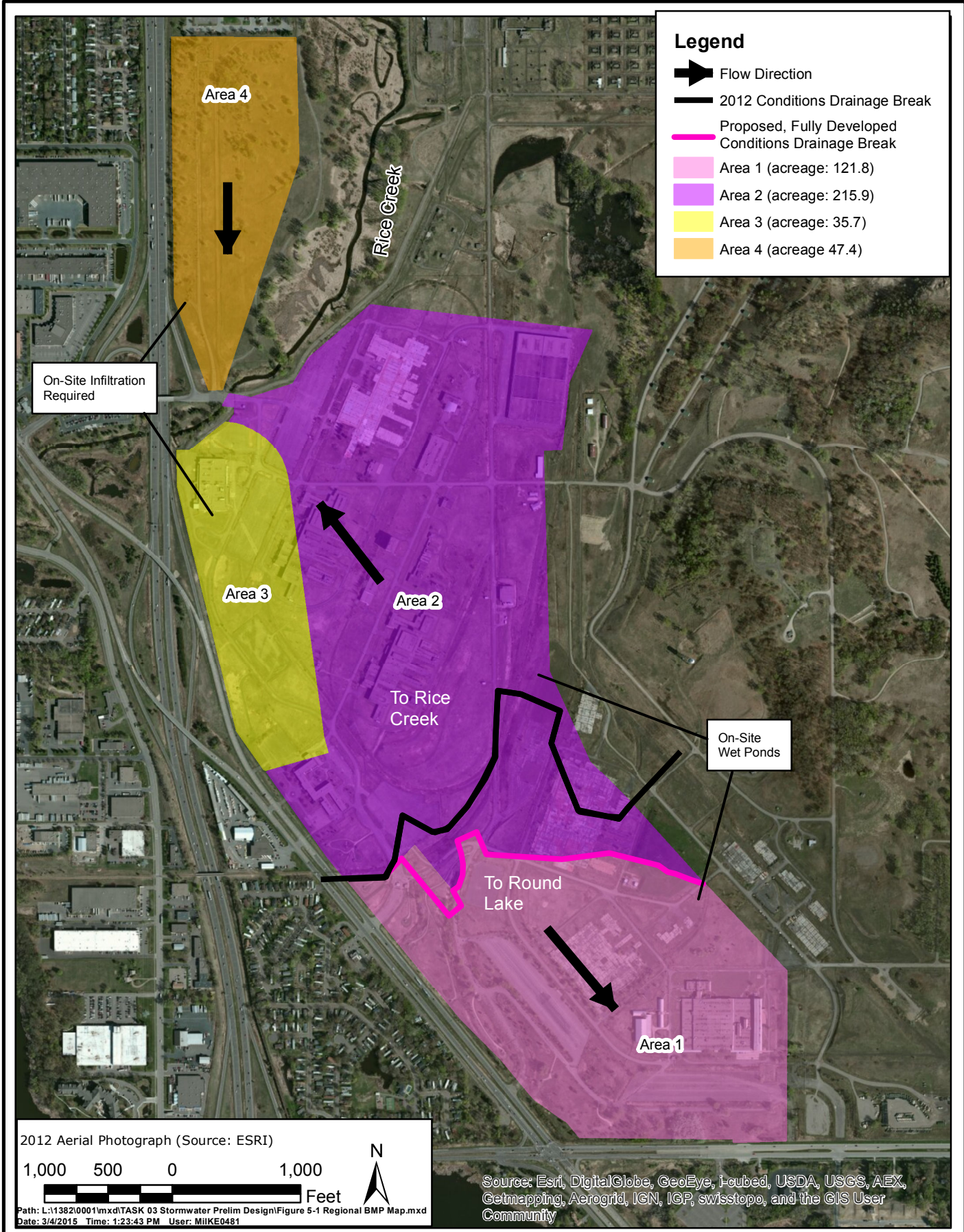
Figure 4-6

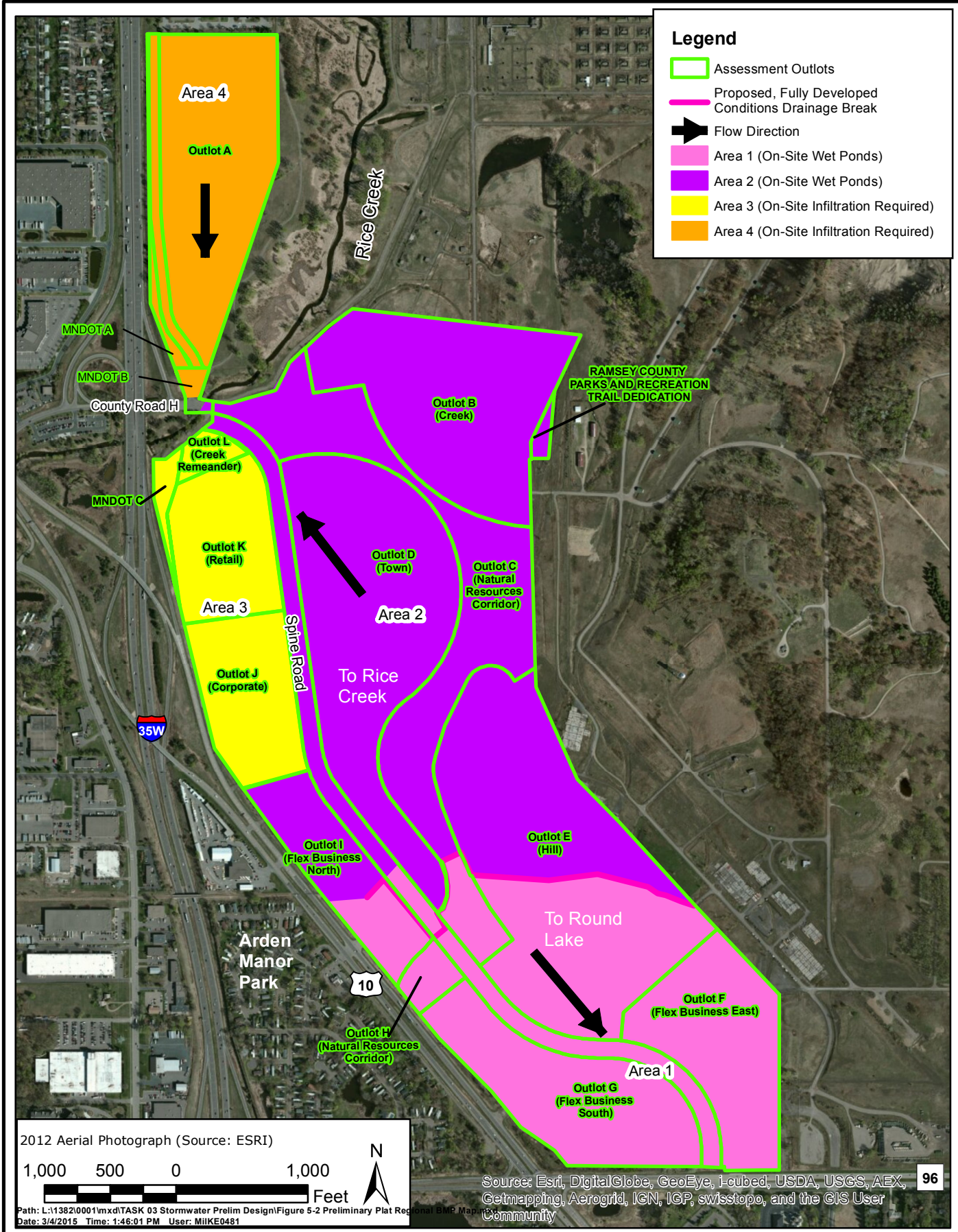


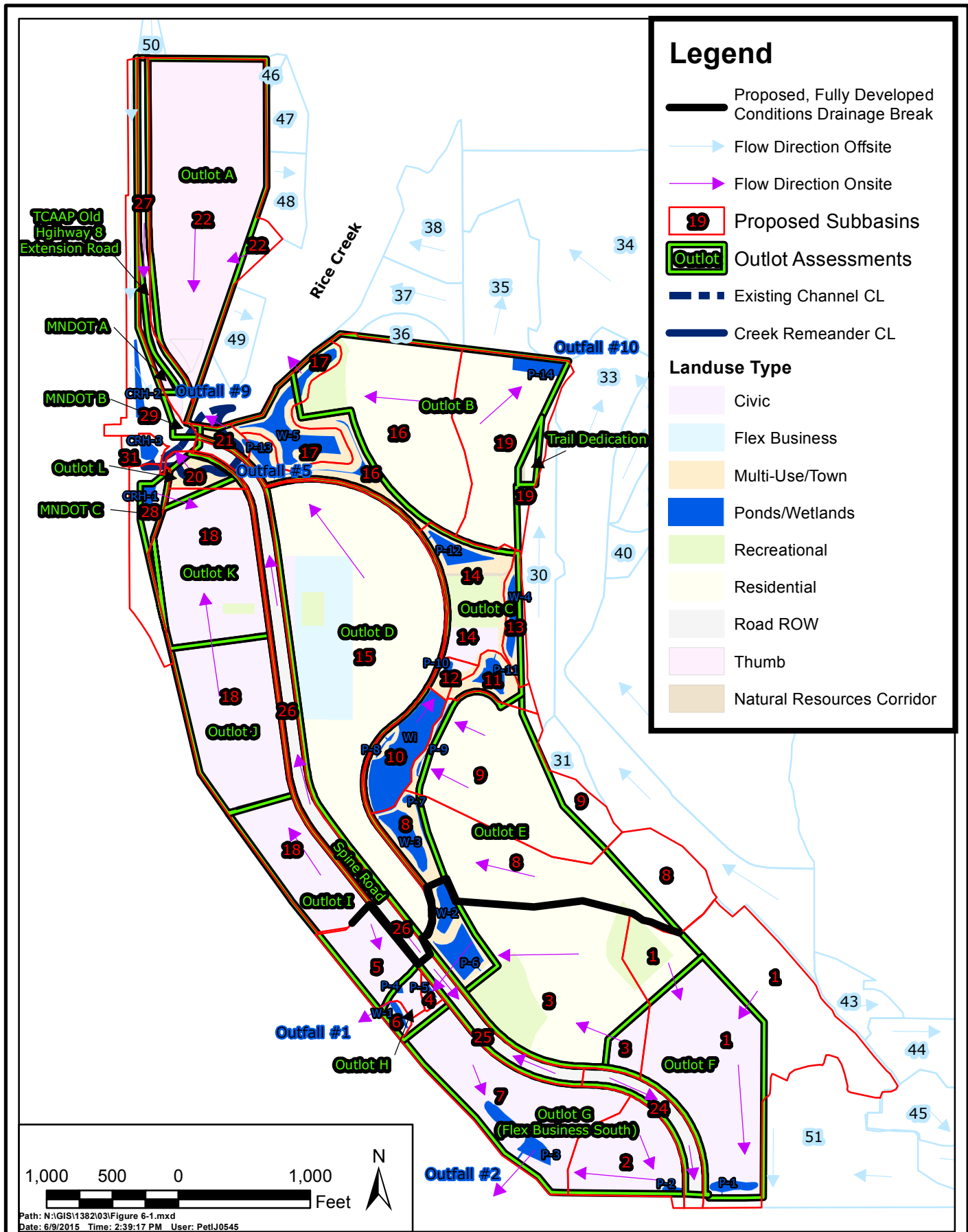
- Legend**
- Proposed, Fully Developed Conditions Drainage Break
 - Stormwater Ponds
 - Proposed Stormwater Piping
 - Proposed Subbasins
 - 427-Acre Boundary
 - Offsite Subbasins

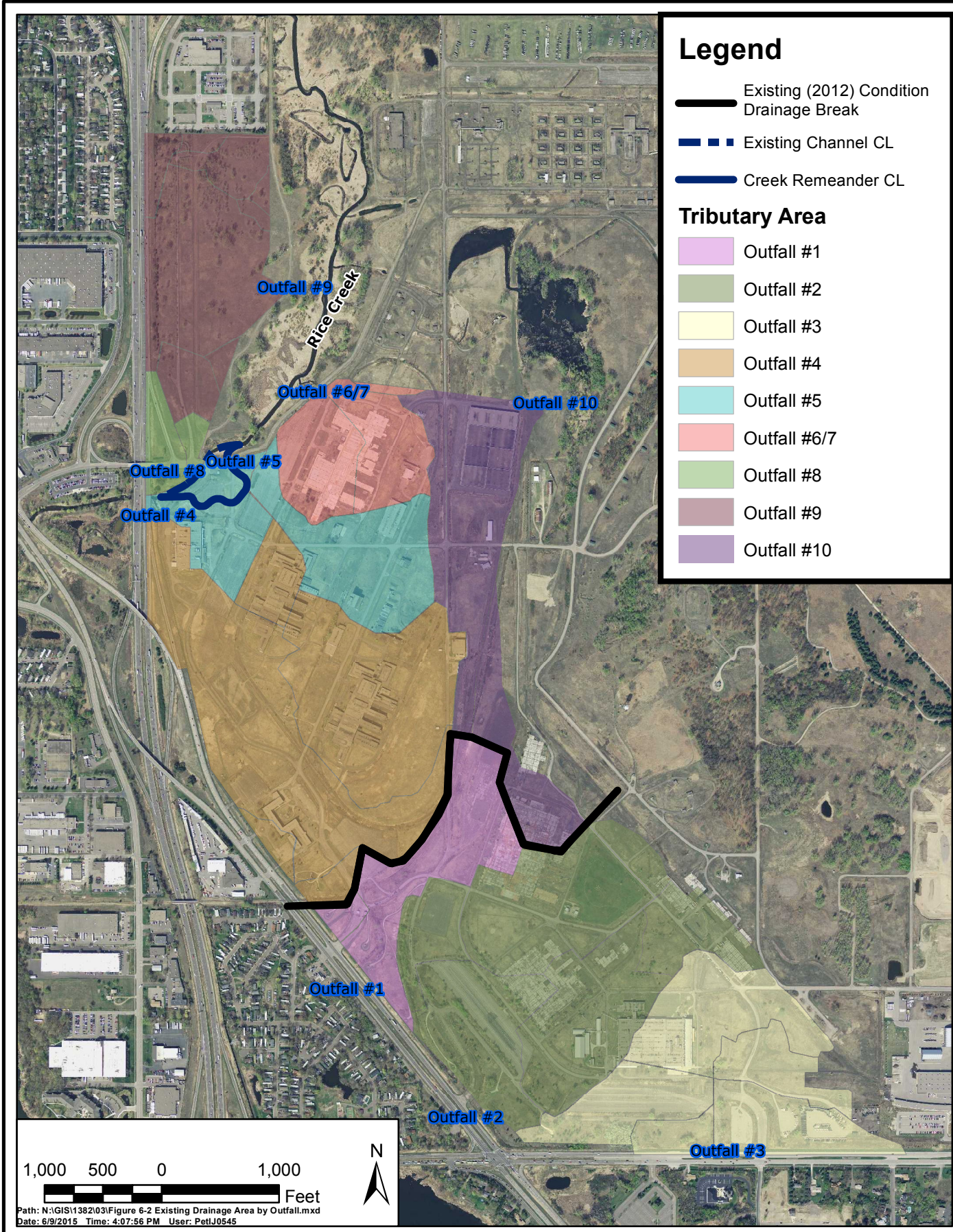
2012 Aerial Photograph (Source: ESRI)
 1,000 500 0 1,000
 Feet
 Path: L:\1382\0001\mxd\TASK 03 Stormwater Prelim Design\Figure 4-7 Ultimate Dev Subwatershed Map with Community.mxd
 Date: 3/4/2015 Time: 1:21:51 PM User: MIKE0481

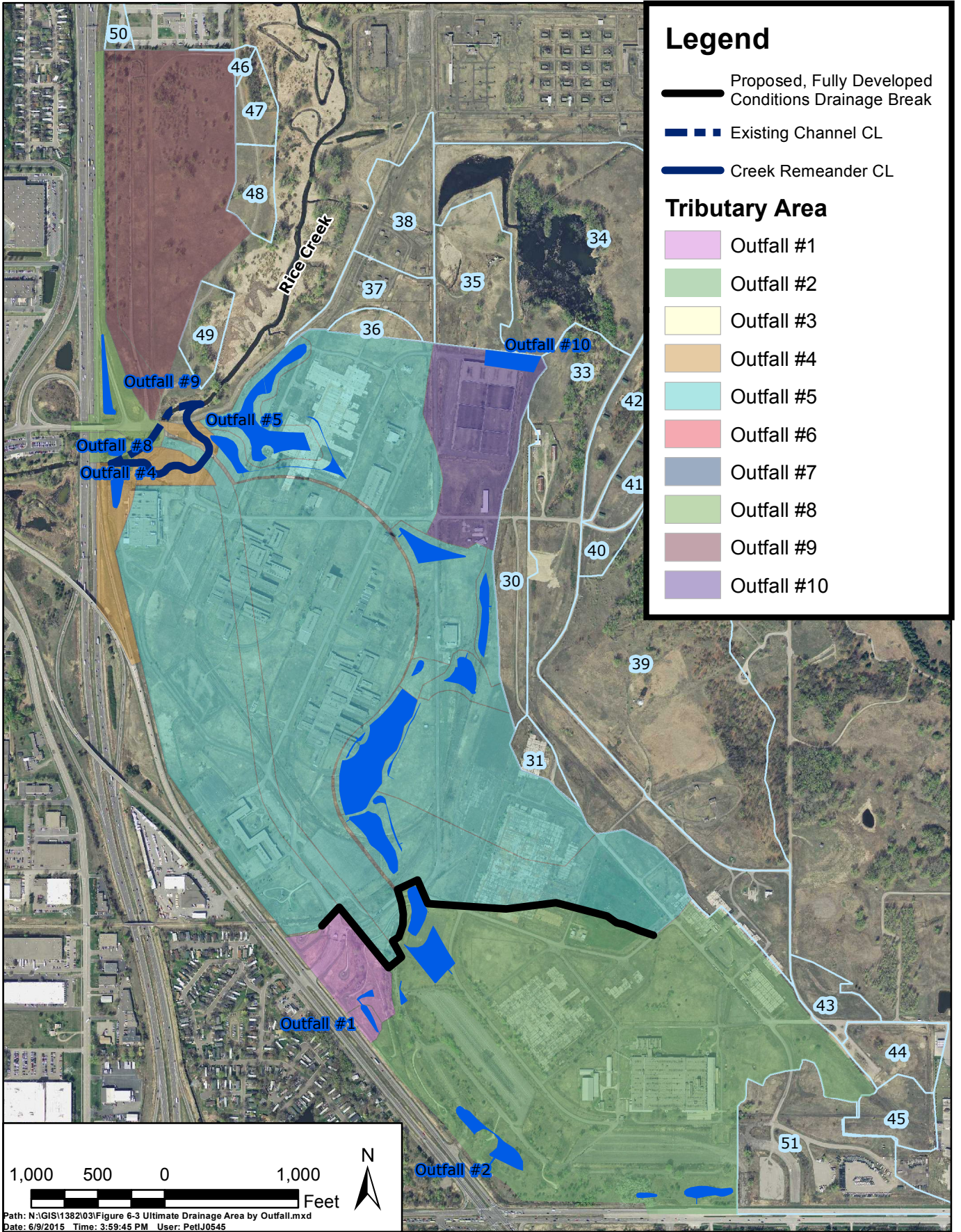
Source: Esri, DigitalGlobe, GeoEye, i-cubed, USDA, USGS, AEX, Getmapping, Aerogrid, IGN, IGP, swisstopo, and the GIS User Community

























Legend

-  Proposed, Fully Developed Conditions Drainage Break
-  Existing Channel CL
-  Creek Remeander CL

Tributary Area

-  Outfall #1
-  Outfall #2
-  Outfall #3
-  Outfall #4
-  Outfall #5
-  Outfall #6
-  Outfall #7
-  Outfall #8
-  Outfall #9
-  Outfall #10

1,000 500 0 1,000 Feet 

Path: N:\GIS\1382\03\Figure 6-3 Ultimate Drainage Area by Outfall.mxd
 Date: 6/9/2015 Time: 3:59:45 PM User: PetlJ0545

Technical Memorandums:

Hydrology and Hydraulics Modeling (HydroCAD)

Water Quality Modeling (P8)

Technical Memo



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To: Beth Kunkel, Kimley-Horn

From: Pamela Massaro, PE, Wenck Associates, Inc.
Adam Marsh, EIT, Wenck Associates, Inc.

Date: August 31, 2015

Subject: HydroCAD Stormwater Modeling
Summary of Changes made August 30, 2015

After discussing the stormwater modeling completed using the HydroCAD model as presented in the Comprehensive Stormwater Management Plan (CSMP) with Rice Creek Watershed District (RCWD) representative Kate MacDonald on Friday August 28, 2015, Ramsey County's consultants (Kimley Horn, Wenck Associates Inc.) revised the interim and fully developed conditions site condition HydroCAD model to address:

- Warning messages, and
- Sub-catchment 51 flows to Outfall 3.

This technical memorandum summarizes the changes made to the *Interim* and *Full Buildout* HydroCAD models transmitted by email to RCWD representative Kate MacDonald.

Changes to Model

In both the *Interim* and *Full Buildout* HydroCAD models, the following changes were made:

- Reaches were removed from the model.
- Model calculation mode was switched from Muskingum-Cunge to Dynamic Storage-Indication in order to capture backwater effects between stormwater ponds.
- Wetland W-3 was disconnected from Pond P-7 to add stability to the model. Excluding this connection makes the model more conservative by reducing the storage volume available during storm events. In actuality, P-7 will feed W-3 via a small pipe located at P-7's and W-3's respective normal water levels.
 - W-3 spill crest - 916 ft
 - P-7 100-yr peak water surface elevation - 915.93 ft.
- The lag time of Offsite Subbasin 51 was adjusted to reflect proposed development at the MN Army National Guard Arden Hills Army Training Site (AHATS) readiness center site, the land upon which Offsite Subbasin 51 is located.
- Where appropriate, links were added to sum the peak discharge from one or more ponds that discharge together.

Model Results

The above changes resulted in discharge rates shown in the table below:

Peak Discharge Rate (cfs) of 100-yr, 24-yr Storm Event

Outfall	Existing Conditions (cfs)	80 % of Existing Conditions (cfs)	Interim Condition		Full Buildout Condition	
			Previous (cfs)	Revised (cfs)	Previous (cfs)	Revised (cfs)
Round Lake Outfall #1	135	108	11	11	16	16
Round Lake Outfall #2	438	350	272	277	261	268
Round Lake Outfall #3	83	67	94	70	94	70
Rice Creek	1219	975	682	678	867	862

Model Warnings

The revised *Interim* condition model contains three warnings. None of these warnings threaten the validity of model results.

First Warning: Pond 17P Exceeded Pond P-5/P-6 by 0.17' @ 36.12 hours (0.20 cfs 0.727 af). The 12-inch RCP pipe with a slope of 0.0437 ft/ft and a 300 ft length between ponds P-5/P-6 limit the flow rates and pond levels take more time to equalize after a precipitation event. This warning is not a concern because it indicates that P17 (wetland W-2) returns to pre-storm conditions less quickly than P-5/P-6. P-5/P-6 is connected to W-2 by a 12-inch pipe intended to maintain the hydrology of W-2 during periods of dry weather. Some storm water is released to W-2 during storm events (peak discharge of 3.09 cfs) and that stormwater is slowly released to W-3 (peak discharge of 1.02 cfs). Because W-2 releases water so slowly, P-5/P-6 returns to pre-storm conditions more rapidly than W-2, thus triggering the HydroCAD warning at simulation hour 36.12, more than a full day after peak storm intensity.

Second Warning: Pond 7P Qout>Qin may require Finer Routing or smaller dt. This is a common warning message that results from using the Dynamic Storage-Indication solution technique. As stated in the HydroCAD User Help Guide this warning message is sometime caused by normal routing conditions. An inspection of the hydrograph and output data from the HydroCAD model indicates that, in this case, no modeling issue exists (i.e. caused by normal routing conditions). There are no oscillations or flow abnormalities in the pond P-7 influent or effluent hydrographs. Nor does the pond summary data indicate that an error exists in the model results for P-7. In an effort to eliminate this warning, the Finer Routing setting was increased from 1 to 2 but this had no effect on model results. The time step used is 0.01 hours, the smallest time increment allowed by HydroCAD.

Technical Memo

Summary of Changes made August
30, 2015
August 31, 2015



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Third Warning: Pond 10P outlet Device #1 rise exceeded. Pond P-10 outlet device #1 (the primary outlet) is designed to be submerged. Once the outlet device #1 rise is exceeded, HydroCAD treats the outlet as an orifice of the modeled dimensions. This is how outlet device #1 was designed to operate.

The revised *Full Buildout* condition model contains two warnings. None of these warnings threaten the validity of model results.

First Warning: Pond 7P Qout>Qin may require Finer Routing or smaller dt. As stated in the HydroCAD User Help Guide this warning message is sometime caused by normal routing conditions. An inspection of the hydrograph and output data from the HydroCAD model indicates that, in this case, no modeling issue exists (i.e. caused by normal routing conditions). There are no oscillations or flow abnormalities in the pond P-7 influent or effluent hydrographs. Nor does the pond summary data indicate that an error exists in the model results for P-7. In an effort to eliminate this warning, the Finer Routing setting was increased from 1 to 2 but this had no effect on model results. The time step used is 0.01 hours, the smallest time increment allowed by HydroCAD.

Second Warning: Pond 10P outlet Device #1 rise exceeded. Pond P-10 outlet device #1 (the primary outlet) is designed to be submerged. Once the outlet device #1 rise is exceeded, HydroCAD treats the outlet as an orifice of the modeled dimensions. This is how outlet device #1 was designed to operate.

Technical Memo



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Exceptional outcomes.

To: Beth Kunkel, Kimley-Horn

From: Pamela Massaro, PE, Wenck Associates, Inc.
Adam Marsh, EIT, Wenck Associates, Inc.

Date: July 30, 2015

Subject: HydroCAD Stormwater Modeling

Introduction

The purpose of this technical memorandum is to summarize the stormwater modeling completed using the HydroCAD model as presented in the Comprehensive Stormwater Management Plan (CSMP) submitted to the Rice Creek Watershed District (RCWD) for the portion of the former Twin Cities Army Ammunition Plant (TCAAP) Site being redeveloped by Ramsey County (County), the City of Arden Hills (City), the County's consultants (Kimley Horn, Wenck Associates Inc.), and future Developers. The Site, under 2012 existing conditions, provides few stormwater control structures to reduce discharge rate and just grassy swales as best management practices (BMPs) to improve water quality treatment before stormwater leaves the Site.

The Rice Creek Commons Site, under fully developed conditions, will meet RCWD's water quality treatment requirements through a combination of onsite infiltration and wet detention ponds. Water quality results are summarized in a separate *P8 Water Quality Modeling* technical memorandum. This technical memorandum details stormwater volume and discharge rate control measures proposed to meet RCWD's peak stormwater runoff control requirements.

Many figures referenced in this technical memorandum are from the CSMP document.

1.0 Existing Site Conditions

1.1 Project Location

The 427-acre Rice Creek Commons Site, known as the former Twin Cities Army Ammunitions Plant (TCAAP), is located in the Ramsey County, Minnesota in the cities of Arden Hills and New Brighton. It is bounded by U.S. Interstate Highway 35 (I-35W) on the west, County State Aid Highway 96 (CSAH 96) to the south and U.S. Highway 10 (Hwy 10) to the southwest (**CSMP Figure 2-1**).

Rice Creek passes through the site near County Road H. Two-thirds of the site drain to Rice Creek and the remaining portion drains south to Round Lake. **CSMP Figure 2-2** shows the dividing line for areas draining to Rice Creek and Round Lake.

1.2 Existing Condition Subwatersheds

Wenck delineated 37 subwatersheds using topography (LiDAR data: Block F: 11-13-11 to 11-17-11, reflight 3-25-12) and information available on the US Army storm sewer network (**Figure 1**). Overland flow direction and drainage swales were identified using the available one foot contours. The existing conditions model explicitly represents 503.4 acres, which includes some off-site drainage.

There are nine subwatersheds (27-35 and County Rd H) that drain from the portion north of Rice Creek directly to Rice Creek. Stormwater exits at various, non-descript locations and discharges directly or ultimately to Rice Creek.

There are nine subwatersheds (12, 14-16 and 20-24), south of Rice Creek, that drain stormwater north discharging directly into Rice Creek by way of a ditch on the western site boundary or through various storm sewers.

There are five subwatersheds (17-19 and 25-26), south of Rice Creek, comprising the northeastern edge of the portion south of Rice Creek, where stormwater flows northeast to a wetland area, which eventually discharges to Rice Creek.

The southern third of the site drains to Round Lake by one of three outfalls (**CSMP Figure 4-7**). There are two subwatersheds (8, 13) that contribute runoff to Outfall #1, a 24-inch culvert passing beneath Hwy 10. There are eight subwatersheds (1, 4-6, 9-11, and 36) that contribute runoff to Outfall #2, a 60-inch culvert passing beneath Hwy 10. There are three subwatersheds (2, 3 and 7) that contribute runoff to Outfall #3, a 30-inch culvert which connects to the new storm sewer installed for CSAH 96.

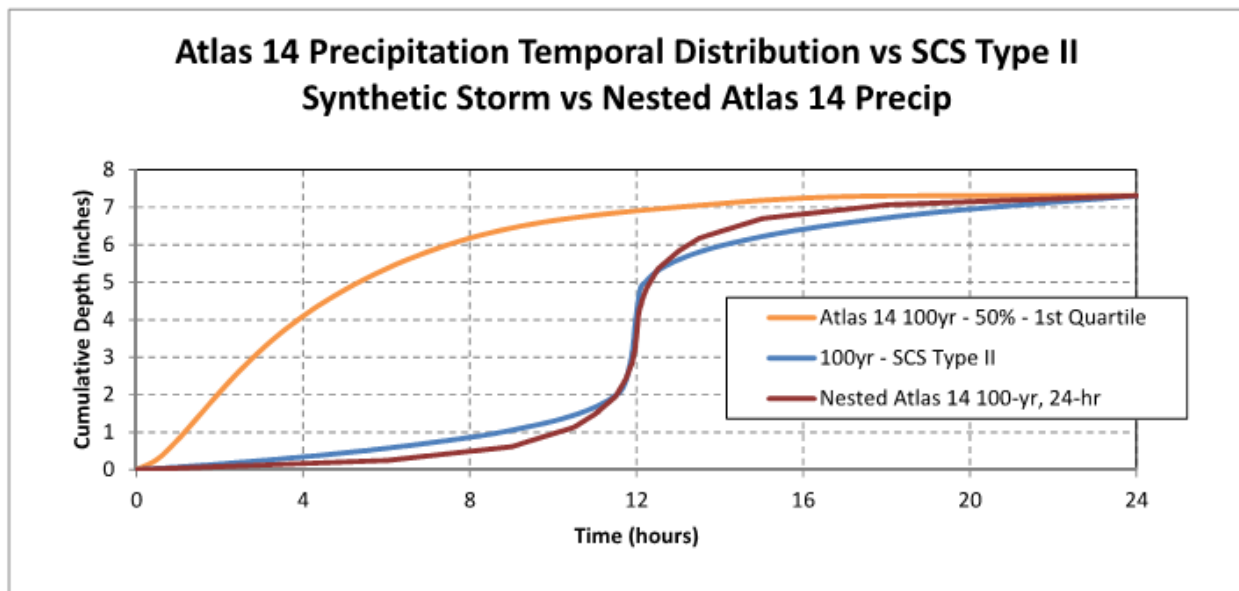
1.3 Subwatershed Runoff

Wenck evaluated stormwater runoff for 2-, 10-, and 100-year 24-hour design rainfall events. Precipitation depths were obtained from the National Oceanic and Atmospheric Administration (NOAA) Atlas 14. The 24-hour nested Atlas 14 rainfall depths used are:

- ▲ 2-Year Event 2.82 inches
- ▲ 10-Year Event 4.22 inches
- ▲ 100-Year Event 7.31 inches

The Atlas 14 nested curve is a hyetograph that encompasses short duration and long duration storms in one distribution (a conservative but effective approach for design). The Soil Conservation Service (SCS) Type II distribution is a nested curve developed using the Technical Paper (TP-40) data set. No areal reduction factor (a factor used to account for special variations in rainfall intensity) is applied based on individual subwatershed size. The rainfall distributions for the 100-year 24-hour design rainfall event are illustrated in **Figure 2**.

Figure 2 – Temporal nested Atlas 14 storm precipitation distribution compared to a SCS Type II distribution for the 100-yr, 24-hr rainstorms



The existing and proposed stormwater management calculations are based on the National Resources Conservation Service (NRCS) methodology using curve numbers (CN), time of concentration (T_c) and a design storm event approach. These variables and the other requisite NRCS inputs are used to complete calculations and to build a model in HydroCAD, a computer-aided design system for modeling the hydrology and hydraulics of stormwater runoff. The SCS Unit Hydrograph method is used to generate the subwatershed runoff hydrographs. A rainfall temporal distribution curve along with the drainage basin area, basin time of concentration, and drainage basin composite CN are required input parameters.

To reflect the 2012 site conditions the existing condition model does not have any stormwater ponds or basins.

1.3.1 Existing Condition Soils

The SCS classifies soils into four Hydrologic Soil Groups based on the soil's potential for runoff. The four Hydrologic Soil Groups (HSG) are A, B, C, and D. Type A soils generally have the least runoff potential and Type D soils have the greatest. Details for these classifications can be found in SCS TR-55 (SCS 1986). Soil data from the NRCS SSURGO database data is incomplete (**CSMP Figure 3-3**), with the bottom two-thirds of the site classified merely as "Urban". The geologic map of surficial geology (**CSMP Figure 3-4**) provides additional information about the site's geology. The portion south of Rice Creek has had the same land use for decades, the soil condition is poor and the soil is classified as Type C. The Type C soil classification is refined using geotechnical investigation information. Open spaces (including lawns, parks, and grasslands) are assigned a CN of 74 according to RCWD rules.

1.3.2 Existing Condition Land Cover

The 2012 aerial photograph provides sufficient land use data for the existing conditions. Roads and buildings are digitized and assigned a CN of 98 in compliance with RCWD rules. The portion of the site north of Rice Creek has Type A soils and is predominately open space, with some access trails.

1.3.3 Existing Condition Curve Number

Wenck determined curve numbers by intersecting the HSG map information with land cover data using GIS (**CSMP Figure 4-2**). Intersecting subwatershed area with curve number attribute information provided the composite curve number for each subwatershed (**Table 1**).

1.3.4 Existing Condition Time of Concentration

Time of concentration is the time it takes runoff to travel from the most distant point in the watershed to its outlet. The time of concentration is determined by a basin's geometry and flow types, which impact flow velocities. Runoff is assumed to travel as sheet flow, shallow concentrated flow, or channel flow. Time of concentration is estimated as the sum of travel times of these flows. The basin lag time is calculated for each subwatershed using **Equation 1**.

Equation 1 – Time of concentration

$$t_c = t_{sheet\ flow} + t_{shallow\ concentrated\ flow} + t_{channel\ flow}$$

where

$$t_{sheet\ flow} = 0.007 * \frac{(flow\ length * n)^{0.8}}{precipitation\ depth^{0.5} slope^{0.4}}$$

$$t_{shallow\ concentrated\ flow} = \frac{flow\ length}{C * slope^{0.5}}$$

where

$$C = 16.1345 \text{ for unpaved surfaces} \quad \text{or} \\ C = 20.3282 \text{ for paved surfaces}$$

$$t_{channel\ flow} = \frac{flow\ length}{\{ [1.49 * (cross\ sectional\ area \div wetted\ perimeter)^{2/3} * slope^{0.5}] \div n \}}$$

Wenck calculates the time of concentration using existing topography, and surface channels. No storm sewer is included in the model for two reasons. First, available information about storm sewer layout and dimensions is insufficient to allow accurate representation in a model. Second, the United States Army ceased operations at TCAAP in 2002 and fitness of the storm sewer system in 2012 is unknown. Runoff travels as sheet flow for a maximum of 100 feet, at which point it transitions to shallow concentrated flow. Shallow concentrated flow then continues until intercepted by a surface channel or the receiving water body.

Table 1 - Input parameters for the Existing Condition HydroCAD Model

Subwatershed	T _c	Area	Composite CN
	<i>min</i>	<i>Acres</i>	-
1	16.3	15.3	87
2	12.2	4.9	77
3	32.8	15.5	75
4	11.3	24.0	85
5	44.5	27.2	83
6	46.4	22.5	79
7	27	9.8	66
8	9.5	28.7	83
9	12.7	9.3	82
10	37.7	30.0	81
11	32.9	4.4	80
12	14	3.3	80
13	36.2	2.3	82
14	8.94	2.5	84
15	28	61.3	82
16	26.3	44.8	81
17	11.5	8.0	88
18	15.8	28.4	83
19	7.3	6.4	78
20	17.1	15.9	85
21	10.8	7.2	90
22	19.6	26.6	82
23	9.42	13.8	92
24	19	13.9	85
25	22.6	6.7	75
26	38.2	0.8	66
27	13	2.0	66
28	23.8	5.8	44
29	26.9	1.3	39
30	26.8	31.6	42
31	30.2	0.9	39
32	27.6	0.9	39
33	19.9	3.2	48
34	12.1	1.2	49
35	16.7	6.2	43
36	61.5	11.2	84
County Road H	29.3	5.9	80
Total	-	503.4	-

2.0 Interim and Fully Developed Site Conditions

Wenck evaluated stormwater runoff for interim and fully developed conditions site conditions.

2.1 Interim Conditions (Interim Scenario 1)

The first phase of site development includes the construction of public infrastructure including Spine Road, I-35W improvements, alterations to County Road H and realignment of Rice Creek (**CSMP Figure 4-5**). During this interim construction phase (Interim Scenario 1), a contractor, selected by Ramsey County will construct all wetlands and ponds except P-1, P-3, and P-14. Interim Scenario 1 reflects the first stage of site development which includes the construction of public infrastructure, described in the CSMP. The Water Quality Model evaluates Interim Scenario 1 and Interim Scenario 2. Interim Scenario 2 is not modeled explicitly in HydroCAD.

2.2 Fully Developed Conditions

The fully developed conditions site plan is only conceptual in nature, at this time, leaving final design for various parts of the Site to developers. Private developers will construct ponds P-1, P-3 and P-14. Fully developed conditions reflect proposed grading plans for initial site development (**CSMP Figure 4-5**) and master land use plan (**CSMP Figure 3-1**).

2.3 Watershed Delineation

For the Interim Scenario 1 and fully developed conditions, Wenck reviewed proposed grading plans and delineated the Site into 30 subwatersheds (**CSMP Figure 4-7**), four subwatersheds in the portion north of Rice Creek and 26 in the portion south of Rice Creek.

There are four subwatersheds (22, 27, 29, and 1S) north of Rice Creek that drain to Rice Creek.

There are 13 subwatersheds (8-18, 26, and 28) south of Rice Creek that drain stormwater runoff to the north. The runoff is routed through a series of stormwater ponds in the Natural Resource Corridor and ultimately discharges to Rice Creek.

Two subwatersheds (20 and 21) encompass the relocated Rice Creek meander and its bank area. These subwatersheds are not included in the model because precipitation falling in these subwatersheds falls directly into Rice Creek.

There is one subwatershed (19), south of Rice Creek, comprising the northeastern edge of the site, where stormwater flows to a stormwater pond (P-14). The pond discharges beneath the trail and drains northeast to an existing wetland area, which eventually discharges to Rice Creek.

The southern third of site drains to Round Lake by one of three outfalls. There are two subwatersheds (5 and 6) that contribute runoff to Outfall #1, a 24-inch culvert passing beneath Hwy 10. There are seven subwatersheds (1-4, 7, 24 and 25) contributing runoff to Outfall #2, a 60-inch culvert passing beneath Hwy 10. Stormwater originating in offsite subwatershed 51 is diverted before entering the Rice Creek Commons property and discharges to Round Lake through an existing storm sewer. Fully developed conditions assume that the stormwater ponds P-1 and P-2 are not connected to the storm sewer catchbasin (CB 5004) which is routes runoff through a 15-inch pipe under CSAH 96. The stormwater ponds P-1 and P-2 are assumed to be connected to stormwater pond P-3 by a 60-inch pipe.

In the Interim Scenario 1 and fully developed condition HydroCAD models, 142.2 acres drain to Round Lake and 332.8 acres drain to Rice Creek. The Interim Scenario 1 and fully developed conditions total tributary area is 500.2 acres, 3.2 acres less than the existing tributary area. Two factors account for the change in total tributary area. First, the remainder of Rice Creek, causes subwatersheds 20 and 21 to include Rice Creek and its banks in the Interim Scenario 1 and fully developed conditions. For this reason, subwatersheds 20 and 21 (3.9 acres) are excluded from the Interim Scenario 1 and fully developed conditions model. Second, in the Interim Scenario 1 and fully developed conditions, 0.7 acres of additional tributary area are added due to expansion of I-35W and County Road H.

2.4 Stormwater Control Structures

To provide stormwater control in Interim Scenario 1, a contractor selected by Ramsey County will construct 14 stormwater ponds. During subsequent development, private developers will construct three additional ponds to provide further rate control (**CSMP Figure 4-6**). For water quality control, the required treatment volume is captures and treated in wet ponds or infiltration basins. For a detailed analysis of water quality controls, see the CSMP Appendix A: P8 Water Quality Modeling Technical Memorandum.

The Interim Scenario 1 and fully developed conditions grading plans include five newly created wetlands (W-1, W-2, W-3, W-4 and W-5) and preservation of one existing wetland (Wi). Wetlands are modeled to receive controlled inputs of water from the dead pool of adjacent stormwater ponds.

Through an iterative process, Wenck sized National Urban Runoff Program compliant stormwater control structures in collaboration with Kimley-Horn and Associates, Inc. Kimley-Horn provided a proposed grading plan indicating the approximate size and location of stormwater control structures, which Wenck translated into HydroCAD and P8 models. Based on model results, Wenck recommended alterations to stormwater control structure size and location. Kimley-Horn then revised the proposed grading plan to address stormwater control and water quality concerns identified by Wenck. After several iterations, Wenck and Kimley-Horn reached a proposed grading plan that satisfies RCWD discharge rate and water quality requirements. Kimley-Horn is preparing a RCWD permit application for Interim Scenario 1.

2.5 Routing

Each subwatershed's runoff is routed individually and directed to a pond and stormwater control structure, or drain directly to the receiving waterbody (**CSMP Figure 4-7**). Wenck did not consider storm sewers, other than what is considered public infrastructure. Developers will design and construct storm sewers conforming to the grading and stormwater control structures presented here.

2.6 Land Cover

Interim Scenario 1 and fully developed conditions plan drawings obtained from Kimley-Horn provided land cover data for the fully developed conditions condition (**CSMP Figure 3-1**). In accordance with SCS TR-55, the following land use designations were used:

- ▲ Residential areas – 38% impervious
- ▲ Commercial areas – 85% impervious
- ▲ Recreational areas – 30% impervious
- ▲ Open spaces – 2% impervious
- ▲ Pavement – 100% impervious

2.7 Curve Number

The Interim Scenario 1 and fully developed conditions curve numbers were assigned by intersecting the HSG map information with land cover data using GIS tools. Intersecting the area of each subwatershed with curve number attribute information provided the curve numbers for each subwatershed (**Table 2 and Table 3**). Wenck assigned separate areas and curve numbers for the pervious and impervious fraction of each subwatershed.

For both conditions, Wenck adjusted the CN of Type A and Type B soils upwards from 39 to 49 and from 61 to 74, respectively. This shift accounts for the effects of compaction and soil stratification that decrease the infiltration rate of Type A and B soils.

2.8 Time of Concentration

Because storm sewer design on Outlots is the responsibility of developers, time of concentration was calculated considering sheet flow and shallow concentrated flow only. Wenck calculated the time of concentration using fully developed conditions condition topography and stormwater control structures. Runoff travels as sheet flow for a maximum of 100 feet, at which point it transitions to shallow concentrated flow. Shallow flow calculations assume that runoff travels entirely over roads, parking lots, businesses and other paved surfaces. These are the most likely flow paths for unrouted flow in the fully developed conditions condition. Shallow concentrated flow then continues until intercepted by a stormwater control structure, or the receiving waterbody.

Table 2 - Input parameter for Interim Scenario 1 condition HydroCAD model

Subwatershed	Tc	Pervious		Impervious	
		Area	CN	Area	CN
1	53.1	52.1	74	0.0	-
2	16.6	11.0	74	0.4	100
3	15.3	34.7	74	2.9	-
4	5.9	0.3	74	0.3	100
5	59.3	7.4	74	0.5	100
6	20.3	0.9	74	0.1	100
7	5.7	21.6	74	0.0	-
8	47.1	27.9	74	1.6	100
9	30	25.8	74	0.0	100
10	7.3	6.1	74	0.3	100
11	11.7	2.1	74	1.2	100
12	9.5	1.1	74	0.3	100
13	9.4	2.2	74	0.8	100
14	4.3	8.6	74	1.6	-
15	31.3	58.5	74	0.0	-
16	12.1	30.6	74	1.9	100
17	4.3	3.9	74	3.7	100
18	33.5	52.8	74	0.0	-
19	24.7	21.2	74	0.0	-
20	0	2.7	74	0.0	-
21	0	1.2	74	0.0	-
22	41	41.9	74	0.0	-
23	No subwatershed assigned the number 23				
24	7.5	0.1	74	4.9	98
25	10.7	0.2	74	4.9	98
26	25.4	0.2	74	14.1	98
27	27.6	0.0	74	5.5	98
28	14.6	3.7	74	3.3	98
29	19.1	6.4	74	3.9	-
51	17.7	20.2	65**	5.0	98
1S		1.1	74	0.5	98
Total	-	442.5^	-	57.6^	-

*Surface of ponds and wetlands assigned a CN of 100. All other impervious area assigned a CN of 98.

**Subwatersheds 20 and 21 are located directly over top of Rice Creek and are excluded from the model because precipitation falling in these subwatersheds falls directly into relocated Rice Creek.

^Excludes area of subwatershed 20 and 21.

Table 3 – Input parameter for fully developed conditions HydroCAD model

Subwatershed	T _c	Pervious Area	CN	Impervious Area	CN*
	<i>min</i>	<i>acres</i>	-	<i>acres</i>	-
1	53.1	27.0	74	25.2	98
2	16.6	1.8	74	9.6	98
3	15.3	22.1	74	15.6	98
4	5.9	0.5	74	0.1	100
5	59.3	2.3	74	5.5	98
6	20.3	0.8	74	0.2	100
7	5.7	3.3	74	18.3	98
8	47.1	20.7	74	8.9	98
9	30	17.2	74	8.6	98
10	7.3	5.9	74	0.5	98
11	11.7	2.2	74	1.1	100
12	9.5	0.8	74	0.5	98
13	9.4	2.1	74	0.9	100
14	4.3	5.9	74	4.4	98
15	31.3	30.3	74	28.2	98
16	12.1	21.6	74	10.9	98
17	4.3	3.9	74	3.7	100
18	33.5	8.2	74	44.7	98
19	24.7	12.7	74	8.5	98
20**	0	2.5	74	0.3	98
21**	0	1.2	74	0.0	98
22	41	7.5	49	34.5	98
23	No subwatershed assigned the number 23				
24	7.5	0.1	74	4.9	98
25	10.7	0.2	74	4.8	98
26	25.4	0.2	74	14.1	98
27	27.6	0.0	49	5.5	98
28	1.6	3.7	74	3.3	98
29	35.1	6.4	74	3.9	98
51	17.7	20.2	65	5.0	98
1S	25.5	1.1	74	0.5	98
Total:		228.5 [^]	73	271.7 [^]	98

*Surface of ponds and wetlands assigned a CN of 100. All other impervious area assigned a CN of 98.

**Subwatersheds 20 and 21 are located directly over top of Rice Creek and are excluded from the model because precipitation falling in these subwatersheds falls directly into relocated Rice Creek.

[^]Excludes area of subwatershed 20 and 21.

3.0 HydroCAD Model Results

The existing and proposed stormwater management calculations are based on NRCS methodology using CN, T_c , and a nested Atlas 14 24-hour rainfall distribution. The 24-hour nested Atlas 14 rainfall depths used are:

▲ 2-Year Event	2.82 inches
▲ 10-Year Event	4.22 inches
▲ 100-Year Event	7.31 inches

These, and other NRCS inputs, were combined in HydroCAD, a stormwater hydraulic and hydrology modeling software, to perform design calculations. Wenck used the Storage-Indication routing method, which routes flow according to storage capacity and neglects travel time, as the pond routing method. Reach routing utilized the Muskingum-Cunge channel flow routing method because this method accounts for both reach storage capacity and travel time through the reach. Analysis of the 10-day snowmelt event was not required as all proposed stormwater ponds have a defined outlet at an elevation below the 100-year high water level.

3.1 Existing vs. Proposed Runoff

Wenck evaluated the peak discharge rate of the existing and fully developed site conditions for the 2-, 10-, and 100-year 24-hour design rainfall events. Rice Creek Watershed District Rule C.7 requires that Interim Scenario 1 and fully developed conditions model predicted peak discharge rates not exceed 80% of existing peak discharge.

3.1.1 Summary for Resource of Concern (Round Lake)

Under existing conditions, stormwater is discharged to Round Lake by way of three culverts. The proposed peak discharge rates under Interim Scenario 1 and fully developed conditions at each location have been significantly reduced for all three design storms (**Table 4**). The proposed stormwater management plan eliminates discharges through Outfall #3 (**CSMP Figure 4-7**) for both the Interim Scenario 1 and fully developed conditions. Runoff originating in offsite subwatershed 51 is redirected to the south before entering Rice Creek Commons property and discharges to Round Lake by way of an existing storm sewer. The Interim Scenario 1 and fully developed conditions grading plan redirects stormwater from 29.8 acres, which drain to Round Lake under existing conditions, to the Rice Creek. This 15.2% reduction in Round Lake tributary and the construction of stormwater retention ponds accounts for the peak discharge rate reduction.

Under Interim Scenario 1 and fully developed conditions, the combined peak discharge rate to Round Lake will be reduced by >80% for all three storm events (**Table 4**).

Table 4 – Peak discharge rates to Round Lake for existing and proposed conditions

Outfall #1 to Round Lake	Drainage Area (acres)	Discharge (cfs)		
		2-yr	10-yr	100-yr
Existing	23.3	34	67	135
80% of Existing	-	27	54	108
Interim Scenario 1	8.8	2	3	11
Fully Developed Conditions	8.8	4	6	16
Outfall #2 to Round Lake	Drainage Area (acres)	Discharge (cfs)		
		2-yr	10-yr	100-yr
Existing	143.8	112	217	438
80% of Existing	-	90	174	350
Interim Scenario 1	133.4	37	94	272
Fully Developed Conditions	133.4	47	98	261
Outfall #3 to Round Lake	Drainage Area (acres)	Discharge (cfs)		
		2-yr	10-yr	100-yr
Existing	30.2	13	33	83
80% of Existing	-	10	26	67
Interim Scenario 1	0.0	0	0	0
Fully Developed Conditions	0.0	0	0	0
Offsite Subbasin 51 to Round Lake	Drainage Area (acres)	Discharge (cfs)		
		2-yr	10-yr	100-yr
Existing	25.3	0	0	0
80% of Existing	-	0	0	0
Interim Scenario 1	25.3	16	37	94
Fully Developed Conditions	25.3	16	37	94
Total in Aggregate to Round Lake	Drainage Area (acres)	Discharge (cfs)		
		2-yr	10-yr	100-yr
Existing	197.3	159	317	656
80% of Existing	-	127	254	525
Interim Scenario 1	167.5	55	134	377
Fully Developed Conditions	167.5	67	141	371

3.1.2 Summary for Resource of Concern (Rice Creek)

Under existing conditions, runoff south of Rice Creek reaches Rice Creek by storm sewer outfalls or overland flow discharging to Rice Creek (**CSMP Figure 4-1**). Stormwater from the north of Rice Creek discharges to Rice Creek by way of an overland flow at numerous uncontrolled locations.

Under existing and proposed conditions, Rice Creek Commons stormwater from the north and south of Rice Creek discharges to the Rice Creek within a single 1,500 foot section of stream. The travel time from the most upstream to most downstream outfalls is minimal (approximately two minutes) causing discharge from separate locations to compound one

another. As a result, the aggregate peak discharge to Rice Creek determines the impact to this resource of concern.

The proposed stormwater control structures (**CSMP Figure 4-5**) reduce peak discharge to Rice Creek by >80% for both the Interim Scenario 1 and fully developed conditions condition (**Table 5**). Compared to existing conditions, the Interim Scenario 1 and fully developed conditions, respectively, reduce peak discharge by 66.0% and 22.8% for the 2-yr storm event; 59.3% and 29.3% for the 10-yr storm; and 44.0% and 28.9% for the 100-yr storm event.

Table 7 - Total peak discharge to Rice Creek for existing and fully developed conditions (in aggregate)

Total Peak Discharge to Rice Creek	Drainage Area (acres)	Discharge (cfs)		
		2-yr	10-yr	100-yr
Existing	288.7	315	604	1219
80% of Existing	-	252	483	975
Interim Scenario 1	317.5	107	246	682
Fully developed conditions	317.5	243	427	867

Full supporting calculations for the existing, interim, and fully developed conditions condition HydroCAD Models are included in **Appendix A, B** and **C**, respectively.

3.2 Pond Outlet Control Structures

All pond designs include a multi-stage outlet control structure (OCS) to manage stormwater for each of the design events. Detailed summaries of tributary areas to each stormwater pond, outlet information, normal water elevations, emergency overflow elevations, dead pool storage, and high water levels, live storage and peak discharge rates for each design storm event (2-yr, 10-yr and 100-yr) are included in **Section 6.0**.

3.3 Infiltration Potential

Wenck reviewed the soil borings within the Site (**CSMP Figure 3-7**) and classified the soils in each boring log based on infiltration potential. If a boring had clay soils then it was considered no infiltration potential and labeled on **Figure 3** as a red dot. If there was a seam of sand or silt at depth, then the boring was classified as "infiltration potential at target seam" and labeled with an orange square on the Figure. If a particular soil boring showed favorable soils for infiltration, then it was labeled with a green star. If the soils showed potential at depth or at the surface, then it was labeled with a brown triangle and a yellow pentagon. **Figure 3** also shows the approximate location of the stormwater ponds (P-1 through P-14) in blue, the preliminary Outlots from the concept preliminary plat in purple, and areas excluded due to the potential concern relative to surficial (Unit 1) contaminated groundwater shown in red with diagonal lines. On an Outlot by Outlot basis, the infiltration is deemed feasible based on the majority of borings within an Outlot. Infiltration will be required in the portion north of Rice Creek and an area on the west side of the Spine Road alignment as shown in **CSMP Figure 5-1**.

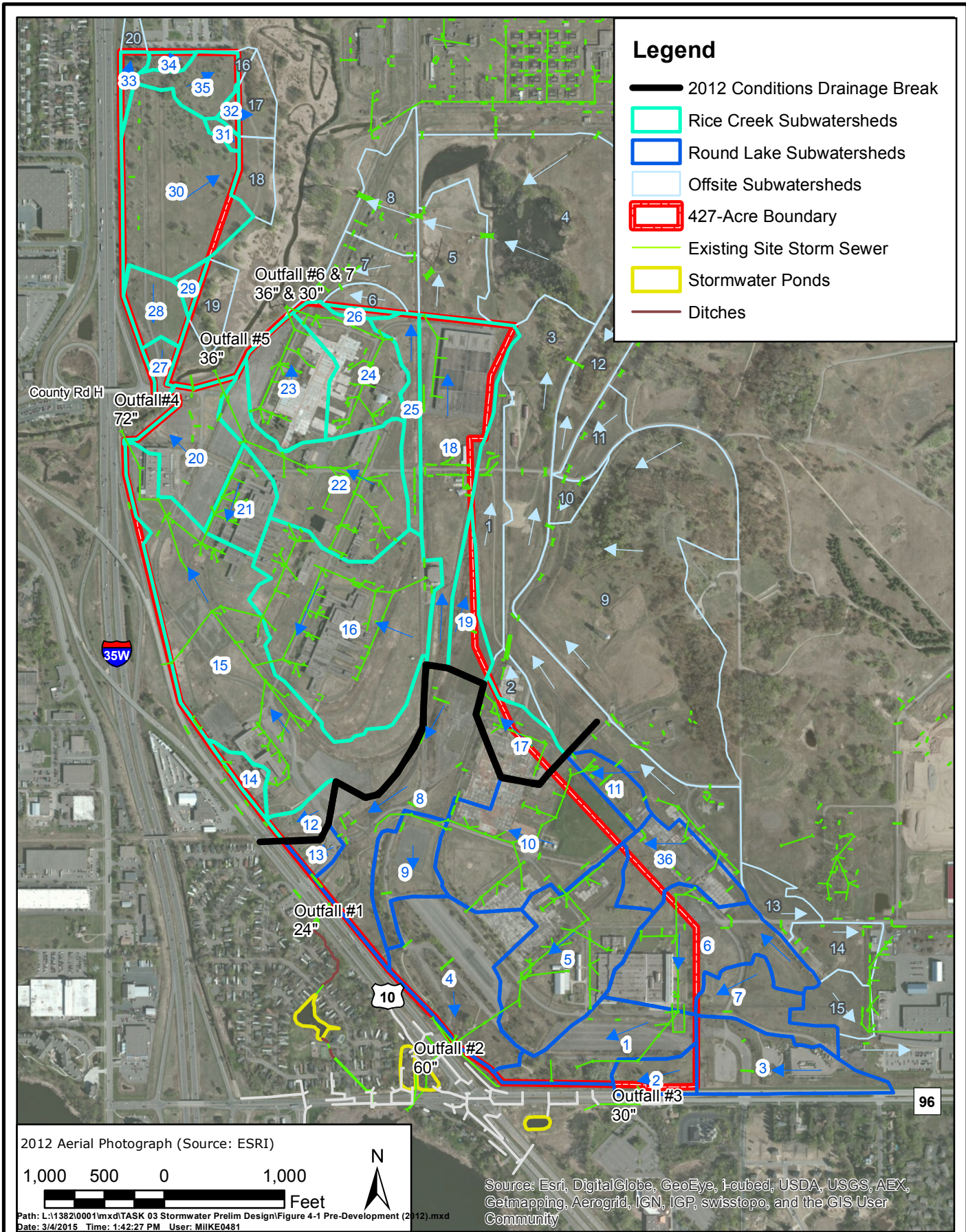
4.0 Summary and Conclusions

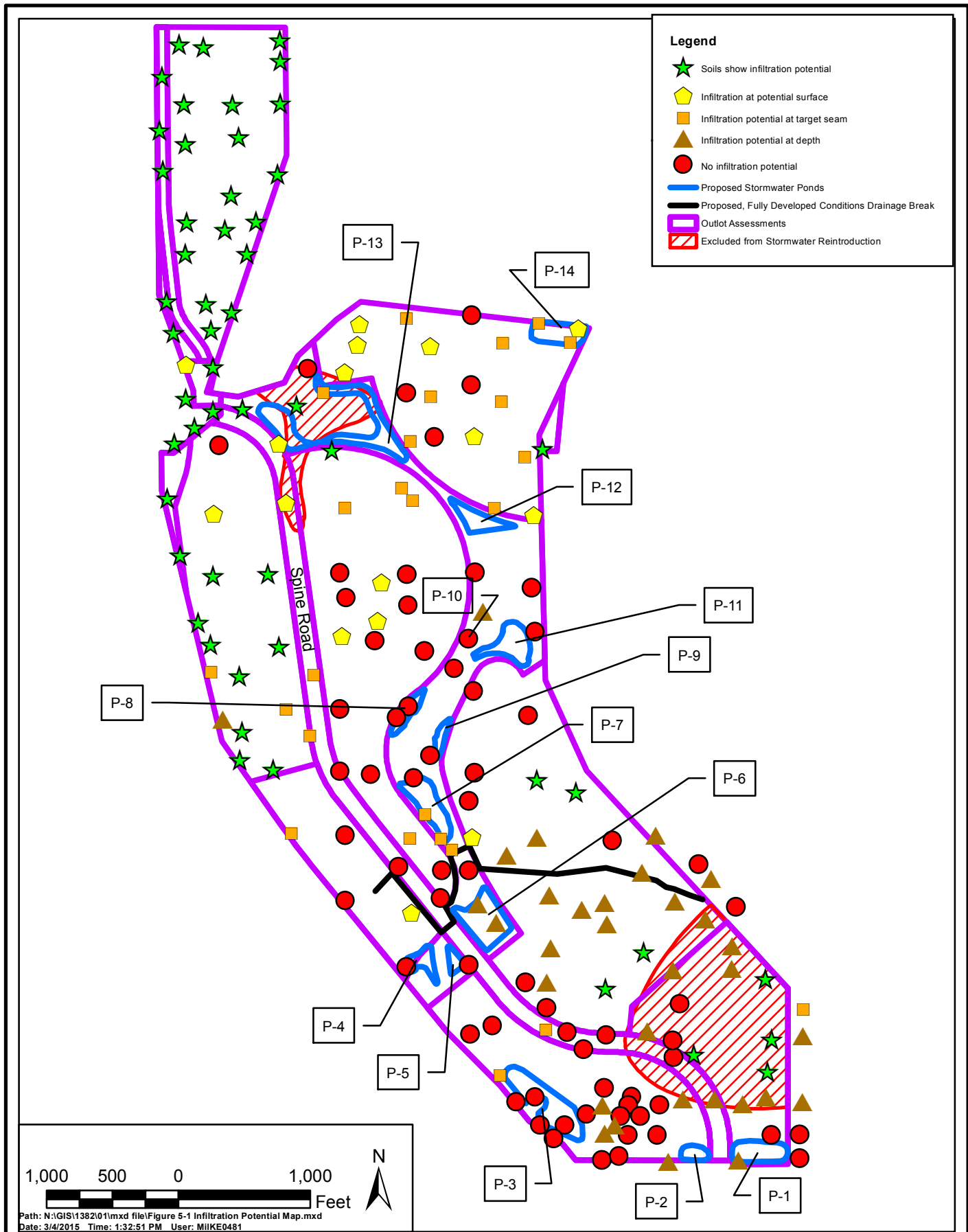
Based on the HydroCAD modeling results presented herein, the RCWD peak runoff rate control standards will be met in aggregate for each Resource of Concern drainage area. Under Interim Scenario 1 and fully developed conditions, the total tributary area discharging to Round Lake decreases 15.2% to 167.4 acres and the total tributary area discharging to Rice Creek increases 8.7% to 332.8 acres when compared to existing conditions.

5. Figures

Figure 1. Pre-development (2012): Subwatershed Map with Storm Sewer

Figure 3. Infiltration Potential Map





6. Summary Tables

Stormwater Pond Data Summary Tables

HydroCAD Tech Memo

Section 6.0 Table 6-1, Pond Outlets

Stormwater Control Structure	Outlet structure	Pipe Size	Length (ft)	Slope	Max Velocity
			ft	ft/ft	ft/sec
P-1/P-2	6" horizontal orifice w/invert @ 924 ft Sharp crested box w/40' perimeter at 924.4 ft	60" RCP	2150	0.0050	10.7
P-3	12" horizontal orifice w/invert @ 914 ft Sharp crested rectangular weir w/invert at 915 ft and crest length = 7 ft Sharp crested rectangular weir w/invert at 918.25 ft and crest length = 10 ft	60" RCP	100	0.0085	13.9
P-4	6" horizontal orifice w/invert @ 915 ft 9" Diameter horizontal orifice w/invert @ 915 ft 24" RCP culvert w/inlet @ 915.95 ft	24" RCP	50	0.0050	5.8
P-5/P-6	12" horizontal orifice w/invert @ 930 ft 9" vertical orifice w/invert @ 930 ft 7 ft sharp crested weir @ 930.5 ft 10 ft sharp crested weir @ 931.5 ft dropping into a 48" pipe	48" RCP	830	0.0070	10.8
P-7	12" RCP W/invert at 915 75' broad crested weir @ 915 ft		Connection to W-3 Overland Flow to P-7 from P-9		
P-8	24" RCP w/invert at 897 ft	24" RCP	380	0.0028	3.9
P-9	80' broad crested weir @ 915 ft		Overland Flow to P-9 from P-11		
P-10	2.5 ft sharp crested rectangular baffle weir w/invert @ 896 ft dumping into 30" pipe 50 ft broad crested rectangular weir w/invert @ 897.4 ft	30" RCP	750	0.0037	6.8
P-11	12" horizontal orifice w/invert @ 909 ft 12" horizontal orifice w/invert at 909 ft Two 24" RCPs w/invert at @ 910 ft 60 ft broad crested rectangular weir w/invert @ 912 ft	two 24" RCP	200	0.0050	10.0
P-12	two 12" horizontal orifice w/invert at 893 ft four 44"x27" RCP arch culverts w/ invert at 893.5 ft		Overland flow via channel to P-13 from P-12		
P-13	five 12" RCP pipes w/invert at 883 ft 55 ft broad crested weir		Connection to W-5 Channel flow to Rice Creek		
P-14	12" horizontal orifice w/invert at 892 ft 18" Round RCP culvert w/invert at 893 ft		18" RCP is sufficient, specific reach not defined in model		
CRH-1	Infiltration two 24" RCP pipes w/invert @ 877 ft	- two 24" RCP	15 acres of infiltration are 155	0.8 in/hr 0.0065	2.5
CRH-2	Infiltration two 24" RCP pipes w/invert @ 881.5 ft	- two 24" RCP	.2 acres of infiltration are 155	0.8 in/hr 0.0065	2.5
CRH-3	Infiltration two 24" RCP pipes w/invert @ 878 ft	- two 24" RCP	15 acres of infiltration are 155	0.8 in/hr 0.0065	2.5
W-1	6" horizontal orifice w/invert @ 914.75 ft	12" RCP	100	0.0010	1.6
W-2	12" RCP w/invert @ 929.1 ft	12" RCP	300	0.0437	2.6
W-3	12" RCP w/invert @ 914.75 ft	12" RCP	50	0.0380	2.5
W-4	12" RCP w/invert @ 908 ft	12" RCP	170	0.0020	2.0
W-5	Two 6 ft sharp crested rectangular baffle weir w/invert @ 882.75 ft	12" RCP	100	0.0050	3.7

HydroCAD Tech Memo

Section 6.0 Table 6-2, Interim Pond Geometry

Basin ID	Tributary Area				Basin Info		
	Tributary	Direct (acres)	Previously Ponded (acres)	Total (acres)	Outlet Elev/ NWL (feet)	Emergency Overflow (feet)	Dead Storage (ac-ft)
Round Lake							
P-1	-	-	-	-	Constructed in Later Phase		
P-2	1, 2, 24	68.3	0.0	68.3	924.0	926.0	0.7
P-3	-	-	-	-	Constructed in Later Phase		
P-4	5	7.9	0.0	7.9	915.0	918.0	0.6
P-5	4, P-6	0.6	42.7	43.3	930.0	933.0	5.1
P-6	3, 25	42.7	0.0	42.7	930.0	933.0	
W-1	6	1.0	0.0	1.0	913.0	915.0	-
W-2	P-6	0.0	42.7	42.7	929.0	930.0	-
Offsite Subbasin 51	Offsite Subbasin 51	25.2	0.0	25.2	-	-	-
Rice Creek/Long Lake							
P-7	8	29.6	0.0	29.6	915.0	916.0	1.1
P-8	10	6.4	0.0	6.4	897.0	900.0	0.5
P-9	9, P-7, W-3	25.8	29.6	55.4	915.0	918.0	0.4
P-10	12, P-8, P-11	1.4	65.1	66.4	896.0	899.0	0.7
P-11	11, P-9	3.3	55.4	58.7	909.0	913.0	3.6
P-12	14, P-10, W-4	10.2	69.4	79.7	893.0	896.0	5.1
P-13	15, 16, 18, 26, P-12	158.1	79.7	237.8	883.0	886.0	4.3
P-14	19	21.2	0.0	21.2	Constructed in Later Phase		
Thumb	22, 27	48.5	0.0	48.5	-	-	-
CRH-1	28	7.0	0.0	5.7	876.0	879.0	-
CRH-2	29	10.2	0.0	10.2	881.5	884.0	-
CRH-3	CRH-2, 1S	1.6	10.2	11.8	876.0	879.0	-
W-3	P7	0.0	29.6	29.6	914.8	916.0	-
W-4	13	3.0	0.0	3.0	908.0	910.0	-
W-5	17	7.6	237.8	245.4	880.0	883.0	-

NOTE: Subbasins 20-22 and 27 are not routed through onsite ponds. No subbasin assigned number 23
 Outlet information in Appendix 3, Table 2

HydroCAD Tech Memo

Section 6.0 Table 6-3, Interim Pond Discharge

Basin ID	Tributary Subbasins and Ponds	100-Year Storm			10-Year Storm			2-Year Storm		
		HWL	Live Storage	Peak Outflow	HWL	Live Storage	Peak Outflow	HWL	Live Storage	Peak Outflow
		(feet)	(ac-ft)	(cfs)	(feet)	(ac-ft)	(cfs)	(feet)	(ac-ft)	(cfs)
Round Lake										
P-1	-	-	-	-	-	-	-	-	-	-
P-2	1, 2, 24	925.5	0.6	146	925.0	0.4	59	924.7	0.3	25
P-3	-	-	-	272	-	-	94	-	-	37
P-4	5	917.3	0.8	8	916.2	0.4	1	915.4	0.1	1
P-5	4, P-6	931.5	5.6	113	930.5	3.2	29	929.9	1.8	10
P-6	3, 25									
W-1	6	915.3	0.4	3	915.1	0.2	2	915.0	0.1	1
W-2	P-6	929.6	0.7	1	929.5	0.6	1	929.4	0.4	0
Offsite Subbasin 51	Offsite Subbasin 51	-	-	94	-	-	37	-	-	16
Rice Creek/Long Lake										
P-7	8	915.9	0.4	77	915.8	0.4	32	915.8	0.4	14
P-8	10	899.0	0.9	12	898.1	0.4	4	897.6	0.2	1
P-9	9, P-7, W-3	915.8	0.2	148	915.4	0.1	60	915.3	0.1	25
P-10	12, P-8, P-11	898.4	0.8	127	897.6	0.5	25	896.8	0.2	5
P-11	11, P-9	912.7	5.2	136	911.4	3.2	26	910.2	1.5	7
P-12	14, P-10, W-4	895.5	4.5	118	894.1	1.9	23	893.6	0.9	6
P-13	15, 16, 18, 26, P-12	855.3	5.3	503	884.2	2.5	189	883.7	1.4	86
P-14	19	-	-	75	-	-	30	-	-	12
Thumb	22, 27	-	-	53	-	-	0	-	-	0
CRH-1	28	878.8	0.8	25	878.1	0.5	12	877.7	0.4	5
CRH-2	29	883.8	1.5	27	882.7	0.9	10	882.1	0.6	2
CRH-3	CRH-2, 1S	879.8	0.8	26	878.9	0.4	8	878.3	2.6	1
W-3	P7	915.2	0.9	1	915.1	0.6	0	915.0	0.5	0
W-4	13	909.3	1.3	4	908.9	0.9	3	908.7	0.6	2
W-5	17	-	-	-	883.1	1.6	7	882.9	0.9	3

NOTE: Subbasins 20-22 and 27 are not routed through onsite ponds. No subbasin assigned number 23

HydroCAD Tech Memo

Section 6.0 Table 6-4, Full Buildout Pond Geometry

Basin ID	Tributary Area				Basin Info		
	Tributary	Direct	Previously Ponded	Total	Outlet Elev/ NWL	Emergency Overflow	Dead Storage
		(acres)	(acres)	(acres)	(feet)	(feet)	(ac-ft)
Round Lake							
P-1	1	52.2	0.0	52.2	924.0	926.0	3.5
P-2	2, 24	16.1	52.2	68.3			
P-3	-	21.6	111.6	133.1	914.0	920.0	5.8
P-4	5	7.9	0.0	7.9	915.0	918.0	0.6
P-5	4, P-6	0.6	42.7	43.3	930.0	933.0	5.1
P-6	3, 25	42.7	0.0	42.7			
W-1	6	1.0	0.0	1.0	913.0	916.0	-
W-2	P-6	0.0	42.7	42.7	929.0	930.0	-
Offsite Subbasin 51	Offsite Subbasin 51	25.2	0.0	25.2	-	-	-
Rice Creek/Long Lake							
P-7	8	29.6	0.0	29.6	915.0	916.0	1.1
P-8	10	6.4	0.0	6.4	897.0	900.0	0.5
P-9	9, P-7, W-3	25.8	29.6	55.4	915.0	918.0	0.4
P-10	12, P-8, P-11	1.4	65.1	66.4	896.0	899.0	0.7
P-11	11, P-9	3.3	55.4	58.7	909.0	913.0	3.6
P-12	14, P-10, W-4	10.2	69.4	79.7	893.0	896.0	5.1
P-13	15, 16, 18, 26, P-12	158.1	79.7	237.8	883.0	886.0	4.3
P-14	19	21.2	0.0	21.2	892.0	895.5	4.5
Thumb	22, 27	48.5	0.0	48.5	-	-	-
CRH-1	28	7.0	0.0	7.0	876.0	879.0	-
CRH-2	29	10.2	0.0	10.2	881.5	884.0	-
CRH-3	CRH-2, 1S	1.6	10.2	11.8	876.0	879.0	-
W-3	P7	0.0	29.6	29.6	914.8	916.0	-
W-4	13	3.0	0.0	3.0	908.0	910.0	-
W-5	17	7.6	237.8	245.4	880.0	883.0	-

NOTE: Subbasins 20-22 and 27 are not routed through onsite ponds. No subbasin assigned number 23
Outlet information in Appendix 3, Table 2

HydroCAD Tech Memo

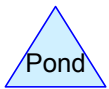
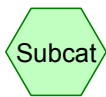
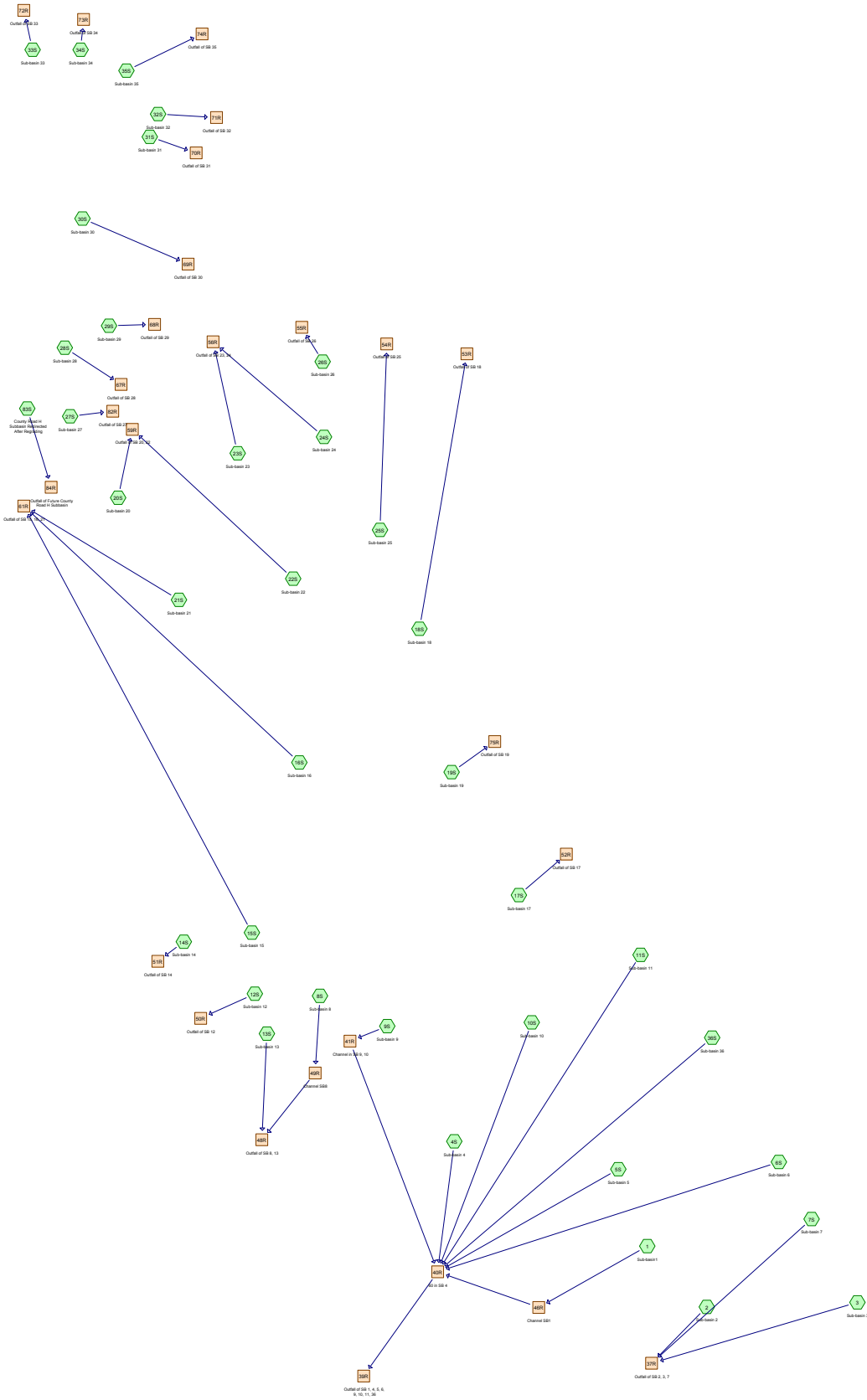
Section 6.0 Table 6-5, Full Buildout Pond Discharge

Basin ID	Tributary Area	100-Year Storm			10-Year Storm			2-Year Storm		
	Tributary	HWL (feet)	Live Storage (ac-ft)	Peak Outflow (cfs)	HWL (feet)	Live Storage (ac-ft)	Peak Outflow (cfs)	HWL (feet)	Live Storage (ac-ft)	Peak Outflow (cfs)
Round Lake										
P-1	1	925.7	2.4	174	925.2	1.6	90	924.9	1.3	53
P-2	2, 24									
P-3	-	919.7	13.3	261	918.0	7.9	98	916.5	5.2	47
P-4	5	917.8	1.0	13	916.8	0.6	4	916.2	0.4	2
P-5	4, P-6	932.6	8.4	123	931.5	5.6	29	930.7	3.8	7
P-6	3, 25									
W-1	6	915.4	0.5	3	915.2	0.3	2	915.1	0.2	2
W-2	P-6	929.6	0.7	1	929.5	0.6	1	929.4	0.4	0
Offsite Subbasin 51	Offsite Subbasin 51	-	-	94	-	-	37	-	-	16
Rice Creek/Long Lake										
P-7	8	915.9	0.4	86	915.8	0.4	40	915.8	0.4	21
P-8	10	899.1	0.9	12	898.1	0.4	4	897.6	0.2	1
P-9	9, P-7, W-3	915.9	0.2	167	915.5	0.1	78	915.4	0.1	42
P-10	12, P-8, P-11	898.5	0.9	169	897.8	0.6	39	897.5	0.5	22
P-11	11, P-9	912.8	5.4	161	912.0	4.2	40	910.9	2.5	16
P-12	14, P-10, W-4	895.8	5.2	141	894.4	2.3	37	893.8	1.4	12
P-13	15, 16, 18, 26, P-12	885.7	6.3	631	884.6	3.4	295	884.1	2.2	165
P-14	19	895.2	4.9	16	893.7	2.4	7	893.0	1.3	4
Thumb	22, 27	-	-	169	-	-	94	-	-	61
CRH-1	28	878.8	0.8	25	878.1	0.5	12	877.7	0.4	5
CRH-2	29	883.8	1.5	27	882.7	0.9	10	882.1	0.6	2
CRH-3	CRH-2, 1S	879.8	0.8	26	878.9	0.4	8	878.3	2.6	1
W-3	P7	915.2	0.9	1	915.1	0.7	1	915.0	0.5	0
W-4	13	909.4	1.4	4	909.0	0.9	3	908.8	0.8	3
W-5	17	-	-	-	883.2	2.4	12	883.1	1.7	8

NOTE: Subbasins 20-22 and 27 are not routed through onsite ponds. No subbasin assigned number 23

Appendix A

Existing Conditions (2012) Hydrology and Hydraulics Modeling (HydroCAD)



Routing Diagram for Existing Conditions_HydroCAD Model
 Prepared by Wenck Associates, Inc., Printed 6/8/2015
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Existing Conditions_HydroCAD Model

Prepared by Wenck Associates, Inc.

Printed 6/8/2015

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Page 2

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
15.328	87	(1)
27.092	75	(2, 3, 25S)
53.741	85	(4S, 20S, 24S)
27.171	83	(5S)
28.829	78	(6S, 19S)
9.790	62	(7S)
95.827	81	(8S, 10S, 16S)
94.629	82	(9S, 13S, 15S, 22S)
4.343	76	(11S)
9.187	80	(12S, 83S)
38.195	84	(14S, 18S)
8.010	86	(17S)
7.156	90	(21S)
13.825	92	(23S)
2.861	66	(26S, 27S)
5.784	44	(28S)
3.017	39	(29S, 31S, 32S)
31.577	42	(30S)
3.237	48	(33S)
1.241	49	(34S)
6.229	43	(35S)
11.210	79	(36S)
498.279	78	TOTAL AREA

Existing Conditions_HydroCAD Model

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Page 3

Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
498.279	Other	1, 2, 3, 4S, 5S, 6S, 7S, 8S, 9S, 10S, 11S, 12S, 13S, 14S, 15S, 16S, 17S, 18S, 19S, 20S, 21S, 22S, 23S, 24S, 25S, 26S, 27S, 28S, 29S, 30S, 31S, 32S, 33S, 34S, 35S, 36S, 83S
498.279		TOTAL AREA

Existing Conditions_HydroCAD Model

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Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	0.000	0.000	498.279	498.279		1, 2, 3, 4S, 5S, 6S, 7S, 8S, 9S, 10S, 11S, 12S, 13S, 14S, 15S, 16S, 17S, 18S, 19S, 20S, 21S, 22S, 23S, 24S, 25S, 26S, 27S, 28S, 29S, 30S, 31S, 32S, 33S, 34S, 35S, 36S, 83S
0.000	0.000	0.000	0.000	498.279	498.279	TOTAL AREA	

Existing Conditions_HydroCAD Model

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Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	40R	0.00	-23.69	718.0	0.0330	0.013	60.0	0.0	0.0

Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Time span=0.00-48.00 hrs, dt=0.02 hrs, 2401 points

Runoff by SCS TR-20 method, UH=SCS

Reach routing by Muskingum-Cunge method - Pond routing by Stor-Ind method

Subcatchment1: Sub-basin1	Runoff Area=15.328 ac 0.00% Impervious Runoff Depth=1.58" Tc=16.3 min CN=87 Runoff=26.50 cfs 2.022 af
Subcatchment2: Sub-basin 2	Runoff Area=4.913 ac 0.00% Impervious Runoff Depth=0.85" Tc=12.2 min CN=75 Runoff=4.67 cfs 0.346 af
Subcatchment3: Sub-basin 3	Runoff Area=15.522 ac 0.00% Impervious Runoff Depth=0.85" Tc=32.8 min CN=75 Runoff=9.18 cfs 1.093 af
Subcatchment4S: Sub-basin 4	Runoff Area=23.961 ac 0.00% Impervious Runoff Depth=1.44" Tc=11.3 min CN=85 Runoff=44.38 cfs 2.872 af
Subcatchment5S: Sub-basin 5	Runoff Area=27.171 ac 0.00% Impervious Runoff Depth=1.30" Tc=40.5 min CN=83 Runoff=23.85 cfs 2.950 af
Subcatchment6S: Sub-basin 6	Runoff Area=22.467 ac 0.00% Impervious Runoff Depth=1.00" Tc=46.4 min CN=78 Runoff=13.57 cfs 1.877 af
Subcatchment7S: Sub-basin 7	Runoff Area=9.790 ac 0.00% Impervious Runoff Depth=0.33" Tc=27.0 min CN=62 Runoff=1.66 cfs 0.268 af
Subcatchment8S: Sub-basin 8	Runoff Area=21.017 ac 0.00% Impervious Runoff Depth=1.18" Tc=9.5 min CN=81 Runoff=33.39 cfs 2.061 af
Subcatchment9S: Sub-basin 9	Runoff Area=9.296 ac 0.00% Impervious Runoff Depth=1.24" Tc=12.7 min CN=82 Runoff=13.79 cfs 0.960 af
Subcatchment10S: Sub-basin 10	Runoff Area=30.014 ac 0.00% Impervious Runoff Depth=1.18" Tc=37.7 min CN=81 Runoff=24.50 cfs 2.943 af
Subcatchment11S: Sub-basin 11	Runoff Area=4.343 ac 0.00% Impervious Runoff Depth=0.90" Tc=32.9 min CN=76 Runoff=2.75 cfs 0.324 af
Subcatchment12S: Sub-basin 12	Runoff Area=3.310 ac 0.00% Impervious Runoff Depth=1.12" Tc=14.0 min CN=80 Runoff=4.17 cfs 0.308 af
Subcatchment13S: Sub-basin 13	Runoff Area=2.279 ac 0.00% Impervious Runoff Depth=1.24" Tc=36.2 min CN=82 Runoff=2.01 cfs 0.235 af
Subcatchment14S: Sub-basin 14	Runoff Area=2.518 ac 0.00% Impervious Runoff Depth=1.37" Tc=8.9 min CN=84 Runoff=4.89 cfs 0.287 af
Subcatchment15S: Sub-basin 15	Runoff Area=56.506 ac 0.00% Impervious Runoff Depth=1.24" Tc=28.0 min CN=82 Runoff=57.14 cfs 5.833 af
Subcatchment16S: Sub-basin 16	Runoff Area=44.796 ac 0.00% Impervious Runoff Depth=1.18" Tc=26.3 min CN=81 Runoff=44.07 cfs 4.393 af

Subcatchment17S: Sub-basin 17	Runoff Area=8.010 ac 0.00% Impervious Runoff Depth=1.51" Tc=11.5 min CN=86 Runoff=15.45 cfs 1.008 af
Subcatchment18S: Sub-basin 18	Runoff Area=35.677 ac 0.00% Impervious Runoff Depth=1.37" Tc=15.8 min CN=84 Runoff=53.72 cfs 4.072 af
Subcatchment19S: Sub-basin 19	Runoff Area=6.362 ac 0.00% Impervious Runoff Depth=1.00" Tc=7.3 min CN=78 Runoff=9.29 cfs 0.531 af
Subcatchment20S: Sub-basin 20	Runoff Area=15.897 ac 0.00% Impervious Runoff Depth=1.44" Tc=17.1 min CN=85 Runoff=24.29 cfs 1.905 af
Subcatchment21S: Sub-basin 21	Runoff Area=7.156 ac 0.00% Impervious Runoff Depth=1.82" Tc=10.8 min CN=90 Runoff=17.27 cfs 1.085 af
Subcatchment22S: Sub-basin 22	Runoff Area=26.548 ac 0.00% Impervious Runoff Depth=1.24" Tc=19.6 min CN=82 Runoff=32.17 cfs 2.741 af
Subcatchment23S: Sub-basin 23	Runoff Area=13.825 ac 0.00% Impervious Runoff Depth=1.99" Tc=9.4 min CN=92 Runoff=38.34 cfs 2.294 af
Subcatchment24S: Sub-basin 24	Runoff Area=13.883 ac 0.00% Impervious Runoff Depth=1.44" Tc=19.0 min CN=85 Runoff=20.15 cfs 1.664 af
Subcatchment25S: Sub-basin 25	Runoff Area=6.657 ac 0.00% Impervious Runoff Depth=0.85" Tc=22.6 min CN=75 Runoff=4.76 cfs 0.469 af
Subcatchment26S: Sub-basin 26	Runoff Area=0.823 ac 0.00% Impervious Runoff Depth=0.46" Tc=38.2 min CN=66 Runoff=0.20 cfs 0.032 af
Subcatchment27S: Sub-basin 27	Runoff Area=2.038 ac 0.00% Impervious Runoff Depth=0.46" Tc=13.0 min CN=66 Runoff=0.77 cfs 0.078 af
Subcatchment28S: Sub-basin 28	Runoff Area=5.784 ac 0.00% Impervious Runoff Depth=0.01" Tc=23.9 min CN=44 Runoff=0.01 cfs 0.003 af
Subcatchment29S: Sub-basin 29	Runoff Area=1.255 ac 0.00% Impervious Runoff Depth=0.00" Tc=26.9 min CN=39 Runoff=0.00 cfs 0.000 af
Subcatchment30S: Sub-basin 30	Runoff Area=31.577 ac 0.00% Impervious Runoff Depth=0.00" Tc=45.9 min CN=42 Runoff=0.01 cfs 0.001 af
Subcatchment31S: Sub-basin 31	Runoff Area=0.884 ac 0.00% Impervious Runoff Depth=0.00" Tc=30.2 min CN=39 Runoff=0.00 cfs 0.000 af
Subcatchment32S: Sub-basin 32	Runoff Area=0.878 ac 0.00% Impervious Runoff Depth=0.00" Tc=27.6 min CN=39 Runoff=0.00 cfs 0.000 af
Subcatchment33S: Sub-basin 33	Runoff Area=3.237 ac 0.00% Impervious Runoff Depth=0.04" Tc=19.9 min CN=48 Runoff=0.01 cfs 0.010 af

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Subcatchment34S: Sub-basin 34	Runoff Area=1.241 ac 0.00% Impervious Runoff Depth=0.05" Tc=12.1 min CN=49 Runoff=0.01 cfs 0.005 af
Subcatchment35S: Sub-basin 35	Runoff Area=6.229 ac 0.00% Impervious Runoff Depth=0.00" Tc=16.7 min CN=43 Runoff=0.00 cfs 0.001 af
Subcatchment36S: Sub-basin 36	Runoff Area=11.210 ac 0.00% Impervious Runoff Depth=1.06" Tc=52.2 min CN=79 Runoff=6.74 cfs 0.989 af
Subcatchment83S: County Road H	Runoff Area=5.877 ac 0.00% Impervious Runoff Depth=1.12" Tc=19.1 min CN=80 Runoff=6.41 cfs 0.547 af
Reach 37R: Outfall of SB 2, 3, 7	Inflow=12.99 cfs 1.708 af Outflow=12.99 cfs 1.708 af
Reach 39R: Outfall of SB 1, 4, 5, 6, 9, 10, 11, 36	Inflow=111.85 cfs 14.937 af Outflow=111.85 cfs 14.937 af
Reach 40R: 60 in SB 4	Avg. Flow Depth=1.37' Max Vel=26.20 fps Inflow=111.84 cfs 14.937 af 60.0" Round Pipe n=0.013 L=718.0' S=0.0330 '/' Capacity=473.08 cfs Outflow=111.85 cfs 14.937 af
Reach 41R: Channel in SB 9, 10	Avg. Flow Depth=0.35' Max Vel=8.22 fps Inflow=13.79 cfs 0.960 af n=0.050 L=1,660.0' S=0.0048 '/' Capacity=280.23 cfs Outflow=12.78 cfs 0.960 af
Reach 46R: Channel SB1	Avg. Flow Depth=0.61' Max Vel=5.16 fps Inflow=26.50 cfs 2.022 af n=0.050 L=841.0' S=0.0071 '/' Capacity=296.86 cfs Outflow=26.13 cfs 2.022 af
Reach 48R: Outfall of SB 8, 13	Inflow=33.61 cfs 2.296 af Outflow=33.61 cfs 2.296 af
Reach 49R: Channel SB8	Avg. Flow Depth=0.54' Max Vel=5.56 fps Inflow=33.39 cfs 2.061 af n=0.050 L=521.0' S=0.0077 '/' Capacity=706.58 cfs Outflow=32.82 cfs 2.061 af
Reach 50R: Outfall of SB 12	Inflow=4.17 cfs 0.308 af Outflow=4.17 cfs 0.308 af
Reach 51R: Outfall of SB 14	Inflow=4.89 cfs 0.287 af Outflow=4.89 cfs 0.287 af
Reach 52R: Outfall of SB 17	Inflow=15.45 cfs 1.008 af Outflow=15.45 cfs 1.008 af
Reach 53R: Outfall of SB 18	Inflow=53.72 cfs 4.072 af Outflow=53.72 cfs 4.072 af
Reach 54R: Outfall of SB 25	Inflow=4.76 cfs 0.469 af Outflow=4.76 cfs 0.469 af
Reach 55R: Outfall of SB 26	Inflow=0.20 cfs 0.032 af Outflow=0.20 cfs 0.032 af

Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Reach 56R: Outfall of SB 23, 24	Inflow=51.42 cfs 3.958 af Outflow=51.42 cfs 3.958 af
Reach 59R: Outfall of SB 20, 22	Inflow=56.04 cfs 4.646 af Outflow=56.04 cfs 4.646 af
Reach 61R: Outfall of SB 15, 16, 21	Inflow=108.23 cfs 11.311 af Outflow=108.23 cfs 11.311 af
Reach 67R: Outfall of SB 28	Inflow=0.01 cfs 0.003 af Outflow=0.01 cfs 0.003 af
Reach 68R: Outfall of SB 29	Inflow=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af
Reach 69R: Outfall of SB 30	Inflow=0.01 cfs 0.001 af Outflow=0.01 cfs 0.001 af
Reach 70R: Outfall of SB 31	Inflow=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af
Reach 71R: Outfall of SB 32	Inflow=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af
Reach 72R: Outfall of SB 33	Inflow=0.01 cfs 0.010 af Outflow=0.01 cfs 0.010 af
Reach 73R: Outfall of SB 34	Inflow=0.01 cfs 0.005 af Outflow=0.01 cfs 0.005 af
Reach 74R: Outfall of SB 35	Inflow=0.00 cfs 0.001 af Outflow=0.00 cfs 0.001 af
Reach 75R: Outfall of SB 19	Inflow=9.29 cfs 0.531 af Outflow=9.29 cfs 0.531 af
Reach 82R: Outfall of SB 27	Inflow=0.77 cfs 0.078 af Outflow=0.77 cfs 0.078 af
Reach 84R: Outfall of Future County Road H Subbasin	Inflow=6.41 cfs 0.547 af Outflow=6.41 cfs 0.547 af

Total Runoff Area = 498.279 ac Runoff Volume = 46.208 af Average Runoff Depth = 1.11"
100.00% Pervious = 498.279 ac 0.00% Impervious = 0.000 ac

Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 1: Sub-basin1

Runoff = 26.50 cfs @ 12.18 hrs, Volume= 2.022 af, Depth= 1.58"

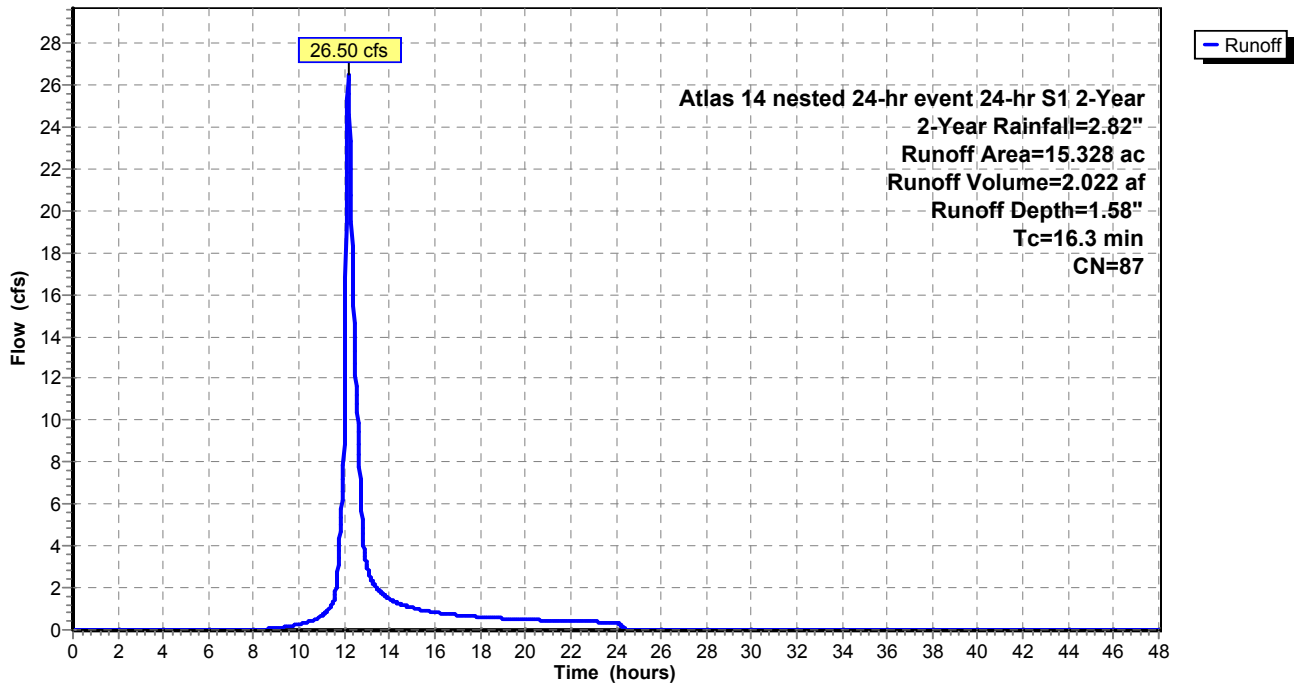
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 15.328	87	
15.328		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.3					Direct Entry,

Subcatchment 1: Sub-basin1

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 2: Sub-basin 2

Runoff = 4.67 cfs @ 12.14 hrs, Volume= 0.346 af, Depth= 0.85"

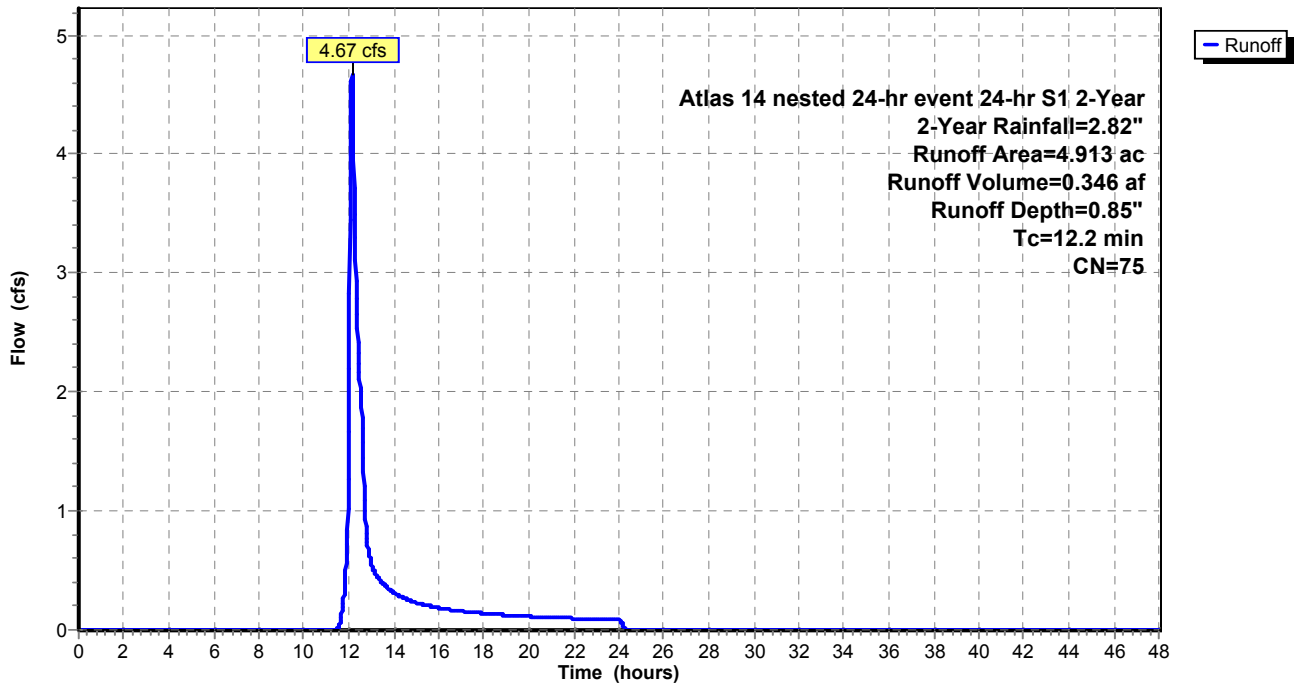
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 4.913	75	
4.913		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.2					Direct Entry,

Subcatchment 2: Sub-basin 2

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 3: Sub-basin 3

Runoff = 9.18 cfs @ 12.46 hrs, Volume= 1.093 af, Depth= 0.85"

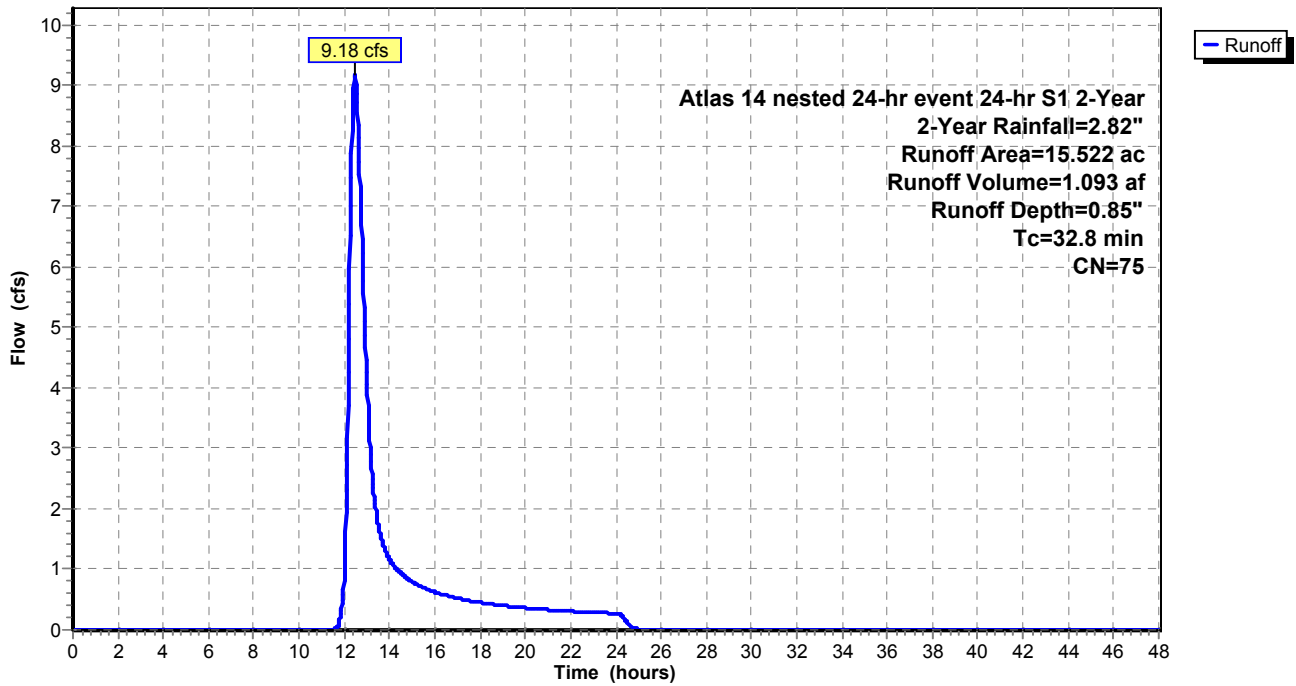
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 15.522	75	
15.522		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.8					Direct Entry,

Subcatchment 3: Sub-basin 3

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 4S: Sub-basin 4

Runoff = 44.38 cfs @ 12.11 hrs, Volume= 2.872 af, Depth= 1.44"

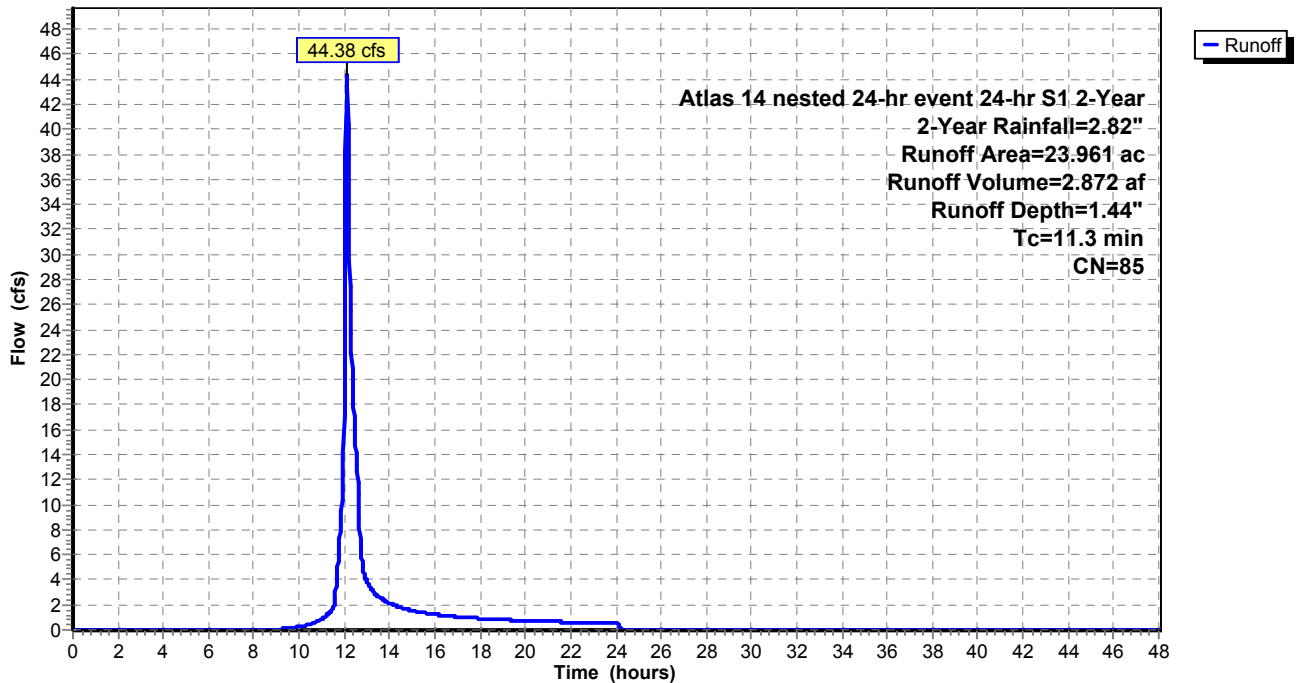
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 23.961	85	
23.961		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.3					Direct Entry,

Subcatchment 4S: Sub-basin 4

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 5S: Sub-basin 5

Runoff = 23.85 cfs @ 12.55 hrs, Volume= 2.950 af, Depth= 1.30"

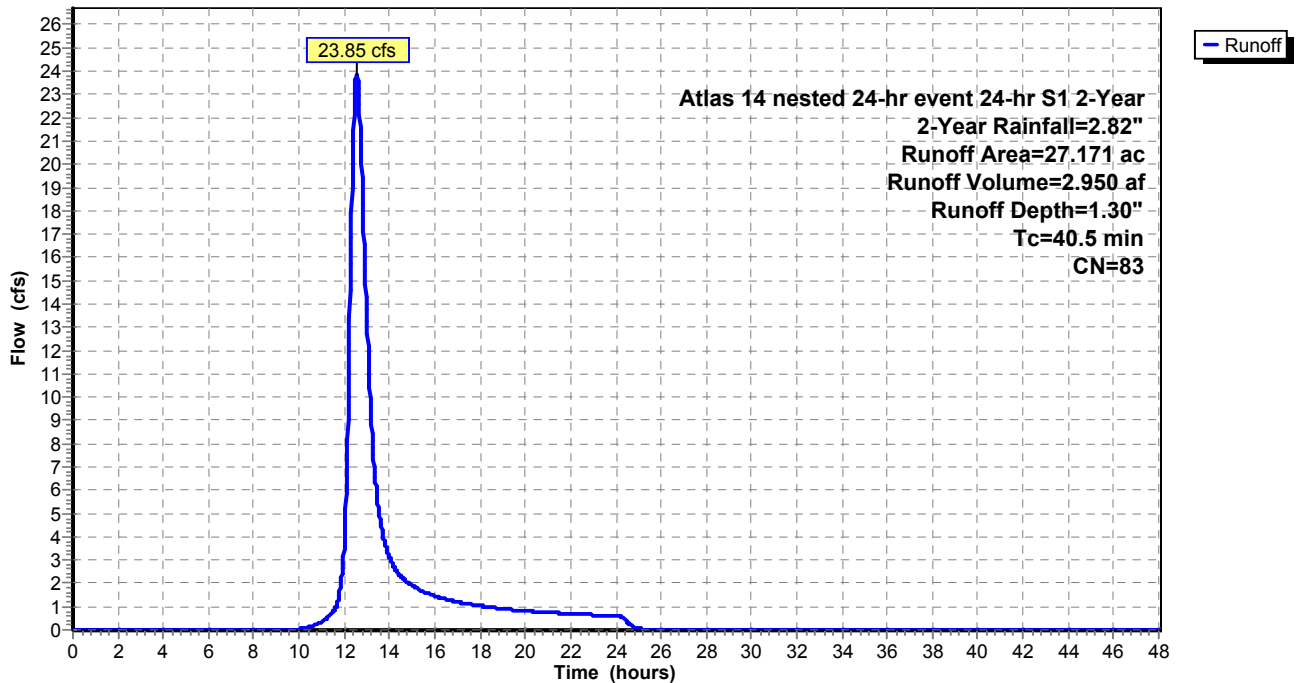
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 27.171	83	
27.171		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
40.5					Direct Entry,

Subcatchment 5S: Sub-basin 5

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 6S: Sub-basin 6

Runoff = 13.57 cfs @ 12.65 hrs, Volume= 1.877 af, Depth= 1.00"

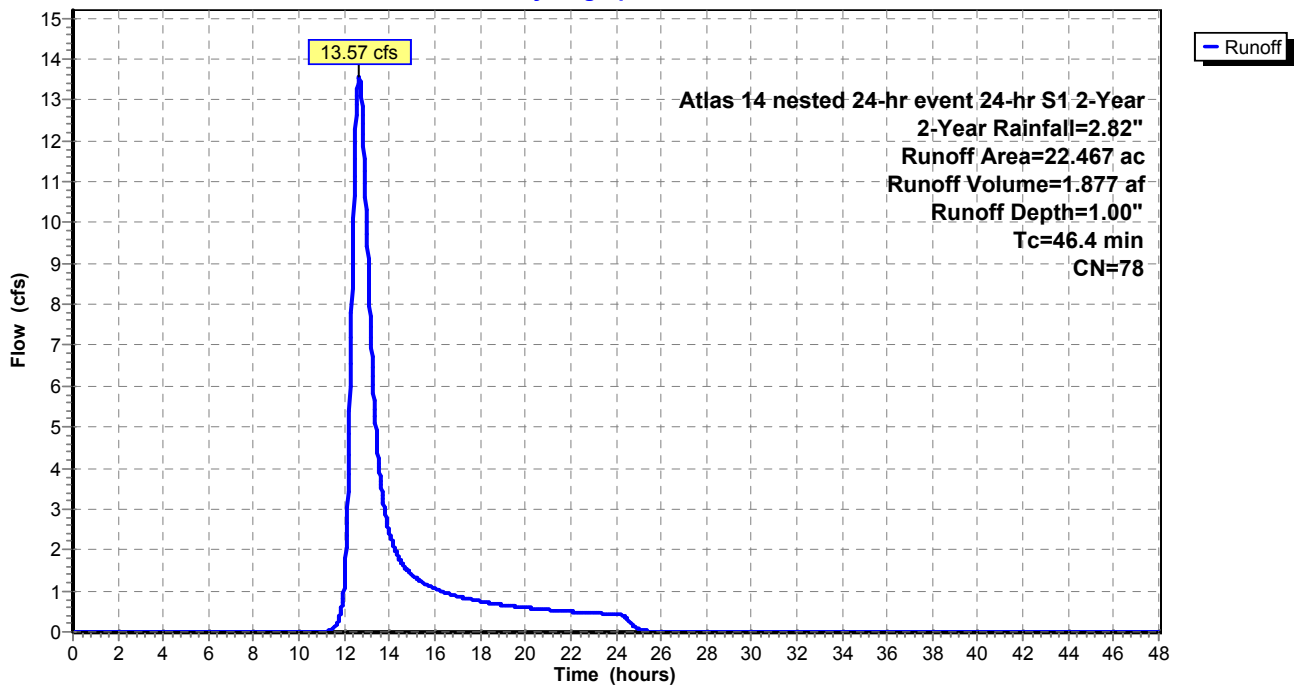
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 22.467	78	
22.467		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
46.4					Direct Entry,

Subcatchment 6S: Sub-basin 6

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 7S: Sub-basin 7

Runoff = 1.66 cfs @ 12.55 hrs, Volume= 0.268 af, Depth= 0.33"

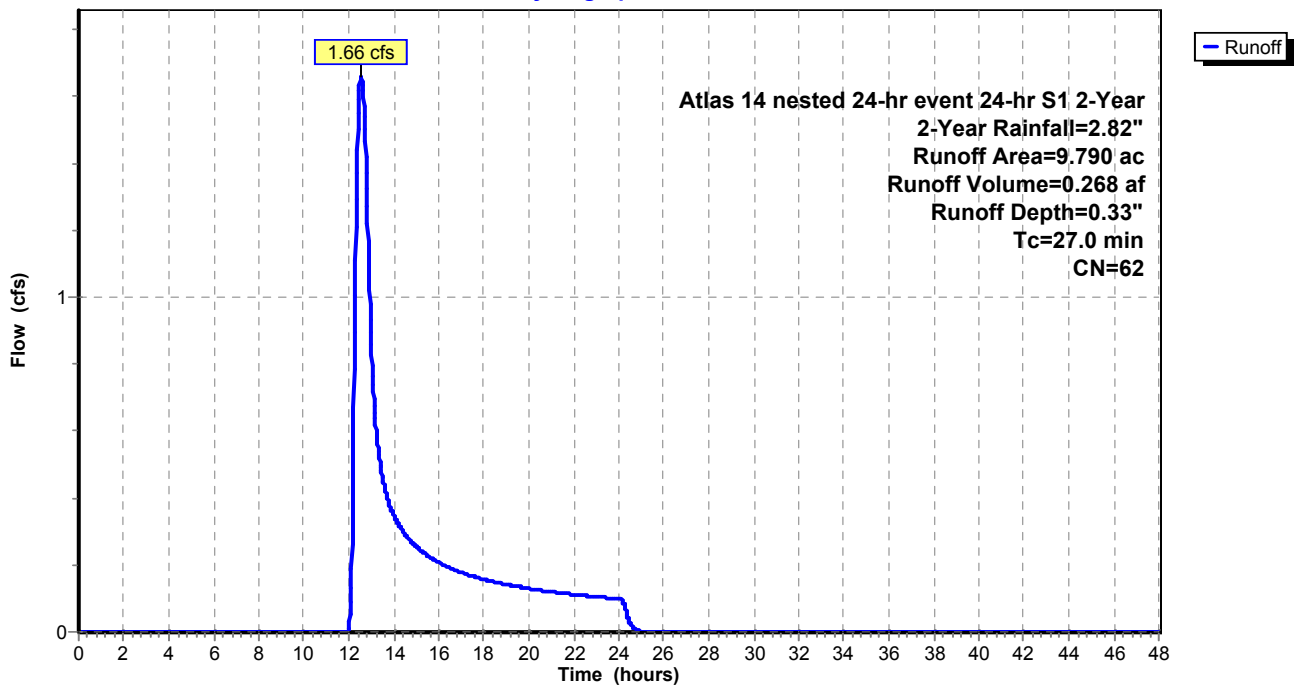
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 9.790	62	
9.790		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.0					Direct Entry,

Subcatchment 7S: Sub-basin 7

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 8S: Sub-basin 8

Runoff = 33.39 cfs @ 12.09 hrs, Volume= 2.061 af, Depth= 1.18"

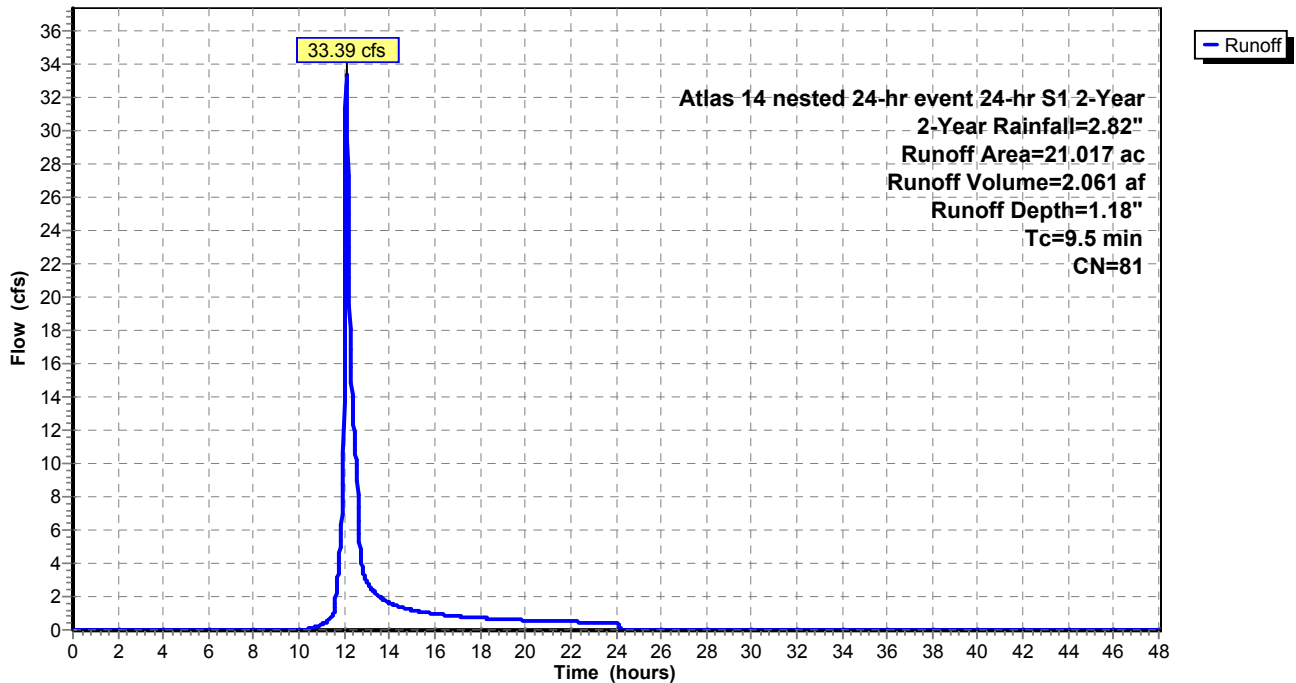
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 21.017	81	
21.017		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.5					Direct Entry,

Subcatchment 8S: Sub-basin 8

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 9S: Sub-basin 9

Runoff = 13.79 cfs @ 12.13 hrs, Volume= 0.960 af, Depth= 1.24"

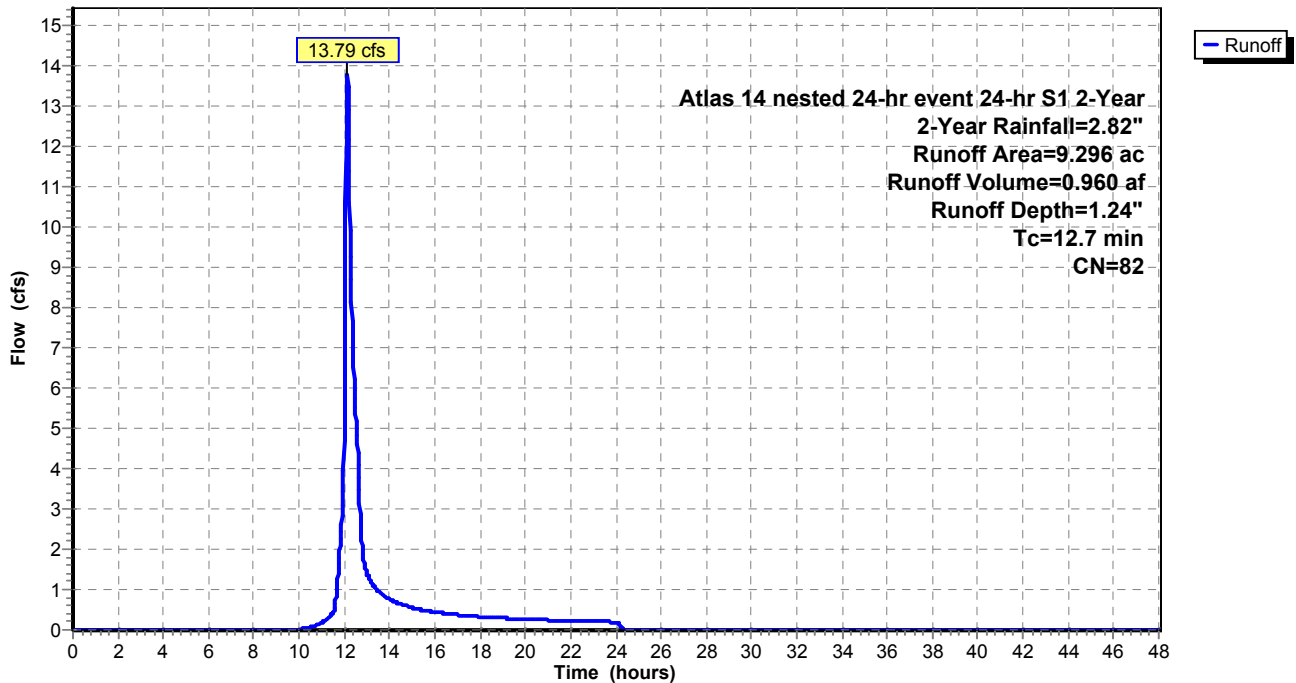
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 9.296	82	
9.296		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.7					Direct Entry,

Subcatchment 9S: Sub-basin 9

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 10S: Sub-basin 10

Runoff = 24.50 cfs @ 12.52 hrs, Volume= 2.943 af, Depth= 1.18"

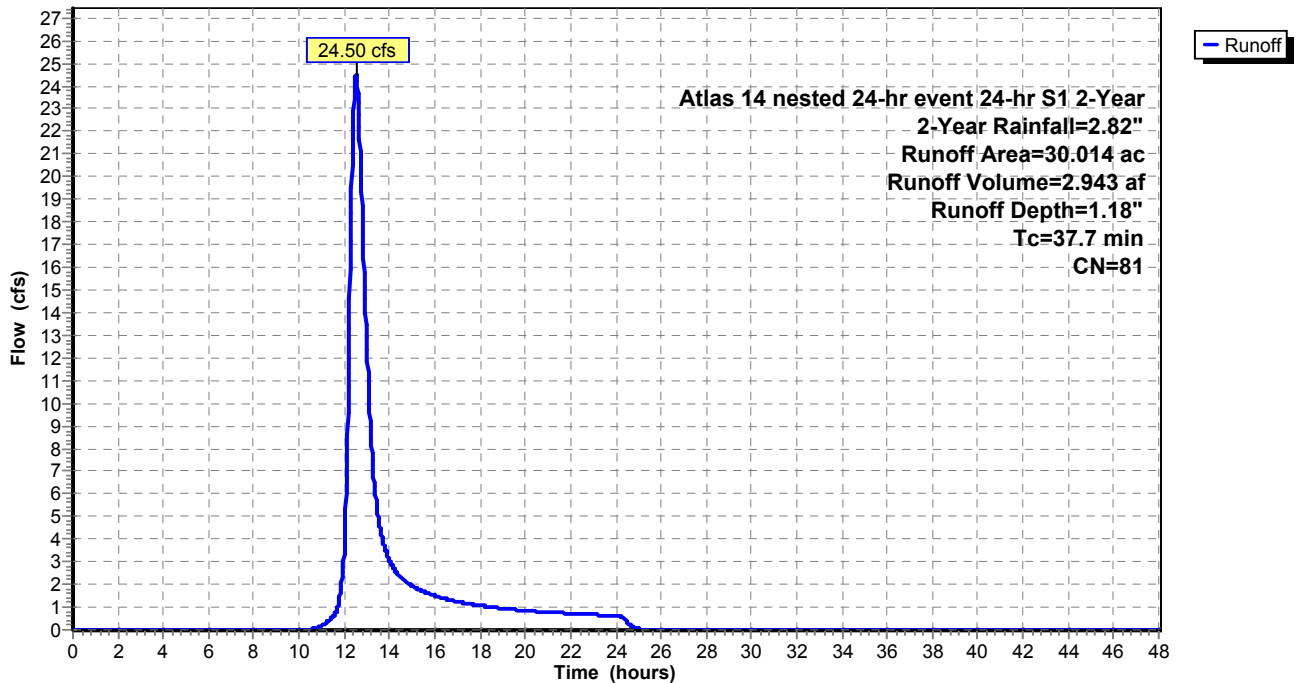
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 30.014	81	
30.014		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
37.7					Direct Entry,

Subcatchment 10S: Sub-basin 10

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 11S: Sub-basin 11

Runoff = 2.75 cfs @ 12.46 hrs, Volume= 0.324 af, Depth= 0.90"

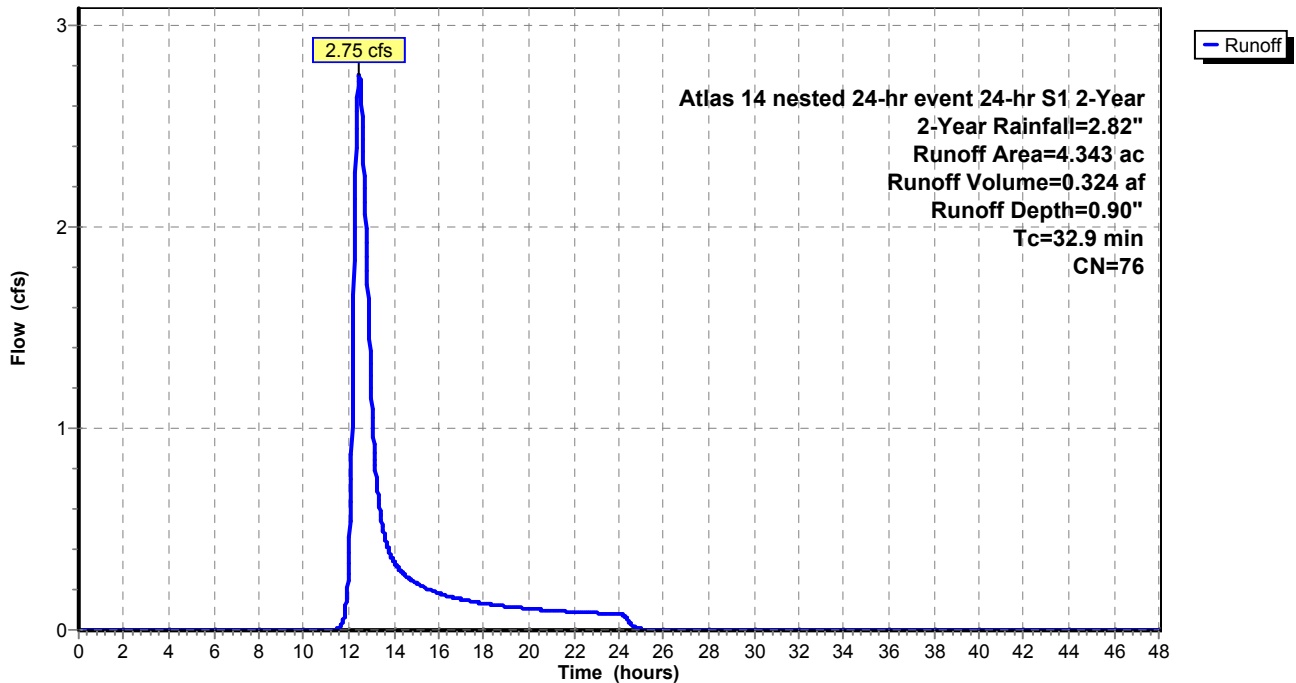
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 4.343	76	
4.343		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.9					Direct Entry,

Subcatchment 11S: Sub-basin 11

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 12S: Sub-basin 12

Runoff = 4.17 cfs @ 12.16 hrs, Volume= 0.308 af, Depth= 1.12"

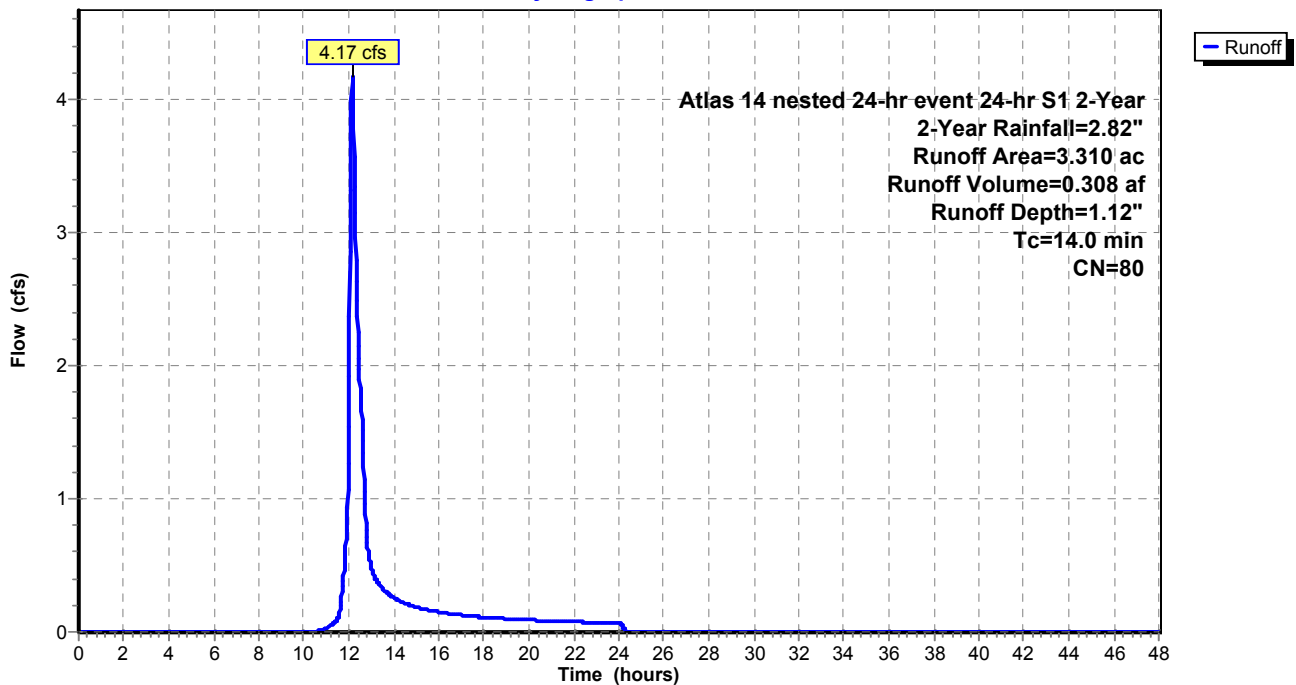
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 3.310	80	
3.310		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.0					Direct Entry,

Subcatchment 12S: Sub-basin 12

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 13S: Sub-basin 13

Runoff = 2.01 cfs @ 12.49 hrs, Volume= 0.235 af, Depth= 1.24"

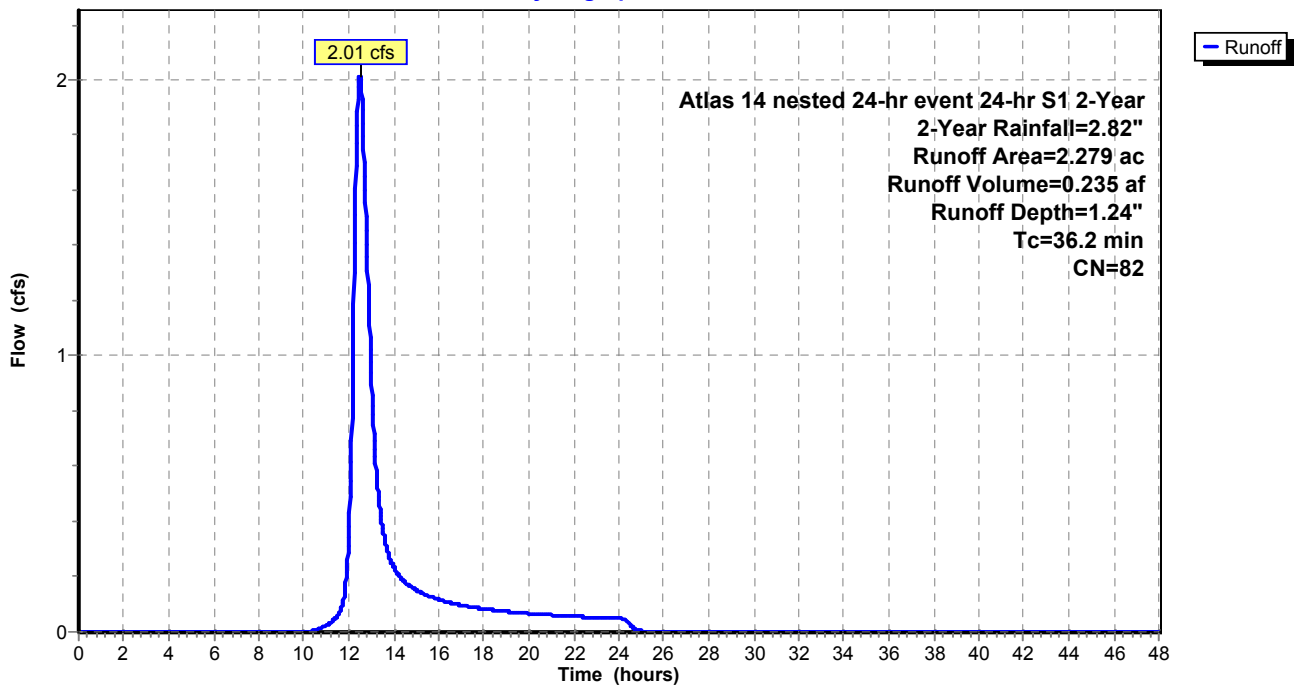
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 2.279	82	
2.279		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
36.2					Direct Entry,

Subcatchment 13S: Sub-basin 13

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 14S: Sub-basin 14

Runoff = 4.89 cfs @ 12.08 hrs, Volume= 0.287 af, Depth= 1.37"

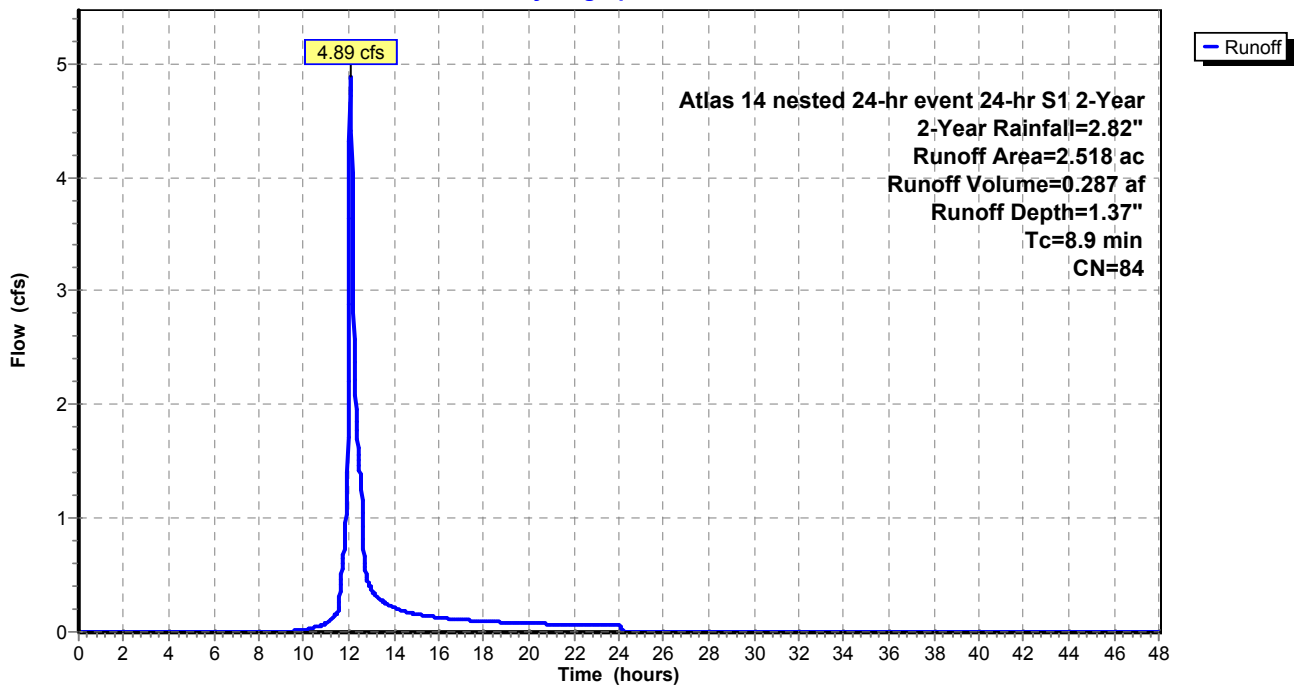
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 2.518	84	
2.518		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.9					Direct Entry,

Subcatchment 14S: Sub-basin 14

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 15S: Sub-basin 15

Runoff = 57.14 cfs @ 12.36 hrs, Volume= 5.833 af, Depth= 1.24"

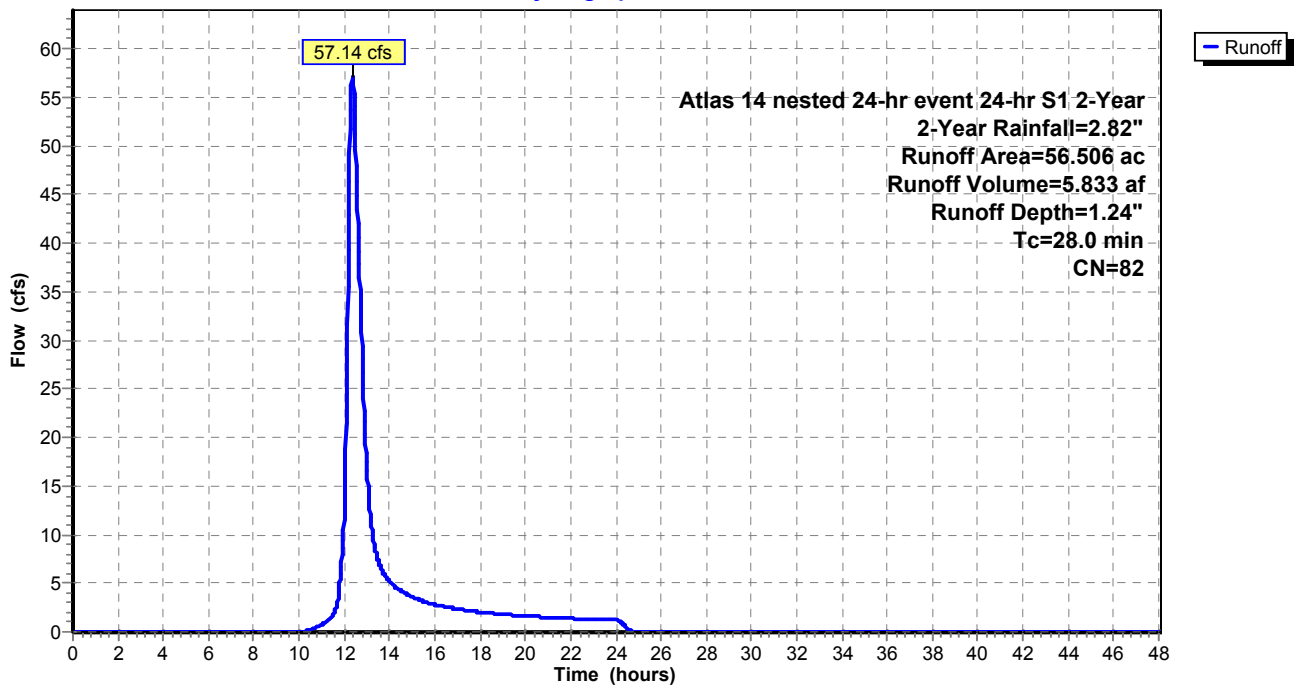
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 56.506	82	
56.506		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.0					Direct Entry,

Subcatchment 15S: Sub-basin 15

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 16S: Sub-basin 16

Runoff = 44.07 cfs @ 12.34 hrs, Volume= 4.393 af, Depth= 1.18"

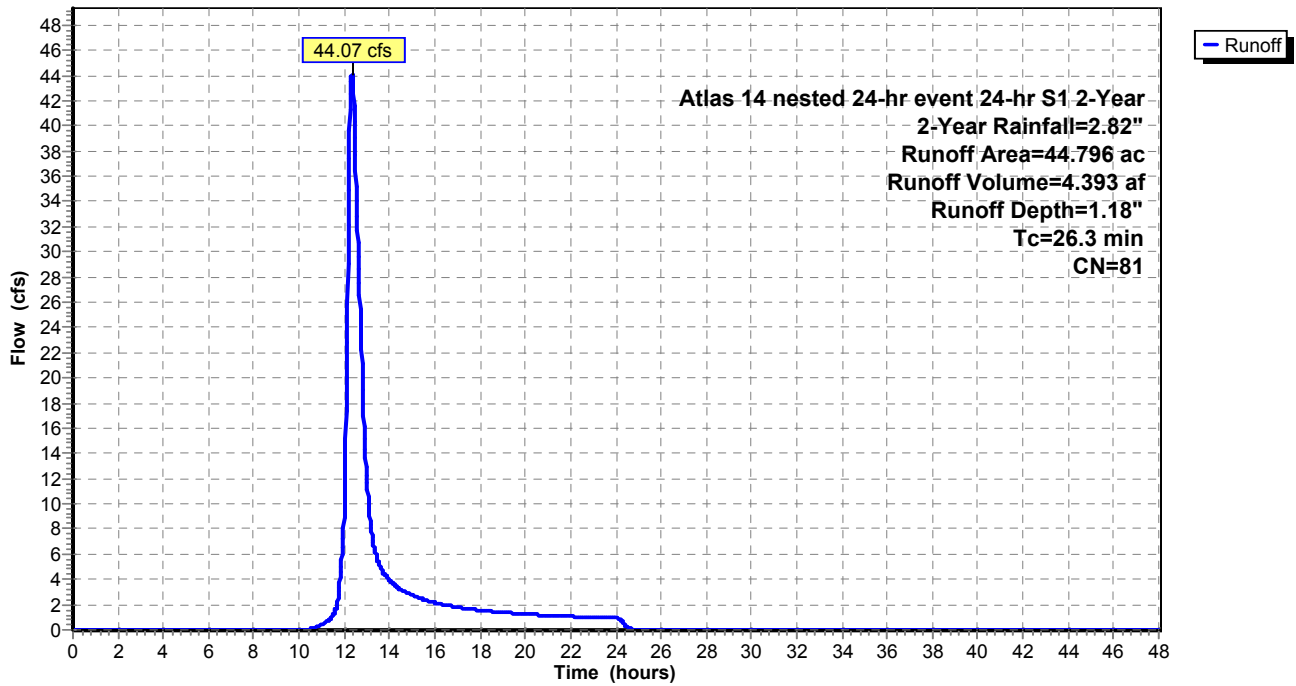
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 44.796	81	
44.796		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
26.3					Direct Entry,

Subcatchment 16S: Sub-basin 16

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 17S: Sub-basin 17

Runoff = 15.45 cfs @ 12.11 hrs, Volume= 1.008 af, Depth= 1.51"

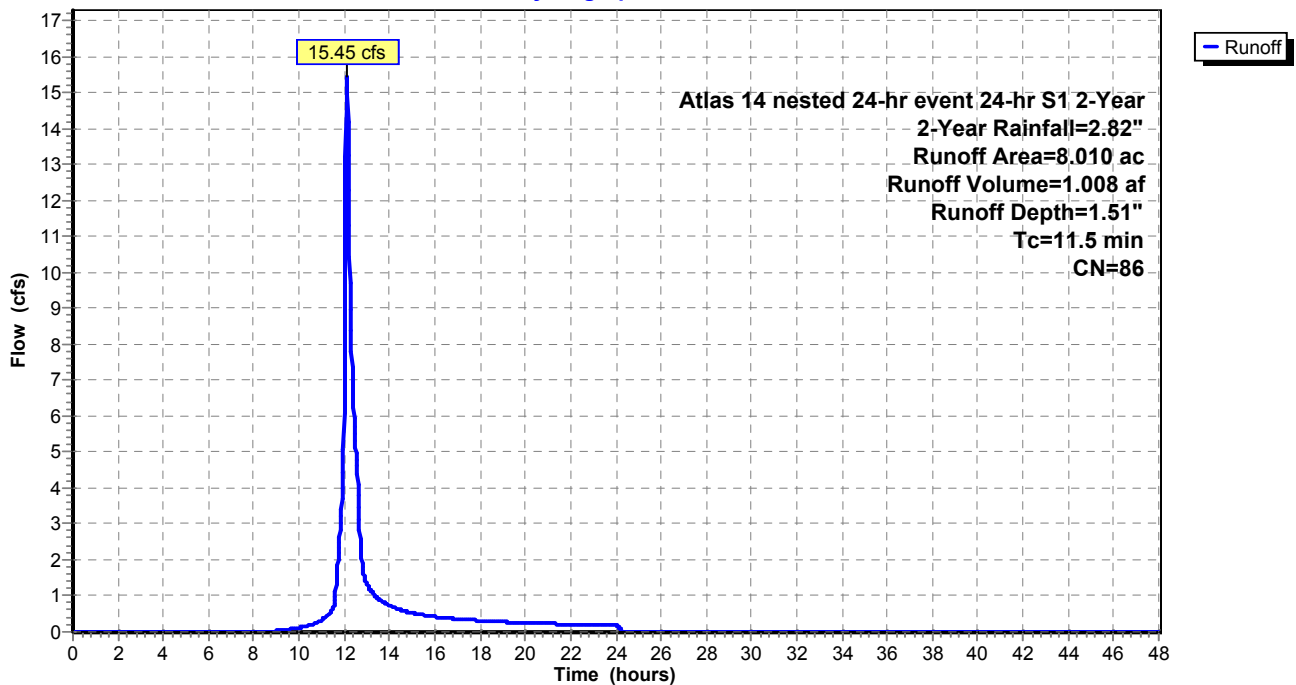
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 8.010	86	
8.010		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.5					Direct Entry,

Subcatchment 17S: Sub-basin 17

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 18S: Sub-basin 18

Runoff = 53.72 cfs @ 12.18 hrs, Volume= 4.072 af, Depth= 1.37"

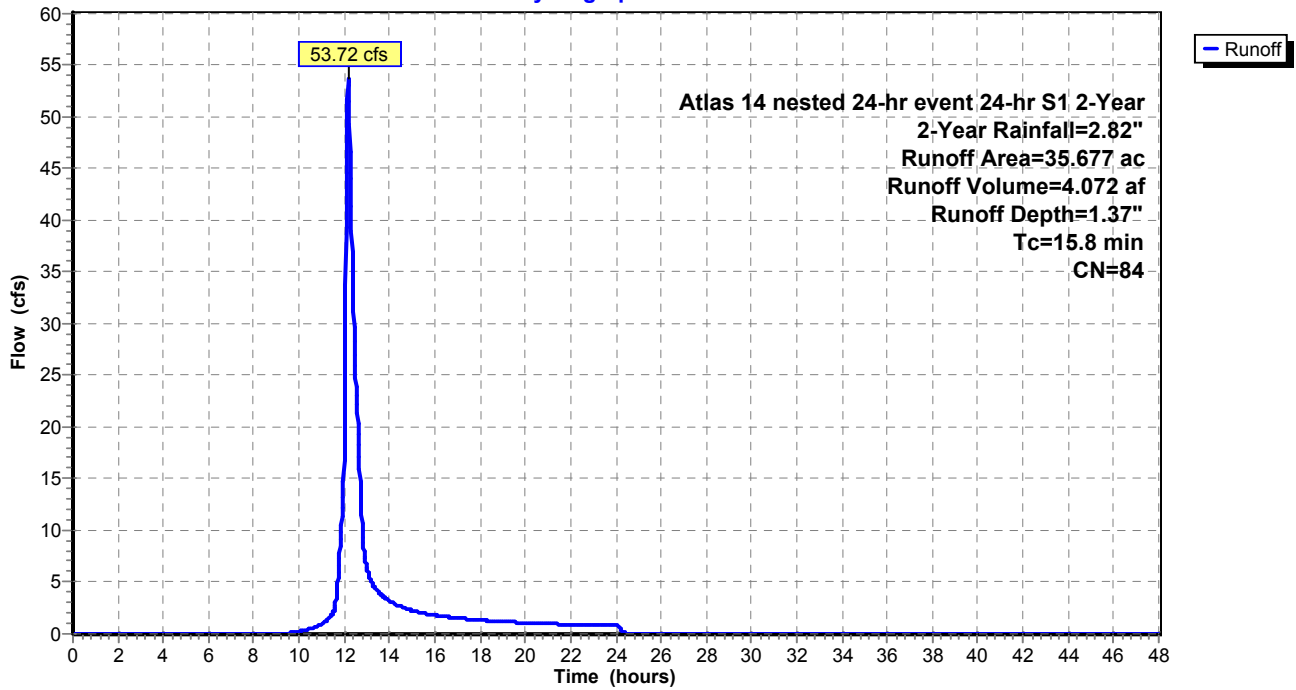
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 35.677	84	
35.677		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.8					Direct Entry,

Subcatchment 18S: Sub-basin 18

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 19S: Sub-basin 19

Runoff = 9.29 cfs @ 12.06 hrs, Volume= 0.531 af, Depth= 1.00"

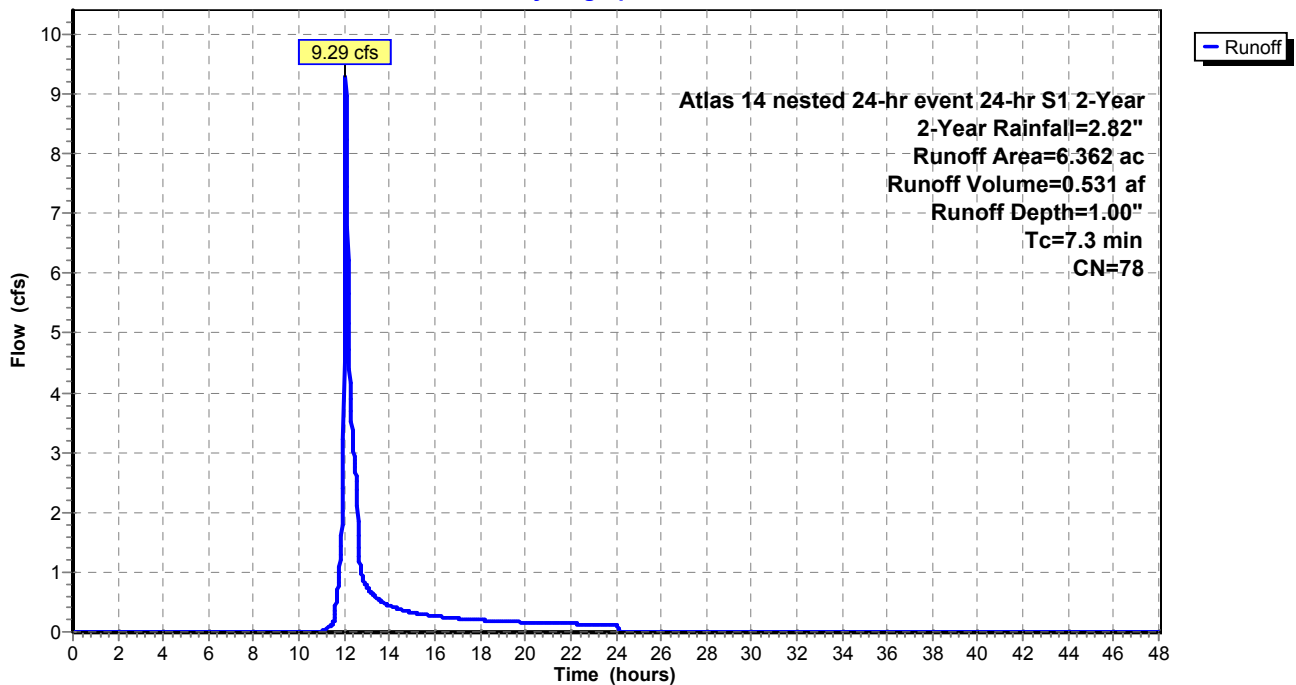
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 6.362	78	
6.362		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.3					Direct Entry,

Subcatchment 19S: Sub-basin 19

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 20S: Sub-basin 20

Runoff = 24.29 cfs @ 12.19 hrs, Volume= 1.905 af, Depth= 1.44"

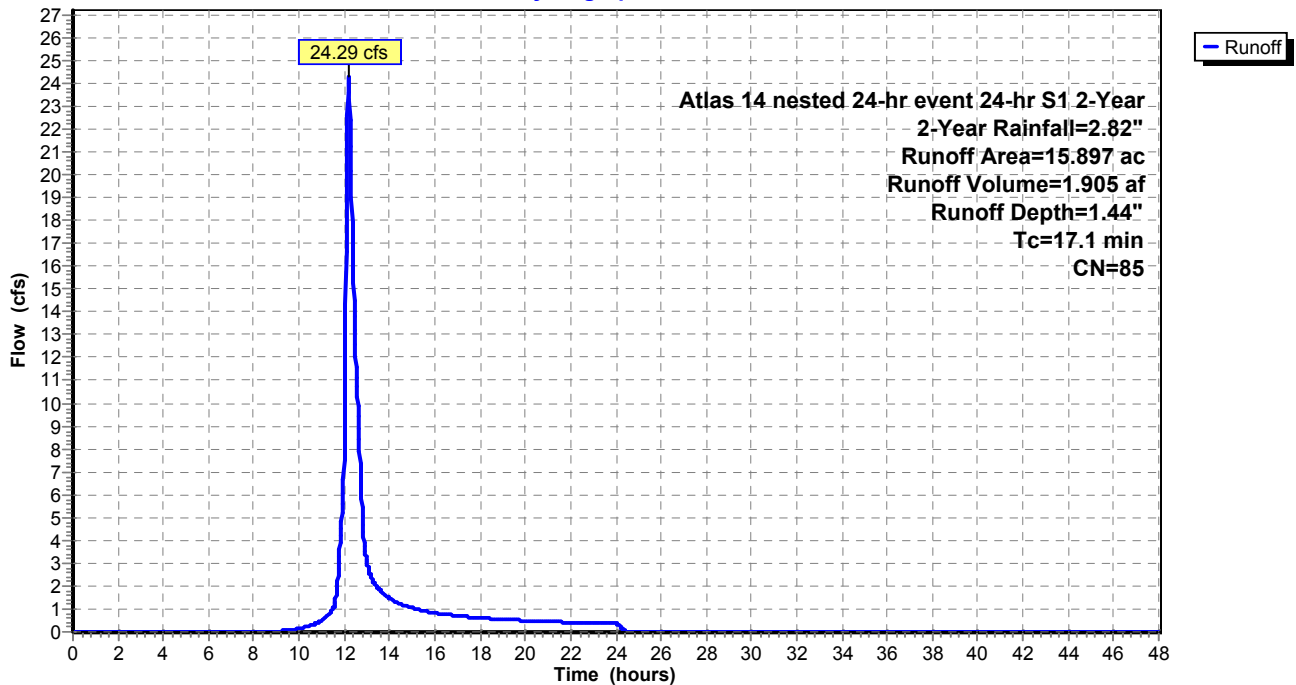
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 15.897	85	
15.897		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
17.1					Direct Entry,

Subcatchment 20S: Sub-basin 20

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 21S: Sub-basin 21

Runoff = 17.27 cfs @ 12.10 hrs, Volume= 1.085 af, Depth= 1.82"

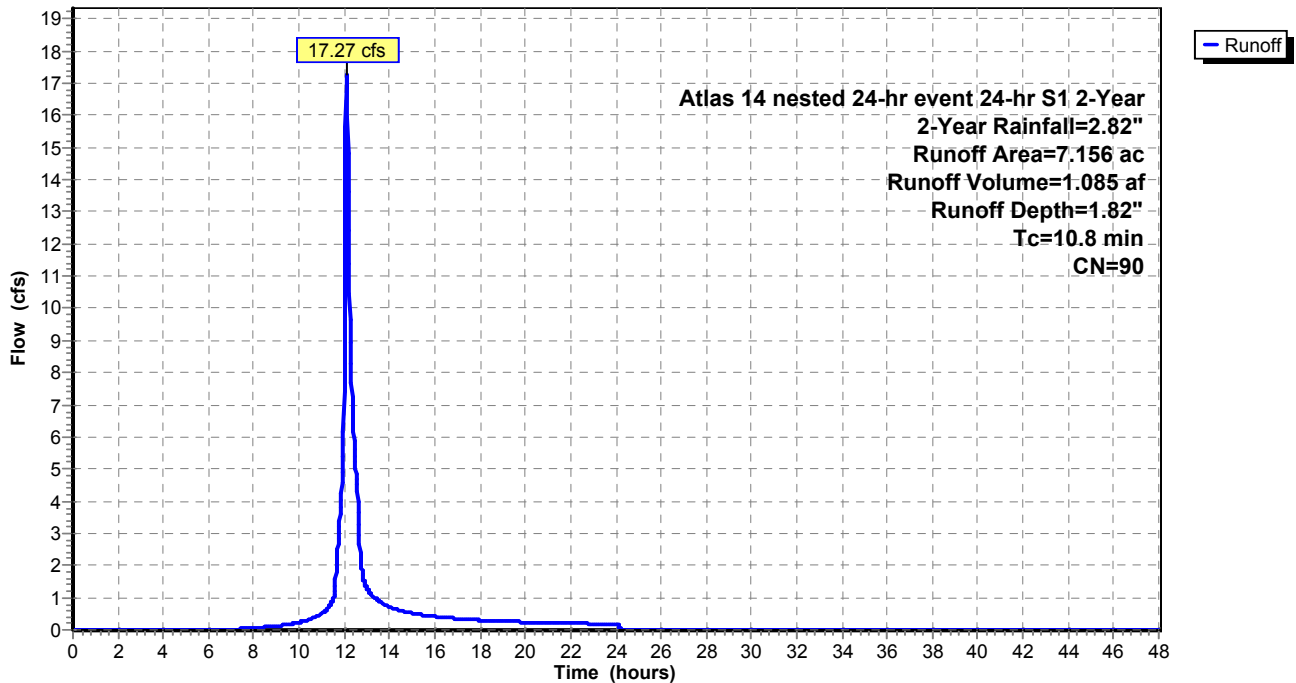
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 7.156	90	
7.156		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.8					Direct Entry,

Subcatchment 21S: Sub-basin 21

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 22S: Sub-basin 22

Runoff = 32.17 cfs @ 12.24 hrs, Volume= 2.741 af, Depth= 1.24"

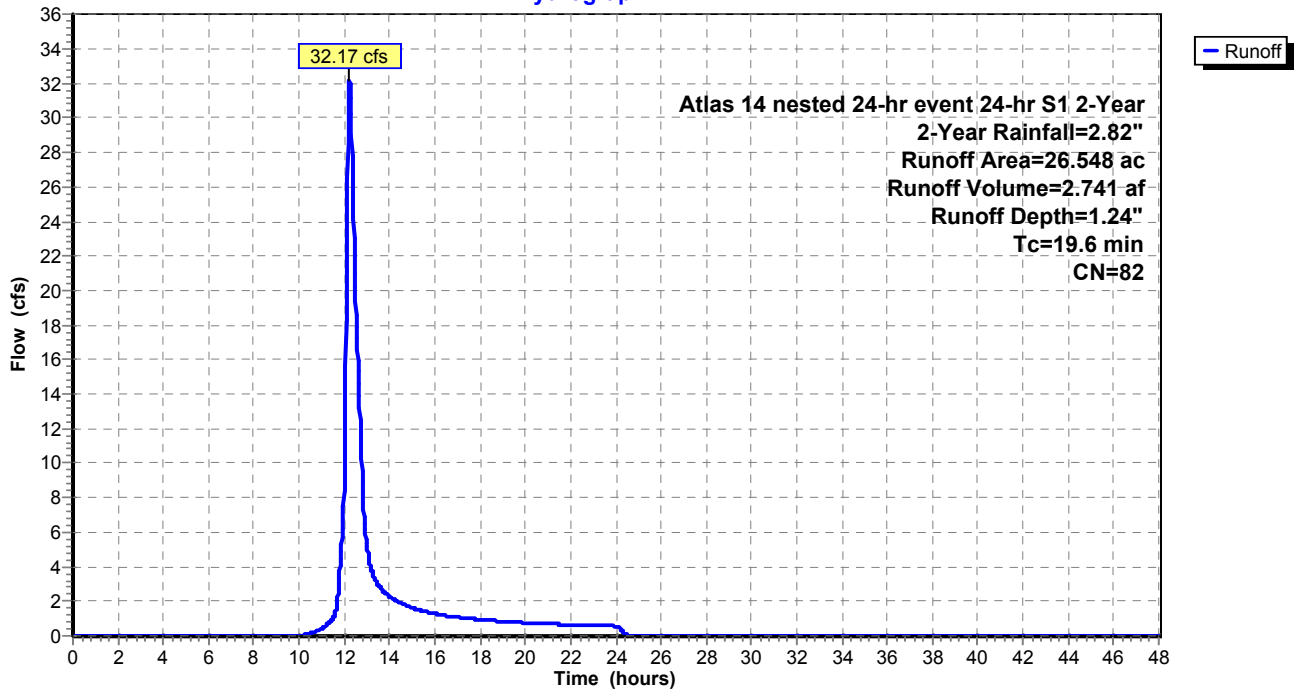
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 26.548	82	
26.548		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.6					Direct Entry,

Subcatchment 22S: Sub-basin 22

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 23S: Sub-basin 23

Runoff = 38.34 cfs @ 12.08 hrs, Volume= 2.294 af, Depth= 1.99"

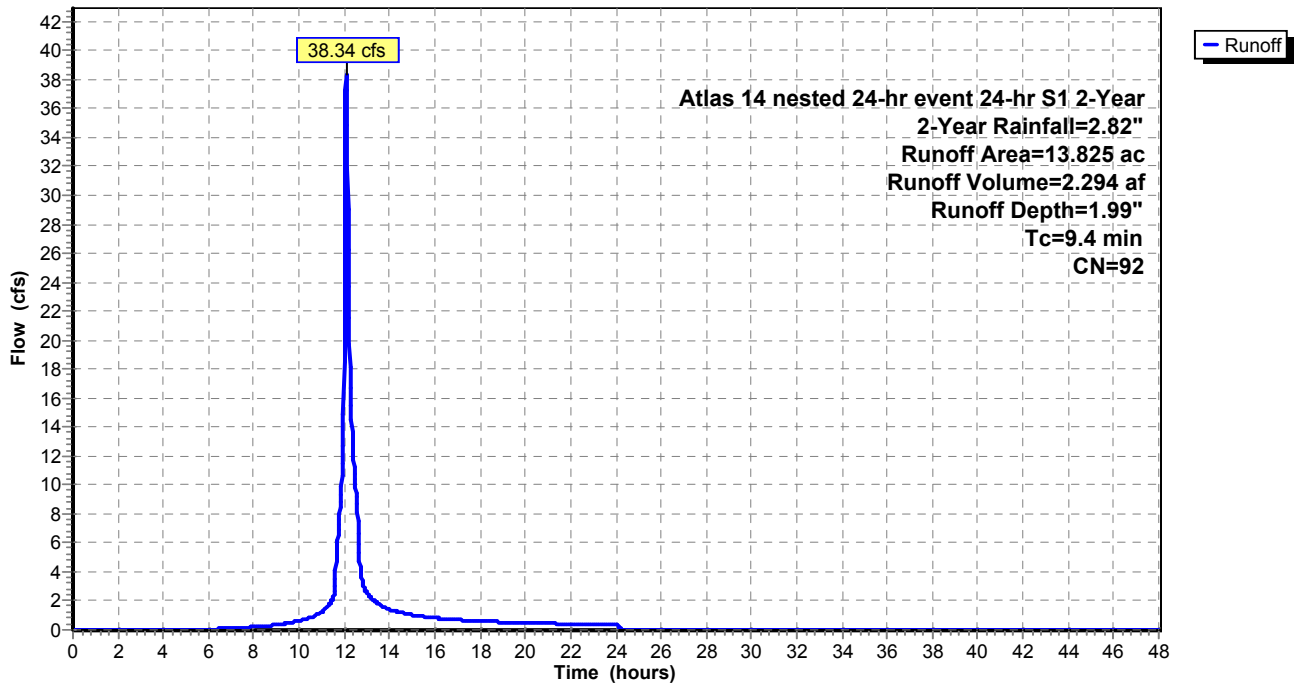
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 13.825	92	
13.825		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.4					Direct Entry,

Subcatchment 23S: Sub-basin 23

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 24S: Sub-basin 24

Runoff = 20.15 cfs @ 12.22 hrs, Volume= 1.664 af, Depth= 1.44"

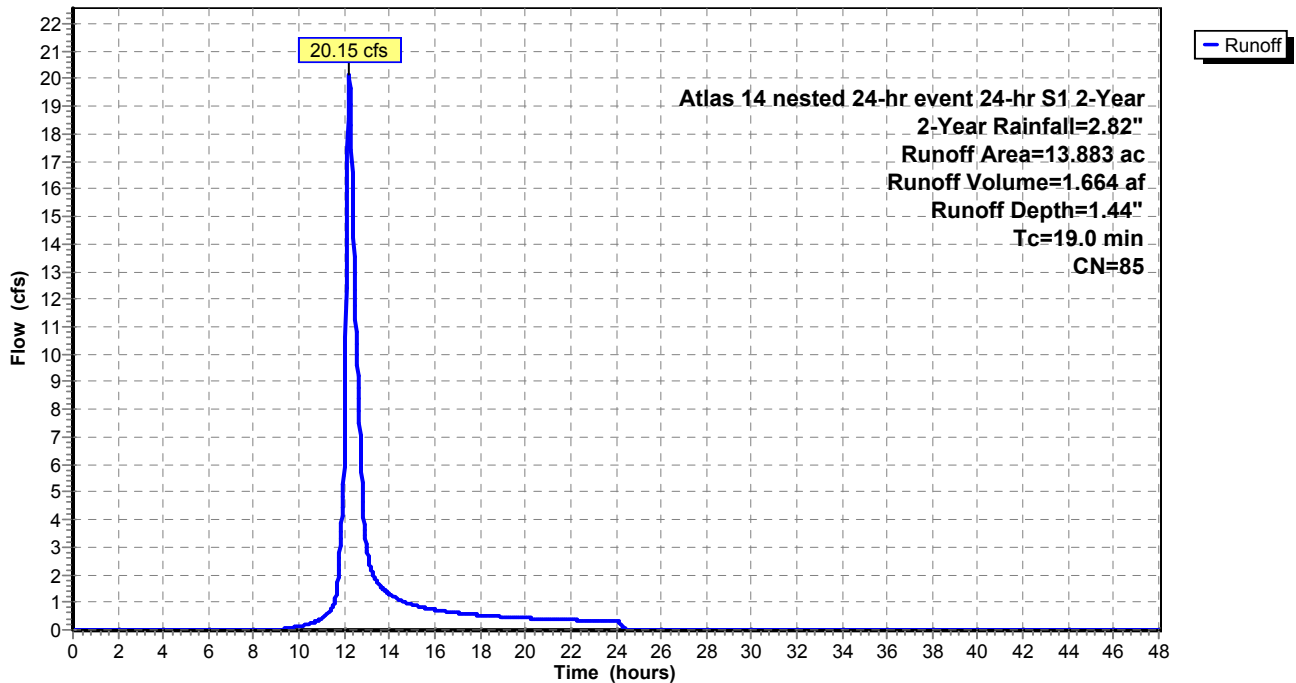
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 13.883	85	
13.883		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.0					Direct Entry,

Subcatchment 24S: Sub-basin 24

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 25S: Sub-basin 25

Runoff = 4.76 cfs @ 12.30 hrs, Volume= 0.469 af, Depth= 0.85"

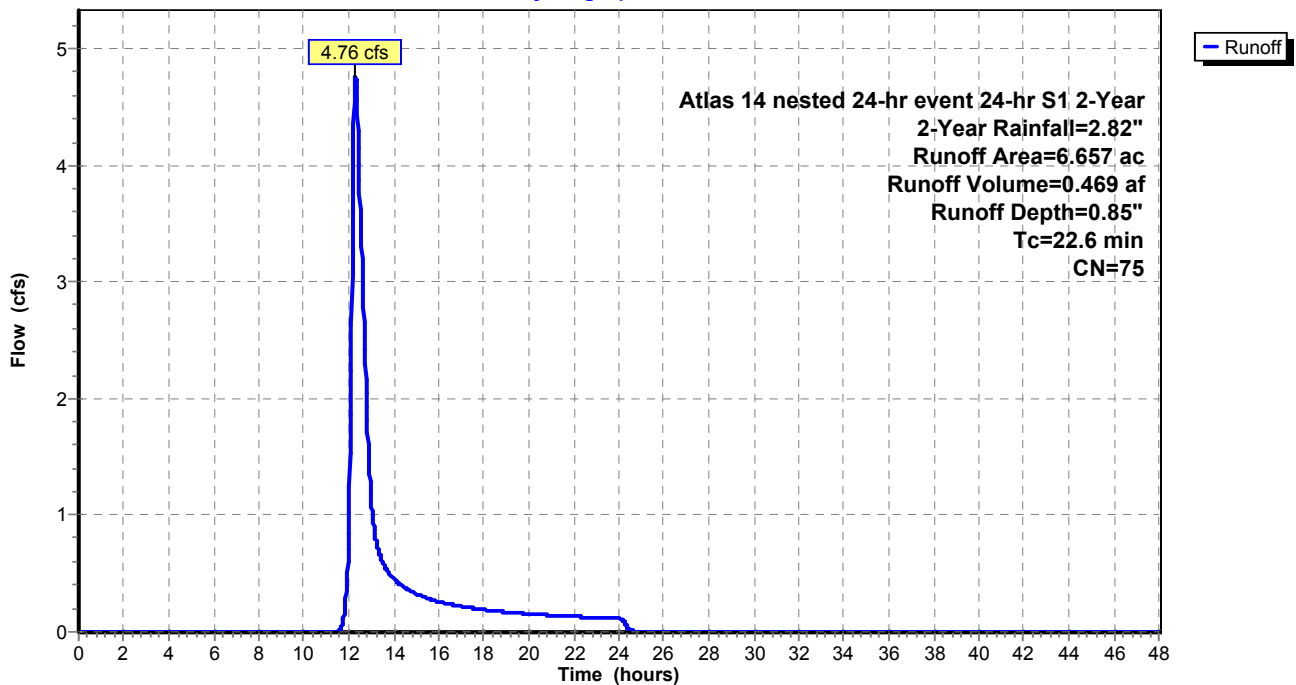
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 6.657	75	
6.657		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
22.6					Direct Entry,

Subcatchment 25S: Sub-basin 25

Hydrograph



Summary for Subcatchment 26S: Sub-basin 26

Runoff = 0.20 cfs @ 12.64 hrs, Volume= 0.032 af, Depth= 0.46"

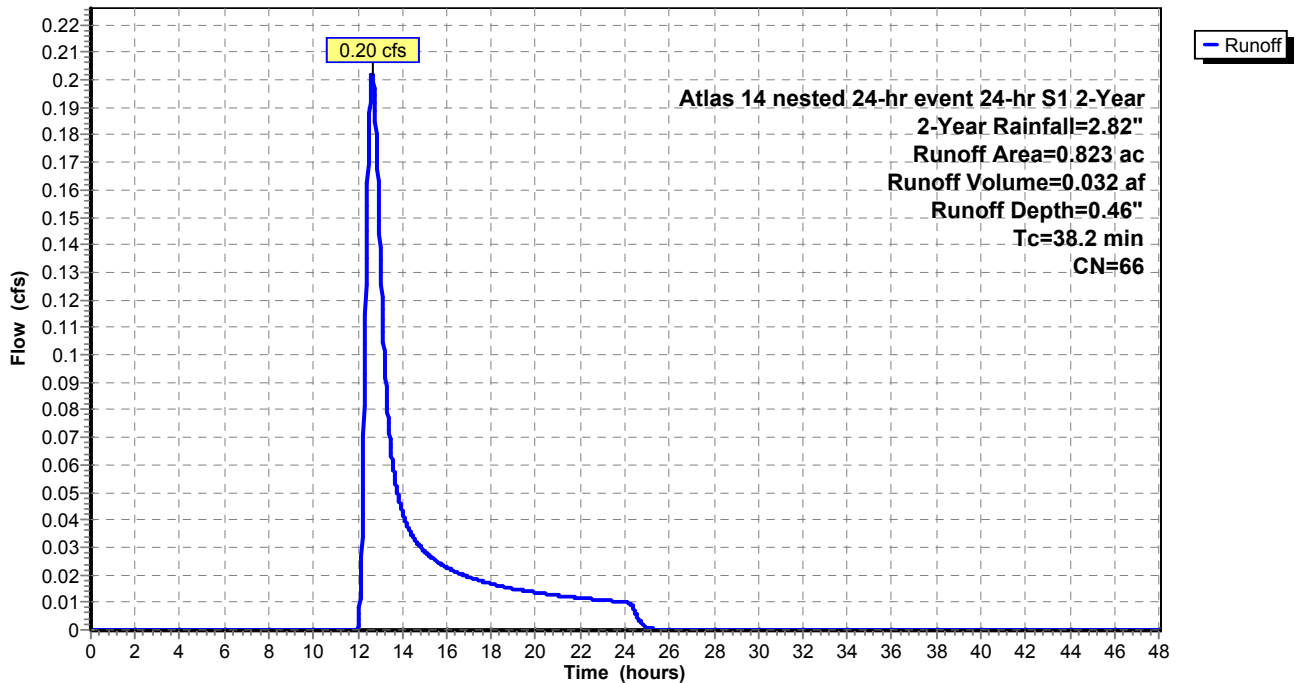
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 0.823	66	
0.823		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
38.2					Direct Entry,

Subcatchment 26S: Sub-basin 26

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 27S: Sub-basin 27

Runoff = 0.77 cfs @ 12.19 hrs, Volume= 0.078 af, Depth= 0.46"

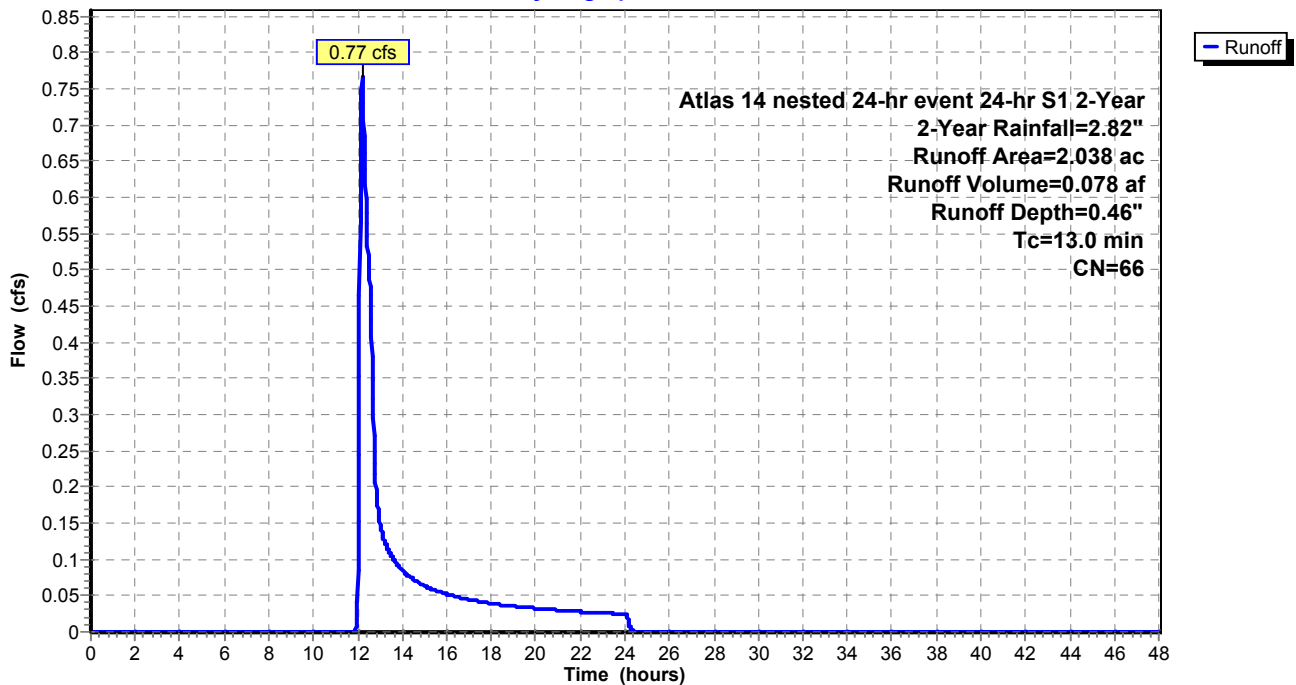
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 2.038	66	
2.038		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.0					Direct Entry,

Subcatchment 27S: Sub-basin 27

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 28S: Sub-basin 28

Runoff = 0.01 cfs @ 24.03 hrs, Volume= 0.003 af, Depth= 0.01"

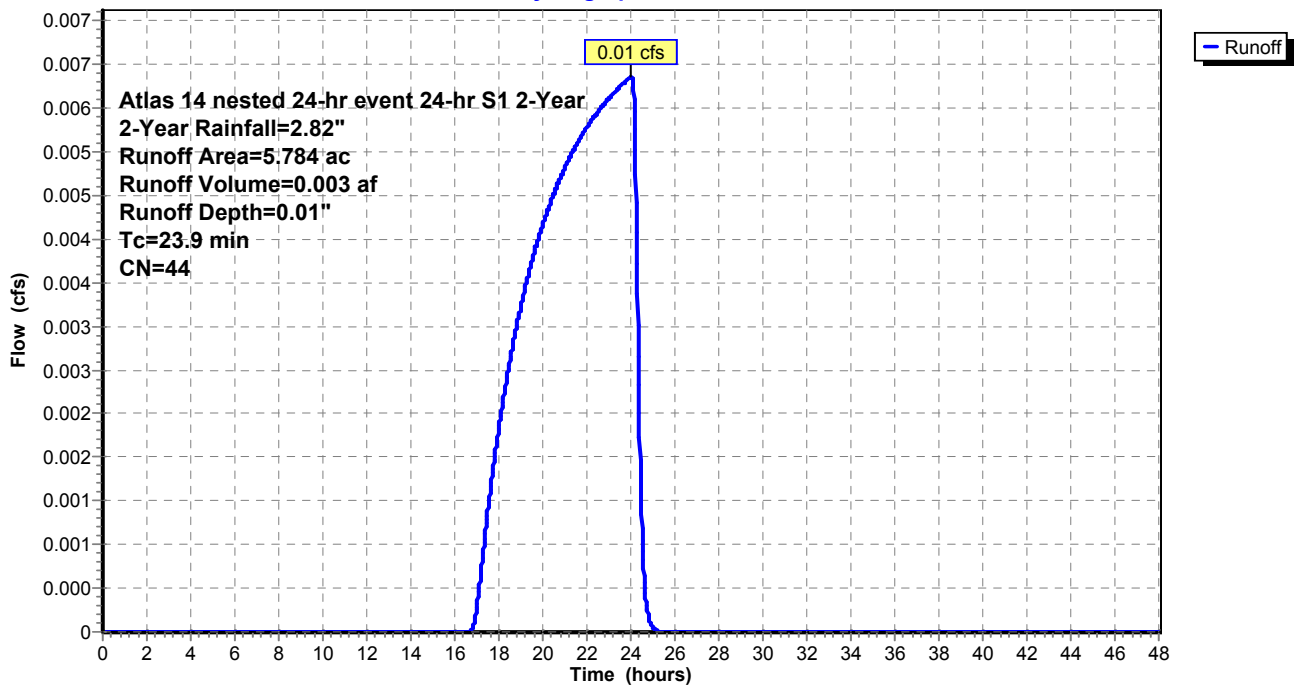
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 5.784	44	
5.784		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.9					Direct Entry,

Subcatchment 28S: Sub-basin 28

Hydrograph



Summary for Subcatchment 29S: Sub-basin 29

[45] Hint: Runoff=Zero

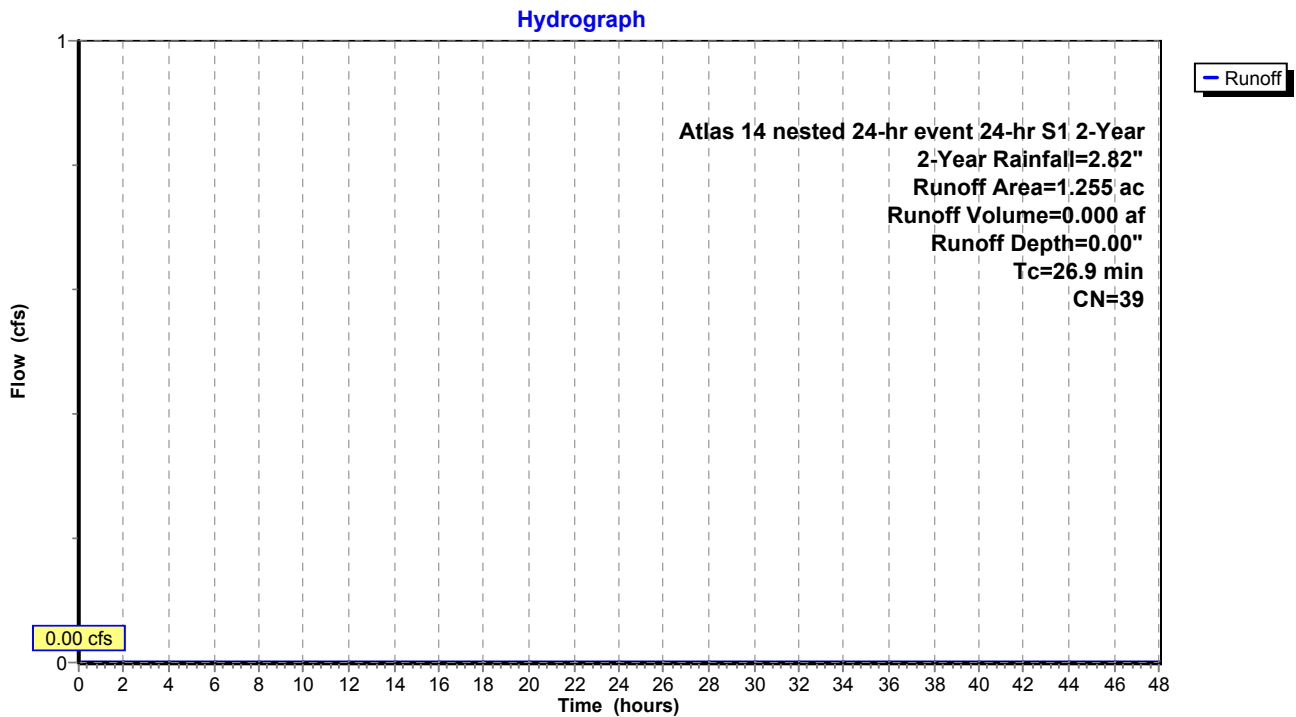
Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 1.255	39	
1.255		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
26.9					Direct Entry,

Subcatchment 29S: Sub-basin 29



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 30S: Sub-basin 30

Runoff = 0.01 cfs @ 24.23 hrs, Volume= 0.001 af, Depth= 0.00"

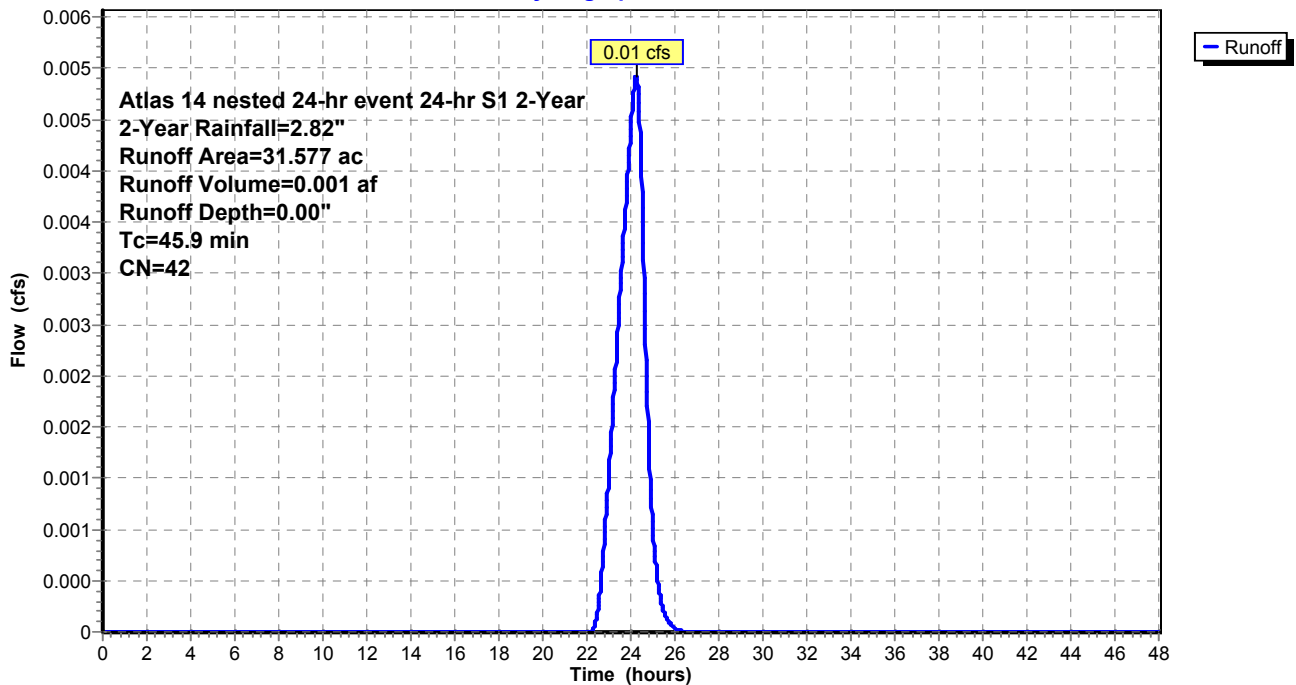
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 31.577	42	
31.577		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
45.9					Direct Entry,

Subcatchment 30S: Sub-basin 30

Hydrograph



Summary for Subcatchment 31S: Sub-basin 31

[45] Hint: Runoff=Zero

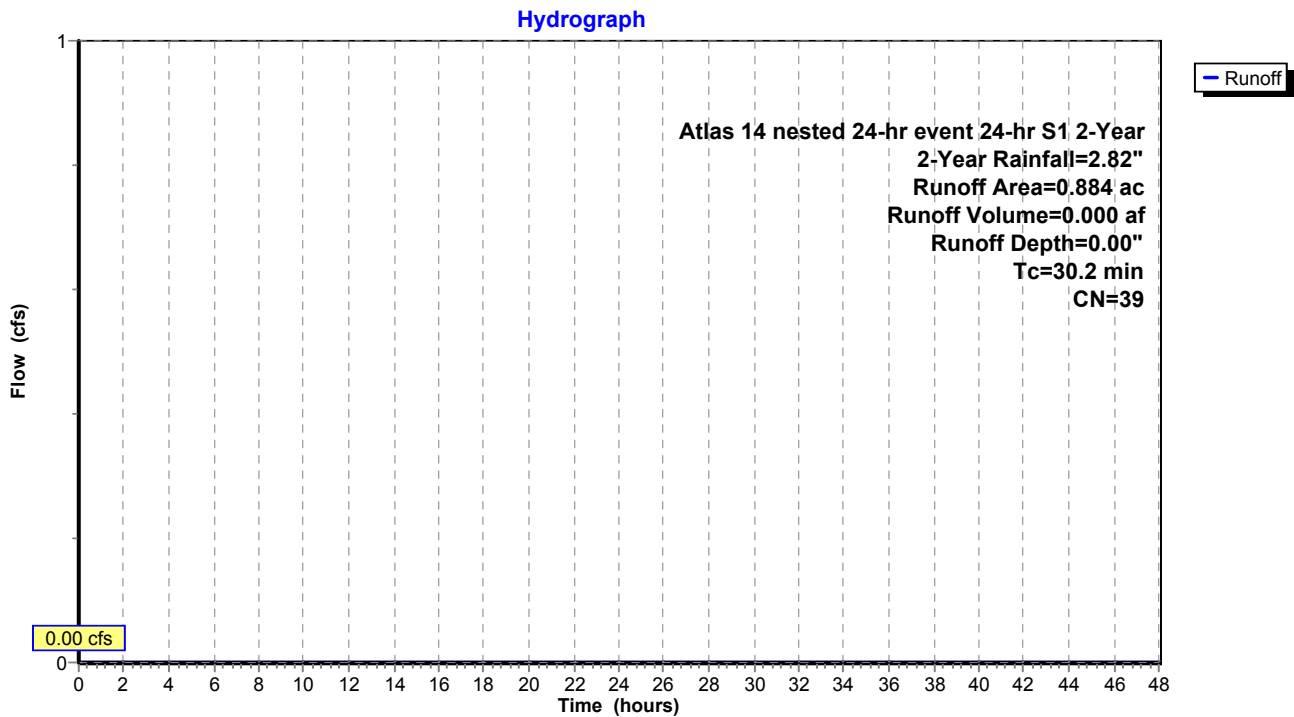
Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 0.884	39	
0.884		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.2					Direct Entry,

Subcatchment 31S: Sub-basin 31



Summary for Subcatchment 32S: Sub-basin 32

[45] Hint: Runoff=Zero

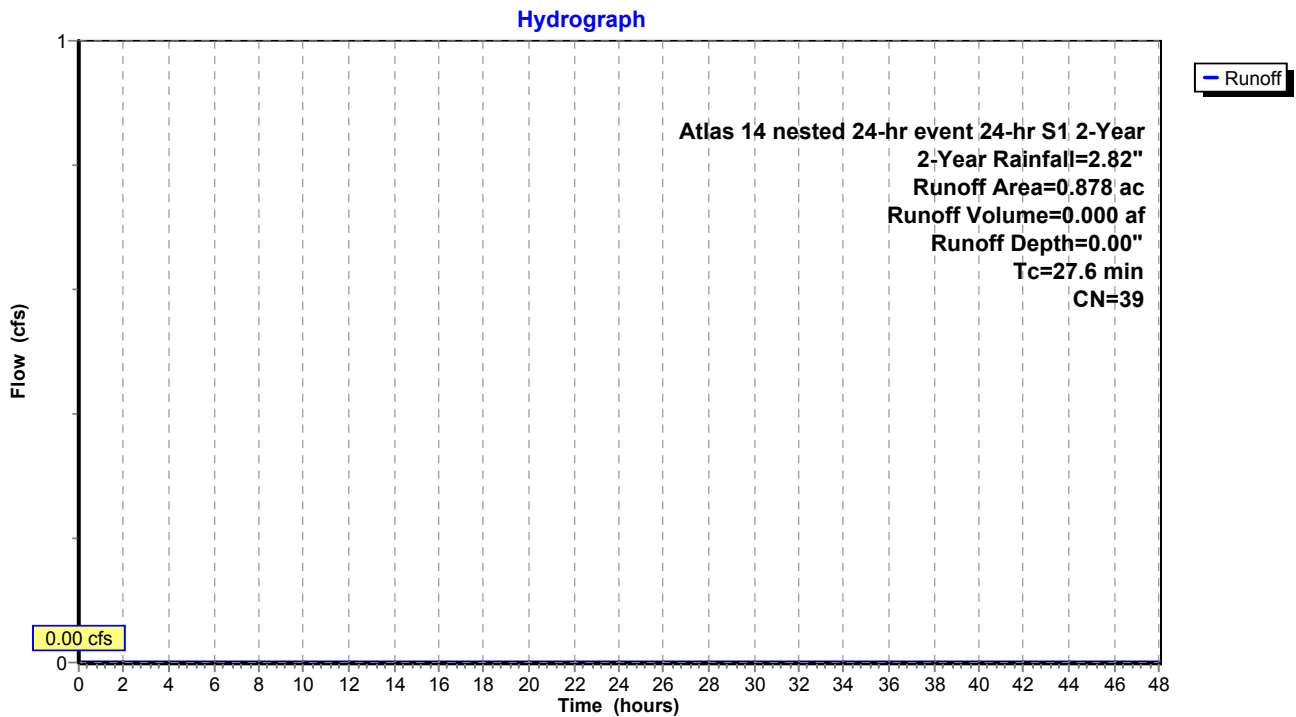
Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 0.878	39	
0.878		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.6					Direct Entry,

Subcatchment 32S: Sub-basin 32



Summary for Subcatchment 33S: Sub-basin 33

Runoff = 0.01 cfs @ 15.50 hrs, Volume= 0.010 af, Depth= 0.04"

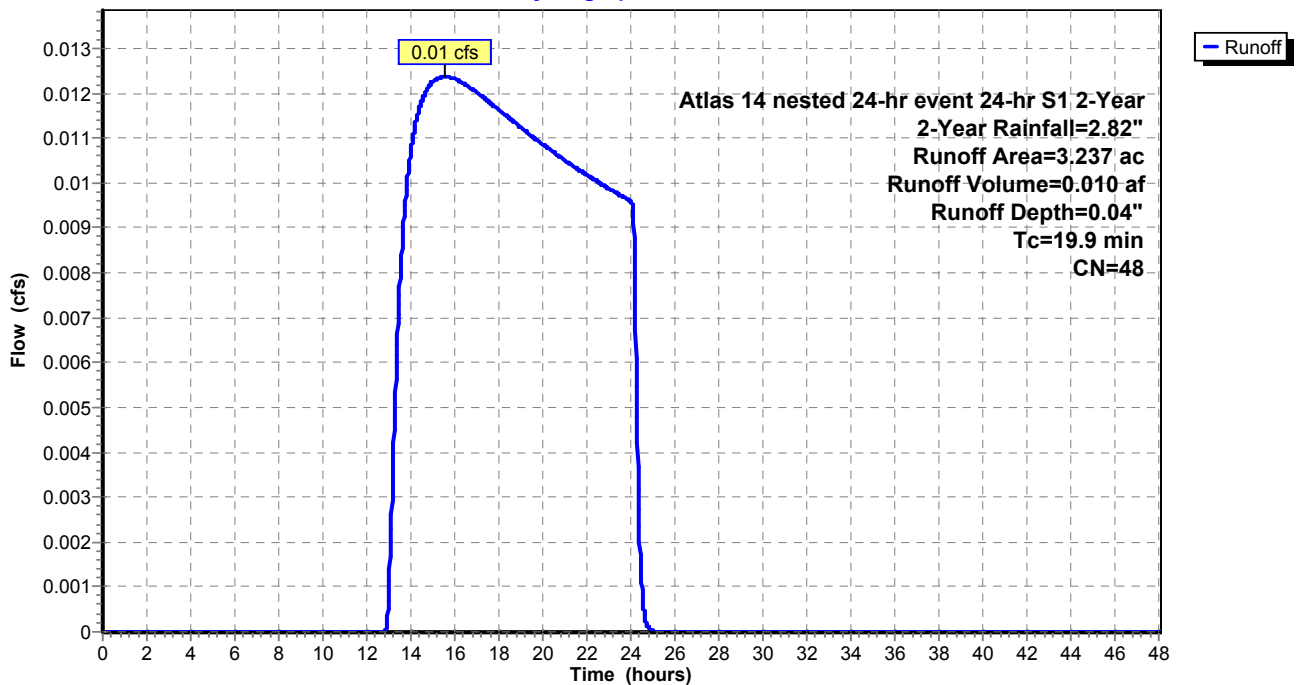
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 3.237	48	
3.237		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.9					Direct Entry,

Subcatchment 33S: Sub-basin 33

Hydrograph



Summary for Subcatchment 34S: Sub-basin 34

Runoff = 0.01 cfs @ 14.09 hrs, Volume= 0.005 af, Depth= 0.05"

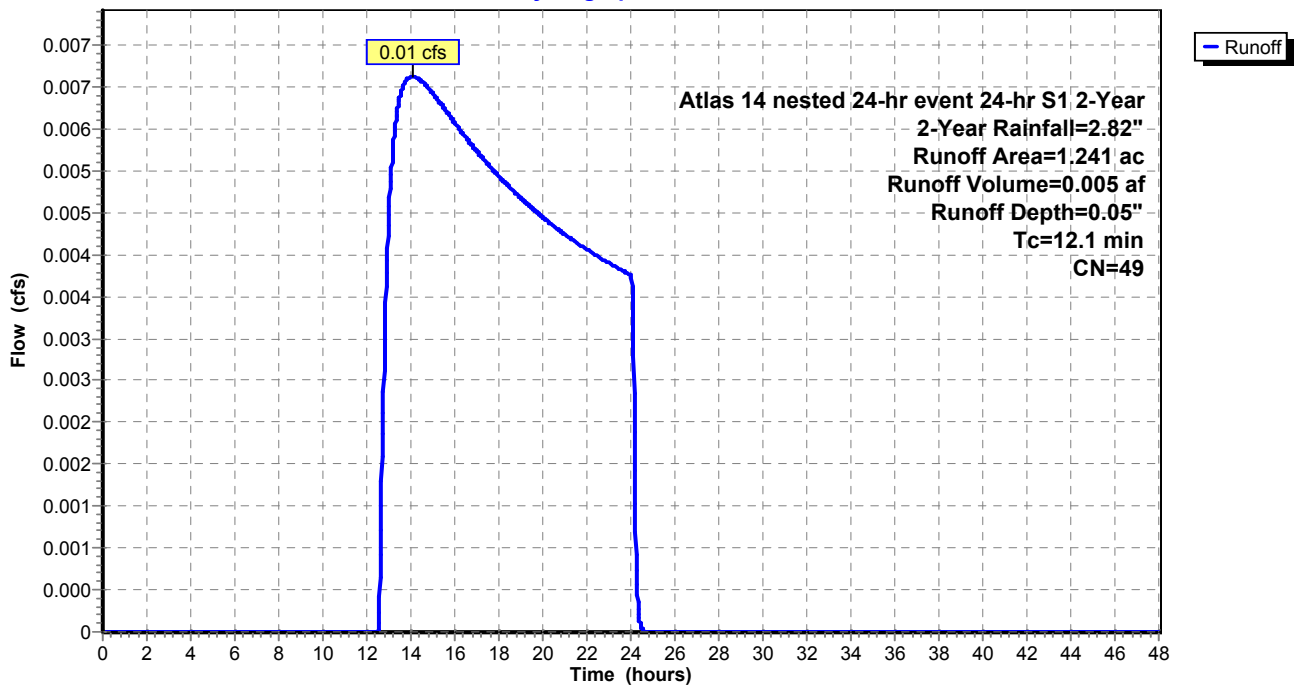
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 1.241	49	
1.241		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.1					Direct Entry,

Subcatchment 34S: Sub-basin 34

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 35S: Sub-basin 35

Runoff = 0.00 cfs @ 24.02 hrs, Volume= 0.001 af, Depth= 0.00"

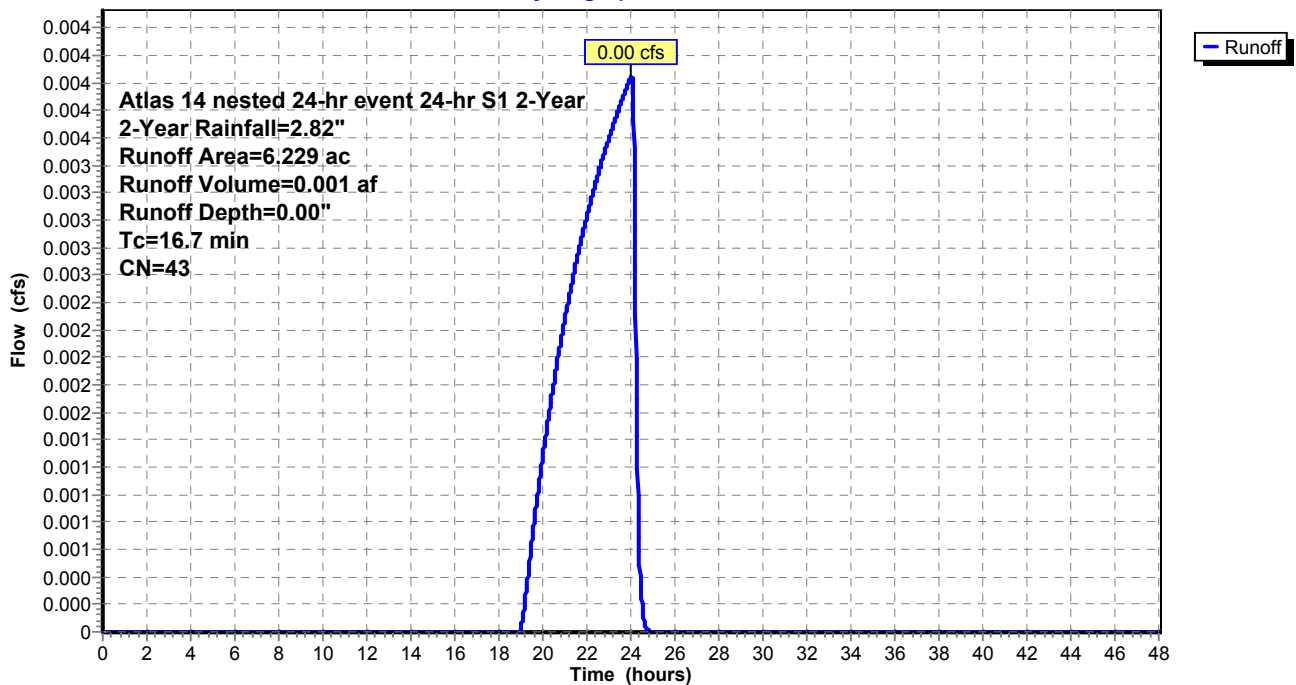
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 6.229	43	
6.229		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.7					Direct Entry,

Subcatchment 35S: Sub-basin 35

Hydrograph



Summary for Subcatchment 36S: Sub-basin 36

Runoff = 6.74 cfs @ 12.72 hrs, Volume= 0.989 af, Depth= 1.06"

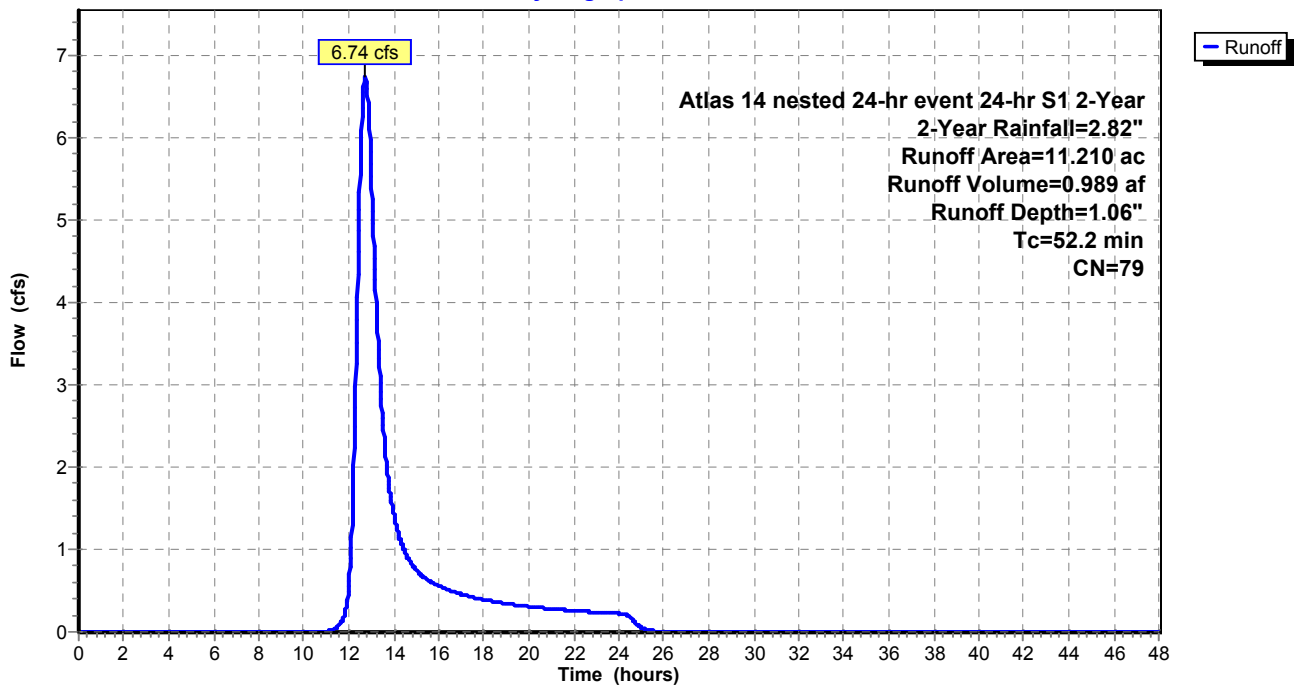
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 11.210	79	
11.210		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
52.2					Direct Entry,

Subcatchment 36S: Sub-basin 36

Hydrograph



Existing Conditions_Hydro Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

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Summary for Subcatchment 83S: County Road H Subbasin Redirected After Regrading

Runoff = 6.41 cfs @ 12.23 hrs, Volume= 0.547 af, Depth= 1.12"

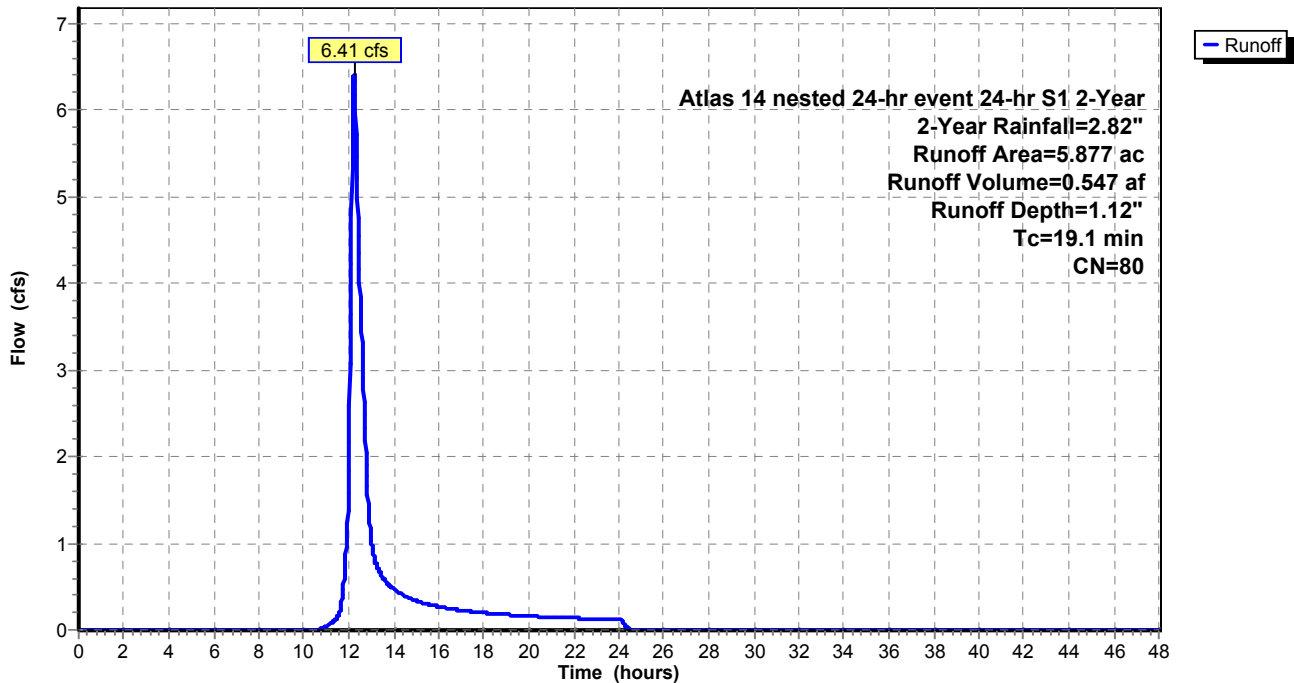
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 2-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 5.877	80	
5.877		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.1					Direct Entry,

Subcatchment 83S: County Road H Subbasin Redirected After Regrading

Hydrograph



Summary for Reach 37R: Outfall of SB 2, 3, 7

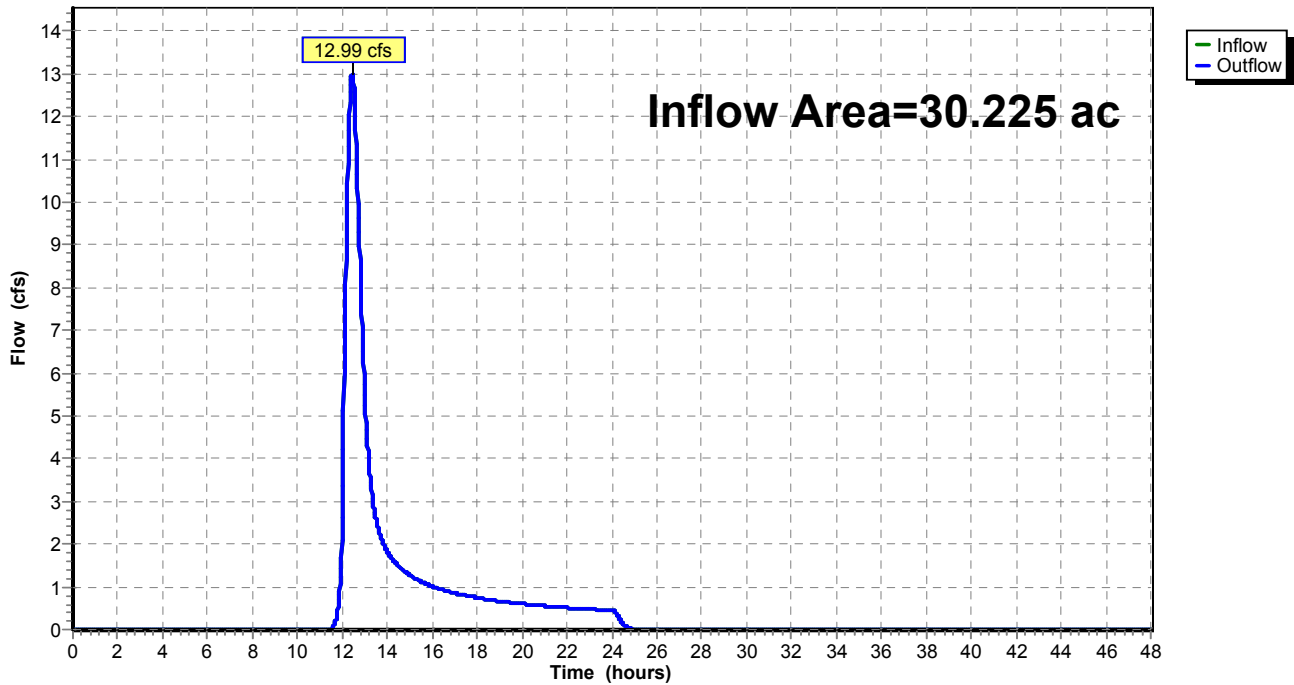
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 30.225 ac, 0.00% Impervious, Inflow Depth = 0.68" for 2-Year event
Inflow = 12.99 cfs @ 12.44 hrs, Volume= 1.708 af
Outflow = 12.99 cfs @ 12.44 hrs, Volume= 1.708 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 37R: Outfall of SB 2, 3, 7

Hydrograph



Summary for Reach 39R: Outfall of SB 1, 4, 5, 6, 9, 10, 11, 36

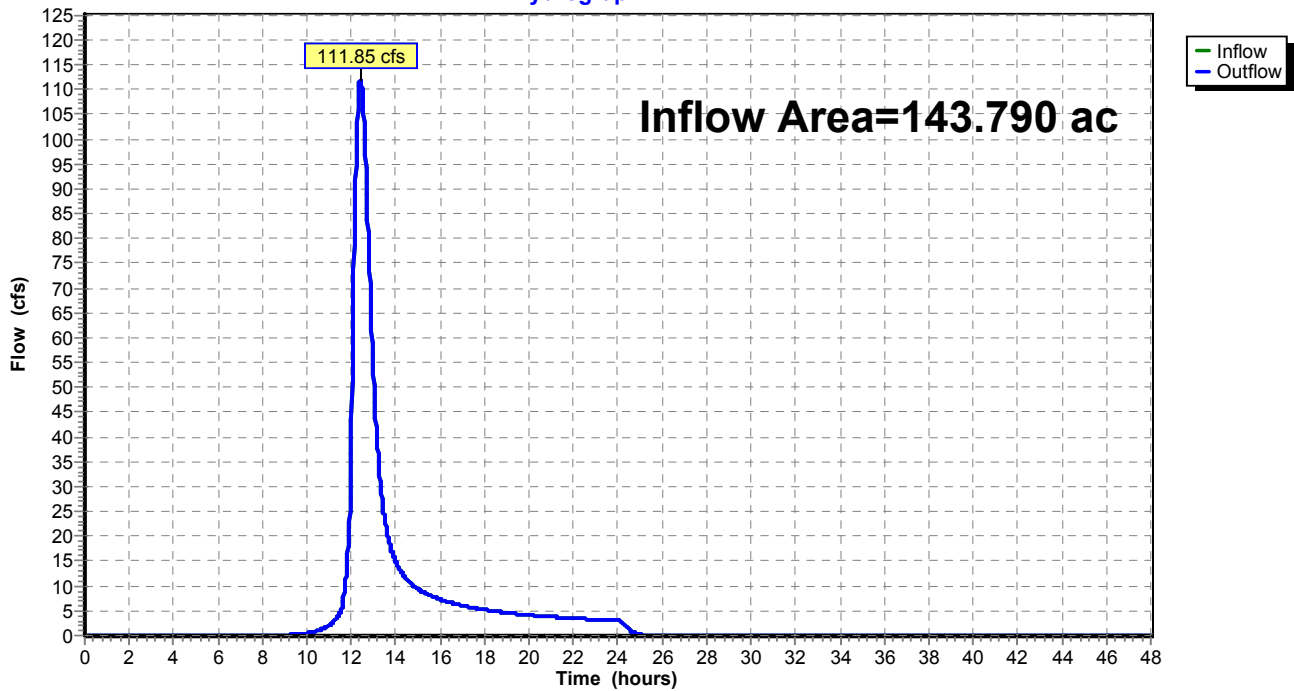
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 143.790 ac, 0.00% Impervious, Inflow Depth = 1.25" for 2-Year event
Inflow = 111.85 cfs @ 12.41 hrs, Volume= 14.937 af
Outflow = 111.85 cfs @ 12.41 hrs, Volume= 14.937 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 39R: Outfall of SB 1, 4, 5, 6, 9, 10, 11, 36

Hydrograph



Summary for Reach 40R: 60 in SB 4

[52] Hint: Inlet/Outlet conditions not evaluated

[65] Warning: Inlet elevation not specified

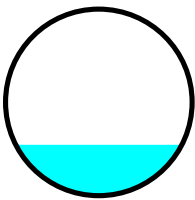
[88] Warning: Qout>Qin may require Finer Routing>1

Inflow Area = 143.790 ac, 0.00% Impervious, Inflow Depth = 1.25" for 2-Year event
Inflow = 111.84 cfs @ 12.40 hrs, Volume= 14.937 af
Outflow = 111.85 cfs @ 12.41 hrs, Volume= 14.937 af, Atten= 0%, Lag= 0.4 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
Reference Flow= 83.88 cfs Estimated Depth= 1.43' Velocity= 18.18 fps
m= 1.416, c= 25.74 fps, dt= 1.2 min, dx= 718.0' / 1 = 718.0', K= 0.5 min, X= 0.479
Max. Velocity= 26.20 fps, Min. Travel Time= 0.5 min
Avg. Velocity = 25.74 fps, Avg. Travel Time= 0.5 min

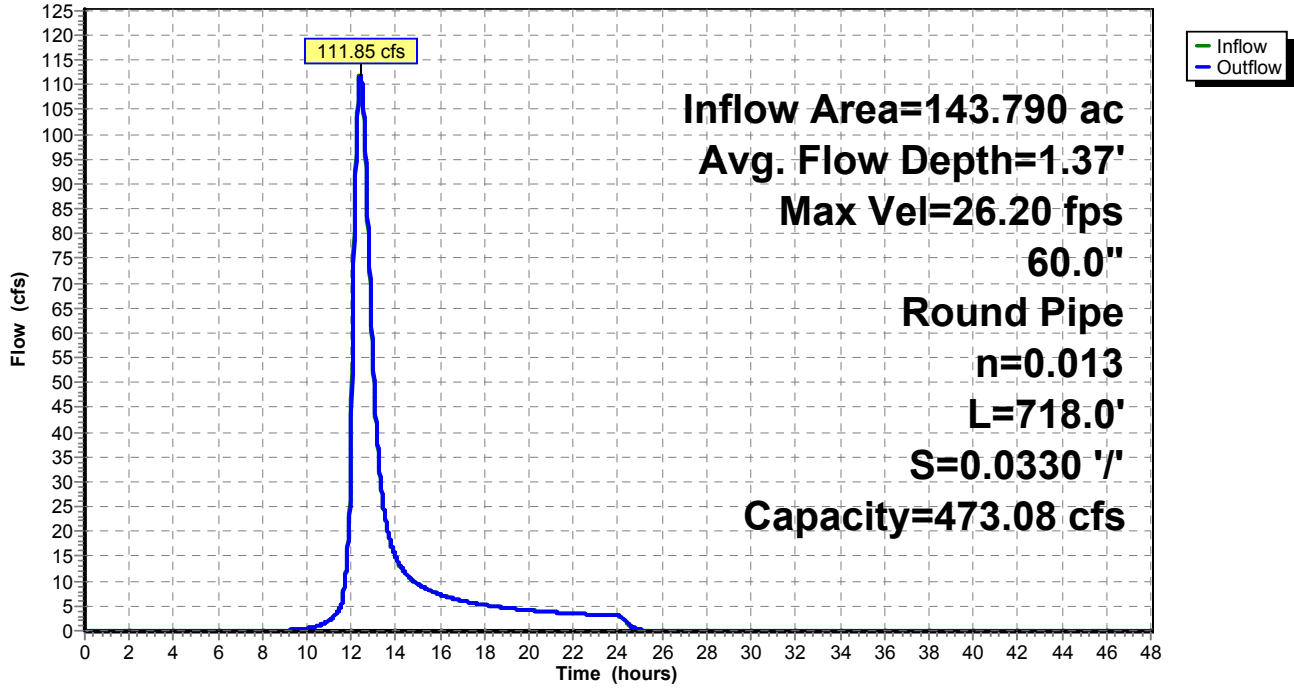
Peak Storage= 3,119 cf @ 12.40 hrs
Average Depth at Peak Storage= 1.37'
Bank-Full Depth= 5.00' Flow Area= 19.6 sf, Capacity= 473.08 cfs

60.0" Round Pipe
n= 0.013
Length= 718.0' Slope= 0.0330 '/'
Inlet Invert= 0.00', Outlet Invert= -23.69'



Reach 40R: 60 in SB 4

Hydrograph



Summary for Reach 41R: Channel in SB 9, 10

[65] Warning: Inlet elevation not specified

Inflow Area = 9.296 ac, 0.00% Impervious, Inflow Depth = 1.24" for 2-Year event
Inflow = 13.79 cfs @ 12.13 hrs, Volume= 0.960 af
Outflow = 12.78 cfs @ 12.39 hrs, Volume= 0.960 af, Atten= 7%, Lag= 15.3 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
Reference Flow= 10.34 cfs Estimated Depth= 0.49' Velocity= 1.18 fps
m= 1.584, c= 1.88 fps, dt= 1.2 min, dx= 1,660.0' / 12 = 138.3', K= 1.2 min, X= 0.269
Max. Velocity= 8.22 fps, Min. Travel Time= 3.4 min
Avg. Velocity = 1.96 fps, Avg. Travel Time= 14.1 min

Peak Storage= 10,181 cf @ 12.29 hrs
Average Depth at Peak Storage= 0.35'
Bank-Full Depth= 3.00' Flow Area= 84.0 sf, Capacity= 280.23 cfs

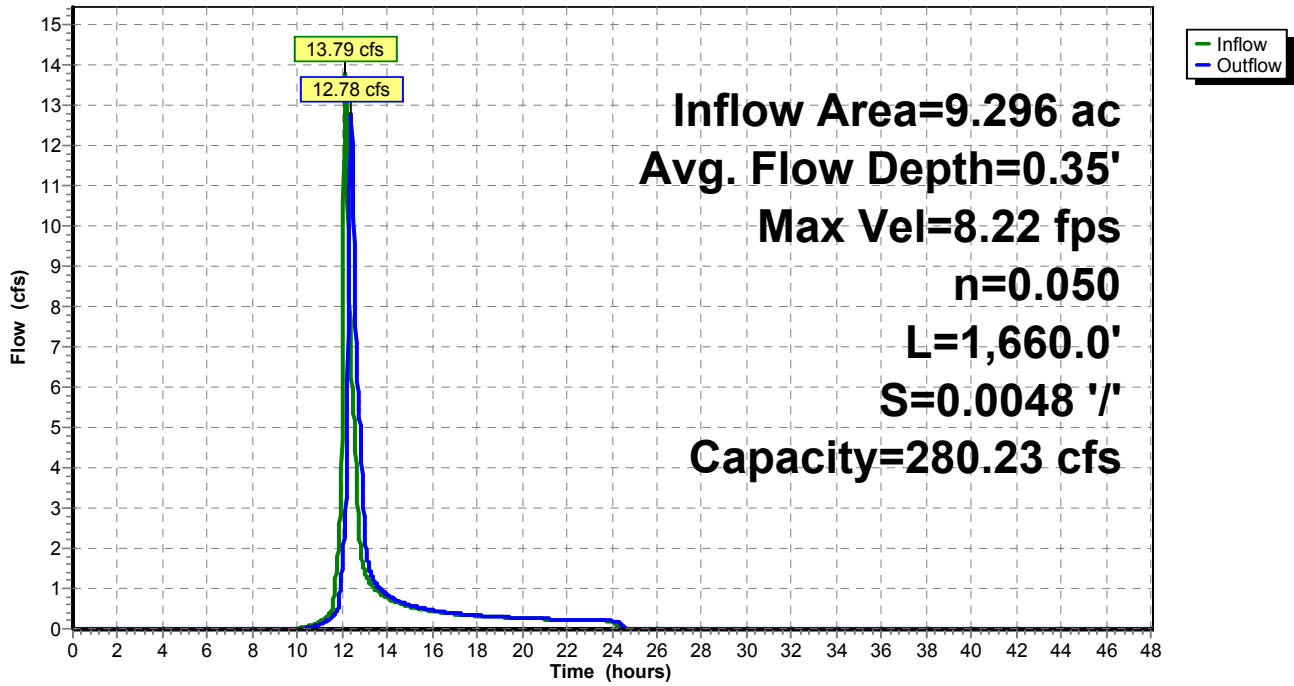
16.00' x 3.00' deep channel, n= 0.050
Side Slope Z-value= 4.0 '/' Top Width= 40.00'
Length= 1,660.0' Slope= 0.0048 '/'
Inlet Invert= 0.00', Outlet Invert= -7.97'



‡

Reach 41R: Channel in SB 9, 10

Hydrograph



Summary for Reach 46R: Channel SB1

[65] Warning: Inlet elevation not specified

Inflow Area = 15.328 ac, 0.00% Impervious, Inflow Depth = 1.58" for 2-Year event
Inflow = 26.50 cfs @ 12.18 hrs, Volume= 2.022 af
Outflow = 26.13 cfs @ 12.27 hrs, Volume= 2.022 af, Atten= 1%, Lag= 5.2 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
Reference Flow= 19.87 cfs Estimated Depth= 0.71' Velocity= 1.77 fps
m= 1.540, c= 2.73 fps, dt= 1.2 min, dx= 841.0' / 4 = 210.3', K= 1.3 min, X= 0.346
Max. Velocity= 5.16 fps, Min. Travel Time= 2.7 min
Avg. Velocity = 2.74 fps, Avg. Travel Time= 5.1 min

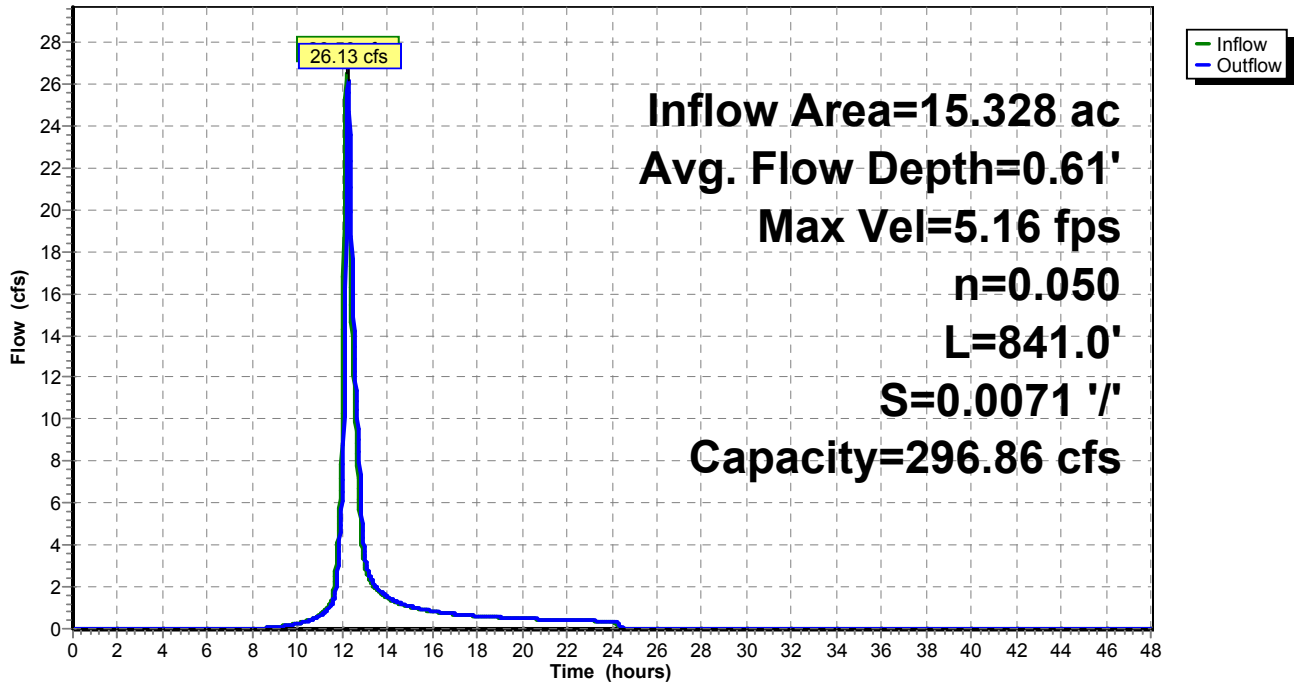
Peak Storage= 7,973 cf @ 12.23 hrs
Average Depth at Peak Storage= 0.61'
Bank-Full Depth= 3.00' Flow Area= 75.0 sf, Capacity= 296.86 cfs

13.00' x 3.00' deep channel, n= 0.050
Side Slope Z-value= 4.0 ' ' Top Width= 37.00'
Length= 841.0' Slope= 0.0071 ' '
Inlet Invert= 0.00', Outlet Invert= -5.97'



Reach 46R: Channel SB1

Hydrograph



Summary for Reach 48R: Outfall of SB 8, 13

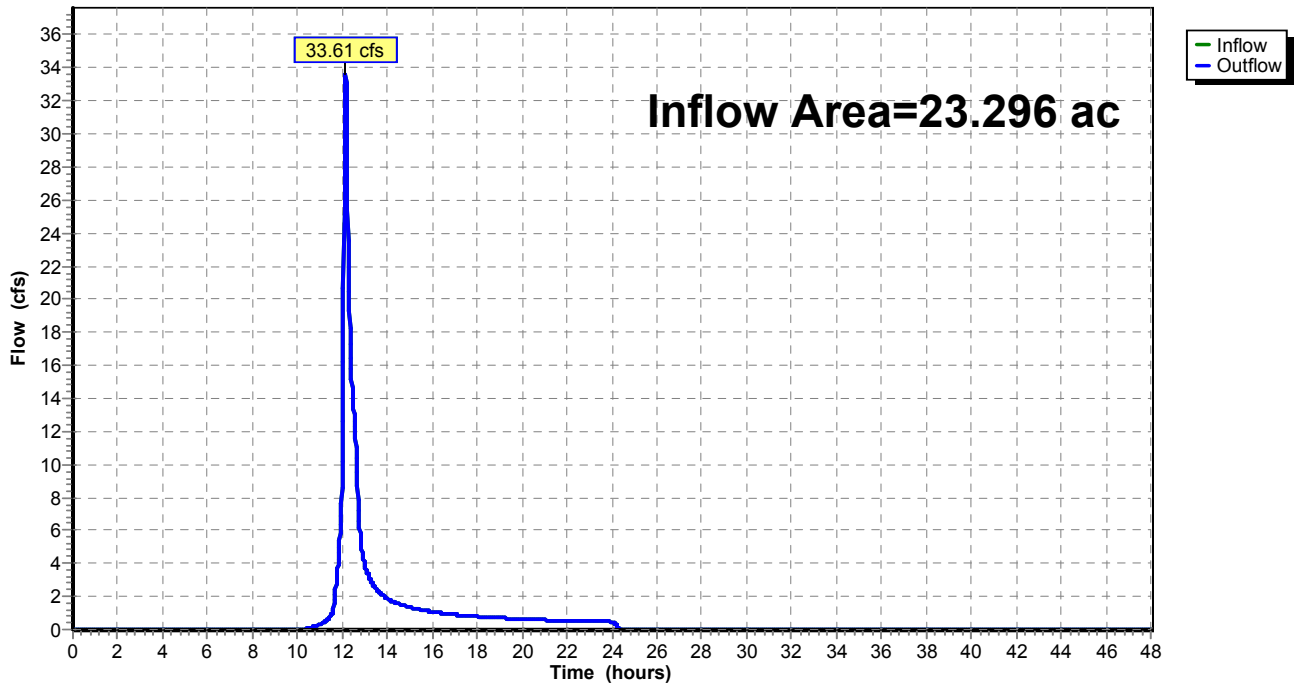
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 23.296 ac, 0.00% Impervious, Inflow Depth = 1.18" for 2-Year event
Inflow = 33.61 cfs @ 12.14 hrs, Volume= 2.296 af
Outflow = 33.61 cfs @ 12.14 hrs, Volume= 2.296 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 48R: Outfall of SB 8, 13

Hydrograph



Summary for Reach 49R: Channel SB8

[65] Warning: Inlet elevation not specified

Inflow Area = 21.017 ac, 0.00% Impervious, Inflow Depth = 1.18" for 2-Year event
Inflow = 33.39 cfs @ 12.09 hrs, Volume= 2.061 af
Outflow = 32.82 cfs @ 12.14 hrs, Volume= 2.061 af, Atten= 2%, Lag= 3.2 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
Reference Flow= 25.04 cfs Estimated Depth= 0.65' Velocity= 1.80 fps
m= 1.577, c= 2.83 fps, dt= 1.2 min, dx= 521.0' / 3 = 173.7', K= 1.0 min, X= 0.347
Max. Velocity= 5.56 fps, Min. Travel Time= 1.6 min
Avg. Velocity = 2.84 fps, Avg. Travel Time= 3.1 min

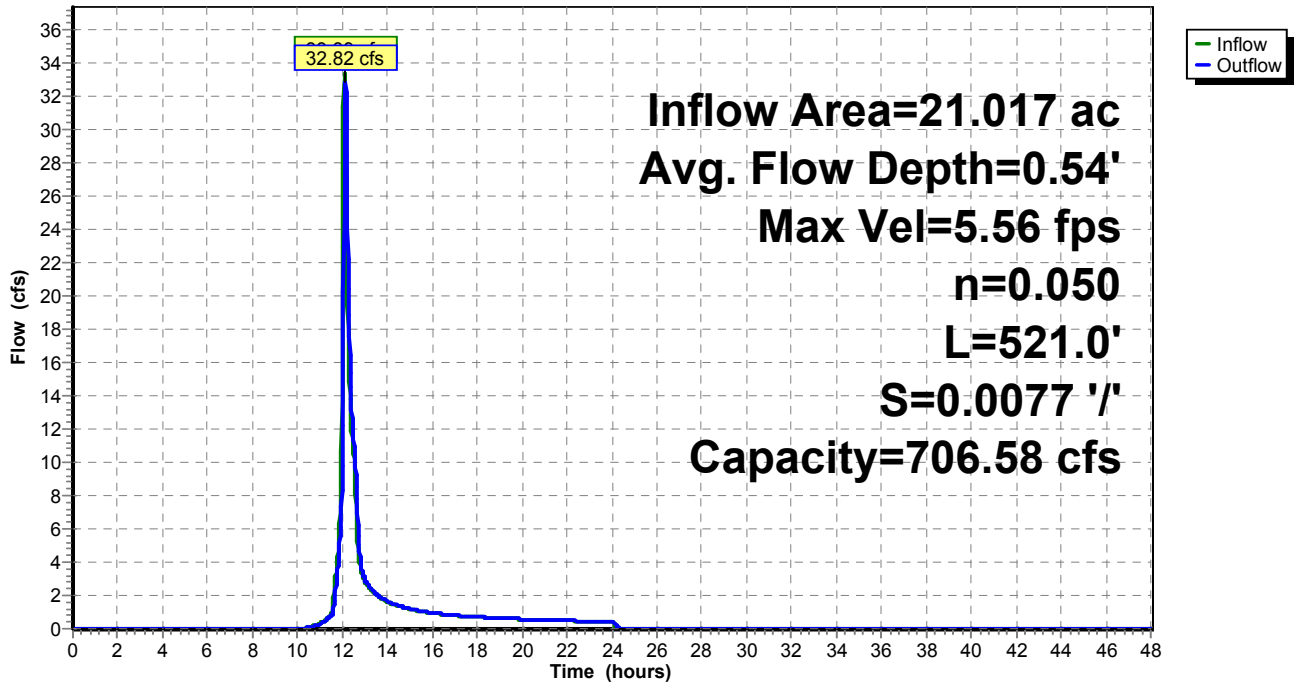
Peak Storage= 5,972 cf @ 12.12 hrs
Average Depth at Peak Storage= 0.54'
Bank-Full Depth= 4.00' Flow Area= 140.0 sf, Capacity= 706.58 cfs

19.00' x 4.00' deep channel, n= 0.050
Side Slope Z-value= 4.0 ' ' Top Width= 51.00'
Length= 521.0' Slope= 0.0077 ' '
Inlet Invert= 0.00', Outlet Invert= -4.01'



Reach 49R: Channel SB8

Hydrograph



Summary for Reach 50R: Outfall of SB 12

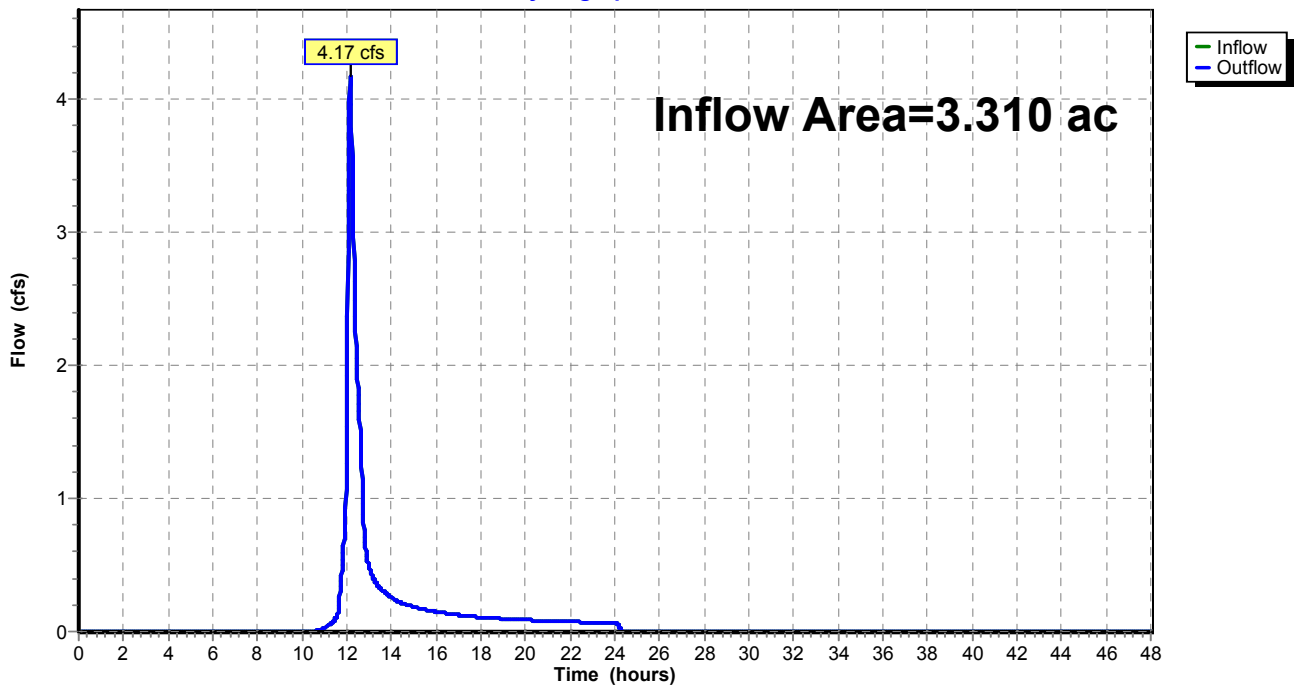
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 3.310 ac, 0.00% Impervious, Inflow Depth = 1.12" for 2-Year event
Inflow = 4.17 cfs @ 12.16 hrs, Volume= 0.308 af
Outflow = 4.17 cfs @ 12.16 hrs, Volume= 0.308 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 50R: Outfall of SB 12

Hydrograph



Summary for Reach 51R: Outfall of SB 14

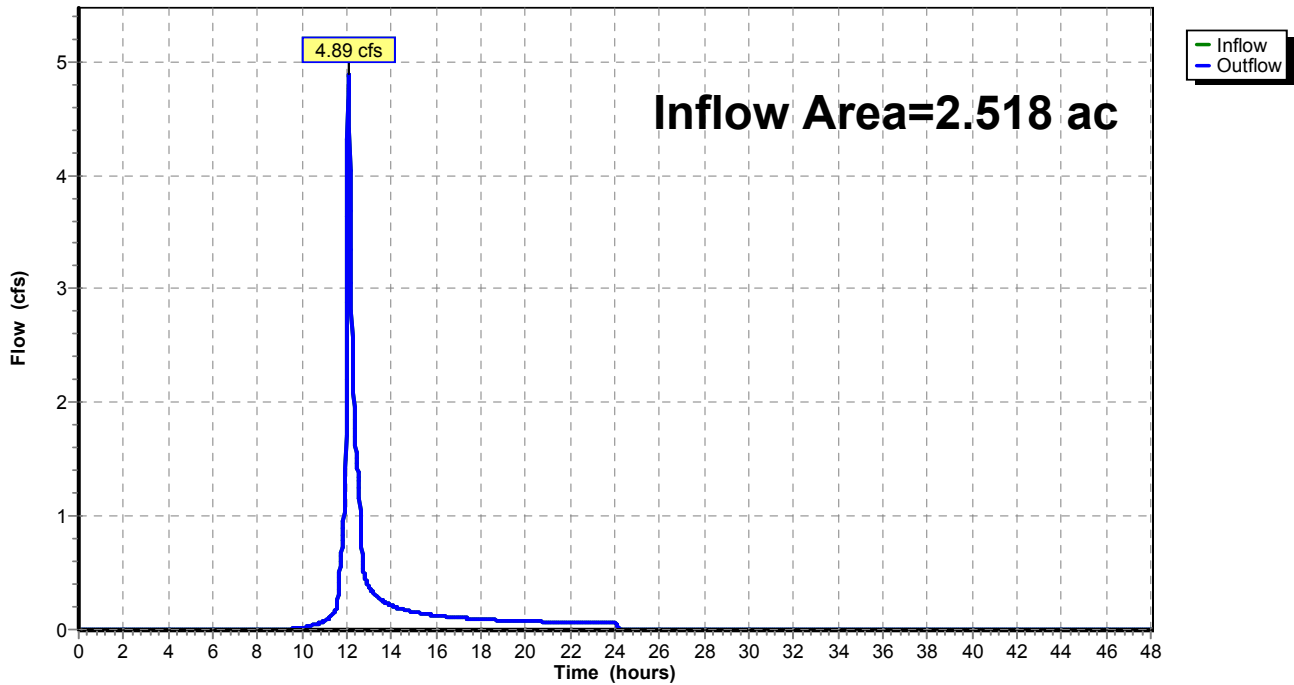
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 2.518 ac, 0.00% Impervious, Inflow Depth = 1.37" for 2-Year event
Inflow = 4.89 cfs @ 12.08 hrs, Volume= 0.287 af
Outflow = 4.89 cfs @ 12.08 hrs, Volume= 0.287 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 51R: Outfall of SB 14

Hydrograph



Summary for Reach 52R: Outfall of SB 17

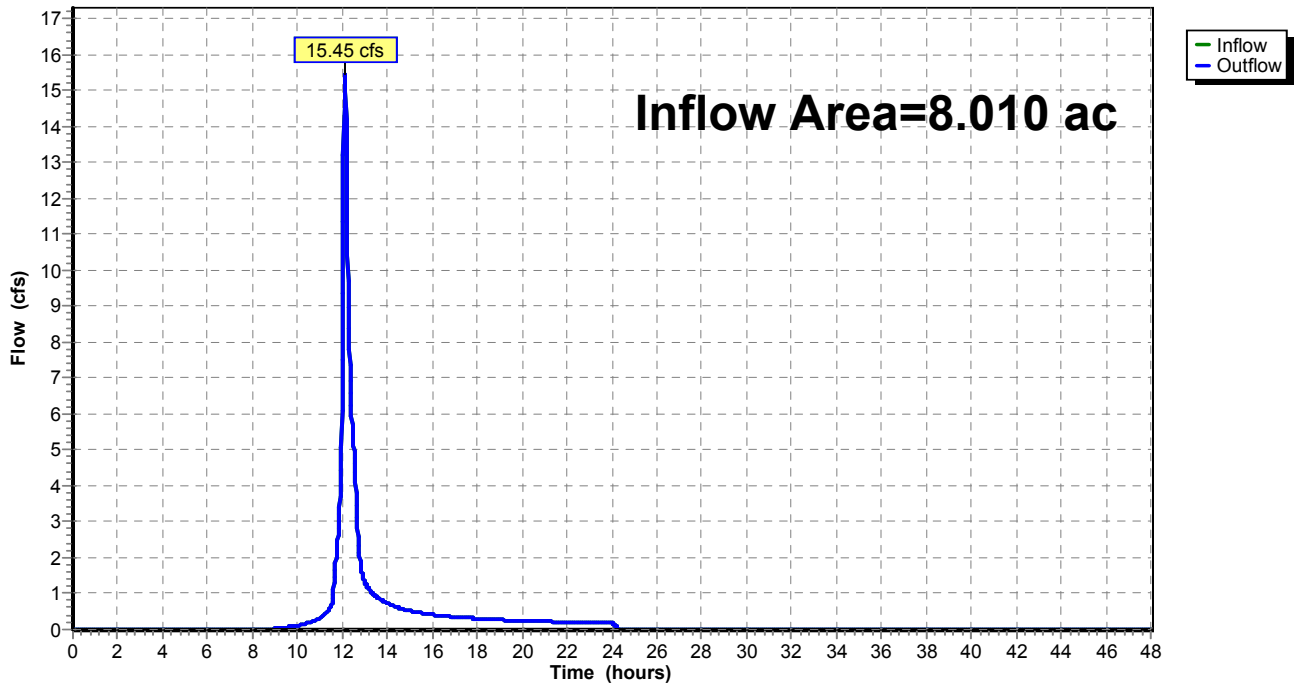
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 8.010 ac, 0.00% Impervious, Inflow Depth = 1.51" for 2-Year event
Inflow = 15.45 cfs @ 12.11 hrs, Volume= 1.008 af
Outflow = 15.45 cfs @ 12.11 hrs, Volume= 1.008 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 52R: Outfall of SB 17

Hydrograph



Summary for Reach 53R: Outfall of SB 18

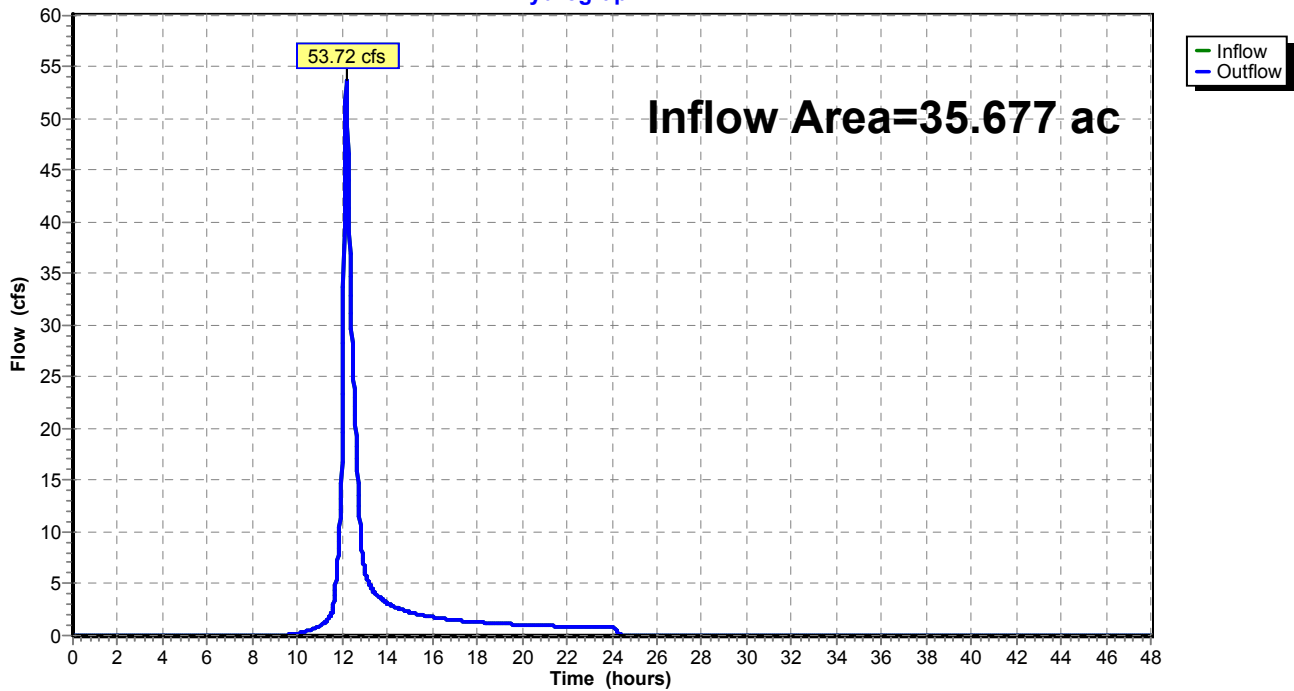
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 35.677 ac, 0.00% Impervious, Inflow Depth = 1.37" for 2-Year event
Inflow = 53.72 cfs @ 12.18 hrs, Volume= 4.072 af
Outflow = 53.72 cfs @ 12.18 hrs, Volume= 4.072 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 53R: Outfall of SB 18

Hydrograph



Summary for Reach 54R: Outfall of SB 25

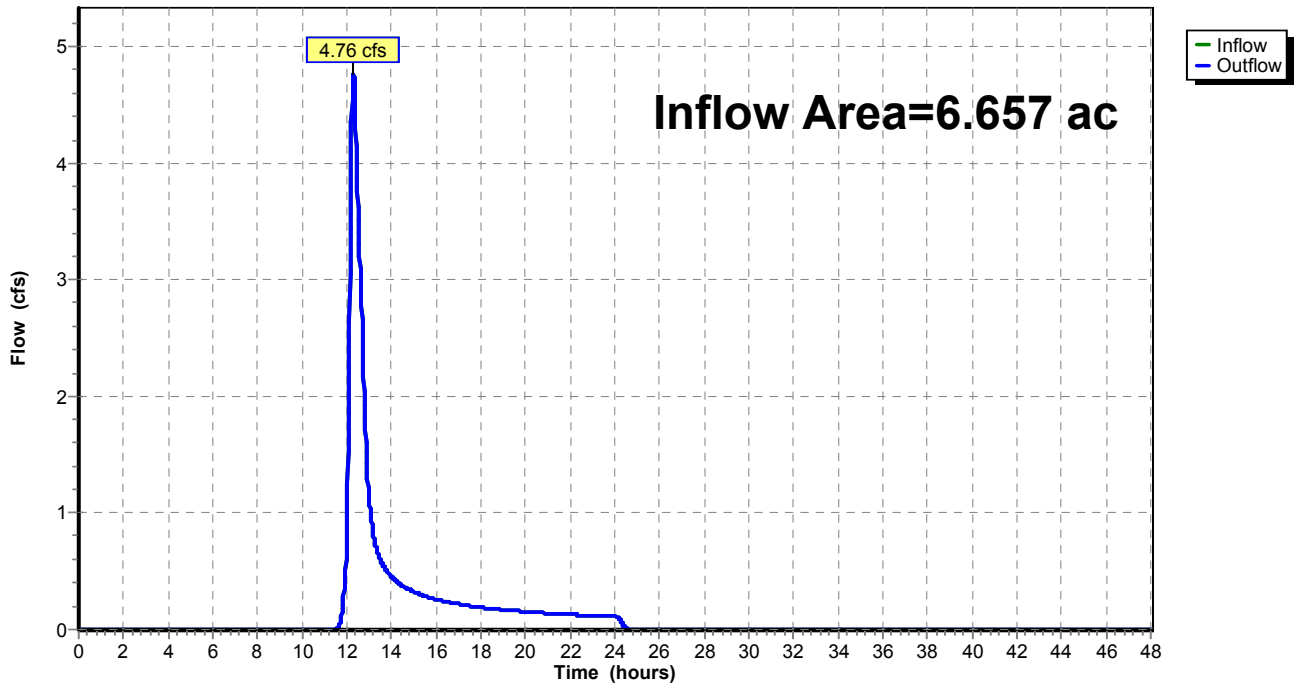
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 6.657 ac, 0.00% Impervious, Inflow Depth = 0.85" for 2-Year event
Inflow = 4.76 cfs @ 12.30 hrs, Volume= 0.469 af
Outflow = 4.76 cfs @ 12.30 hrs, Volume= 0.469 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 54R: Outfall of SB 25

Hydrograph



Summary for Reach 55R: Outfall of SB 26

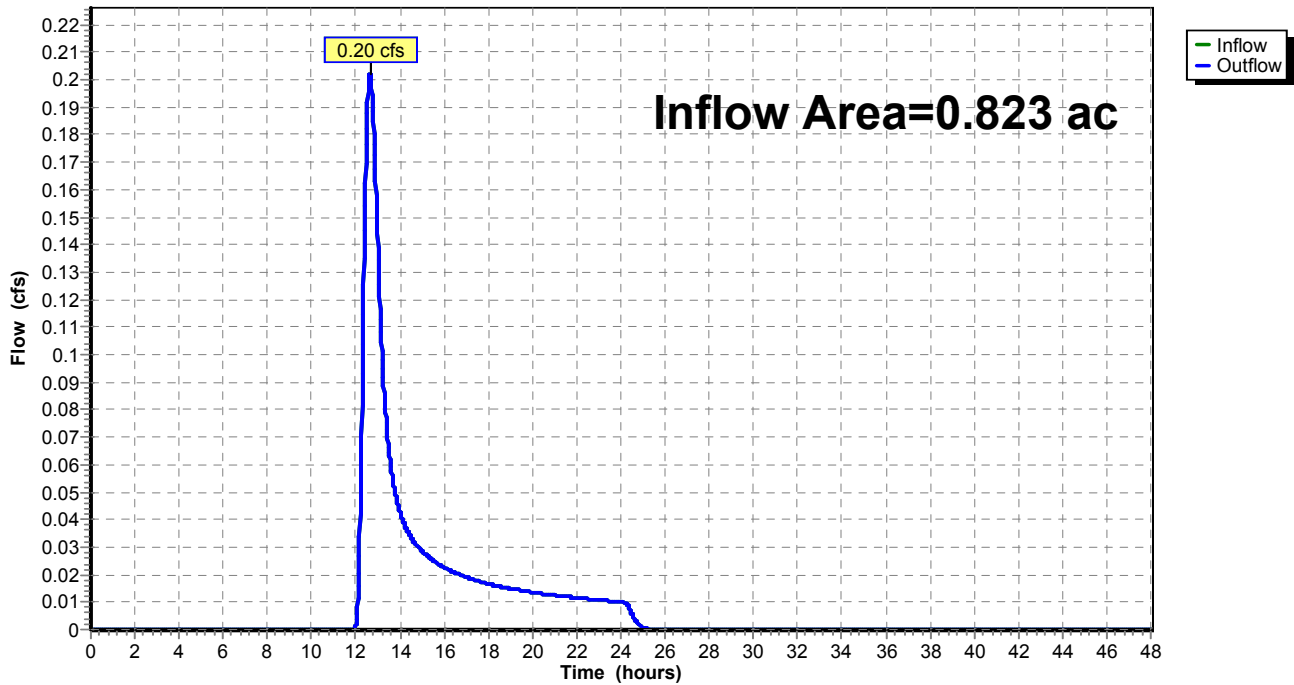
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.823 ac, 0.00% Impervious, Inflow Depth = 0.46" for 2-Year event
Inflow = 0.20 cfs @ 12.64 hrs, Volume= 0.032 af
Outflow = 0.20 cfs @ 12.64 hrs, Volume= 0.032 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 55R: Outfall of SB 26

Hydrograph



Summary for Reach 56R: Outfall of SB 23, 24

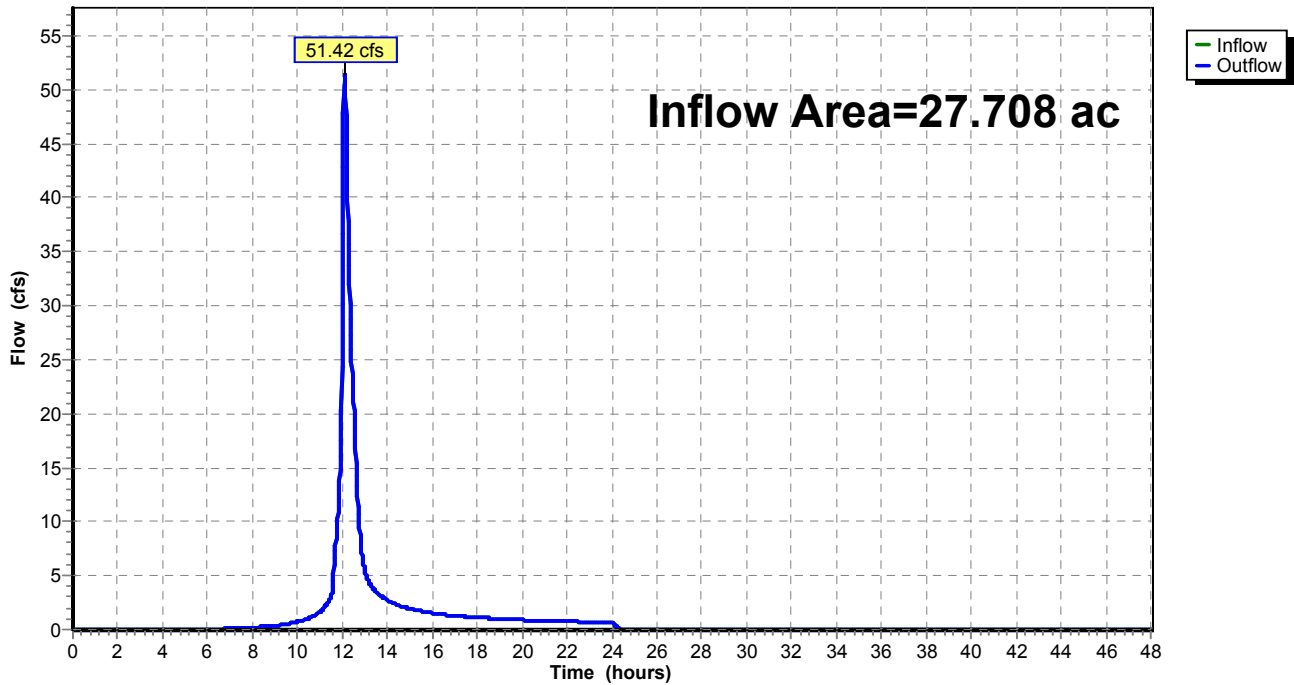
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 27.708 ac, 0.00% Impervious, Inflow Depth = 1.71" for 2-Year event
Inflow = 51.42 cfs @ 12.10 hrs, Volume= 3.958 af
Outflow = 51.42 cfs @ 12.10 hrs, Volume= 3.958 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 56R: Outfall of SB 23, 24

Hydrograph



Summary for Reach 59R: Outfall of SB 20, 22

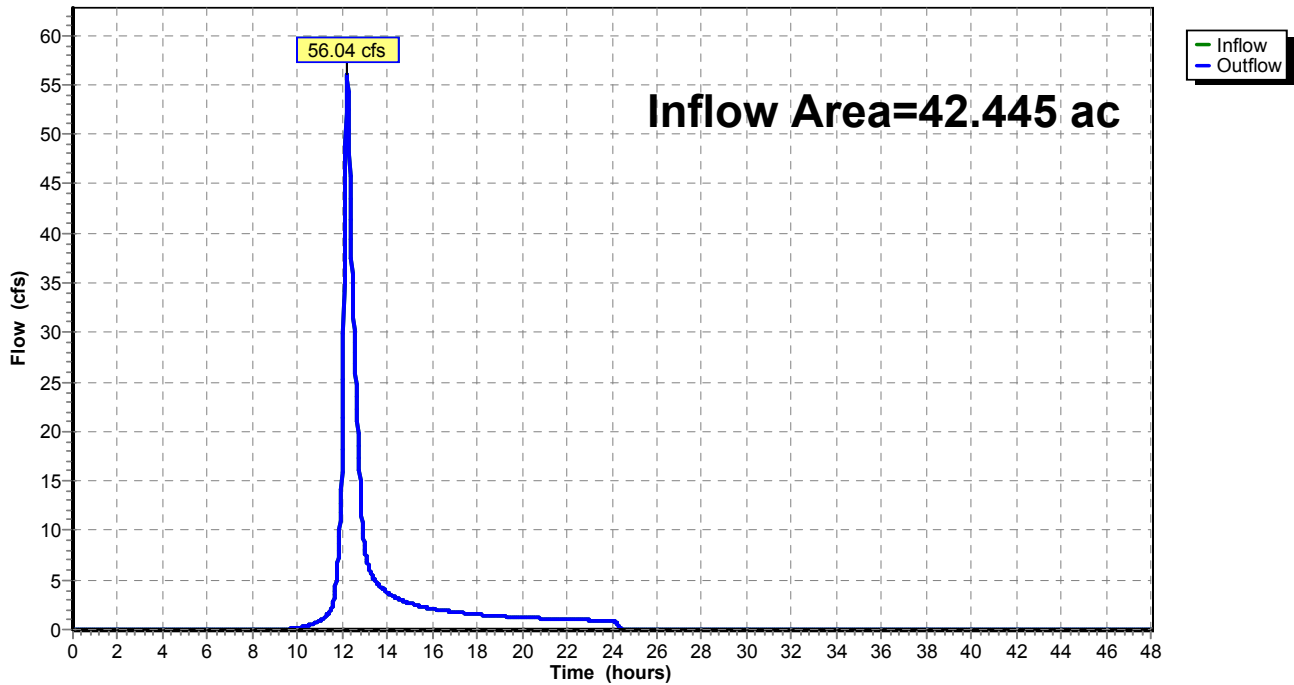
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 42.445 ac, 0.00% Impervious, Inflow Depth = 1.31" for 2-Year event
Inflow = 56.04 cfs @ 12.22 hrs, Volume= 4.646 af
Outflow = 56.04 cfs @ 12.22 hrs, Volume= 4.646 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 59R: Outfall of SB 20, 22

Hydrograph



Summary for Reach 61R: Outfall of SB 15, 16, 21

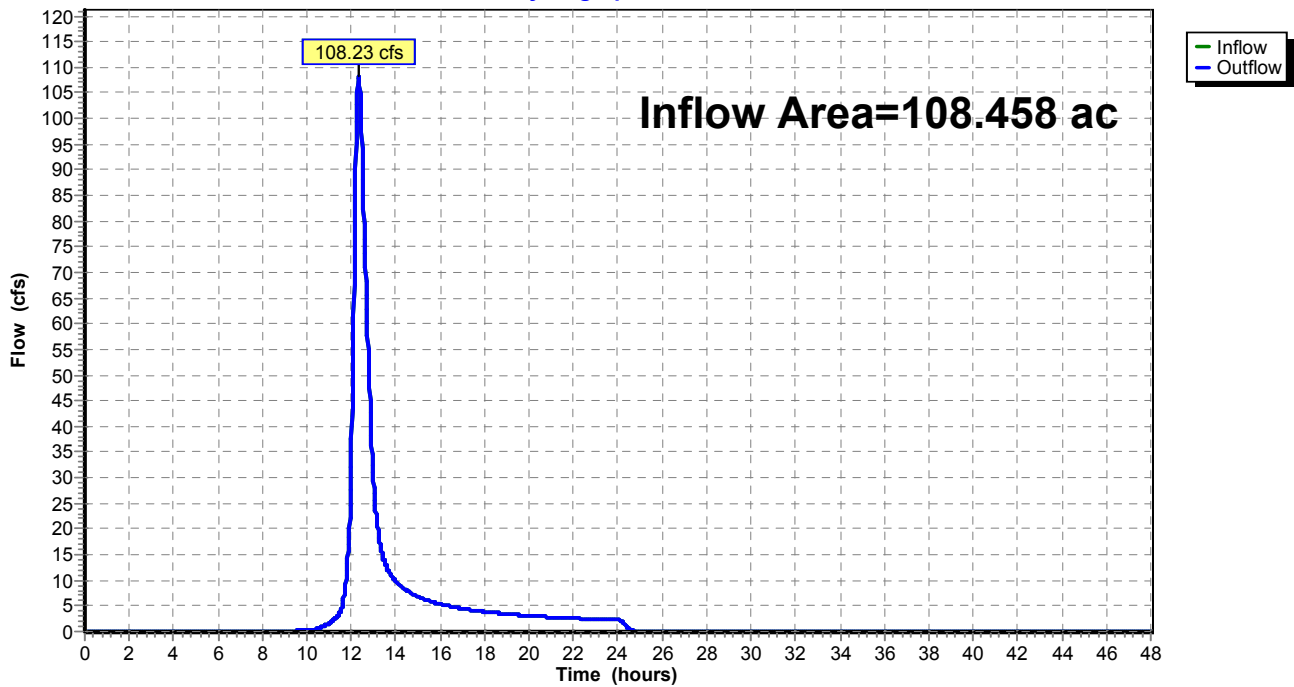
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 108.458 ac, 0.00% Impervious, Inflow Depth = 1.25" for 2-Year event
Inflow = 108.23 cfs @ 12.34 hrs, Volume= 11.311 af
Outflow = 108.23 cfs @ 12.34 hrs, Volume= 11.311 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 61R: Outfall of SB 15, 16, 21

Hydrograph



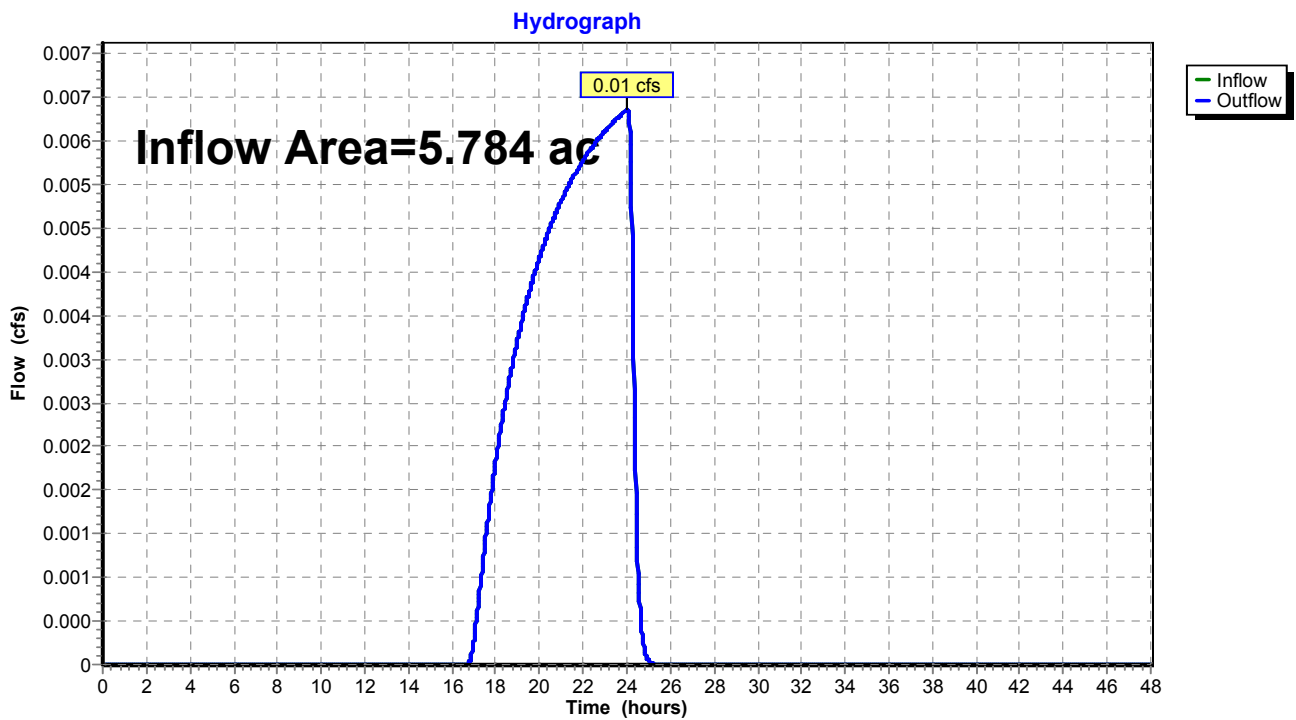
Summary for Reach 67R: Outfall of SB 28

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 5.784 ac, 0.00% Impervious, Inflow Depth = 0.01" for 2-Year event
Inflow = 0.01 cfs @ 24.03 hrs, Volume= 0.003 af
Outflow = 0.01 cfs @ 24.03 hrs, Volume= 0.003 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 67R: Outfall of SB 28



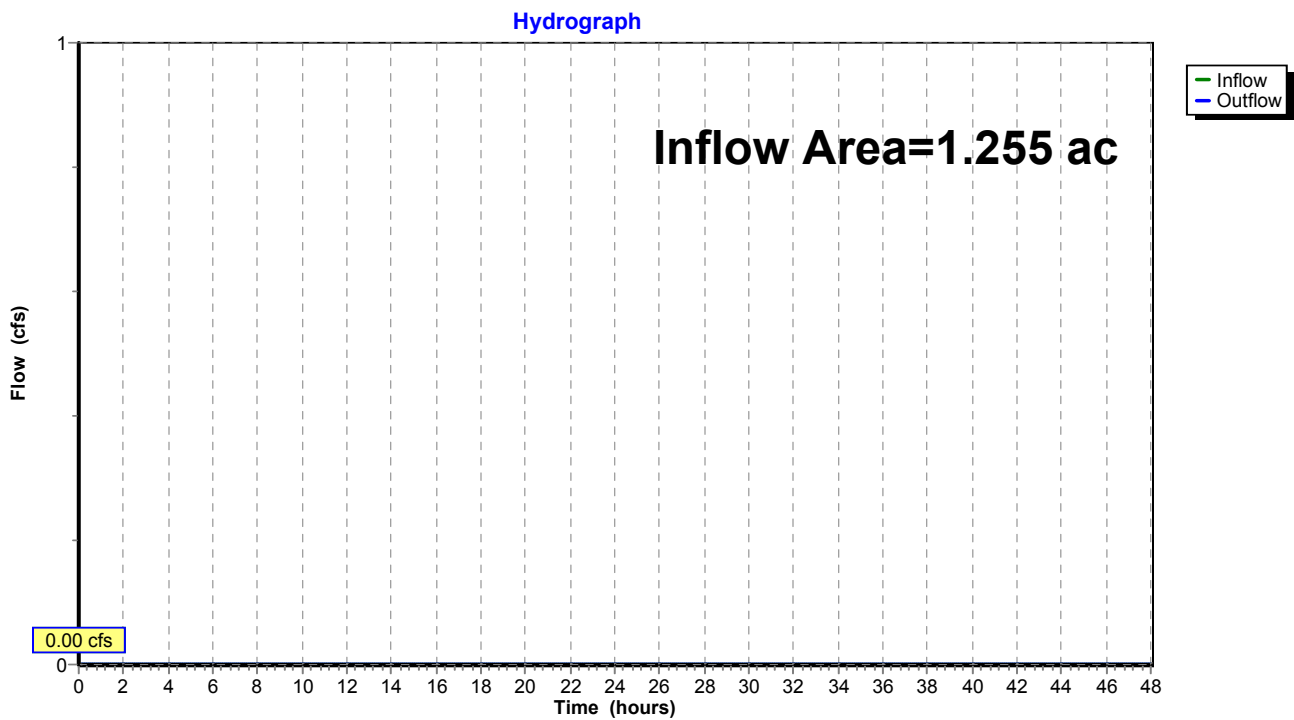
Summary for Reach 68R: Outfall of SB 29

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.255 ac, 0.00% Impervious, Inflow Depth = 0.00" for 2-Year event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 68R: Outfall of SB 29



Summary for Reach 69R: Outfall of SB 30

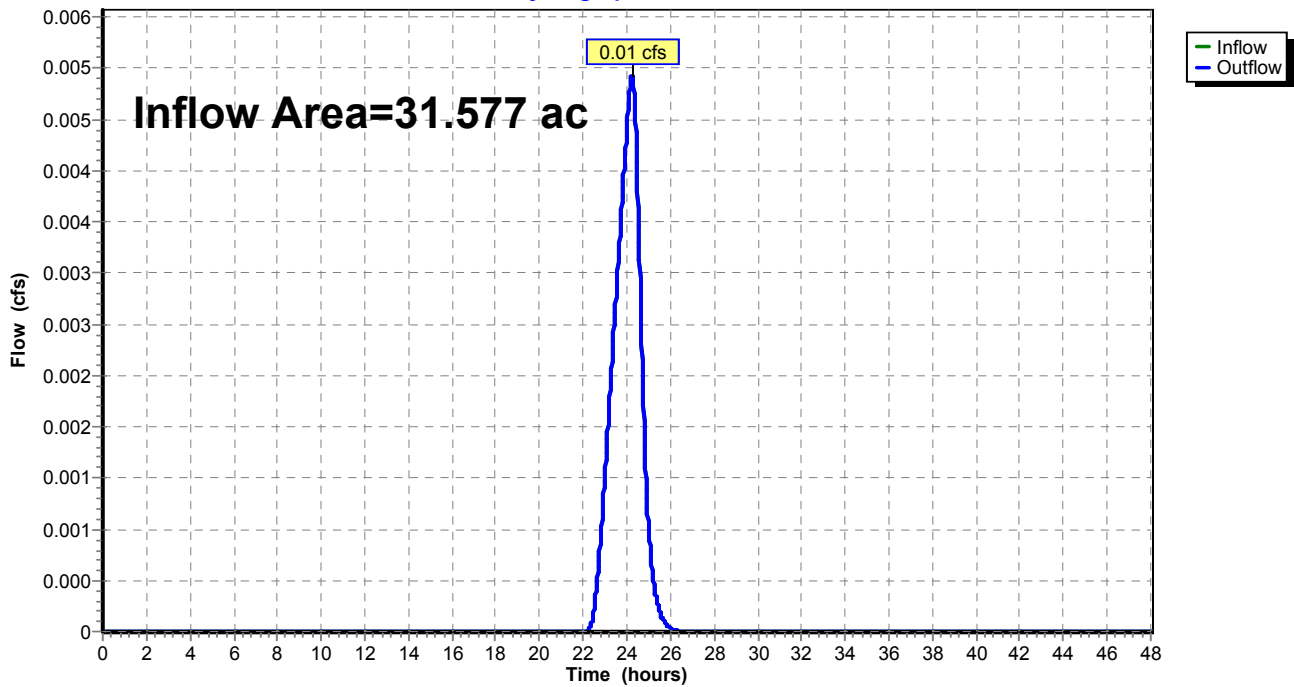
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 31.577 ac, 0.00% Impervious, Inflow Depth = 0.00" for 2-Year event
Inflow = 0.01 cfs @ 24.23 hrs, Volume= 0.001 af
Outflow = 0.01 cfs @ 24.23 hrs, Volume= 0.001 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 69R: Outfall of SB 30

Hydrograph



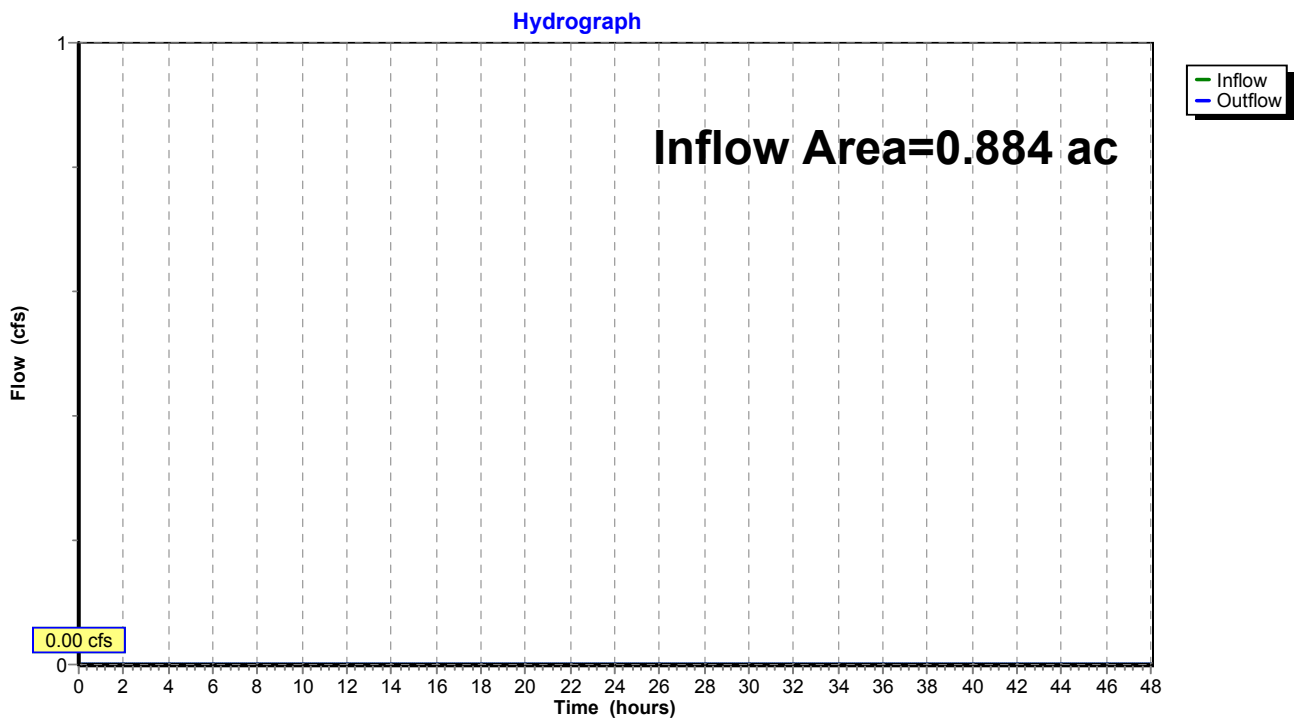
Summary for Reach 70R: Outfall of SB 31

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.884 ac, 0.00% Impervious, Inflow Depth = 0.00" for 2-Year event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 70R: Outfall of SB 31



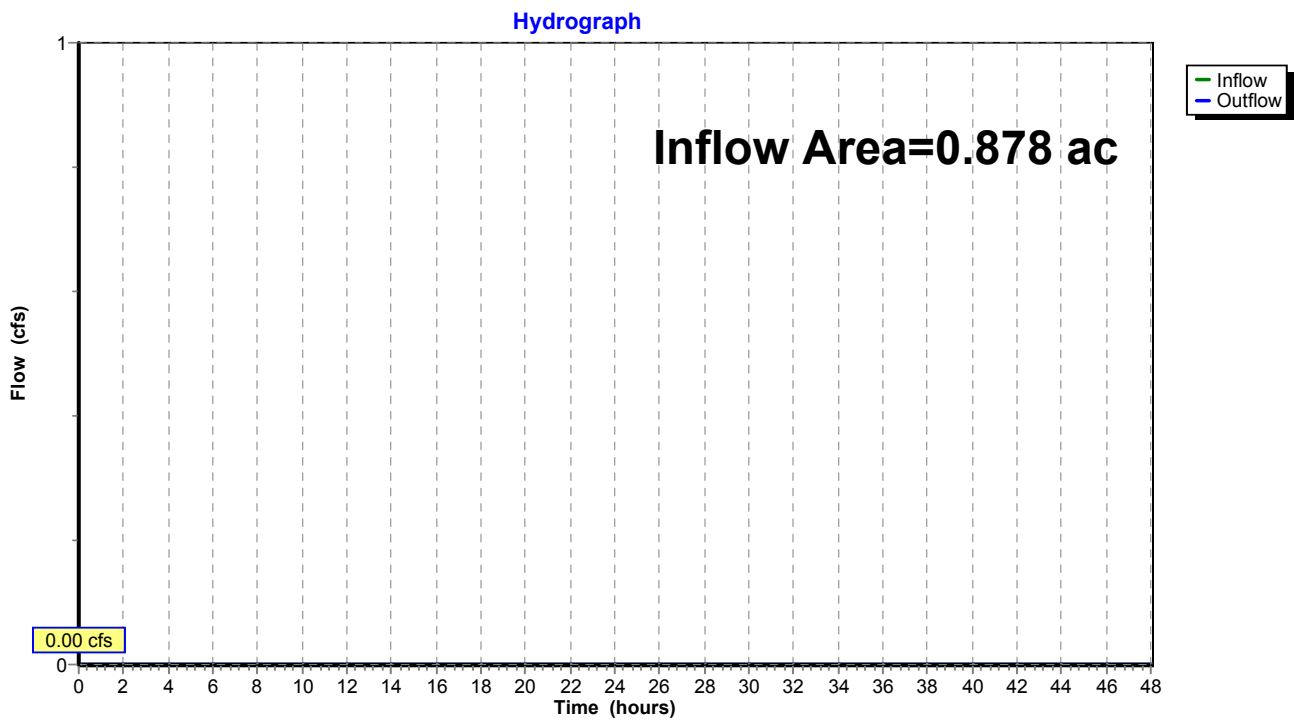
Summary for Reach 71R: Outfall of SB 32

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.878 ac, 0.00% Impervious, Inflow Depth = 0.00" for 2-Year event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 71R: Outfall of SB 32



Summary for Reach 72R: Outfall of SB 33

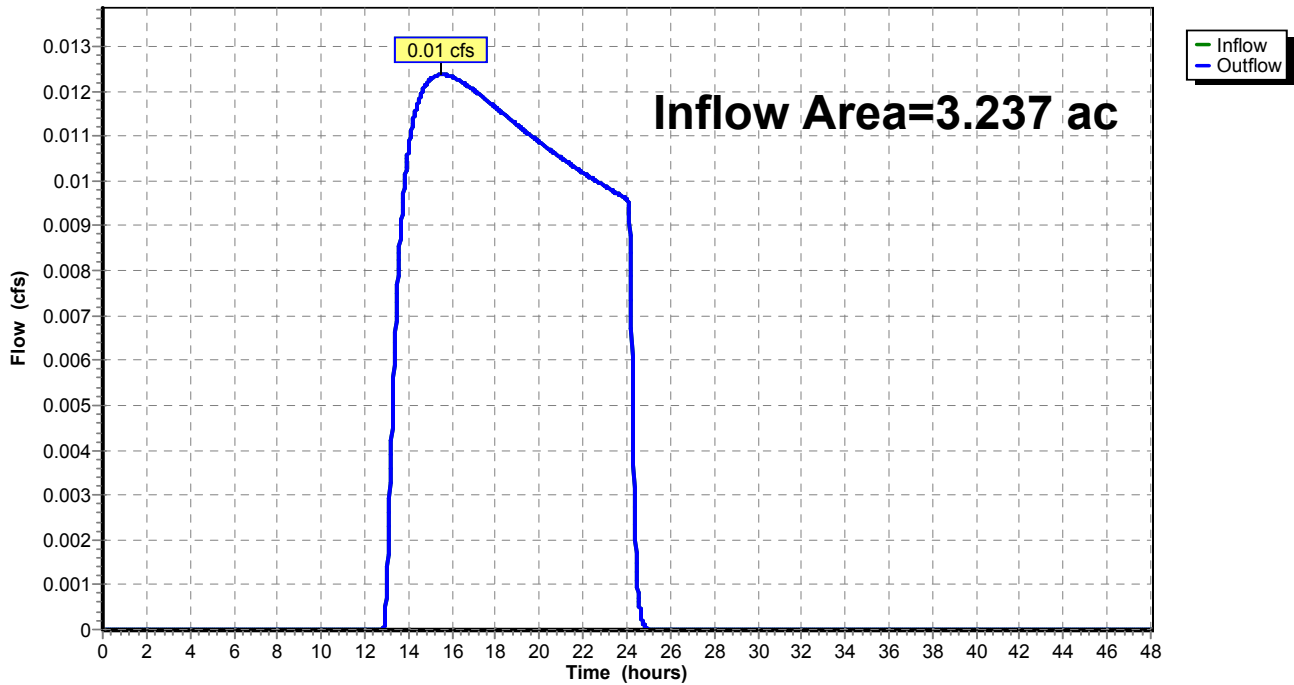
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 3.237 ac, 0.00% Impervious, Inflow Depth = 0.04" for 2-Year event
Inflow = 0.01 cfs @ 15.50 hrs, Volume= 0.010 af
Outflow = 0.01 cfs @ 15.50 hrs, Volume= 0.010 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 72R: Outfall of SB 33

Hydrograph



Summary for Reach 73R: Outfall of SB 34

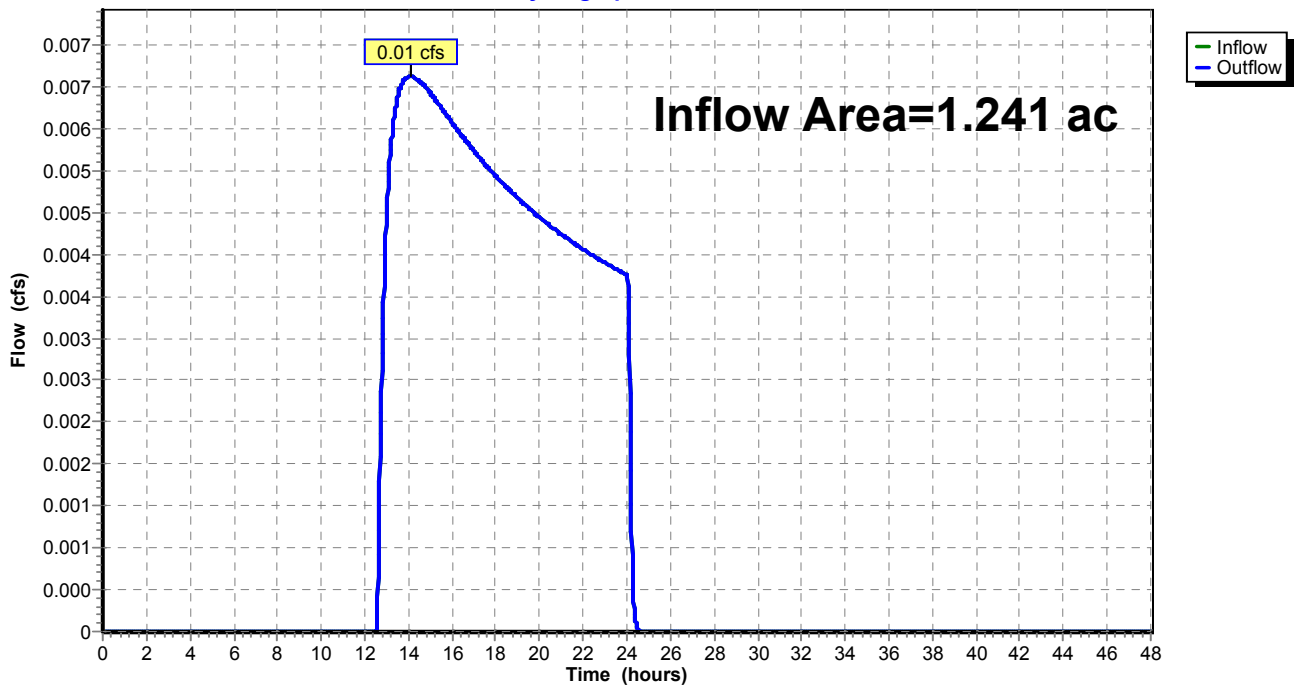
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.241 ac, 0.00% Impervious, Inflow Depth = 0.05" for 2-Year event
Inflow = 0.01 cfs @ 14.09 hrs, Volume= 0.005 af
Outflow = 0.01 cfs @ 14.09 hrs, Volume= 0.005 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 73R: Outfall of SB 34

Hydrograph



Summary for Reach 74R: Outfall of SB 35

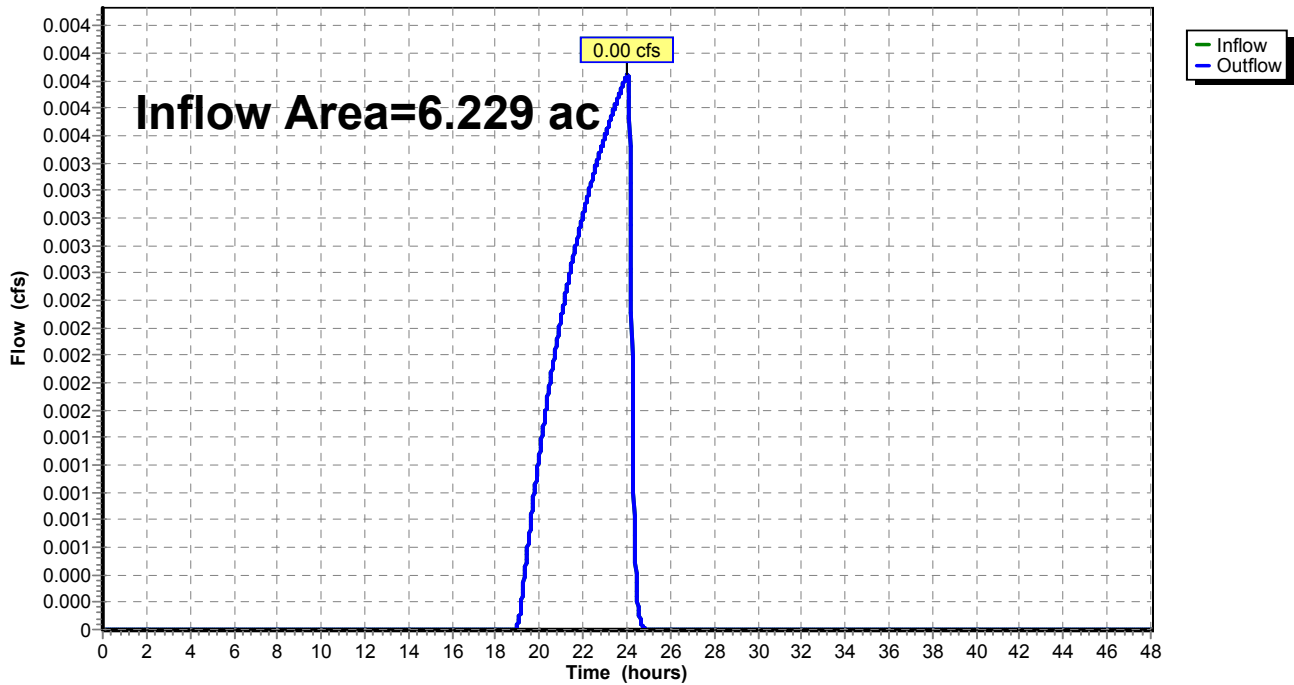
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 6.229 ac, 0.00% Impervious, Inflow Depth = 0.00" for 2-Year event
Inflow = 0.00 cfs @ 24.02 hrs, Volume= 0.001 af
Outflow = 0.00 cfs @ 24.02 hrs, Volume= 0.001 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 74R: Outfall of SB 35

Hydrograph



Summary for Reach 75R: Outfall of SB 19

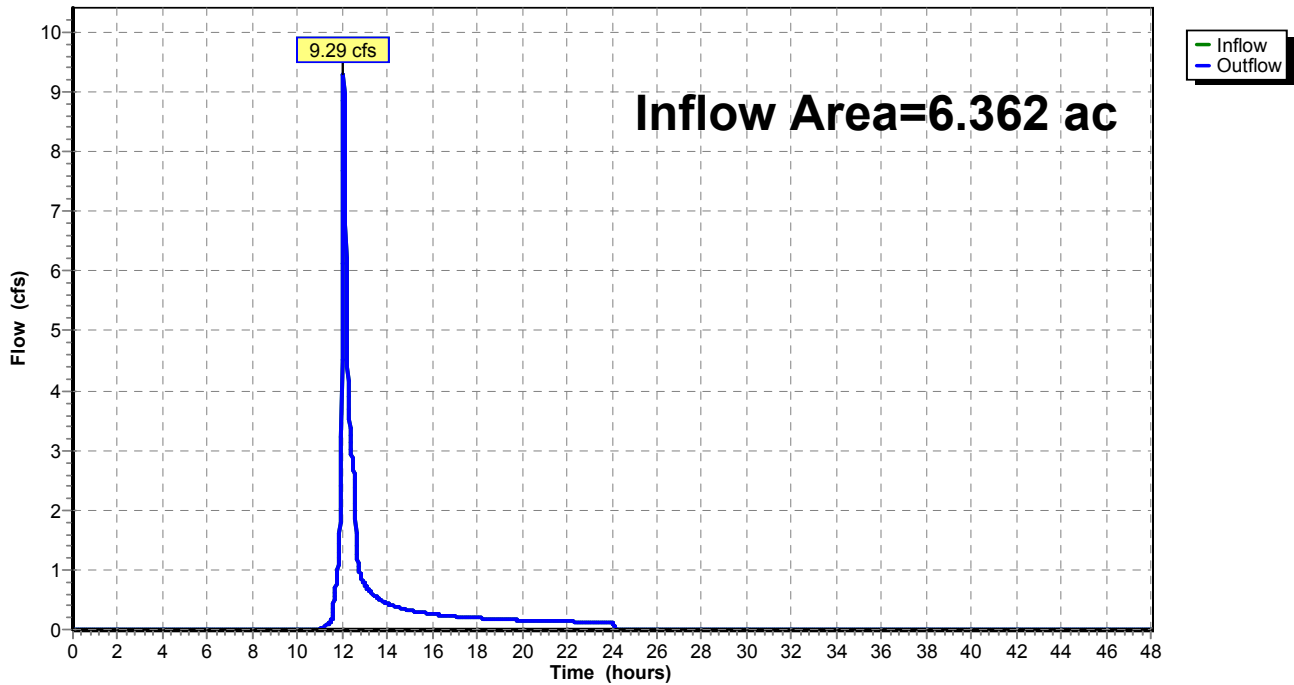
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 6.362 ac, 0.00% Impervious, Inflow Depth = 1.00" for 2-Year event
Inflow = 9.29 cfs @ 12.06 hrs, Volume= 0.531 af
Outflow = 9.29 cfs @ 12.06 hrs, Volume= 0.531 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 75R: Outfall of SB 19

Hydrograph



Summary for Reach 82R: Outfall of SB 27

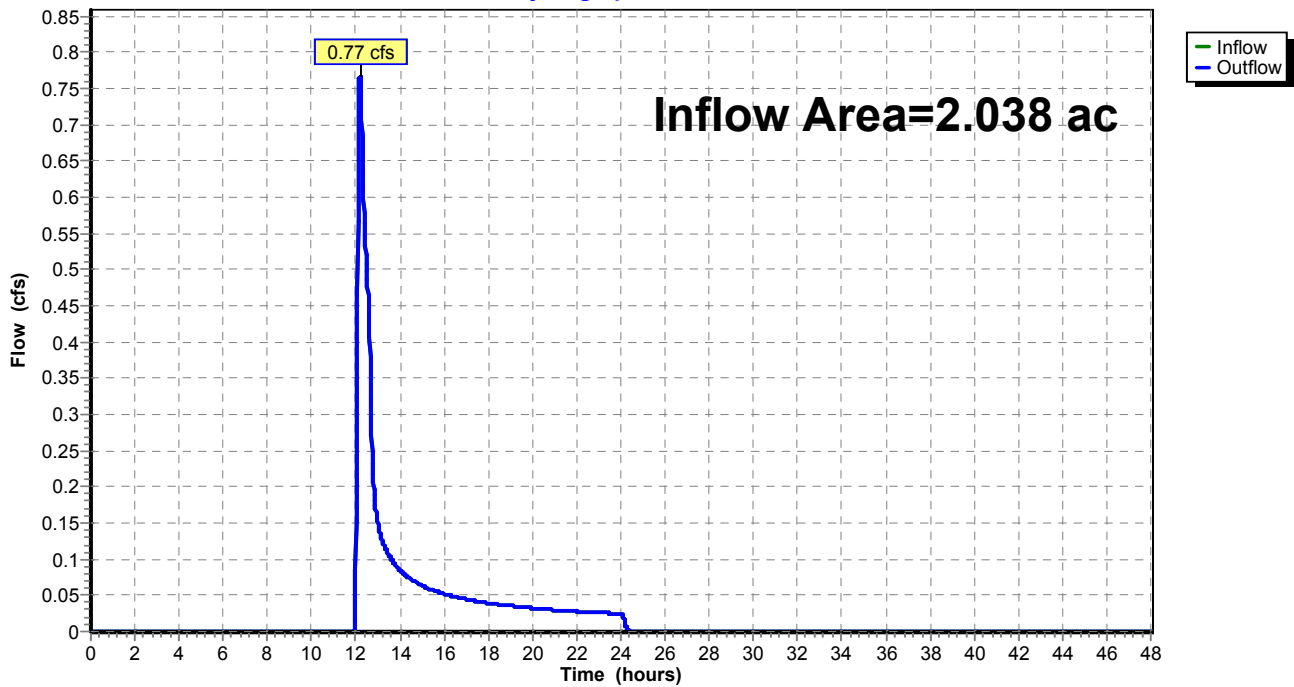
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 2.038 ac, 0.00% Impervious, Inflow Depth = 0.46" for 2-Year event
Inflow = 0.77 cfs @ 12.19 hrs, Volume= 0.078 af
Outflow = 0.77 cfs @ 12.19 hrs, Volume= 0.078 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 82R: Outfall of SB 27

Hydrograph



Summary for Reach 84R: Outfall of Future County Road H Subbasin

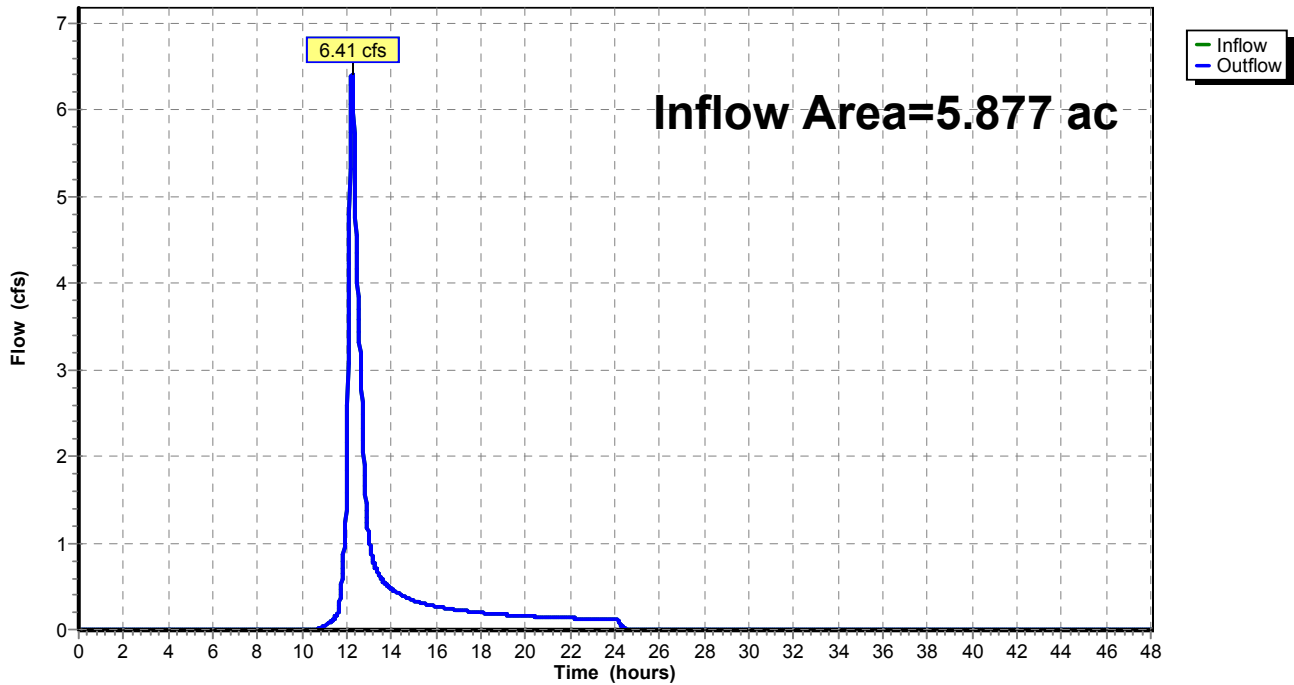
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 5.877 ac, 0.00% Impervious, Inflow Depth = 1.12" for 2-Year event
Inflow = 6.41 cfs @ 12.23 hrs, Volume= 0.547 af
Outflow = 6.41 cfs @ 12.23 hrs, Volume= 0.547 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 84R: Outfall of Future County Road H Subbasin

Hydrograph



Time span=0.00-48.00 hrs, dt=0.02 hrs, 2401 points

Runoff by SCS TR-20 method, UH=SCS

Reach routing by Muskingum-Cunge method - Pond routing by Stor-Ind method

Subcatchment1: Sub-basin1	Runoff Area=15.328 ac 0.00% Impervious Runoff Depth=2.84" Tc=16.3 min CN=87 Runoff=47.16 cfs 3.626 af
Subcatchment2: Sub-basin 2	Runoff Area=4.913 ac 0.00% Impervious Runoff Depth=1.83" Tc=12.2 min CN=75 Runoff=10.91 cfs 0.750 af
Subcatchment3: Sub-basin 3	Runoff Area=15.522 ac 0.00% Impervious Runoff Depth=1.83" Tc=32.8 min CN=75 Runoff=21.31 cfs 2.371 af
Subcatchment4S: Sub-basin 4	Runoff Area=23.961 ac 0.00% Impervious Runoff Depth=2.65" Tc=11.3 min CN=85 Runoff=82.03 cfs 5.301 af
Subcatchment5S: Sub-basin 5	Runoff Area=27.171 ac 0.00% Impervious Runoff Depth=2.48" Tc=40.5 min CN=83 Runoff=45.83 cfs 5.610 af
Subcatchment6S: Sub-basin 6	Runoff Area=22.467 ac 0.00% Impervious Runoff Depth=2.06" Tc=46.4 min CN=78 Runoff=29.03 cfs 3.863 af
Subcatchment7S: Sub-basin 7	Runoff Area=9.790 ac 0.00% Impervious Runoff Depth=0.98" Tc=27.0 min CN=62 Runoff=6.82 cfs 0.801 af
Subcatchment8S: Sub-basin 8	Runoff Area=21.017 ac 0.00% Impervious Runoff Depth=2.31" Tc=9.5 min CN=81 Runoff=66.85 cfs 4.041 af
Subcatchment9S: Sub-basin 9	Runoff Area=9.296 ac 0.00% Impervious Runoff Depth=2.39" Tc=12.7 min CN=82 Runoff=27.07 cfs 1.853 af
Subcatchment10S: Sub-basin 10	Runoff Area=30.014 ac 0.00% Impervious Runoff Depth=2.31" Tc=37.7 min CN=81 Runoff=49.02 cfs 5.770 af
Subcatchment11S: Sub-basin 11	Runoff Area=4.343 ac 0.00% Impervious Runoff Depth=1.91" Tc=32.9 min CN=76 Runoff=6.20 cfs 0.691 af
Subcatchment12S: Sub-basin 12	Runoff Area=3.310 ac 0.00% Impervious Runoff Depth=2.22" Tc=14.0 min CN=80 Runoff=8.55 cfs 0.613 af
Subcatchment13S: Sub-basin 13	Runoff Area=2.279 ac 0.00% Impervious Runoff Depth=2.39" Tc=36.2 min CN=82 Runoff=3.95 cfs 0.454 af
Subcatchment14S: Sub-basin 14	Runoff Area=2.518 ac 0.00% Impervious Runoff Depth=2.57" Tc=8.9 min CN=84 Runoff=9.18 cfs 0.538 af
Subcatchment15S: Sub-basin 15	Runoff Area=56.506 ac 0.00% Impervious Runoff Depth=2.39" Tc=28.0 min CN=82 Runoff=112.23 cfs 11.261 af
Subcatchment16S: Sub-basin 16	Runoff Area=44.796 ac 0.00% Impervious Runoff Depth=2.31" Tc=26.3 min CN=81 Runoff=88.44 cfs 8.612 af

Subcatchment17S: Sub-basin 17	Runoff Area=8.010 ac 0.00% Impervious Runoff Depth=2.75" Tc=11.5 min CN=86 Runoff=28.00 cfs 1.833 af
Subcatchment18S: Sub-basin 18	Runoff Area=35.677 ac 0.00% Impervious Runoff Depth=2.57" Tc=15.8 min CN=84 Runoff=101.21 cfs 7.627 af
Subcatchment19S: Sub-basin 19	Runoff Area=6.362 ac 0.00% Impervious Runoff Depth=2.06" Tc=7.3 min CN=78 Runoff=19.90 cfs 1.094 af
Subcatchment20S: Sub-basin 20	Runoff Area=15.897 ac 0.00% Impervious Runoff Depth=2.65" Tc=17.1 min CN=85 Runoff=44.95 cfs 3.517 af
Subcatchment21S: Sub-basin 21	Runoff Area=7.156 ac 0.00% Impervious Runoff Depth=3.13" Tc=10.8 min CN=90 Runoff=29.08 cfs 1.865 af
Subcatchment22S: Sub-basin 22	Runoff Area=26.548 ac 0.00% Impervious Runoff Depth=2.39" Tc=19.6 min CN=82 Runoff=63.21 cfs 5.291 af
Subcatchment23S: Sub-basin 23	Runoff Area=13.825 ac 0.00% Impervious Runoff Depth=3.33" Tc=9.4 min CN=92 Runoff=62.35 cfs 3.836 af
Subcatchment24S: Sub-basin 24	Runoff Area=13.883 ac 0.00% Impervious Runoff Depth=2.65" Tc=19.0 min CN=85 Runoff=37.26 cfs 3.071 af
Subcatchment25S: Sub-basin 25	Runoff Area=6.657 ac 0.00% Impervious Runoff Depth=1.83" Tc=22.6 min CN=75 Runoff=11.10 cfs 1.017 af
Subcatchment26S: Sub-basin 26	Runoff Area=0.823 ac 0.00% Impervious Runoff Depth=1.22" Tc=38.2 min CN=66 Runoff=0.64 cfs 0.084 af
Subcatchment27S: Sub-basin 27	Runoff Area=2.038 ac 0.00% Impervious Runoff Depth=1.22" Tc=13.0 min CN=66 Runoff=2.67 cfs 0.207 af
Subcatchment28S: Sub-basin 28	Runoff Area=5.784 ac 0.00% Impervious Runoff Depth=0.19" Tc=23.9 min CN=44 Runoff=0.36 cfs 0.094 af
Subcatchment29S: Sub-basin 29	Runoff Area=1.255 ac 0.00% Impervious Runoff Depth=0.07" Tc=26.9 min CN=39 Runoff=0.01 cfs 0.007 af
Subcatchment30S: Sub-basin 30	Runoff Area=31.577 ac 0.00% Impervious Runoff Depth=0.14" Tc=45.9 min CN=42 Runoff=0.75 cfs 0.366 af
Subcatchment31S: Sub-basin 31	Runoff Area=0.884 ac 0.00% Impervious Runoff Depth=0.07" Tc=30.2 min CN=39 Runoff=0.01 cfs 0.005 af
Subcatchment32S: Sub-basin 32	Runoff Area=0.878 ac 0.00% Impervious Runoff Depth=0.07" Tc=27.6 min CN=39 Runoff=0.01 cfs 0.005 af
Subcatchment33S: Sub-basin 33	Runoff Area=3.237 ac 0.00% Impervious Runoff Depth=0.33" Tc=19.9 min CN=48 Runoff=0.49 cfs 0.088 af

Subcatchment34S: Sub-basin 34	Runoff Area=1.241 ac 0.00% Impervious Runoff Depth=0.36" Tc=12.1 min CN=49 Runoff=0.23 cfs 0.038 af
Subcatchment35S: Sub-basin 35	Runoff Area=6.229 ac 0.00% Impervious Runoff Depth=0.17" Tc=16.7 min CN=43 Runoff=0.33 cfs 0.086 af
Subcatchment36S: Sub-basin 36	Runoff Area=11.210 ac 0.00% Impervious Runoff Depth=2.14" Tc=52.2 min CN=79 Runoff=14.13 cfs 2.002 af
Subcatchment83S: County Road H	Runoff Area=5.877 ac 0.00% Impervious Runoff Depth=2.22" Tc=19.1 min CN=80 Runoff=13.12 cfs 1.089 af
Reach 37R: Outfall of SB 2, 3, 7	Inflow=33.01 cfs 3.922 af Outflow=33.01 cfs 3.922 af
Reach 39R: Outfall of SB 1, 4, 5, 6, 9, 10, 11, 36	Inflow=217.12 cfs 28.714 af Outflow=217.12 cfs 28.714 af
Reach 40R: 60 in SB 4	Avg. Flow Depth=1.95' Max Vel=31.08 fps Inflow=217.16 cfs 28.714 af 60.0" Round Pipe n=0.013 L=718.0' S=0.0330 '/' Capacity=473.08 cfs Outflow=217.12 cfs 28.714 af
Reach 41R: Channel in SB 9, 10	Avg. Flow Depth=0.55' Max Vel=9.14 fps Inflow=27.07 cfs 1.853 af n=0.050 L=1,660.0' S=0.0048 '/' Capacity=280.23 cfs Outflow=25.05 cfs 1.853 af
Reach 46R: Channel SB1	Avg. Flow Depth=0.87' Max Vel=6.34 fps Inflow=47.16 cfs 3.626 af n=0.050 L=841.0' S=0.0071 '/' Capacity=296.86 cfs Outflow=46.50 cfs 3.626 af
Reach 48R: Outfall of SB 8, 13	Inflow=67.32 cfs 4.495 af Outflow=67.32 cfs 4.495 af
Reach 49R: Channel SB8	Avg. Flow Depth=0.83' Max Vel=6.23 fps Inflow=66.85 cfs 4.041 af n=0.050 L=521.0' S=0.0077 '/' Capacity=706.58 cfs Outflow=65.69 cfs 4.041 af
Reach 50R: Outfall of SB 12	Inflow=8.55 cfs 0.613 af Outflow=8.55 cfs 0.613 af
Reach 51R: Outfall of SB 14	Inflow=9.18 cfs 0.538 af Outflow=9.18 cfs 0.538 af
Reach 52R: Outfall of SB 17	Inflow=28.00 cfs 1.833 af Outflow=28.00 cfs 1.833 af
Reach 53R: Outfall of SB 18	Inflow=101.21 cfs 7.627 af Outflow=101.21 cfs 7.627 af
Reach 54R: Outfall of SB 25	Inflow=11.10 cfs 1.017 af Outflow=11.10 cfs 1.017 af
Reach 55R: Outfall of SB 26	Inflow=0.64 cfs 0.084 af Outflow=0.64 cfs 0.084 af

Reach 56R: Outfall of SB 23, 24	Inflow=87.74 cfs 6.907 af Outflow=87.74 cfs 6.907 af
Reach 59R: Outfall of SB 20, 22	Inflow=107.32 cfs 8.807 af Outflow=107.32 cfs 8.807 af
Reach 61R: Outfall of SB 15, 16, 21	Inflow=212.59 cfs 21.738 af Outflow=212.59 cfs 21.738 af
Reach 67R: Outfall of SB 28	Inflow=0.36 cfs 0.094 af Outflow=0.36 cfs 0.094 af
Reach 68R: Outfall of SB 29	Inflow=0.01 cfs 0.007 af Outflow=0.01 cfs 0.007 af
Reach 69R: Outfall of SB 30	Inflow=0.75 cfs 0.366 af Outflow=0.75 cfs 0.366 af
Reach 70R: Outfall of SB 31	Inflow=0.01 cfs 0.005 af Outflow=0.01 cfs 0.005 af
Reach 71R: Outfall of SB 32	Inflow=0.01 cfs 0.005 af Outflow=0.01 cfs 0.005 af
Reach 72R: Outfall of SB 33	Inflow=0.49 cfs 0.088 af Outflow=0.49 cfs 0.088 af
Reach 73R: Outfall of SB 34	Inflow=0.23 cfs 0.038 af Outflow=0.23 cfs 0.038 af
Reach 74R: Outfall of SB 35	Inflow=0.33 cfs 0.086 af Outflow=0.33 cfs 0.086 af
Reach 75R: Outfall of SB 19	Inflow=19.90 cfs 1.094 af Outflow=19.90 cfs 1.094 af
Reach 82R: Outfall of SB 27	Inflow=2.67 cfs 0.207 af Outflow=2.67 cfs 0.207 af
Reach 84R: Outfall of Future County Road H Subbasin	Inflow=13.12 cfs 1.089 af Outflow=13.12 cfs 1.089 af

Total Runoff Area = 498.279 ac Runoff Volume = 89.375 af Average Runoff Depth = 2.15"
100.00% Pervious = 498.279 ac 0.00% Impervious = 0.000 ac

Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 1: Sub-basin1

Runoff = 47.16 cfs @ 12.18 hrs, Volume= 3.626 af, Depth= 2.84"

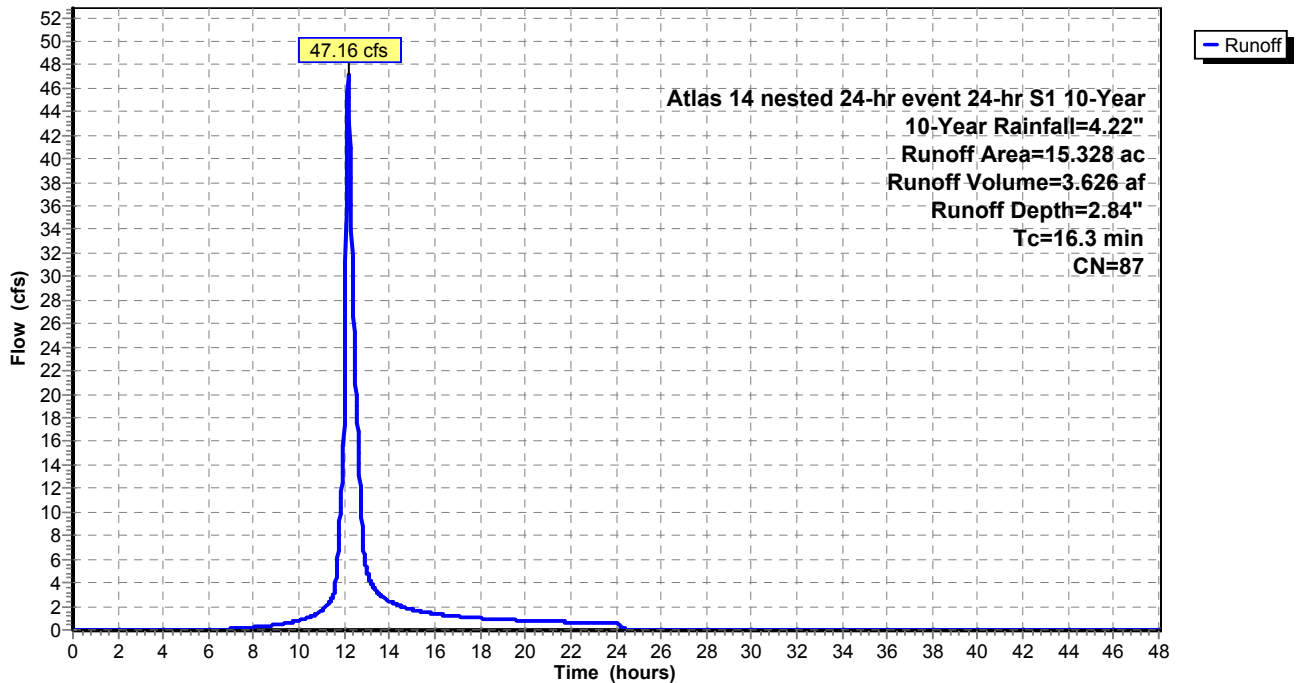
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 15.328	87	
15.328		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.3					Direct Entry,

Subcatchment 1: Sub-basin1

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 2: Sub-basin 2

Runoff = 10.91 cfs @ 12.13 hrs, Volume= 0.750 af, Depth= 1.83"

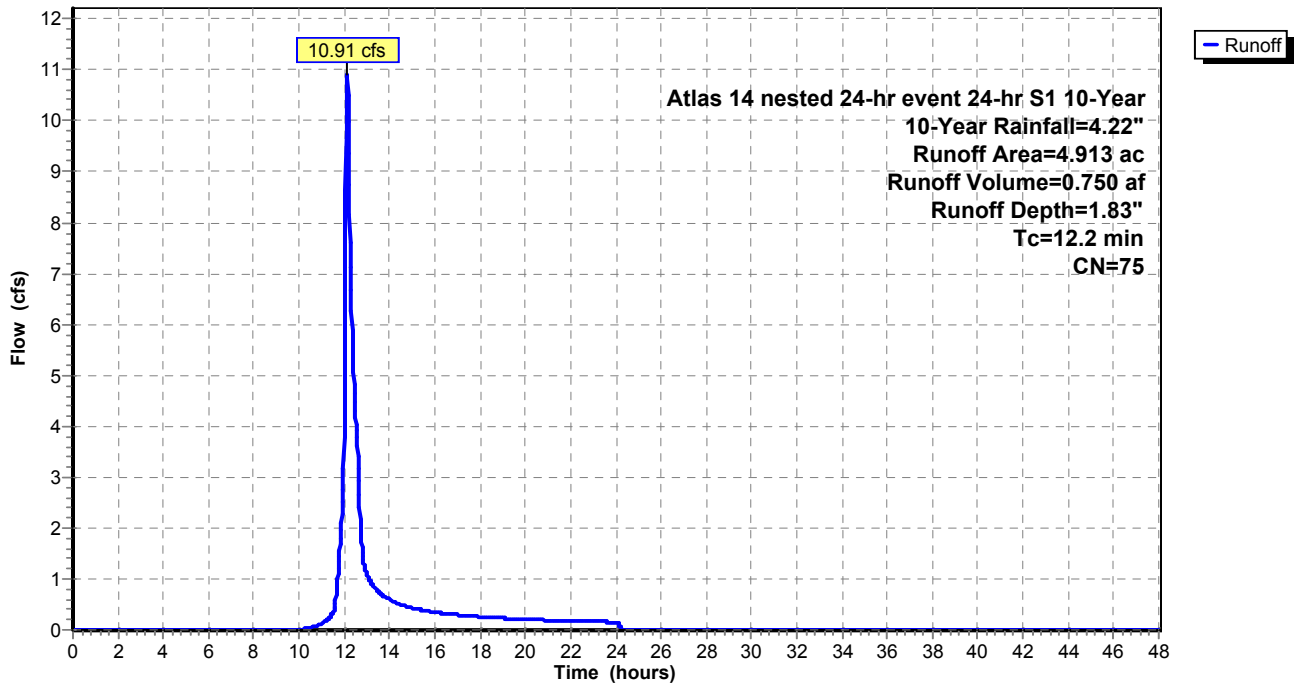
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 4.913	75	
4.913		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.2					Direct Entry,

Subcatchment 2: Sub-basin 2

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 3: Sub-basin 3

Runoff = 21.31 cfs @ 12.43 hrs, Volume= 2.371 af, Depth= 1.83"

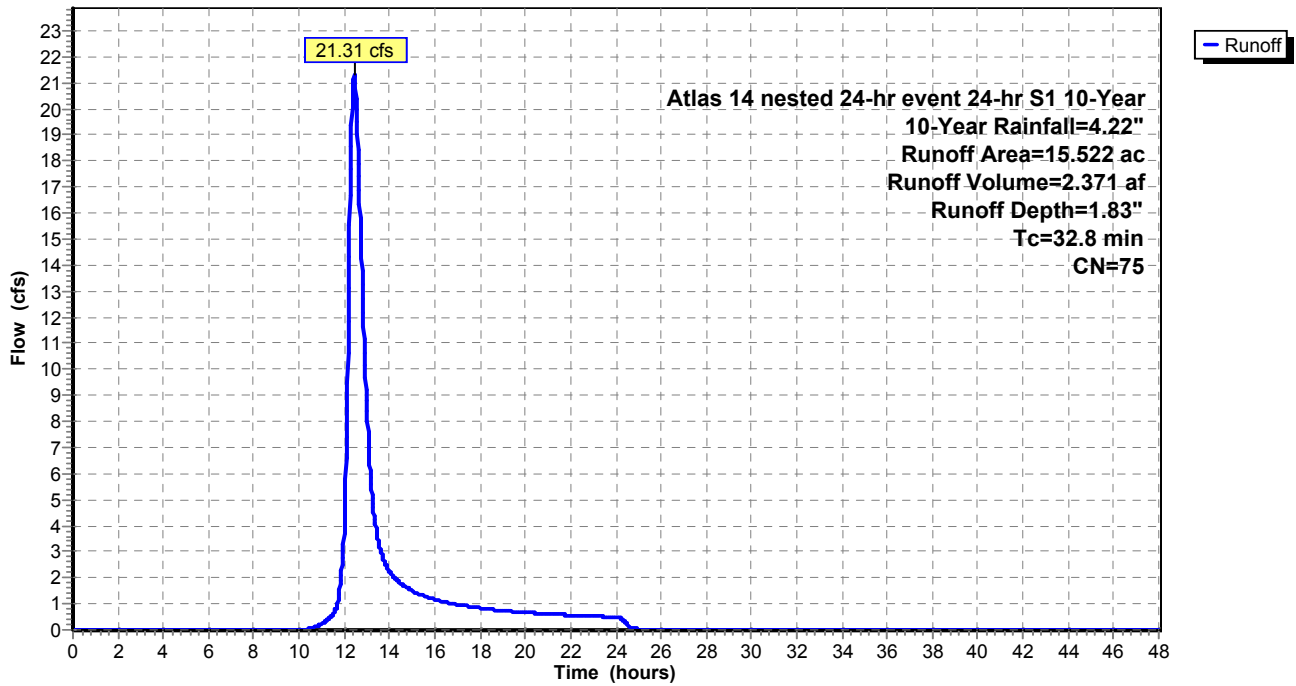
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 15.522	75	
15.522		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.8					Direct Entry,

Subcatchment 3: Sub-basin 3

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 4S: Sub-basin 4

Runoff = 82.03 cfs @ 12.11 hrs, Volume= 5.301 af, Depth= 2.65"

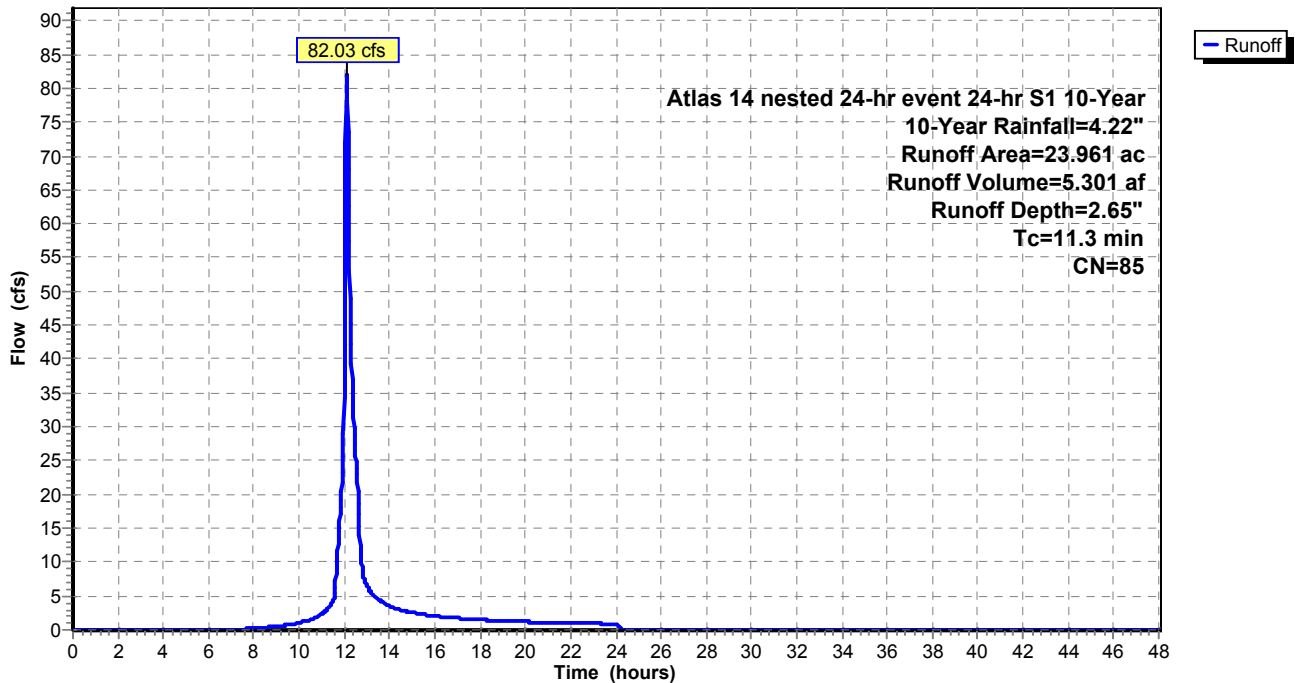
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 23.961	85	
23.961		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.3					Direct Entry,

Subcatchment 4S: Sub-basin 4

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 5S: Sub-basin 5

Runoff = 45.83 cfs @ 12.53 hrs, Volume= 5.610 af, Depth= 2.48"

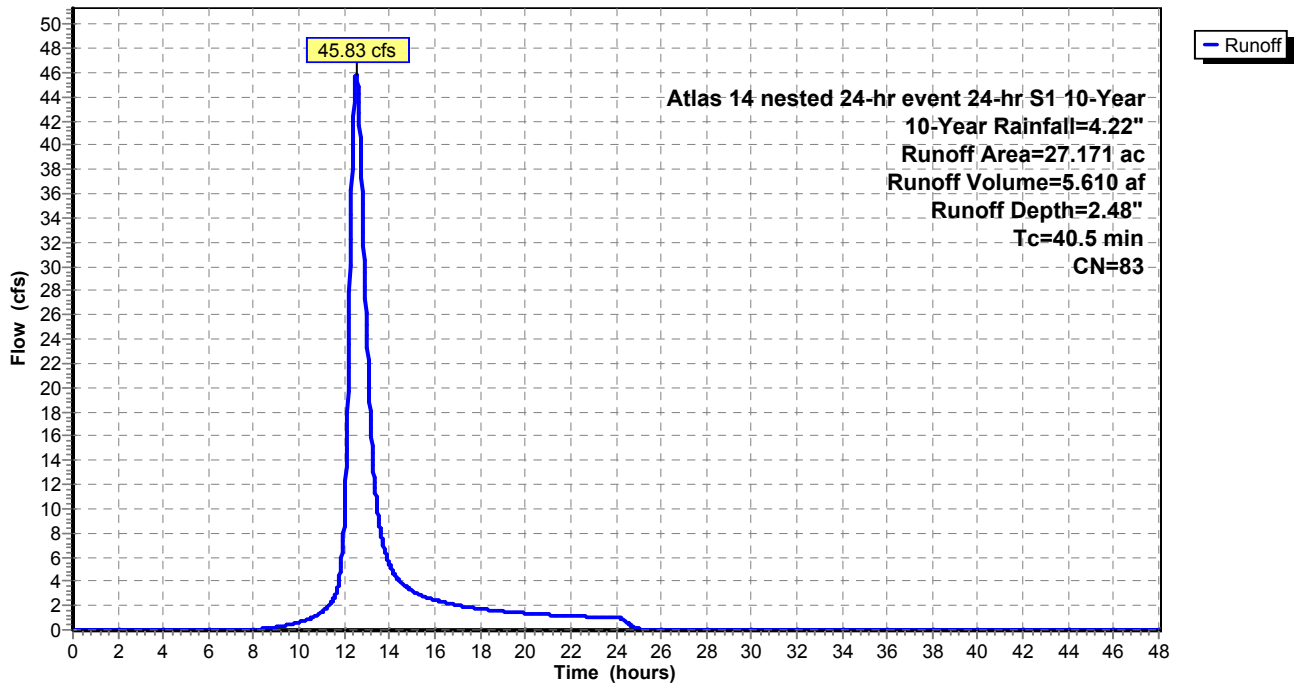
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 27.171	83	
27.171		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
40.5					Direct Entry,

Subcatchment 5S: Sub-basin 5

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 6S: Sub-basin 6

Runoff = 29.03 cfs @ 12.63 hrs, Volume= 3.863 af, Depth= 2.06"

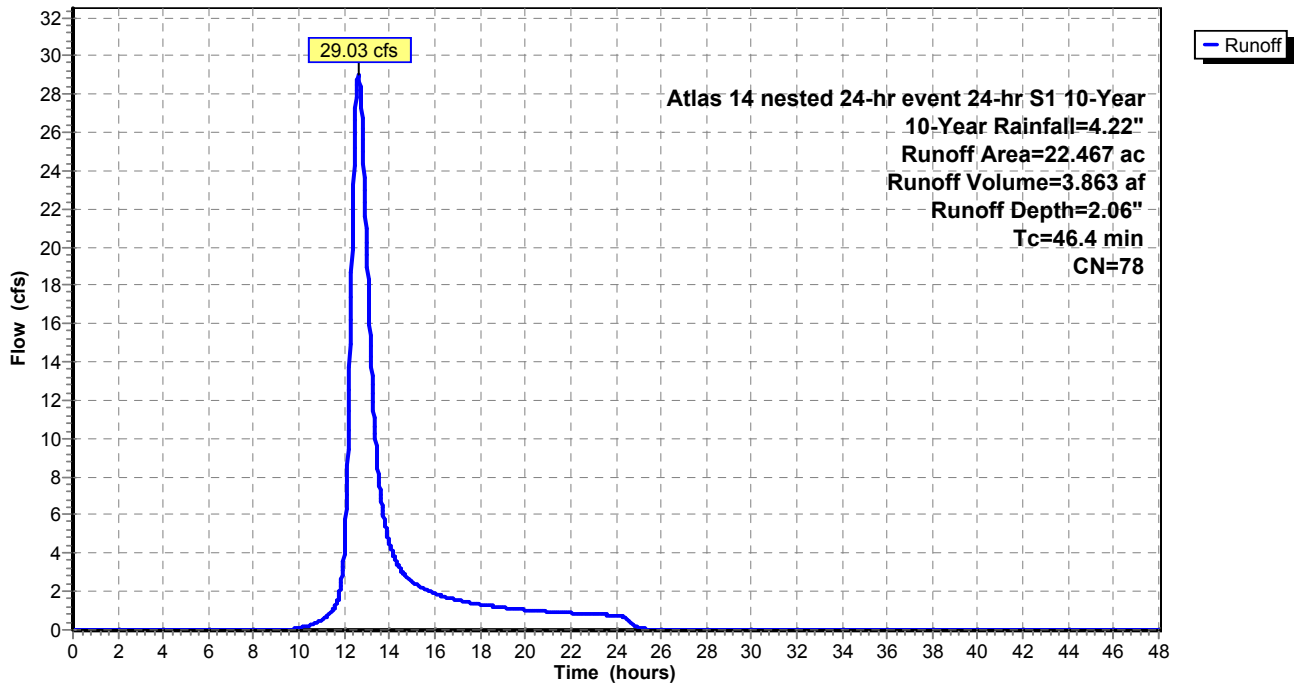
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 22.467	78	
22.467		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
46.4					Direct Entry,

Subcatchment 6S: Sub-basin 6

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 7S: Sub-basin 7

Runoff = 6.82 cfs @ 12.39 hrs, Volume= 0.801 af, Depth= 0.98"

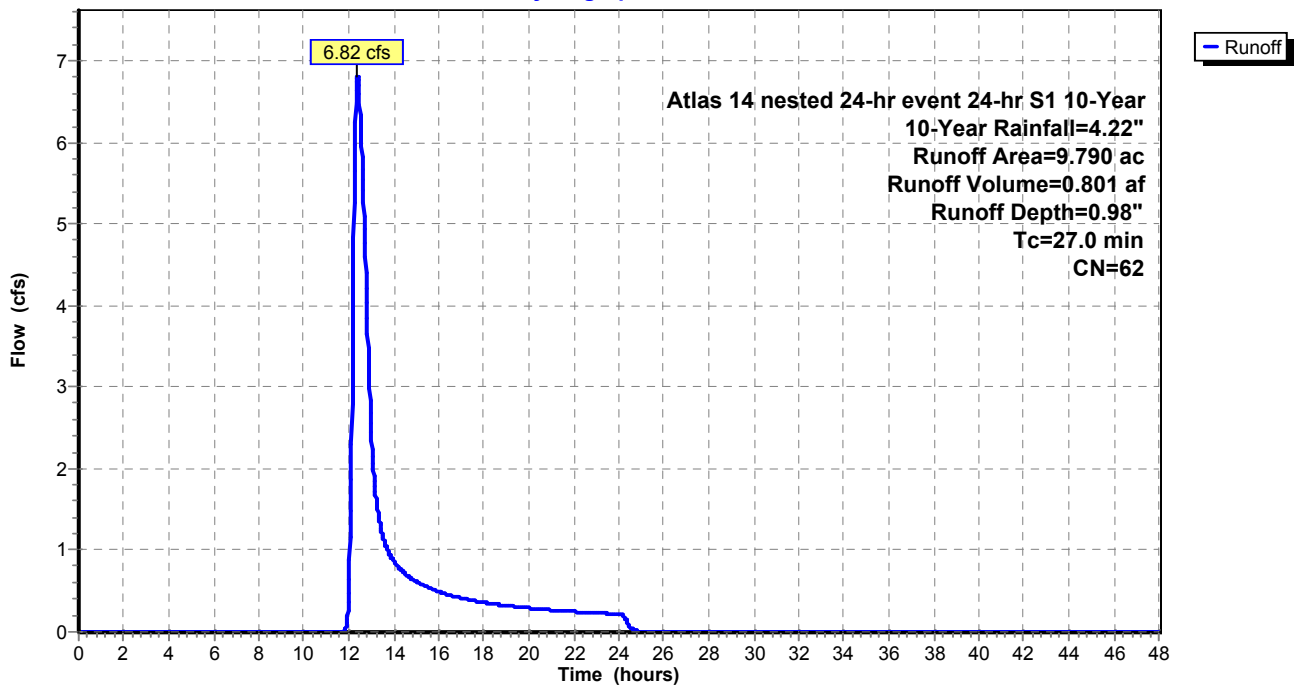
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 9.790	62	
9.790		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.0					Direct Entry,

Subcatchment 7S: Sub-basin 7

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 8S: Sub-basin 8

Runoff = 66.85 cfs @ 12.08 hrs, Volume= 4.041 af, Depth= 2.31"

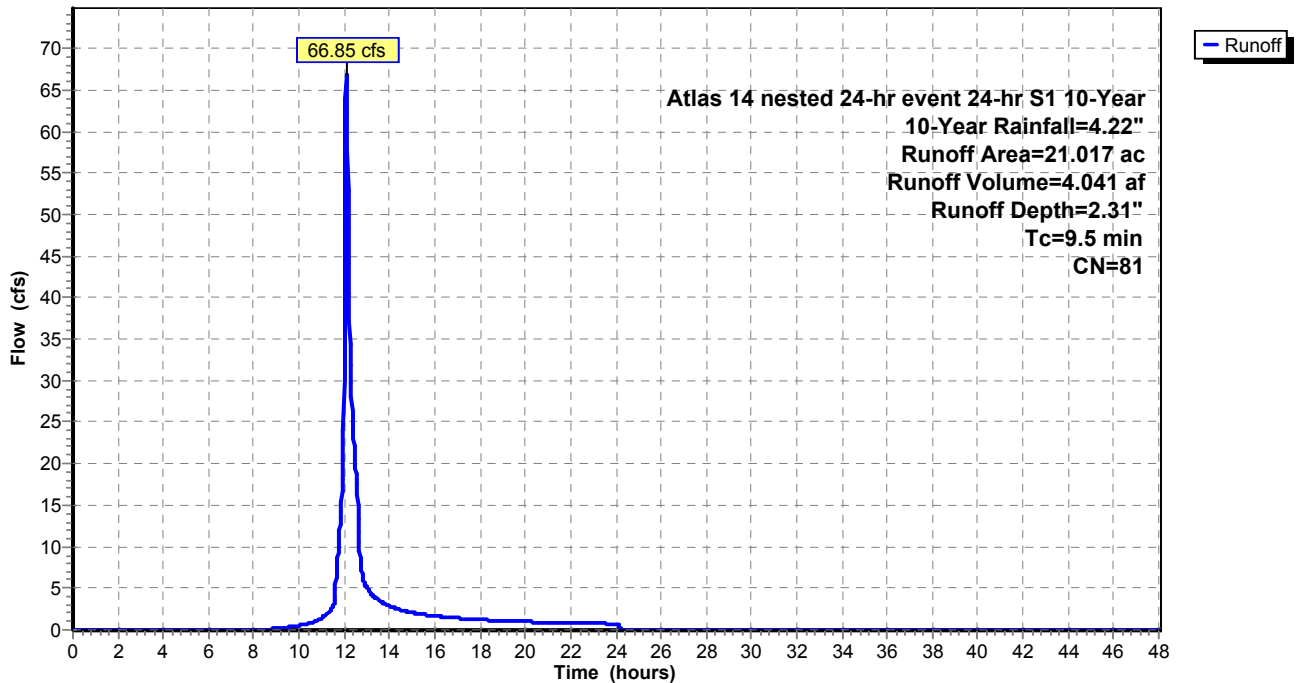
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 21.017	81	
21.017		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.5					Direct Entry,

Subcatchment 8S: Sub-basin 8

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 9S: Sub-basin 9

Runoff = 27.07 cfs @ 12.13 hrs, Volume= 1.853 af, Depth= 2.39"

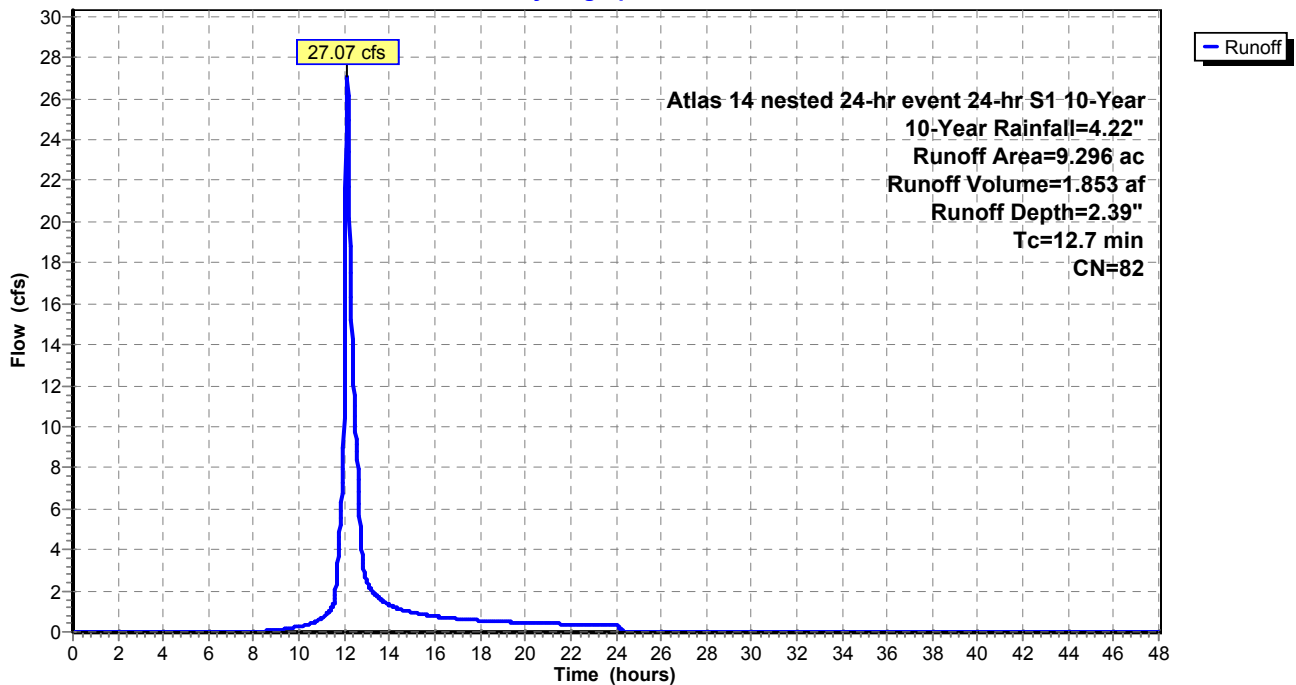
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 9.296	82	
9.296		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.7					Direct Entry,

Subcatchment 9S: Sub-basin 9

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 10S: Sub-basin 10

Runoff = 49.02 cfs @ 12.49 hrs, Volume= 5.770 af, Depth= 2.31"

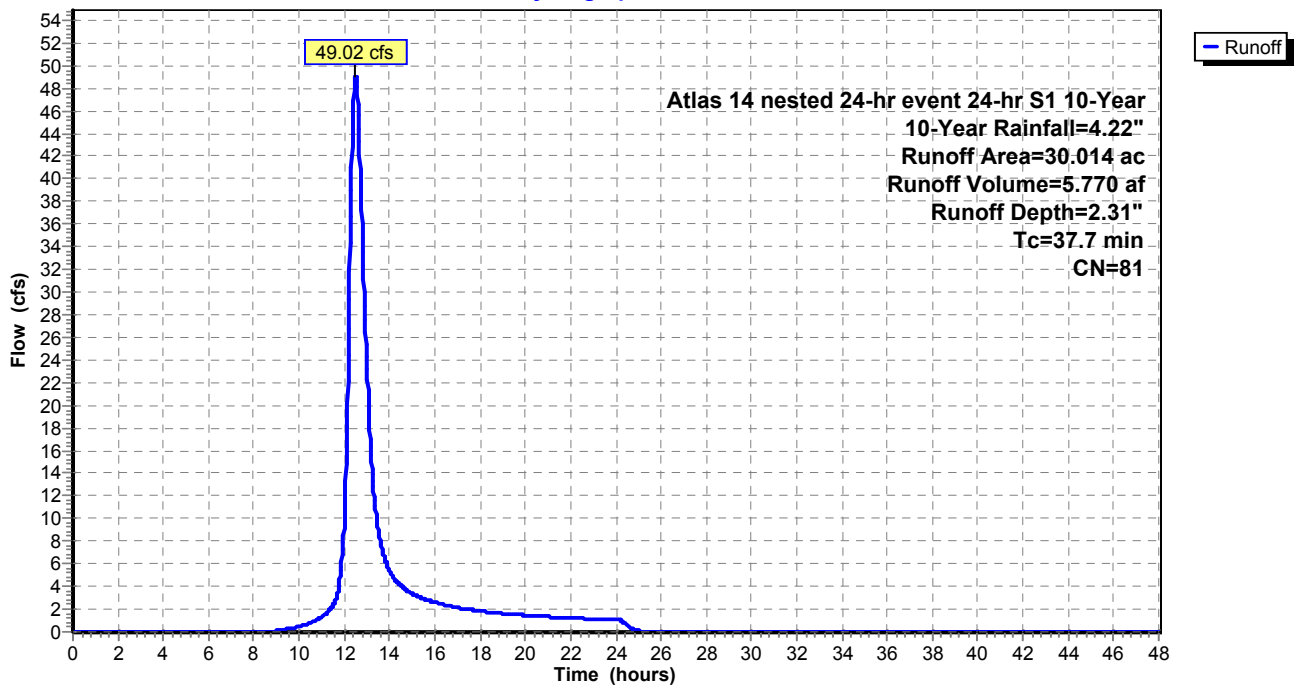
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 30.014	81	
30.014		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
37.7					Direct Entry,

Subcatchment 10S: Sub-basin 10

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 11S: Sub-basin 11

Runoff = 6.20 cfs @ 12.43 hrs, Volume= 0.691 af, Depth= 1.91"

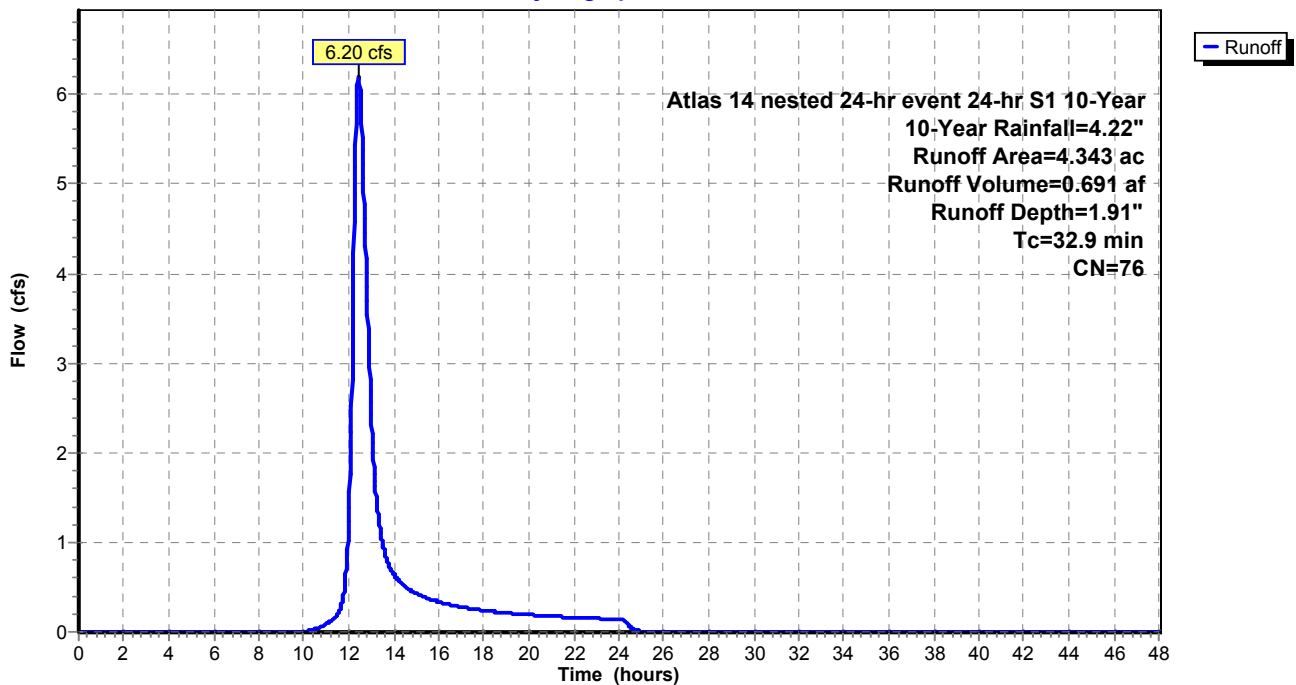
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 4.343	76	
4.343		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.9					Direct Entry,

Subcatchment 11S: Sub-basin 11

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 12S: Sub-basin 12

Runoff = 8.55 cfs @ 12.15 hrs, Volume= 0.613 af, Depth= 2.22"

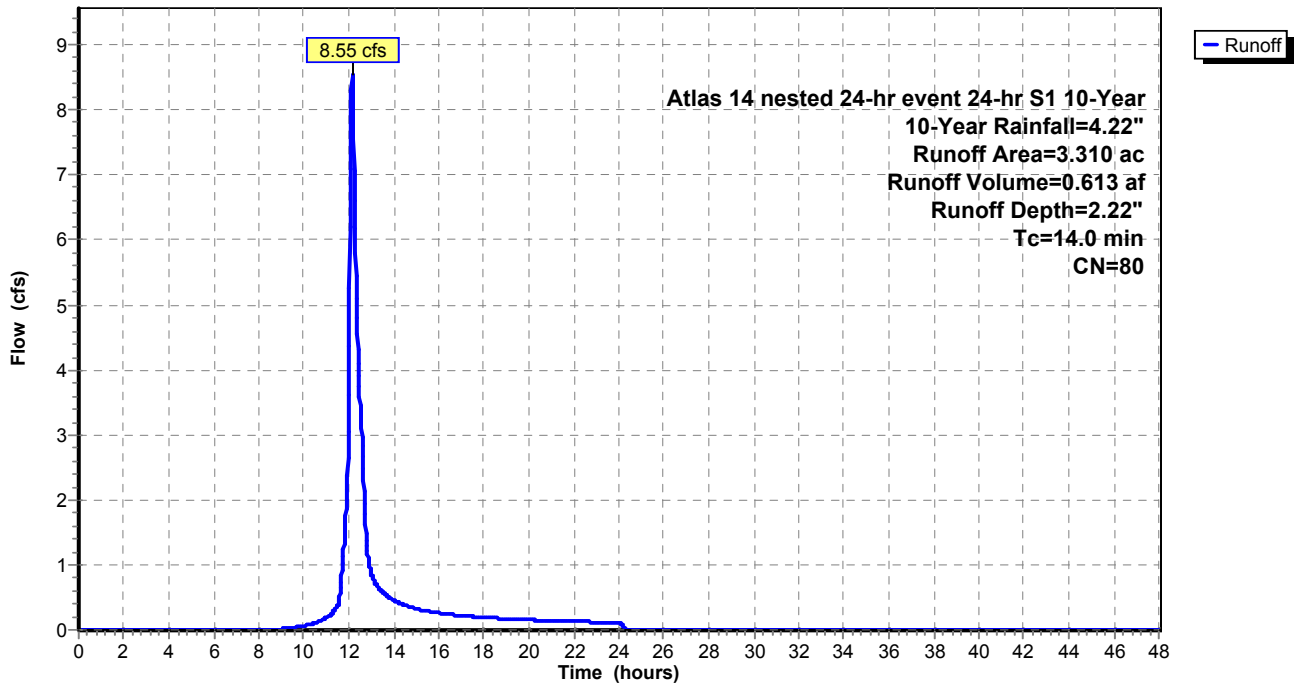
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 3.310	80	
3.310		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.0					Direct Entry,

Subcatchment 12S: Sub-basin 12

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 13S: Sub-basin 13

Runoff = 3.95 cfs @ 12.46 hrs, Volume= 0.454 af, Depth= 2.39"

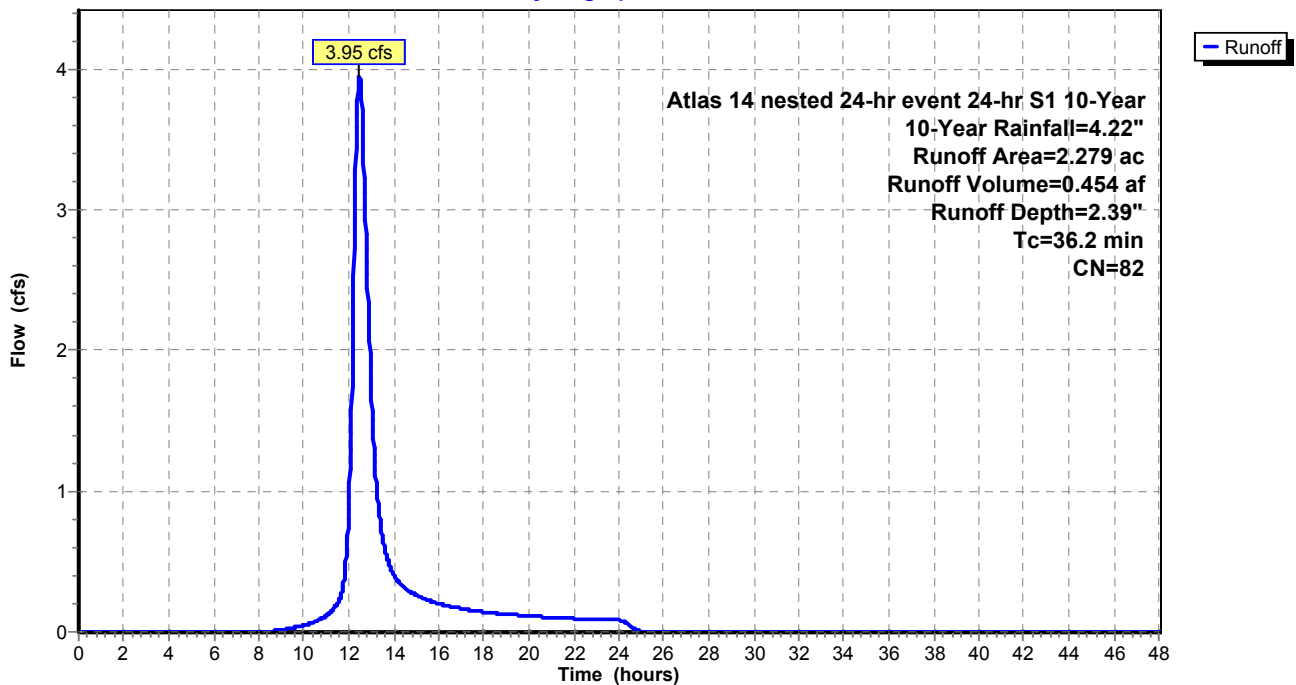
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 2.279	82	
2.279		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
36.2					Direct Entry,

Subcatchment 13S: Sub-basin 13

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 14S: Sub-basin 14

Runoff = 9.18 cfs @ 12.08 hrs, Volume= 0.538 af, Depth= 2.57"

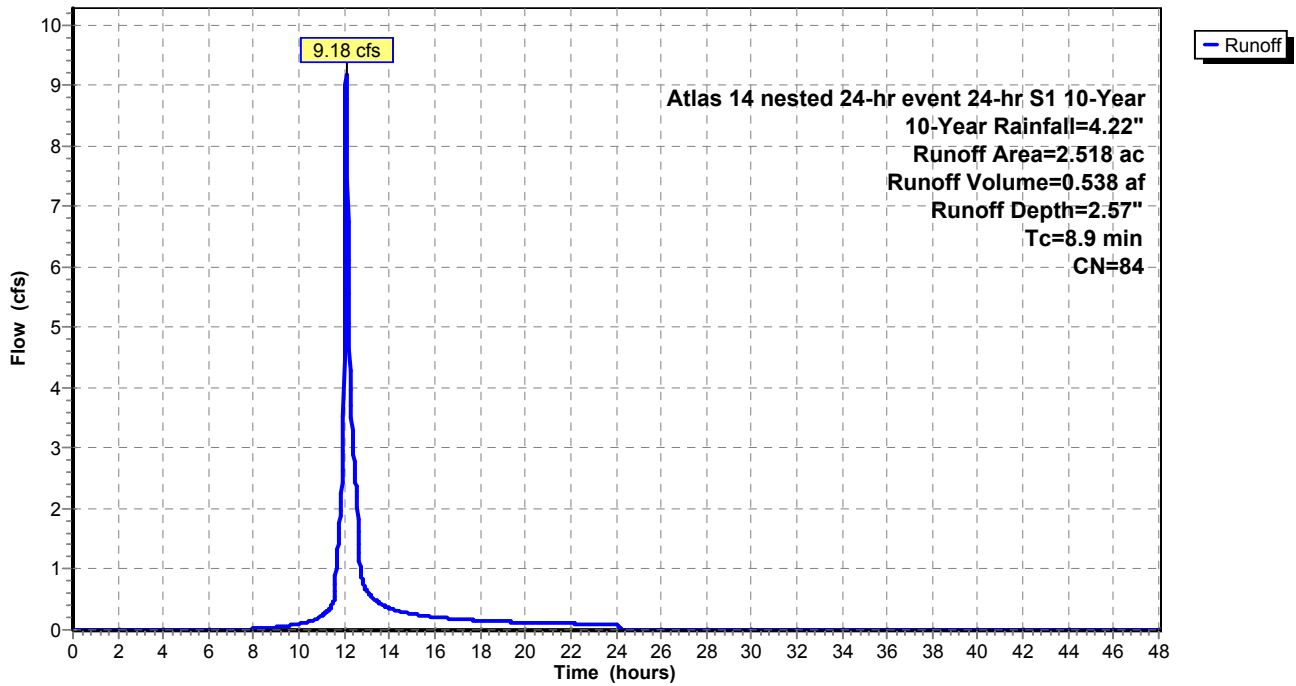
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 2.518	84	
2.518		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.9					Direct Entry,

Subcatchment 14S: Sub-basin 14

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 15S: Sub-basin 15

Runoff = 112.23 cfs @ 12.35 hrs, Volume= 11.261 af, Depth= 2.39"

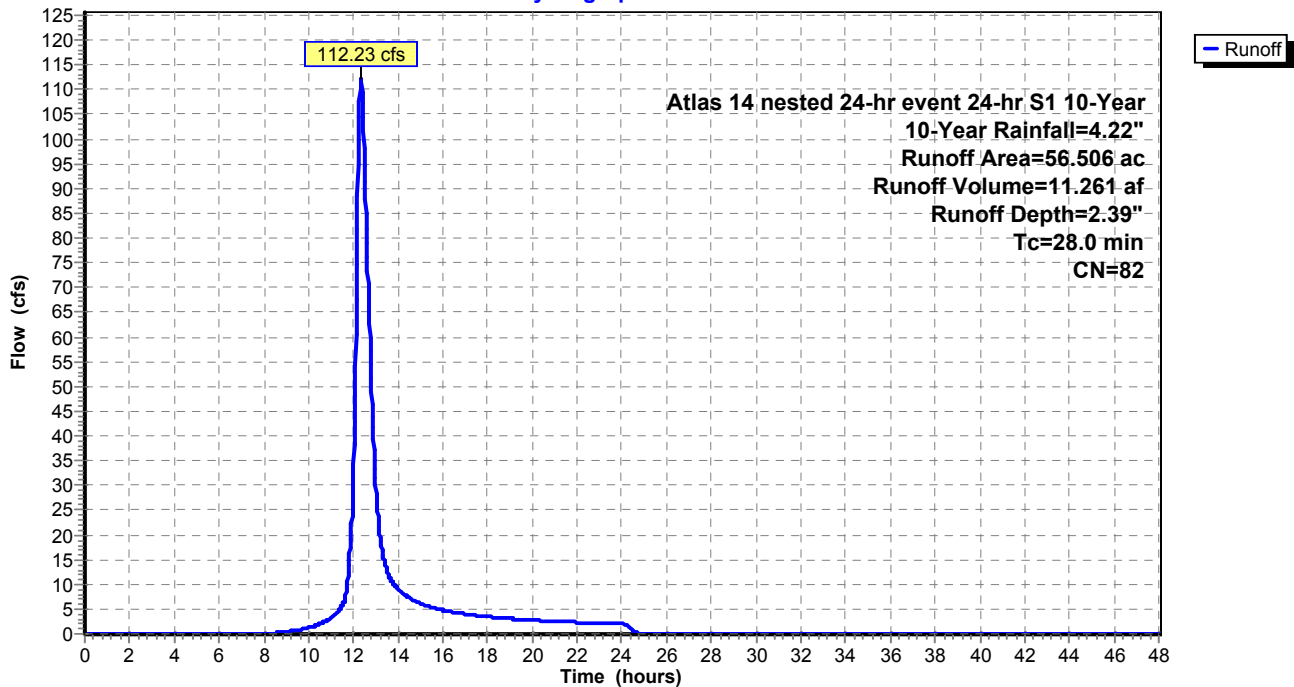
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 56.506	82	
56.506		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.0					Direct Entry,

Subcatchment 15S: Sub-basin 15

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 16S: Sub-basin 16

Runoff = 88.44 cfs @ 12.32 hrs, Volume= 8.612 af, Depth= 2.31"

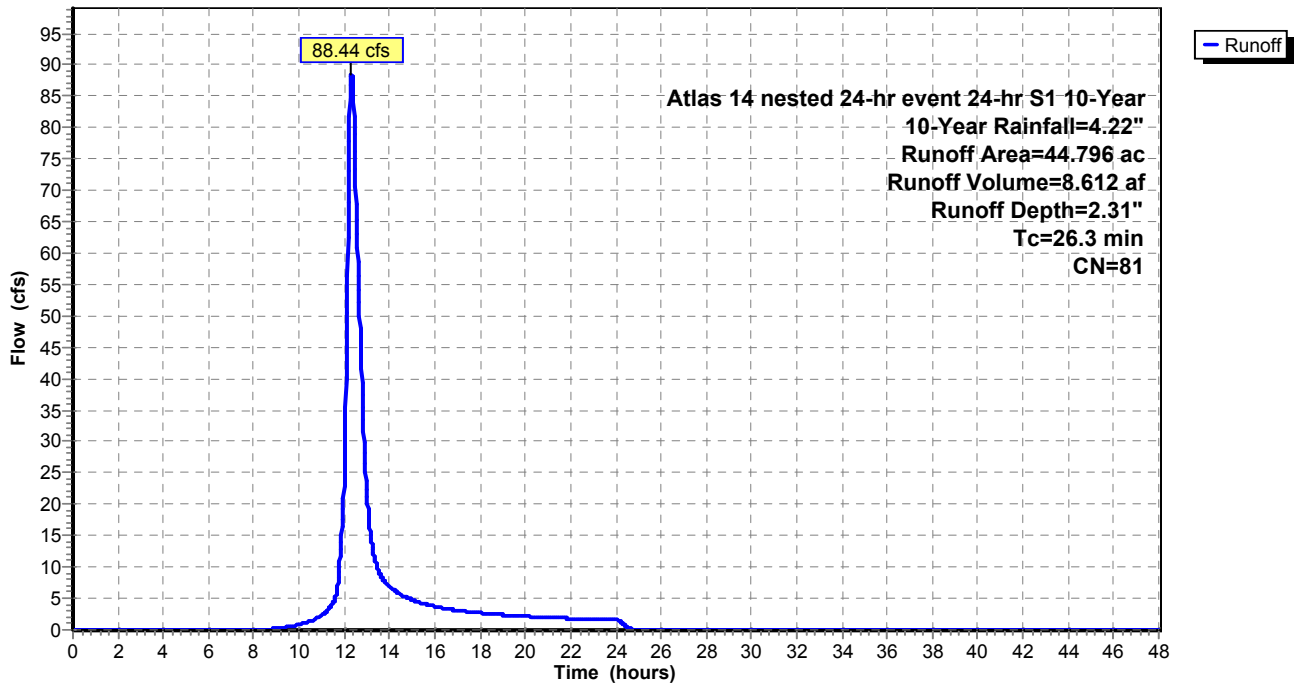
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 44.796	81	
44.796		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
26.3					Direct Entry,

Subcatchment 16S: Sub-basin 16

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 17S: Sub-basin 17

Runoff = 28.00 cfs @ 12.11 hrs, Volume= 1.833 af, Depth= 2.75"

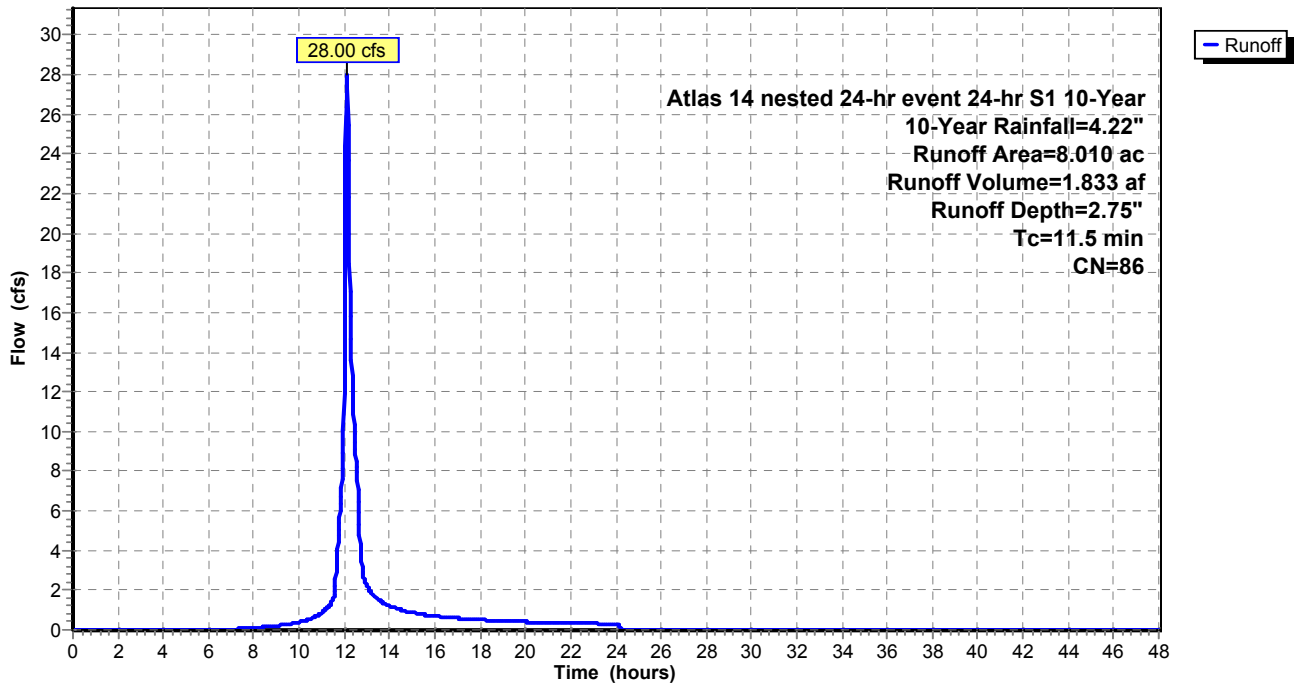
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 8.010	86	
8.010		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.5					Direct Entry,

Subcatchment 17S: Sub-basin 17

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 18S: Sub-basin 18

Runoff = 101.21 cfs @ 12.17 hrs, Volume= 7.627 af, Depth= 2.57"

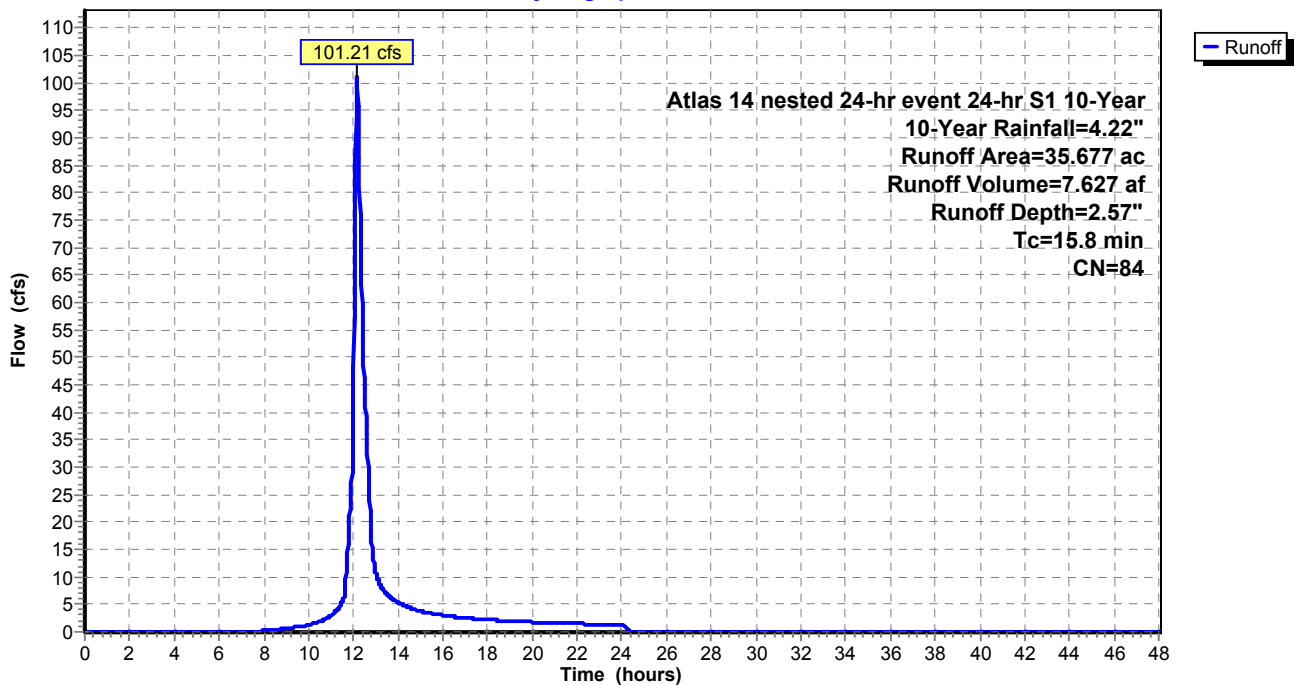
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 35.677	84	
35.677		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.8					Direct Entry,

Subcatchment 18S: Sub-basin 18

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 19S: Sub-basin 19

Runoff = 19.90 cfs @ 12.06 hrs, Volume= 1.094 af, Depth= 2.06"

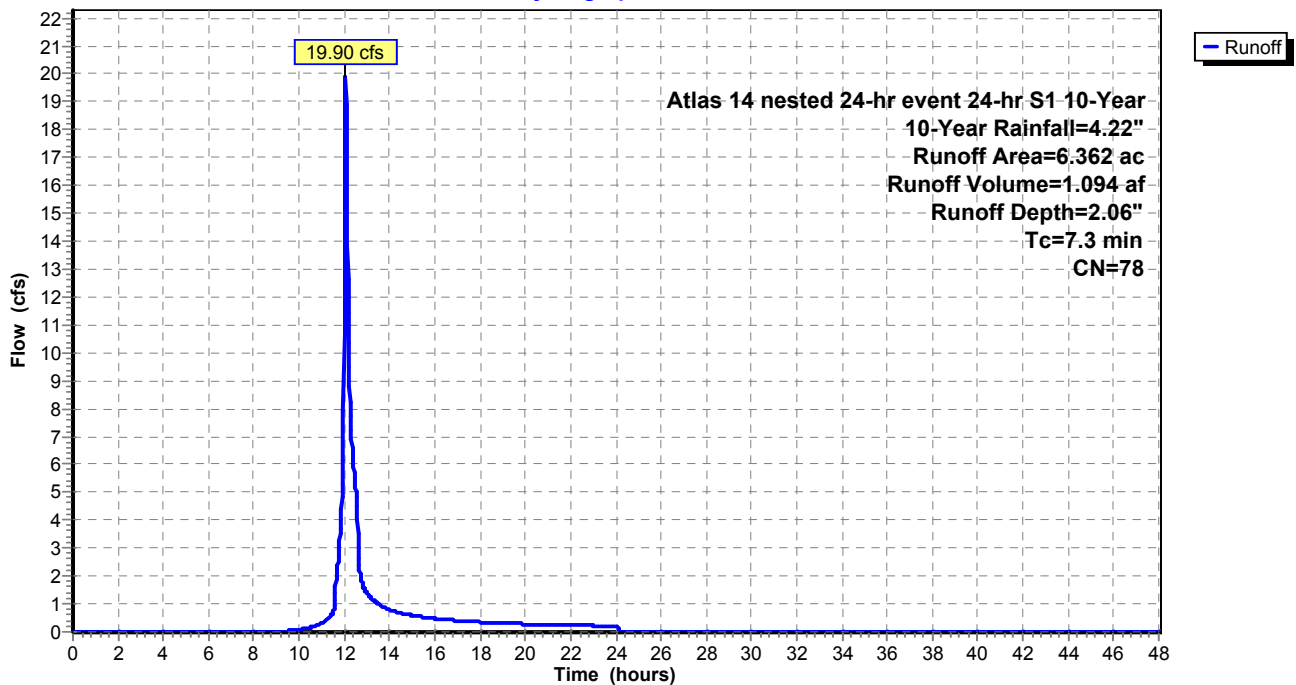
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 6.362	78	
6.362		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.3					Direct Entry,

Subcatchment 19S: Sub-basin 19

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 20S: Sub-basin 20

Runoff = 44.95 cfs @ 12.19 hrs, Volume= 3.517 af, Depth= 2.65"

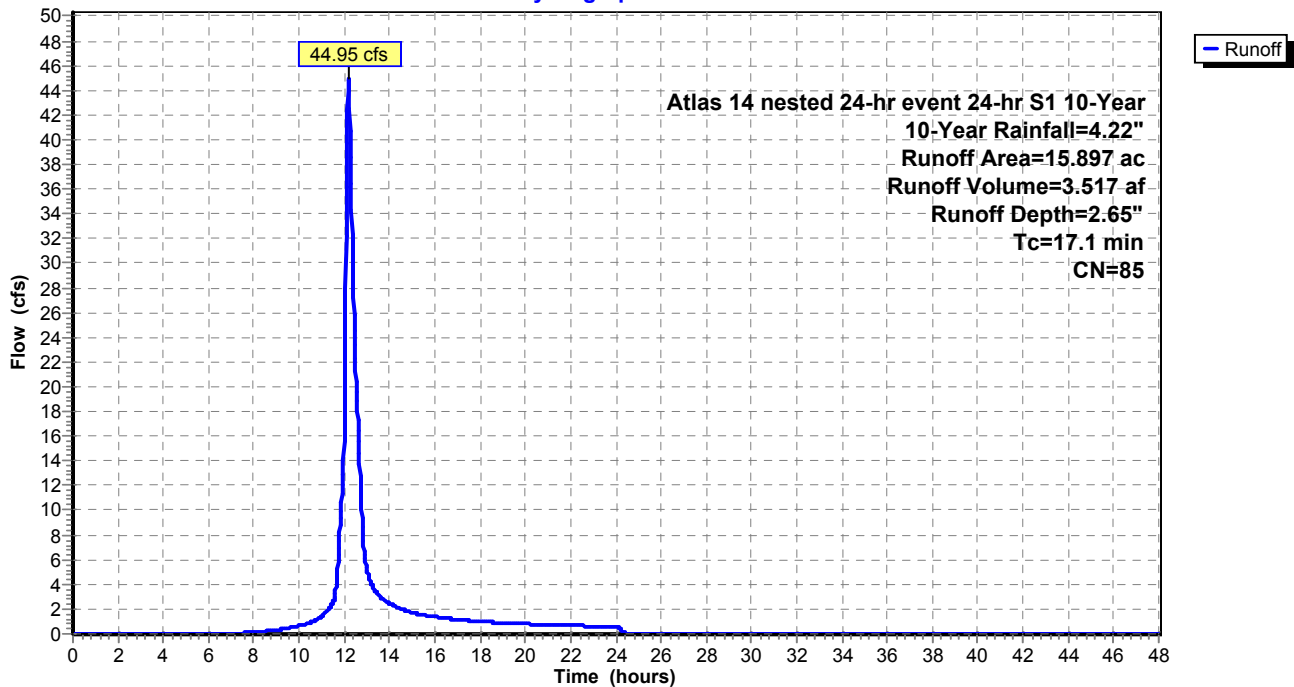
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 15.897	85	
15.897		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
17.1					Direct Entry,

Subcatchment 20S: Sub-basin 20

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 21S: Sub-basin 21

Runoff = 29.08 cfs @ 12.10 hrs, Volume= 1.865 af, Depth= 3.13"

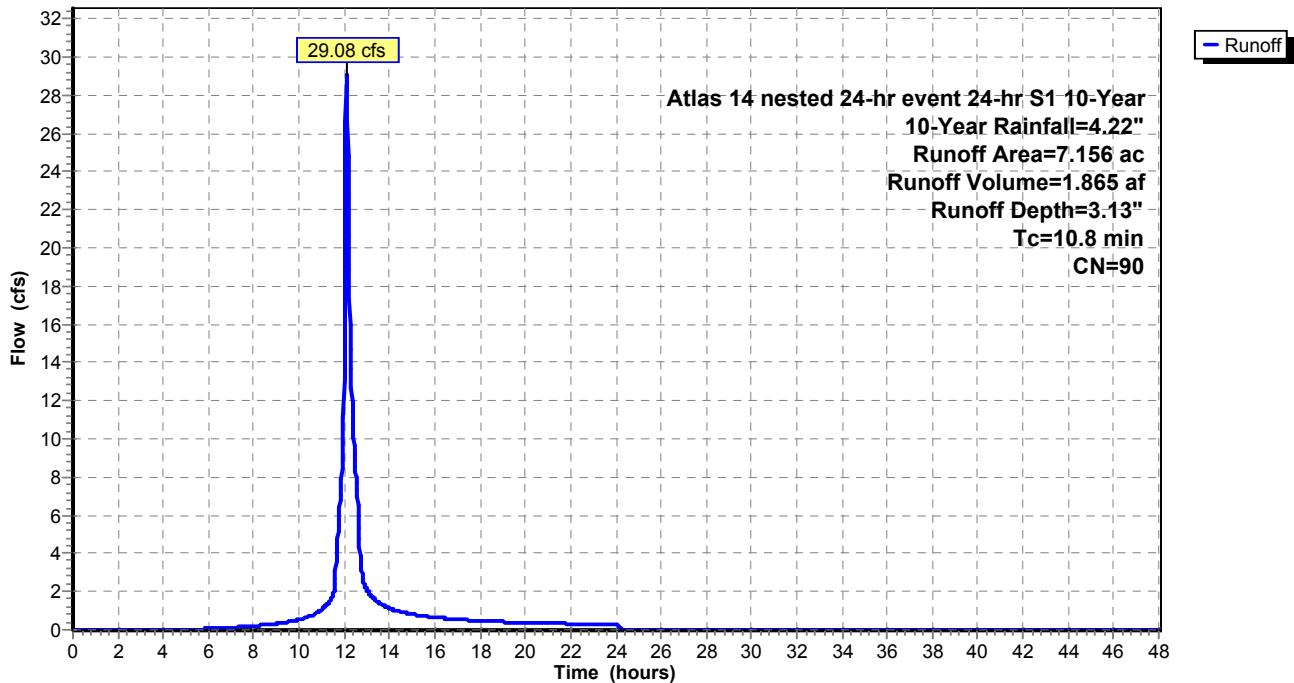
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 7.156	90	
7.156		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.8					Direct Entry,

Subcatchment 21S: Sub-basin 21

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 22S: Sub-basin 22

Runoff = 63.21 cfs @ 12.23 hrs, Volume= 5.291 af, Depth= 2.39"

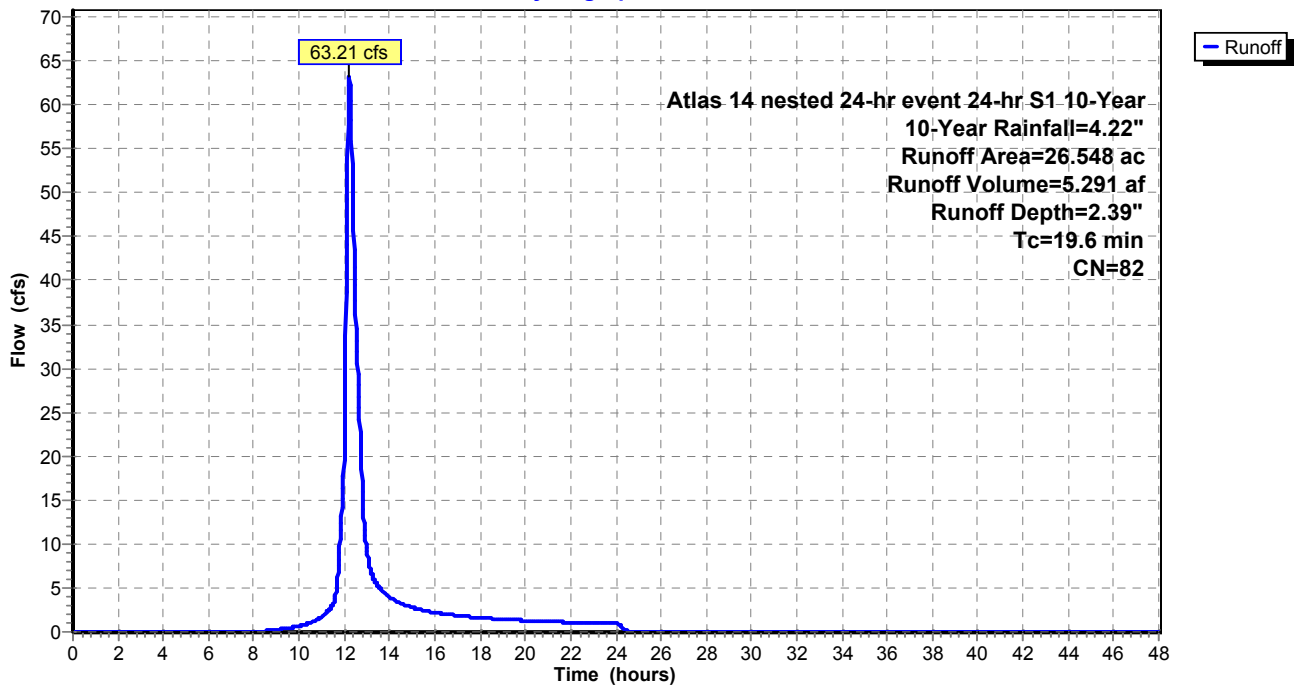
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 26.548	82	
26.548		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.6					Direct Entry,

Subcatchment 22S: Sub-basin 22

Hydrograph



Summary for Subcatchment 23S: Sub-basin 23

Runoff = 62.35 cfs @ 12.08 hrs, Volume= 3.836 af, Depth= 3.33"

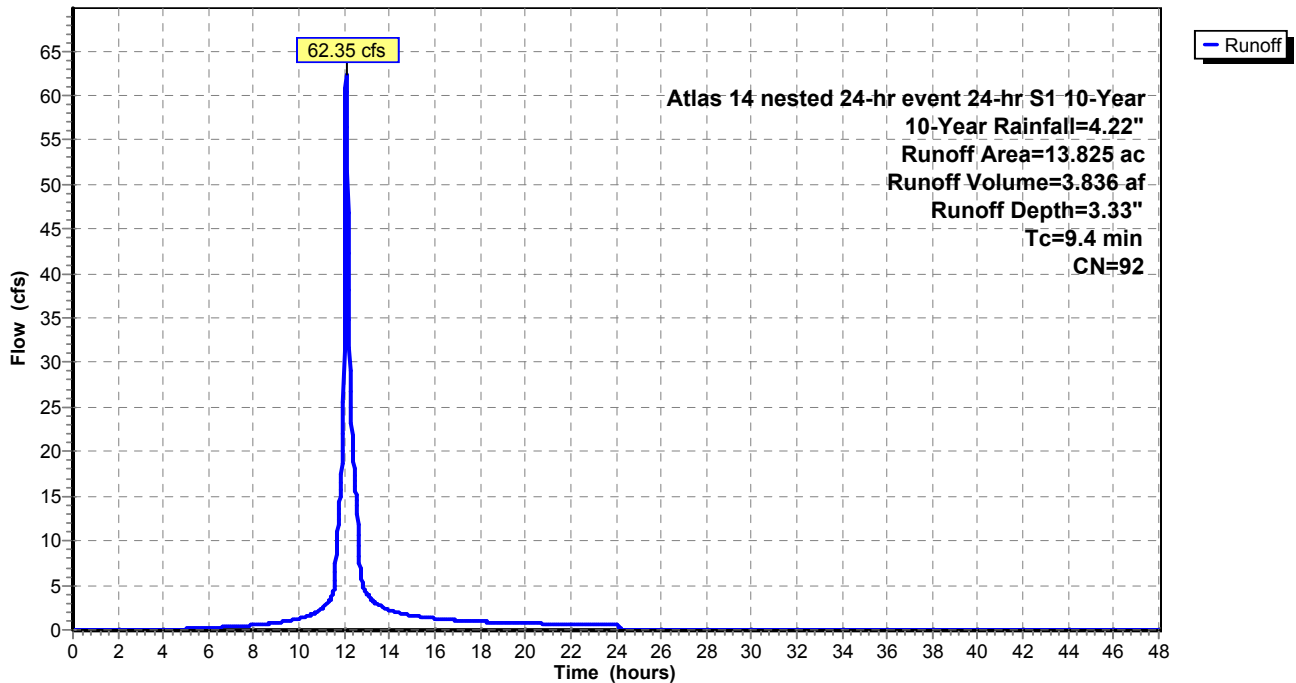
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 13.825	92	
13.825		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.4					Direct Entry,

Subcatchment 23S: Sub-basin 23

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 24S: Sub-basin 24

Runoff = 37.26 cfs @ 12.22 hrs, Volume= 3.071 af, Depth= 2.65"

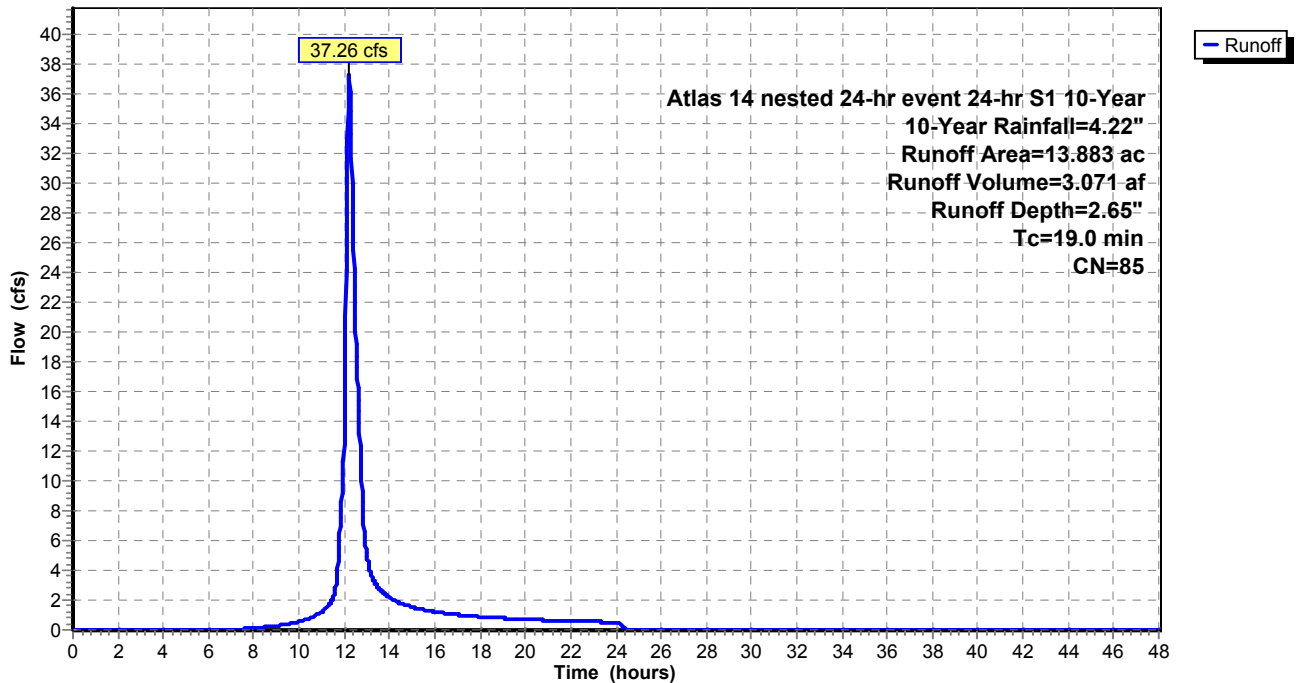
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 13.883	85	
13.883		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.0					Direct Entry,

Subcatchment 24S: Sub-basin 24

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 25S: Sub-basin 25

Runoff = 11.10 cfs @ 12.28 hrs, Volume= 1.017 af, Depth= 1.83"

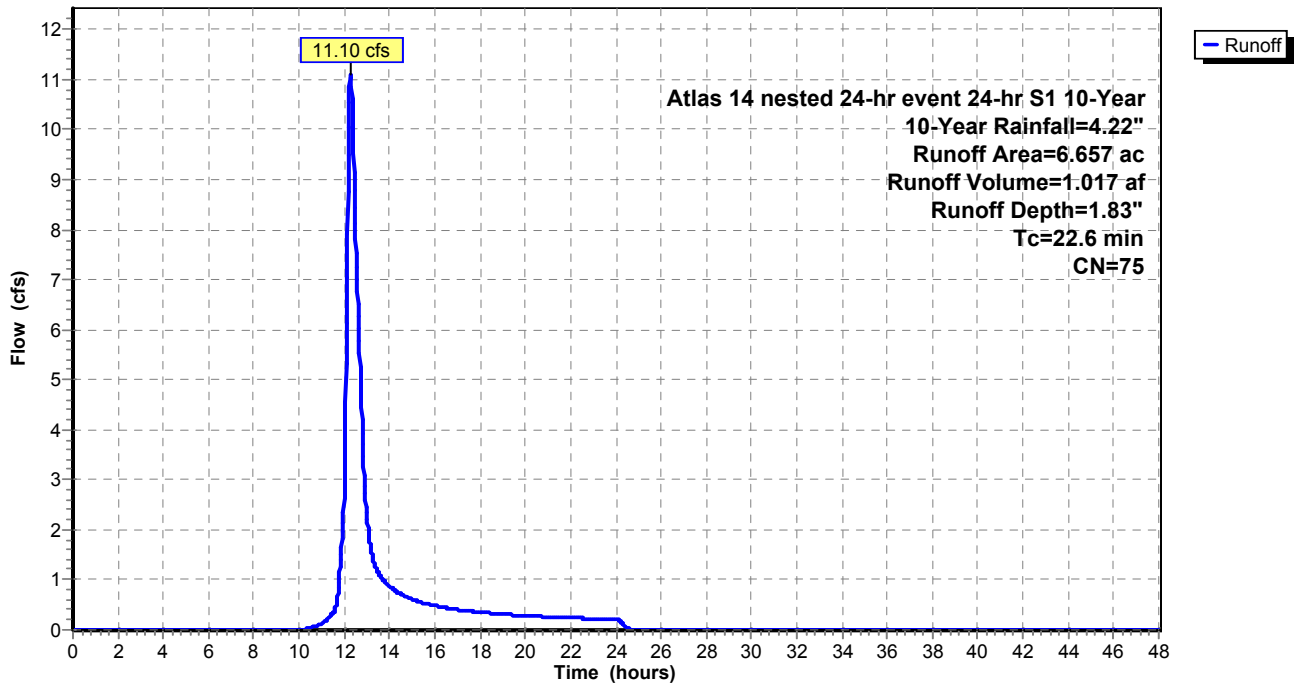
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 6.657	75	
6.657		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
22.6					Direct Entry,

Subcatchment 25S: Sub-basin 25

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 26S: Sub-basin 26

Runoff = 0.64 cfs @ 12.56 hrs, Volume= 0.084 af, Depth= 1.22"

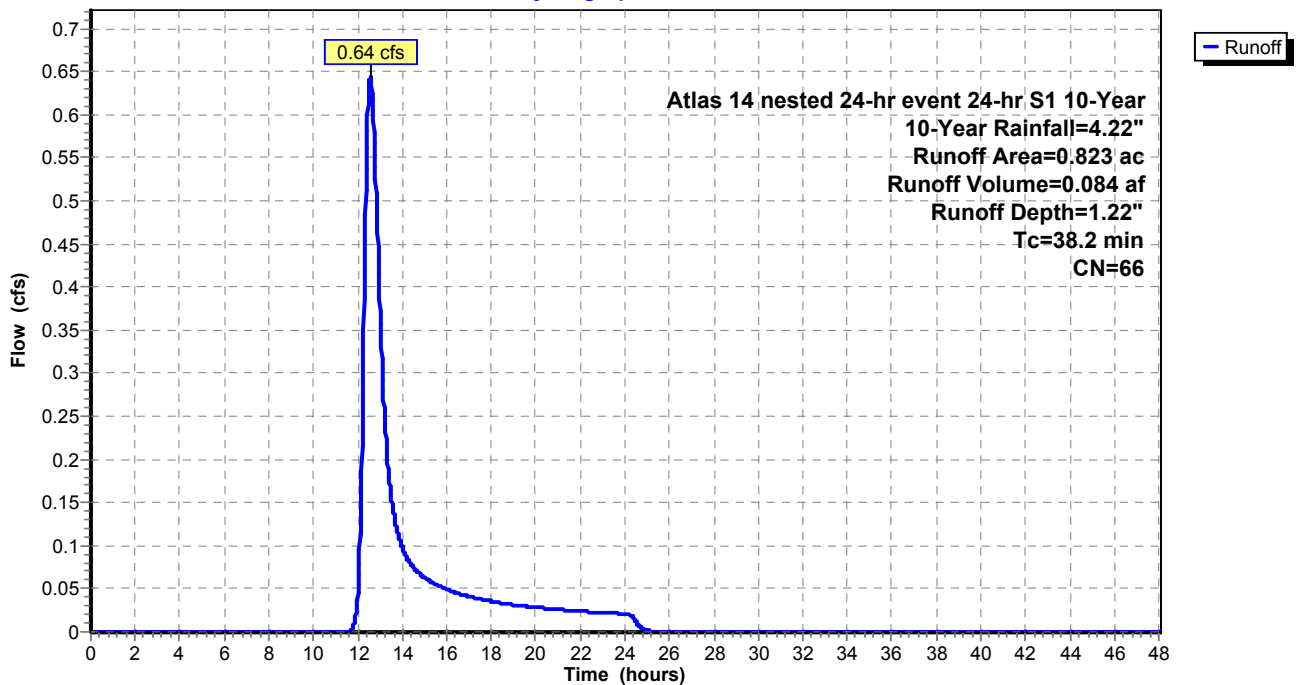
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 0.823	66	
0.823		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
38.2					Direct Entry,

Subcatchment 26S: Sub-basin 26

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 27S: Sub-basin 27

Runoff = 2.67 cfs @ 12.15 hrs, Volume= 0.207 af, Depth= 1.22"

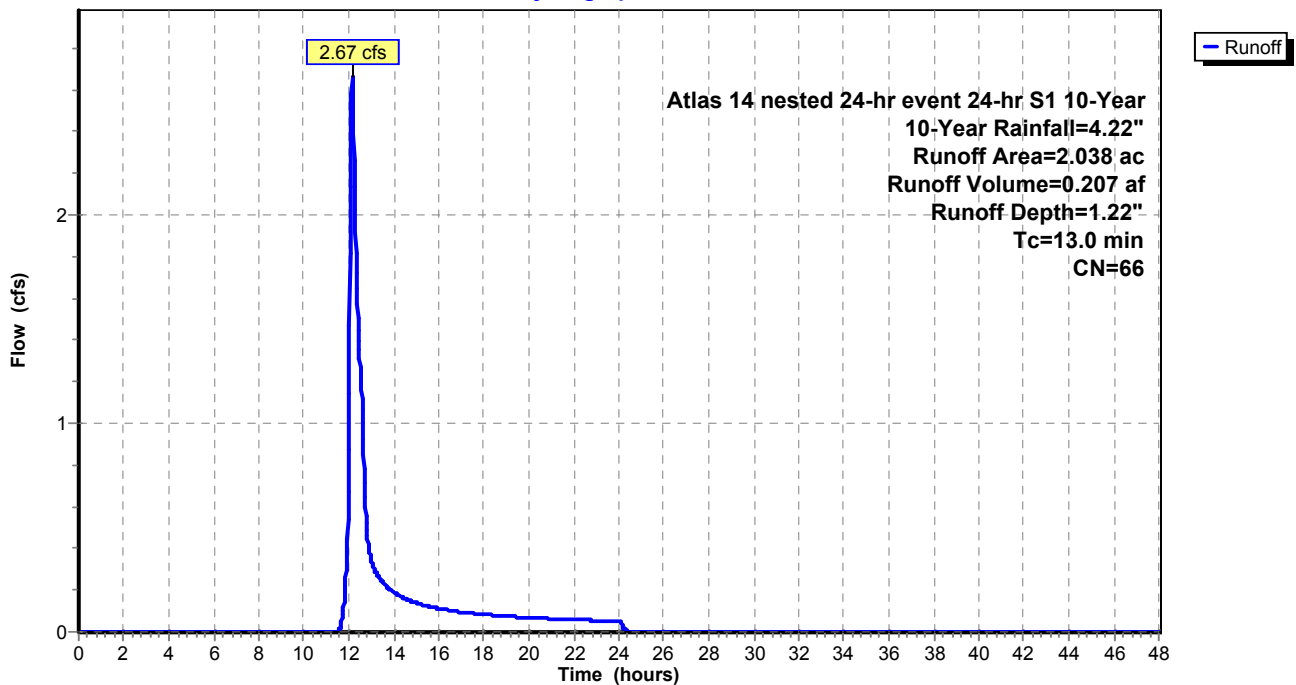
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 2.038	66	
2.038		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.0					Direct Entry,

Subcatchment 27S: Sub-basin 27

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 28S: Sub-basin 28

Runoff = 0.36 cfs @ 12.70 hrs, Volume= 0.094 af, Depth= 0.19"

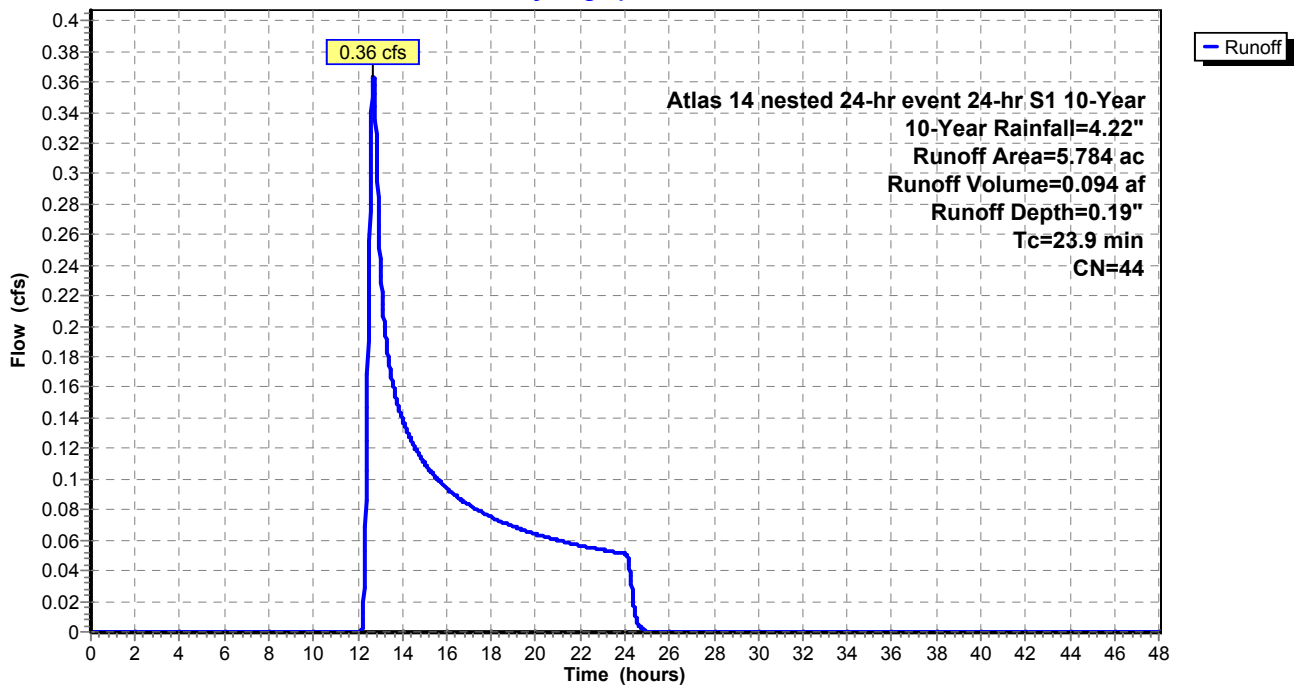
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 5.784	44	
5.784		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.9					Direct Entry,

Subcatchment 28S: Sub-basin 28

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 29S: Sub-basin 29

Runoff = 0.01 cfs @ 14.44 hrs, Volume= 0.007 af, Depth= 0.07"

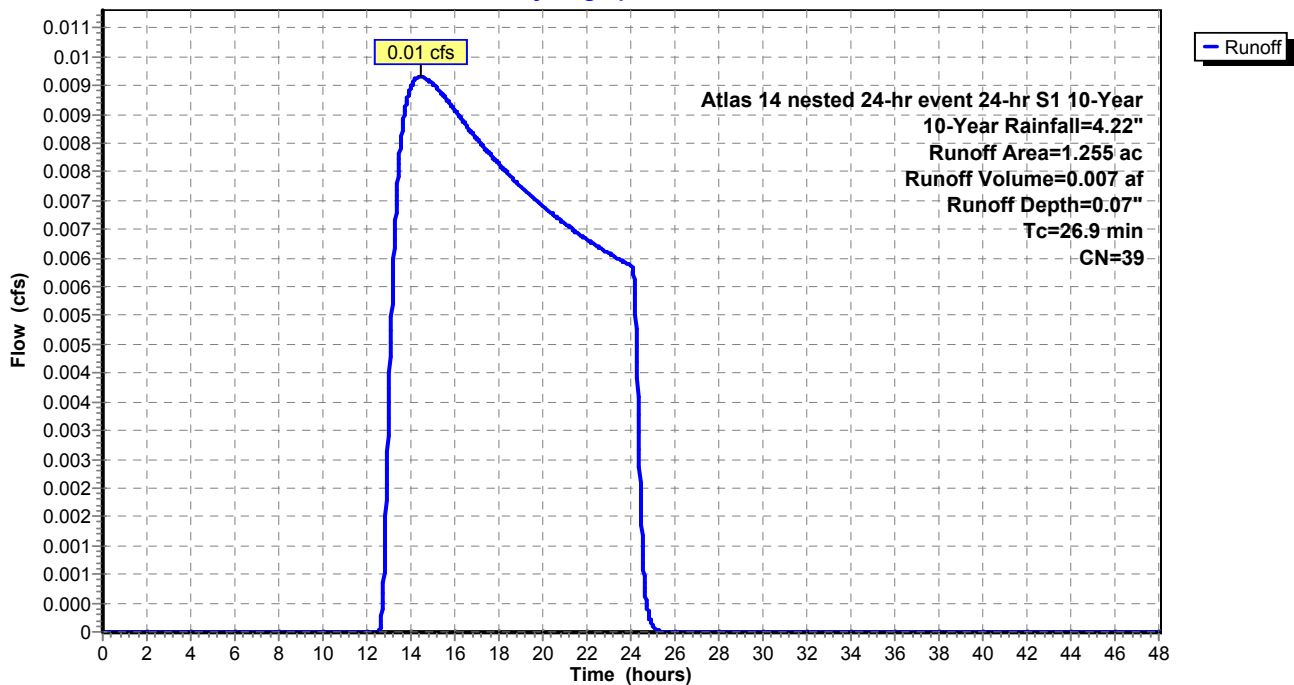
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 1.255	39	
1.255		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
26.9					Direct Entry,

Subcatchment 29S: Sub-basin 29

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 30S: Sub-basin 30

Runoff = 0.75 cfs @ 13.19 hrs, Volume= 0.366 af, Depth= 0.14"

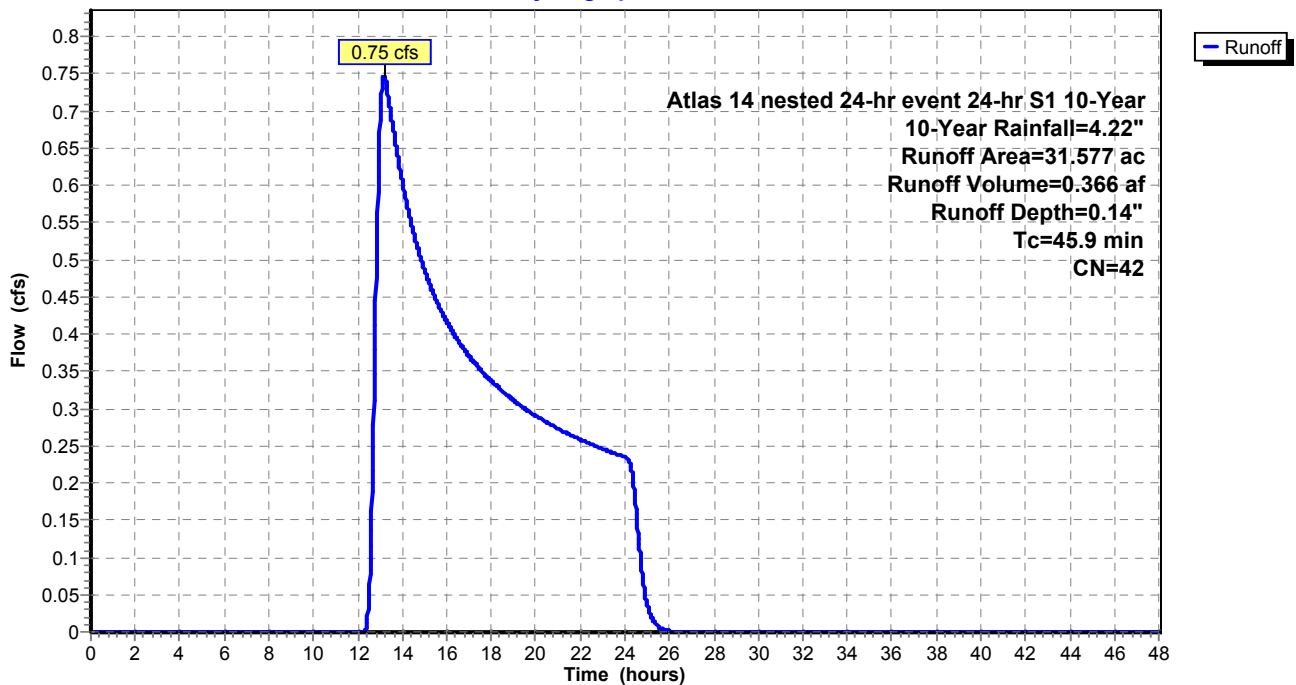
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 31.577	42	
31.577		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
45.9					Direct Entry,

Subcatchment 30S: Sub-basin 30

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 31S: Sub-basin 31

Runoff = 0.01 cfs @ 14.53 hrs, Volume= 0.005 af, Depth= 0.07"

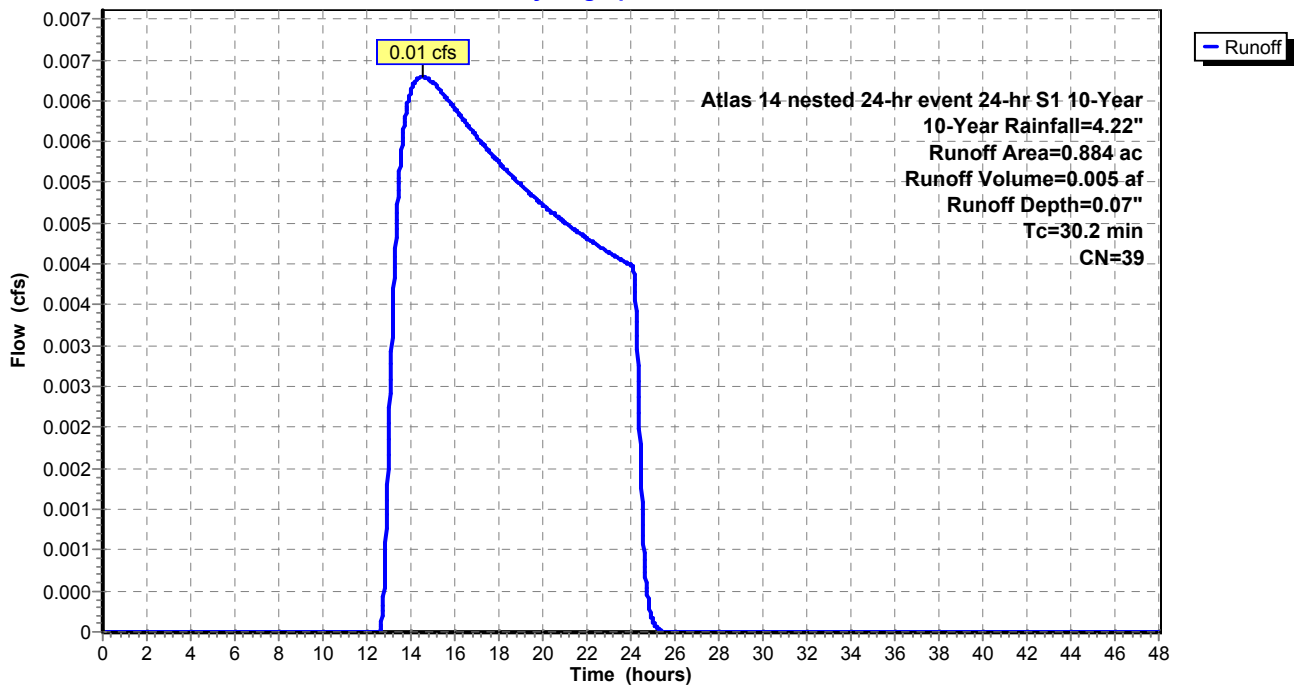
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 0.884	39	
0.884		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.2					Direct Entry,

Subcatchment 31S: Sub-basin 31

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 32S: Sub-basin 32

Runoff = 0.01 cfs @ 14.44 hrs, Volume= 0.005 af, Depth= 0.07"

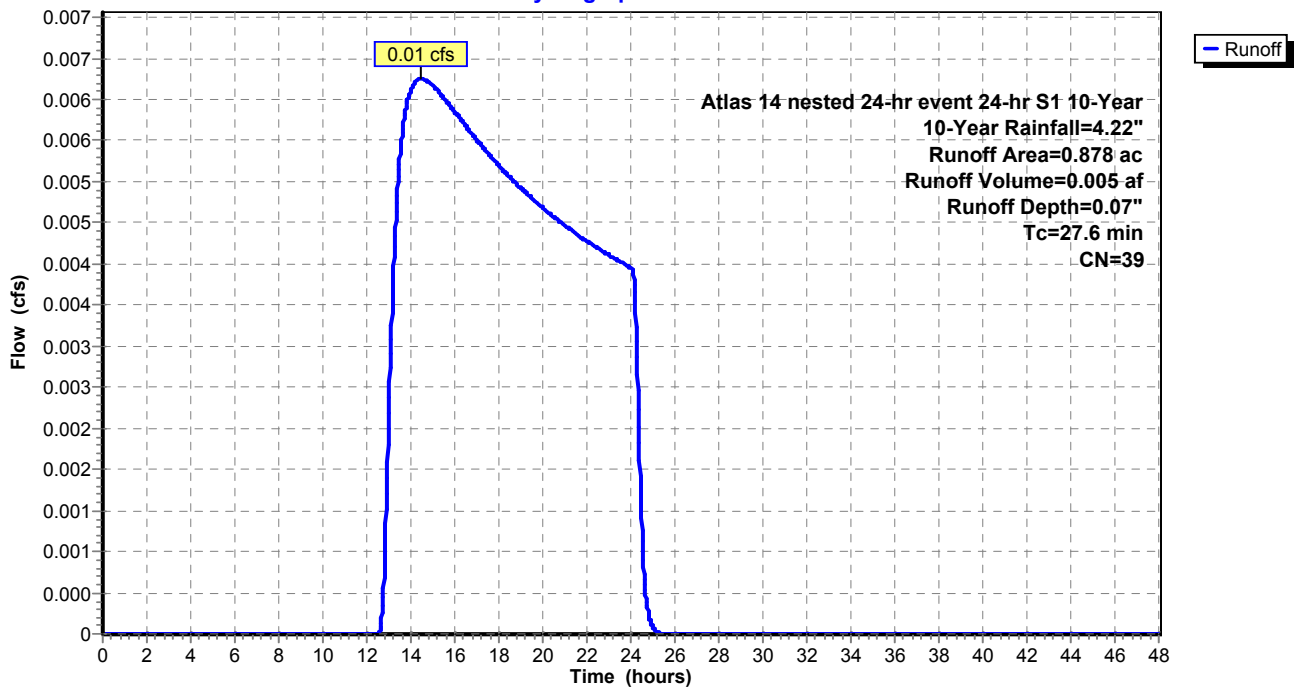
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 0.878	39	
0.878		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.6					Direct Entry,

Subcatchment 32S: Sub-basin 32

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 33S: Sub-basin 33

Runoff = 0.49 cfs @ 12.58 hrs, Volume= 0.088 af, Depth= 0.33"

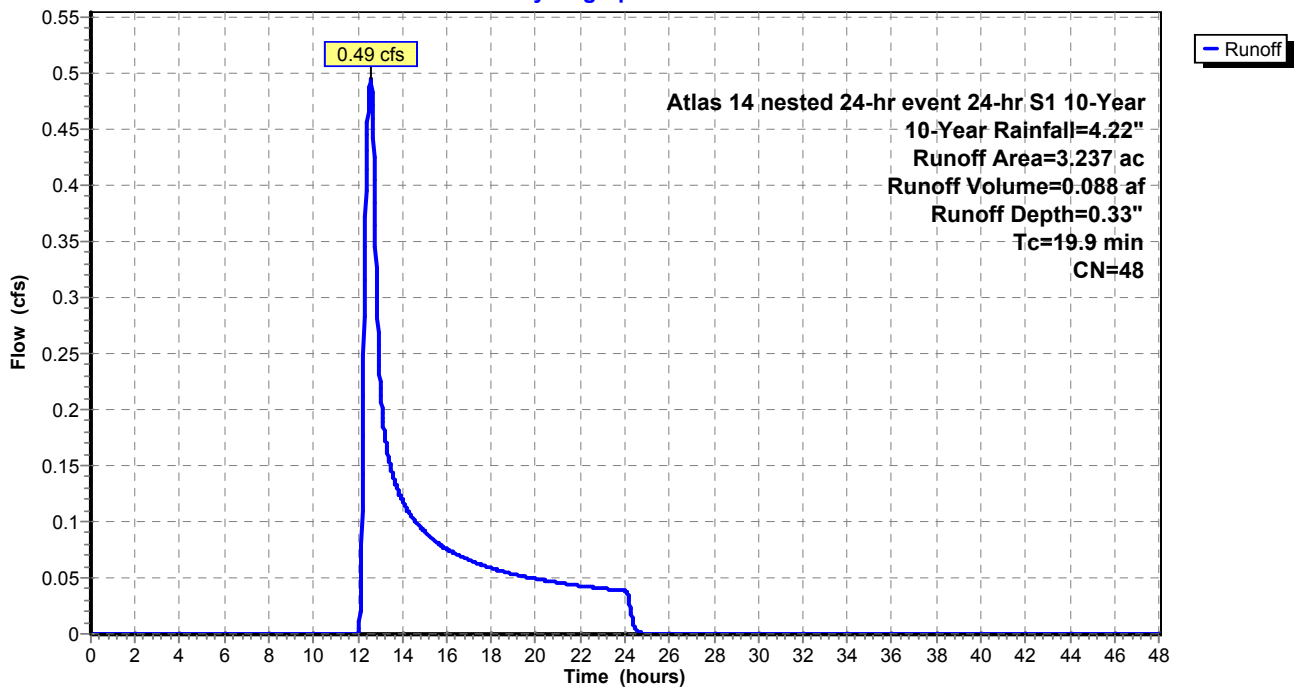
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 3.237	48	
3.237		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.9					Direct Entry,

Subcatchment 33S: Sub-basin 33

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 34S: Sub-basin 34

Runoff = 0.23 cfs @ 12.38 hrs, Volume= 0.038 af, Depth= 0.36"

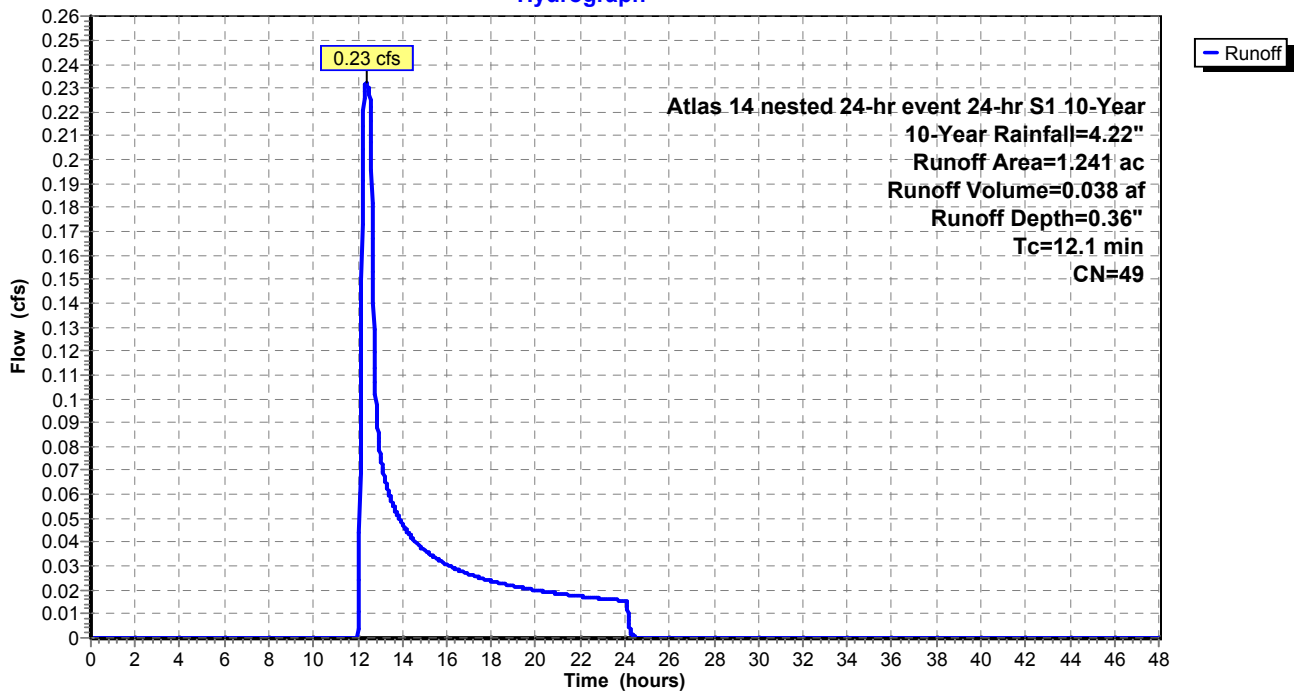
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 1.241	49	
1.241		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.1					Direct Entry,

Subcatchment 34S: Sub-basin 34

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 35S: Sub-basin 35

Runoff = 0.33 cfs @ 12.63 hrs, Volume= 0.086 af, Depth= 0.17"

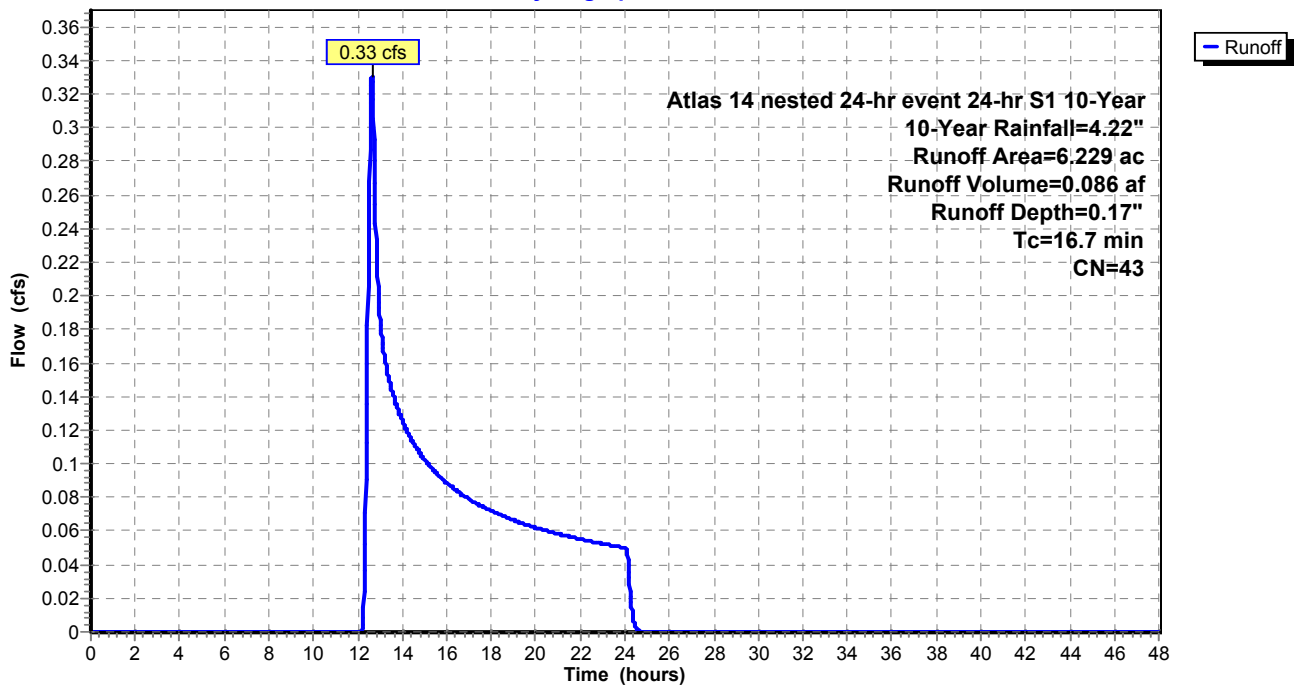
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 6.229	43	
6.229		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.7					Direct Entry,

Subcatchment 35S: Sub-basin 35

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 36S: Sub-basin 36

Runoff = 14.13 cfs @ 12.70 hrs, Volume= 2.002 af, Depth= 2.14"

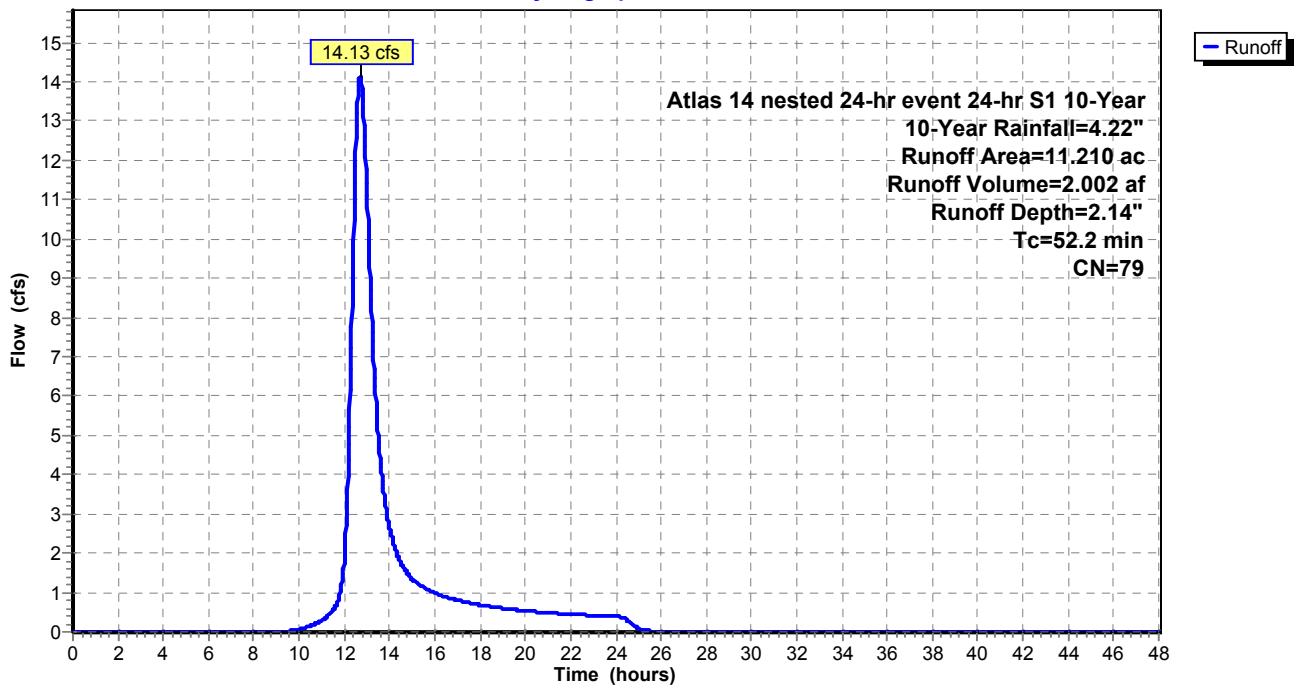
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 11.210	79	
11.210		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
52.2					Direct Entry,

Subcatchment 36S: Sub-basin 36

Hydrograph



Existing Conditions_HydrAtlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

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Summary for Subcatchment 83S: County Road H Subbasin Redirected After Regrading

Runoff = 13.12 cfs @ 12.22 hrs, Volume= 1.089 af, Depth= 2.22"

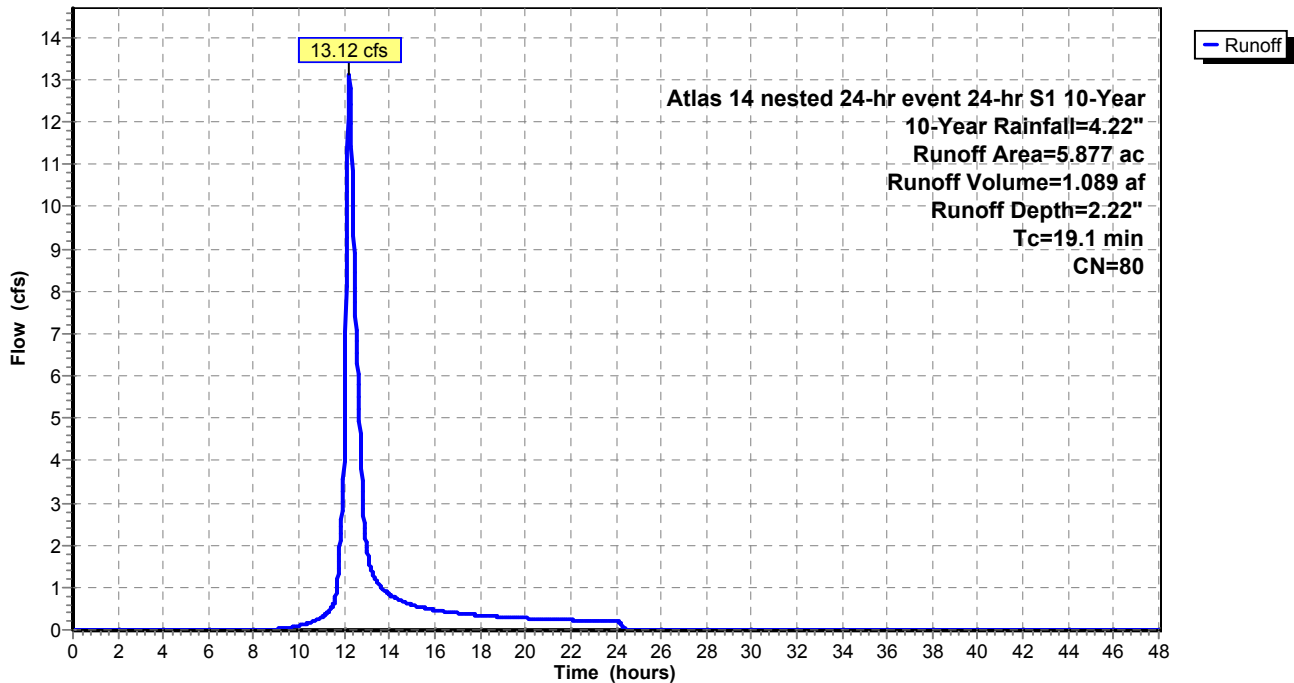
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 10-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 5.877	80	
5.877		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.1					Direct Entry,

Subcatchment 83S: County Road H Subbasin Redirected After Regrading

Hydrograph



Summary for Reach 37R: Outfall of SB 2, 3, 7

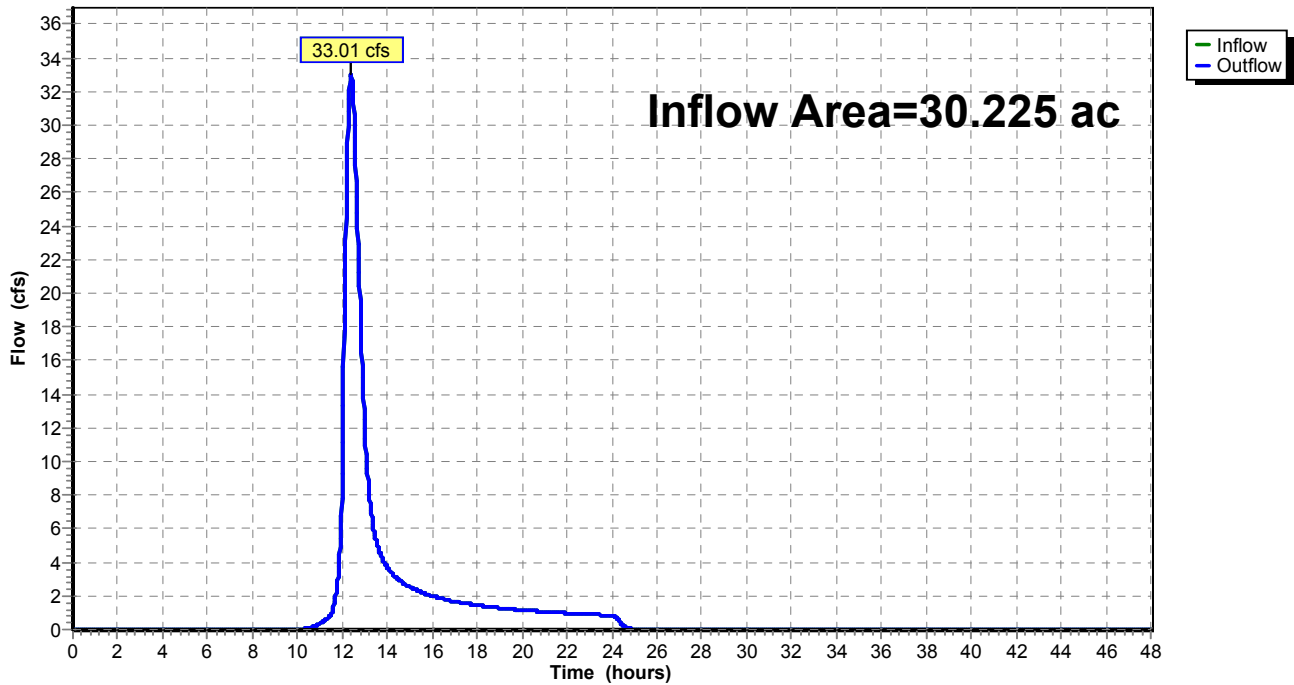
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 30.225 ac, 0.00% Impervious, Inflow Depth = 1.56" for 10-Year event
Inflow = 33.01 cfs @ 12.39 hrs, Volume= 3.922 af
Outflow = 33.01 cfs @ 12.39 hrs, Volume= 3.922 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 37R: Outfall of SB 2, 3, 7

Hydrograph



Summary for Reach 39R: Outfall of SB 1, 4, 5, 6, 9, 10, 11, 36

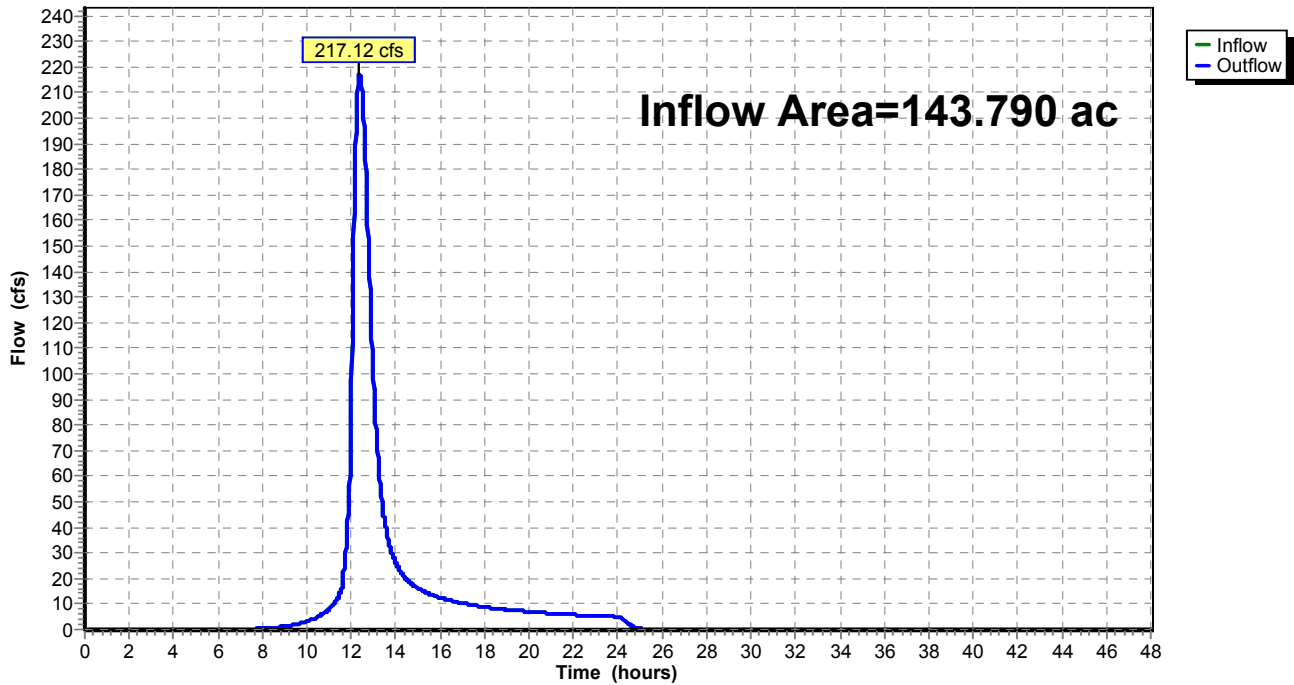
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 143.790 ac, 0.00% Impervious, Inflow Depth = 2.40" for 10-Year event
Inflow = 217.12 cfs @ 12.37 hrs, Volume= 28.714 af
Outflow = 217.12 cfs @ 12.37 hrs, Volume= 28.714 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 39R: Outfall of SB 1, 4, 5, 6, 9, 10, 11, 36

Hydrograph



Summary for Reach 40R: 60 in SB 4

[52] Hint: Inlet/Outlet conditions not evaluated

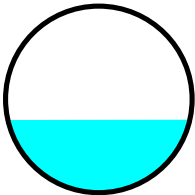
[65] Warning: Inlet elevation not specified

Inflow Area = 143.790 ac, 0.00% Impervious, Inflow Depth = 2.40" for 10-Year event
Inflow = 217.16 cfs @ 12.36 hrs, Volume= 28.714 af
Outflow = 217.12 cfs @ 12.37 hrs, Volume= 28.714 af, Atten= 0%, Lag= 0.5 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
Reference Flow= 162.87 cfs Estimated Depth= 2.02' Velocity= 21.86 fps
m= 1.400, c= 30.61 fps, dt= 1.2 min, dx= 718.0' / 1 = 718.0', K= 0.4 min, X= 0.470
Max. Velocity= 31.08 fps, Min. Travel Time= 0.4 min
Avg. Velocity = 30.61 fps, Avg. Travel Time= 0.4 min

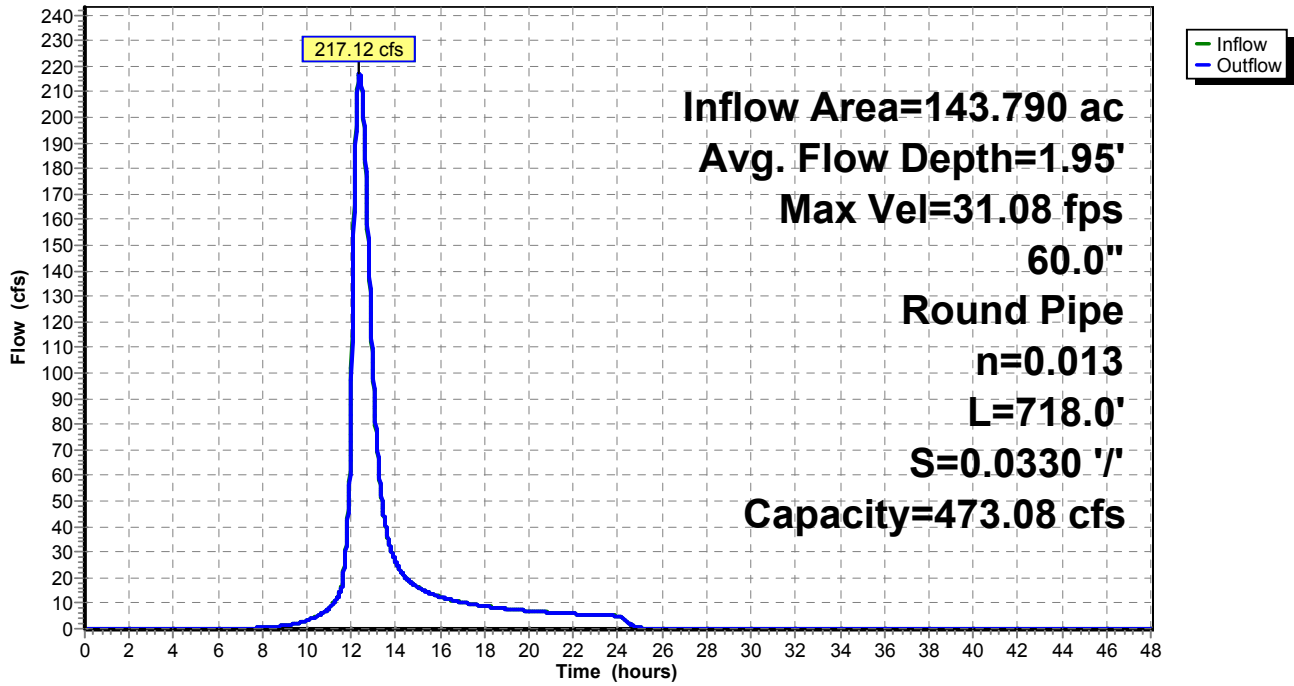
Peak Storage= 5,092 cf @ 12.36 hrs
Average Depth at Peak Storage= 1.95'
Bank-Full Depth= 5.00' Flow Area= 19.6 sf, Capacity= 473.08 cfs

60.0" Round Pipe
n= 0.013
Length= 718.0' Slope= 0.0330 '/'
Inlet Invert= 0.00', Outlet Invert= -23.69'



Reach 40R: 60 in SB 4

Hydrograph



Summary for Reach 41R: Channel in SB 9, 10

[65] Warning: Inlet elevation not specified

Inflow Area = 9.296 ac, 0.00% Impervious, Inflow Depth = 2.39" for 10-Year event
Inflow = 27.07 cfs @ 12.13 hrs, Volume= 1.853 af
Outflow = 25.05 cfs @ 12.34 hrs, Volume= 1.853 af, Atten= 7%, Lag= 12.4 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
Reference Flow= 20.31 cfs Estimated Depth= 0.72' Velocity= 1.50 fps
m= 1.556, c= 2.33 fps, dt= 1.2 min, dx= 1,660.0' / 10 = 166.0', K= 1.2 min, X= 0.210
Max. Velocity= 9.14 fps, Min. Travel Time= 3.0 min
Avg. Velocity = 2.39 fps, Avg. Travel Time= 11.6 min

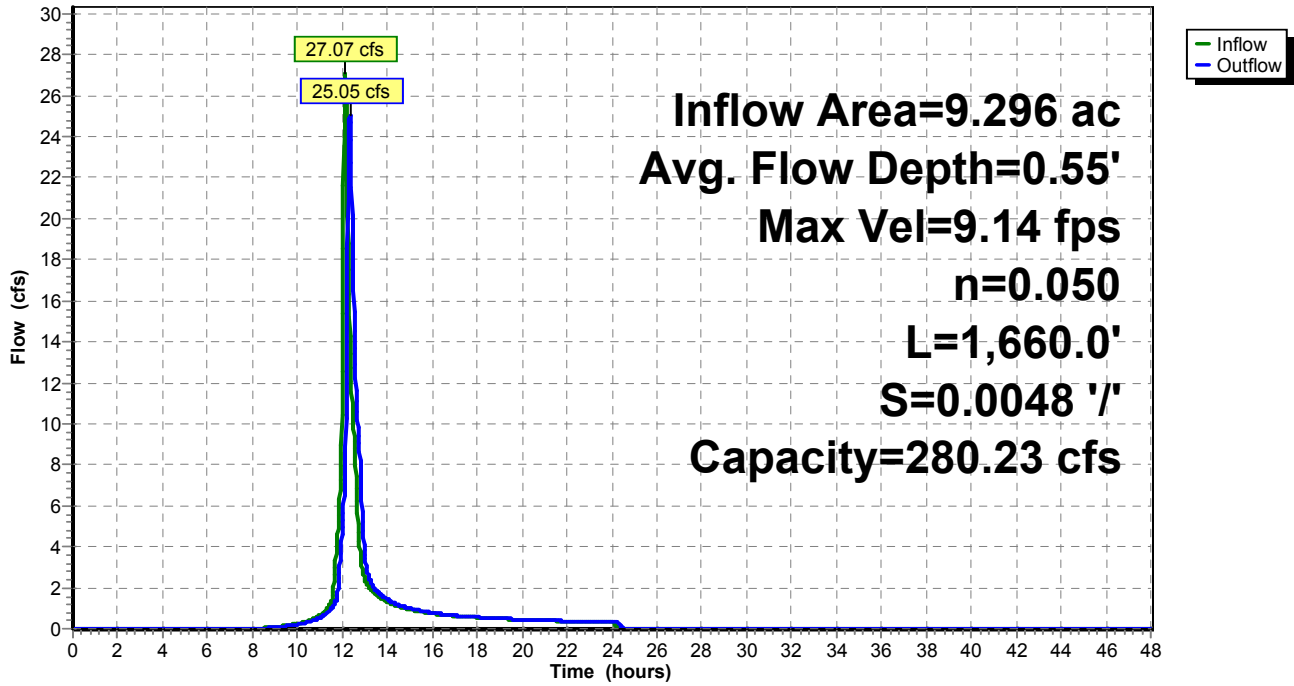
Peak Storage= 16,733 cf @ 12.25 hrs
Average Depth at Peak Storage= 0.55'
Bank-Full Depth= 3.00' Flow Area= 84.0 sf, Capacity= 280.23 cfs

16.00' x 3.00' deep channel, n= 0.050
Side Slope Z-value= 4.0 '/' Top Width= 40.00'
Length= 1,660.0' Slope= 0.0048 '/'
Inlet Invert= 0.00', Outlet Invert= -7.97'



Reach 41R: Channel in SB 9, 10

Hydrograph



Summary for Reach 46R: Channel SB1

[65] Warning: Inlet elevation not specified

Inflow Area = 15.328 ac, 0.00% Impervious, Inflow Depth = 2.84" for 10-Year event
Inflow = 47.16 cfs @ 12.18 hrs, Volume= 3.626 af
Outflow = 46.50 cfs @ 12.25 hrs, Volume= 3.626 af, Atten= 1%, Lag= 4.4 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
Reference Flow= 35.37 cfs Estimated Depth= 0.98' Velocity= 2.13 fps
m= 1.511, c= 3.22 fps, dt= 1.2 min, dx= 841.0' / 4 = 210.3', K= 1.1 min, X= 0.283
Max. Velocity= 6.34 fps, Min. Travel Time= 2.2 min
Avg. Velocity = 3.23 fps, Avg. Travel Time= 4.3 min

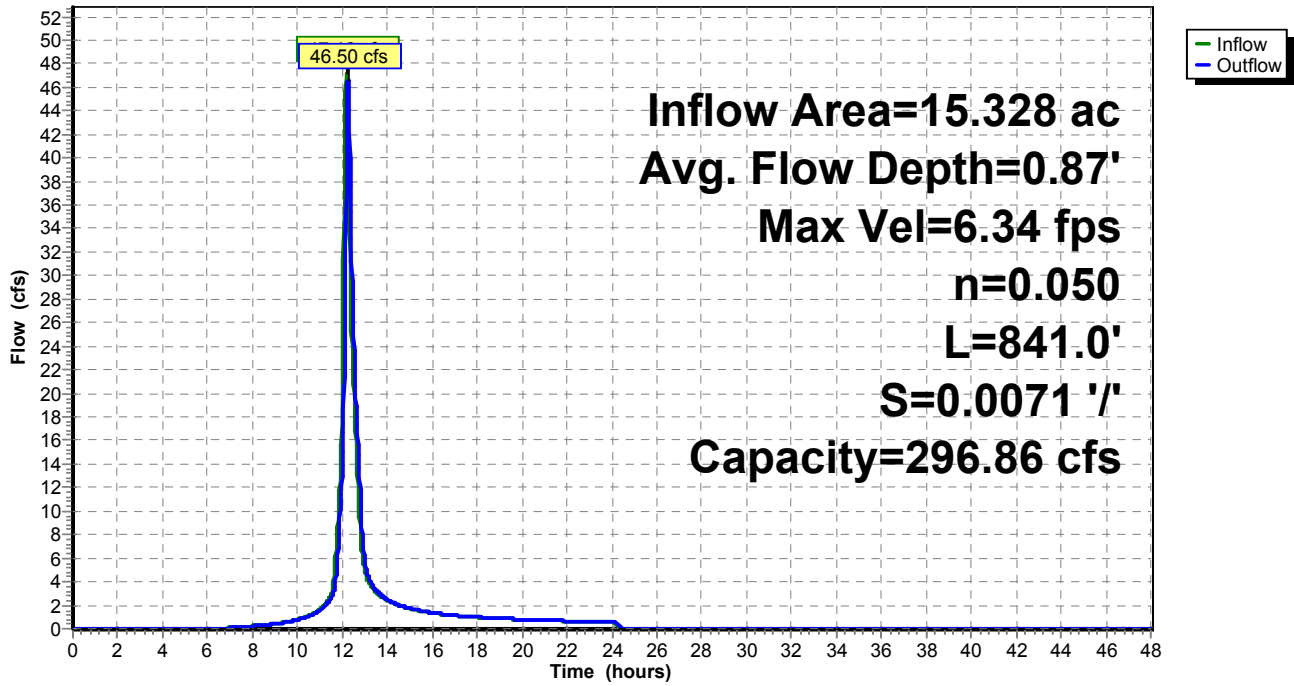
Peak Storage= 12,066 cf @ 12.22 hrs
Average Depth at Peak Storage= 0.87'
Bank-Full Depth= 3.00' Flow Area= 75.0 sf, Capacity= 296.86 cfs

13.00' x 3.00' deep channel, n= 0.050
Side Slope Z-value= 4.0 ' ' Top Width= 37.00'
Length= 841.0' Slope= 0.0071 ' '
Inlet Invert= 0.00', Outlet Invert= -5.97'



Reach 46R: Channel SB1

Hydrograph



Summary for Reach 48R: Outfall of SB 8, 13

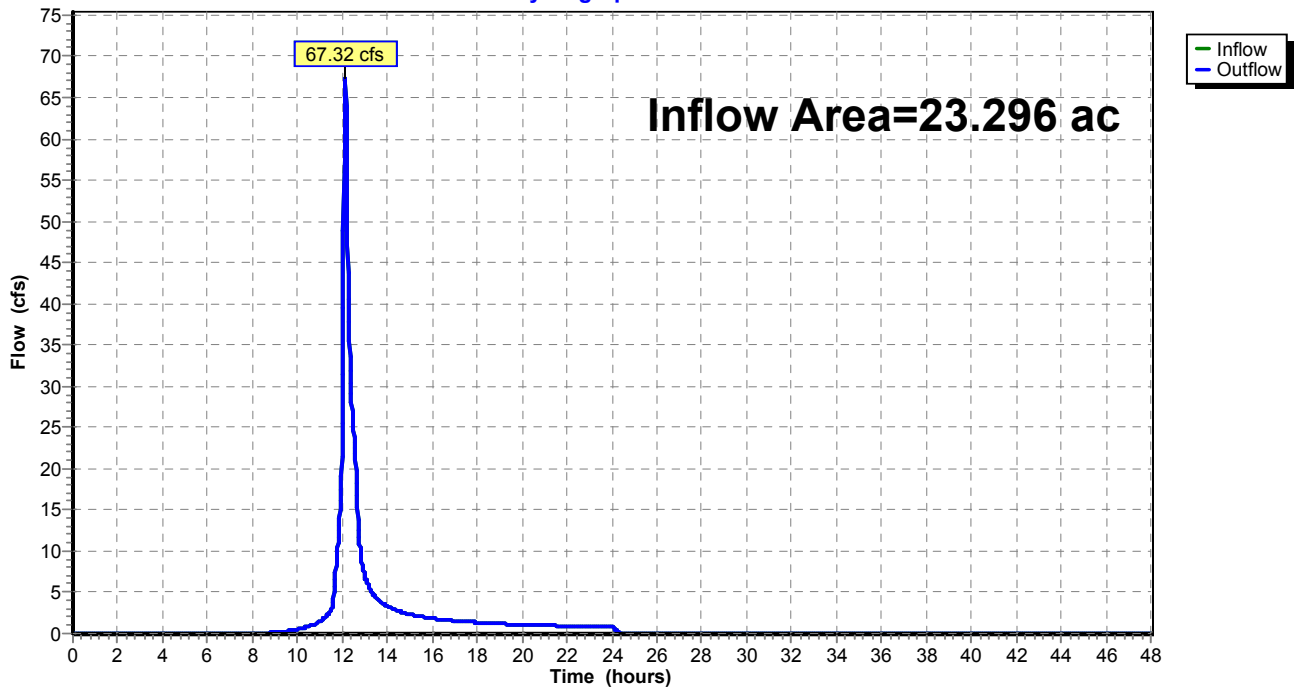
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 23.296 ac, 0.00% Impervious, Inflow Depth = 2.32" for 10-Year event
Inflow = 67.32 cfs @ 12.13 hrs, Volume= 4.495 af
Outflow = 67.32 cfs @ 12.13 hrs, Volume= 4.495 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 48R: Outfall of SB 8, 13

Hydrograph



Summary for Reach 49R: Channel SB8

[65] Warning: Inlet elevation not specified

Inflow Area = 21.017 ac, 0.00% Impervious, Inflow Depth = 2.31" for 10-Year event
Inflow = 66.85 cfs @ 12.08 hrs, Volume= 4.041 af
Outflow = 65.69 cfs @ 12.13 hrs, Volume= 4.041 af, Atten= 2%, Lag= 2.5 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
Reference Flow= 50.14 cfs Estimated Depth= 0.96' Velocity= 2.28 fps
m= 1.546, c= 3.52 fps, dt= 1.2 min, dx= 521.0' / 2 = 260.5', K= 1.2 min, X= 0.345
Max. Velocity= 6.23 fps, Min. Travel Time= 1.4 min
Avg. Velocity = 3.53 fps, Avg. Travel Time= 2.5 min

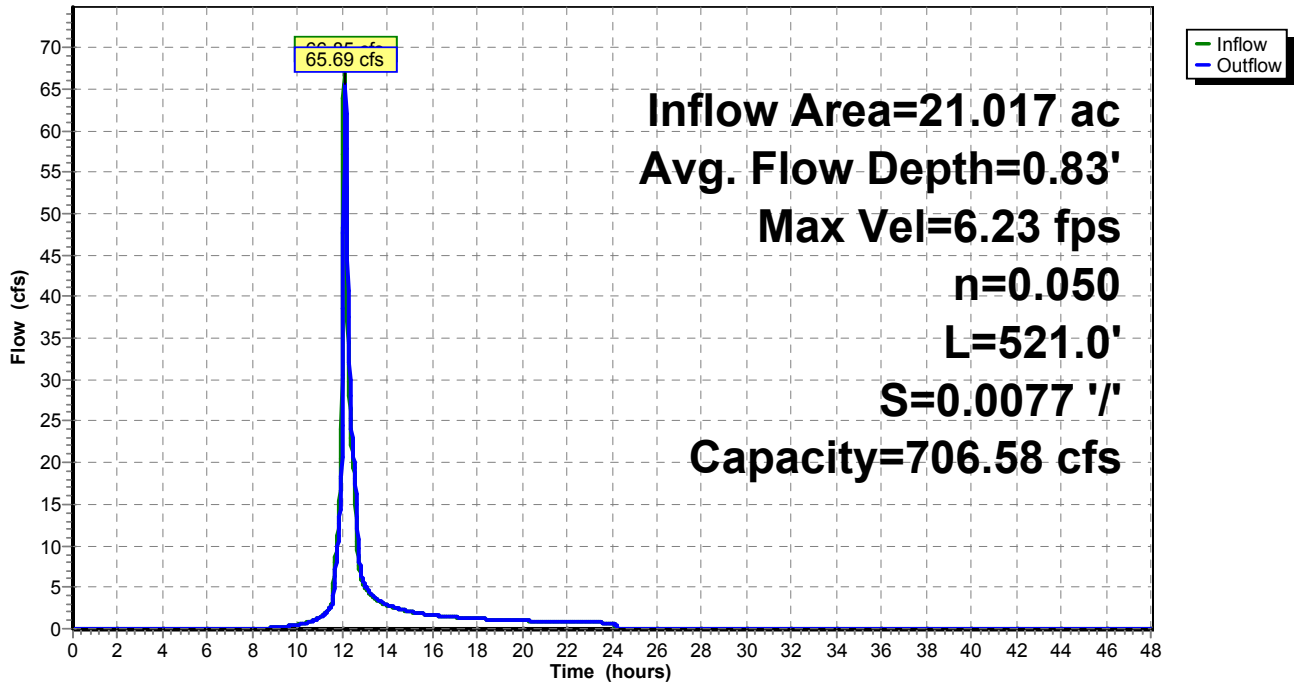
Peak Storage= 9,639 cf @ 12.11 hrs
Average Depth at Peak Storage= 0.83'
Bank-Full Depth= 4.00' Flow Area= 140.0 sf, Capacity= 706.58 cfs

19.00' x 4.00' deep channel, n= 0.050
Side Slope Z-value= 4.0 ' ' Top Width= 51.00'
Length= 521.0' Slope= 0.0077 ' '
Inlet Invert= 0.00', Outlet Invert= -4.01'



Reach 49R: Channel SB8

Hydrograph



Summary for Reach 50R: Outfall of SB 12

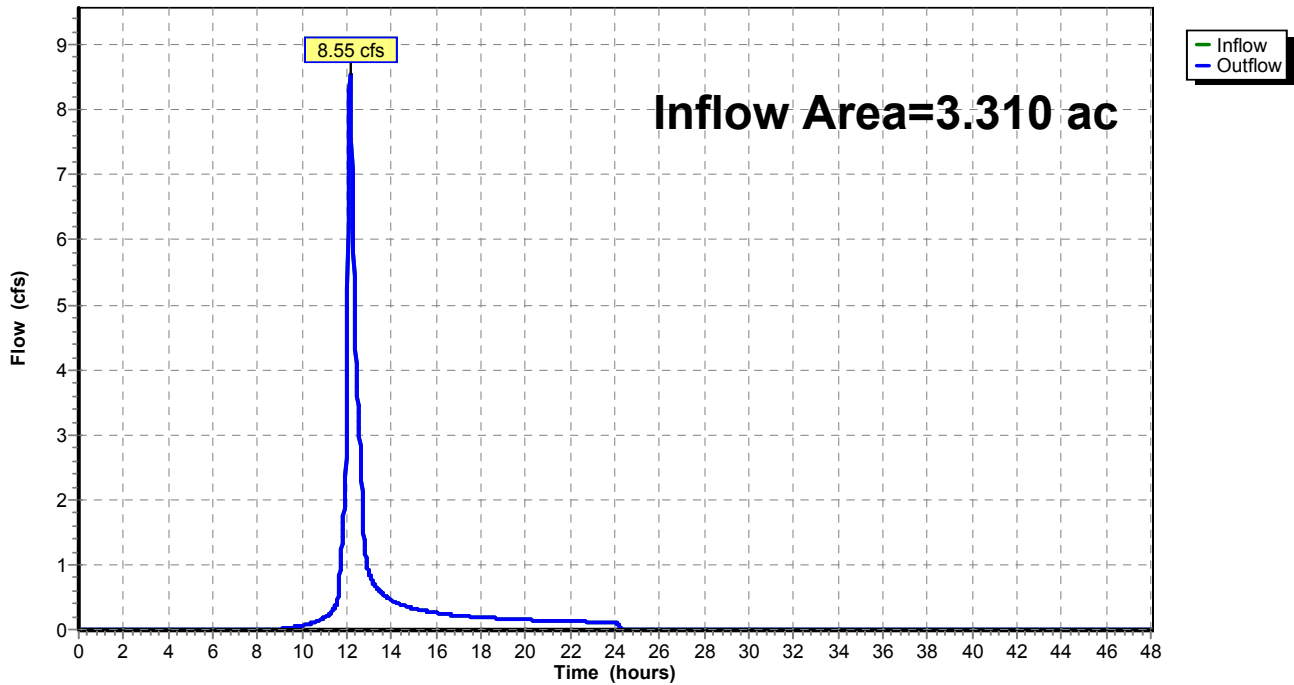
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 3.310 ac, 0.00% Impervious, Inflow Depth = 2.22" for 10-Year event
Inflow = 8.55 cfs @ 12.15 hrs, Volume= 0.613 af
Outflow = 8.55 cfs @ 12.15 hrs, Volume= 0.613 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 50R: Outfall of SB 12

Hydrograph



Summary for Reach 51R: Outfall of SB 14

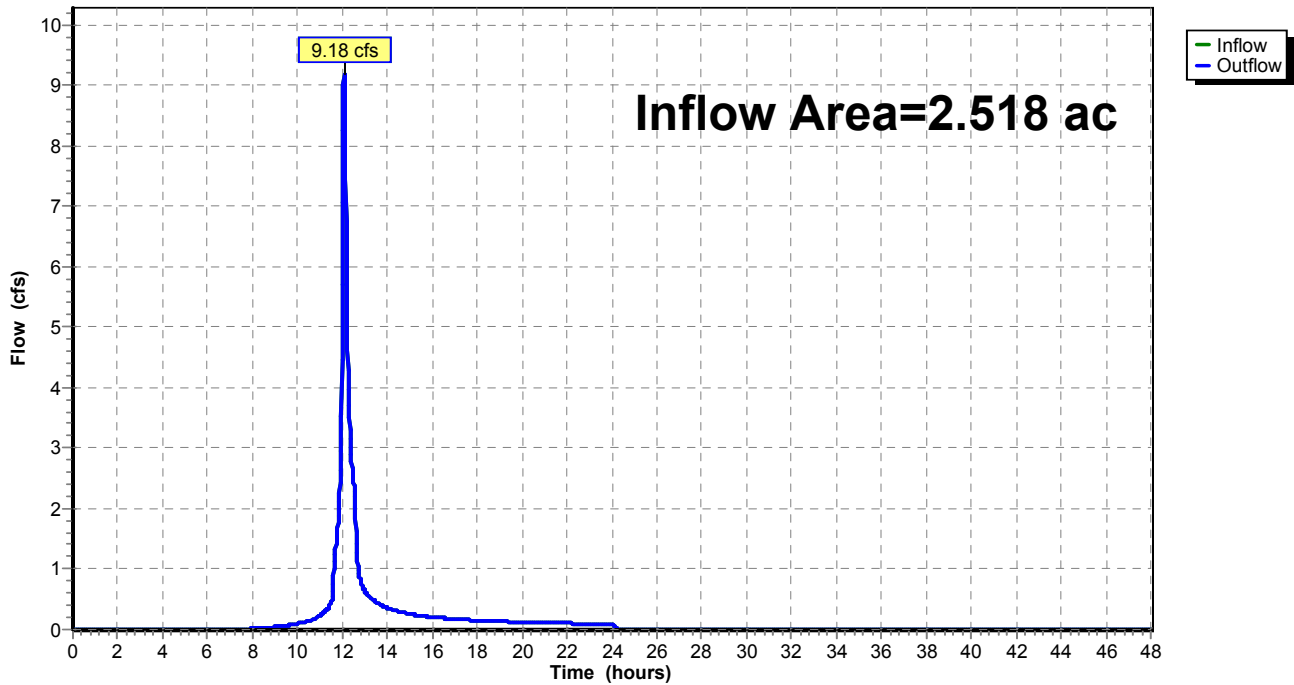
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 2.518 ac, 0.00% Impervious, Inflow Depth = 2.57" for 10-Year event
Inflow = 9.18 cfs @ 12.08 hrs, Volume= 0.538 af
Outflow = 9.18 cfs @ 12.08 hrs, Volume= 0.538 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 51R: Outfall of SB 14

Hydrograph



Summary for Reach 52R: Outfall of SB 17

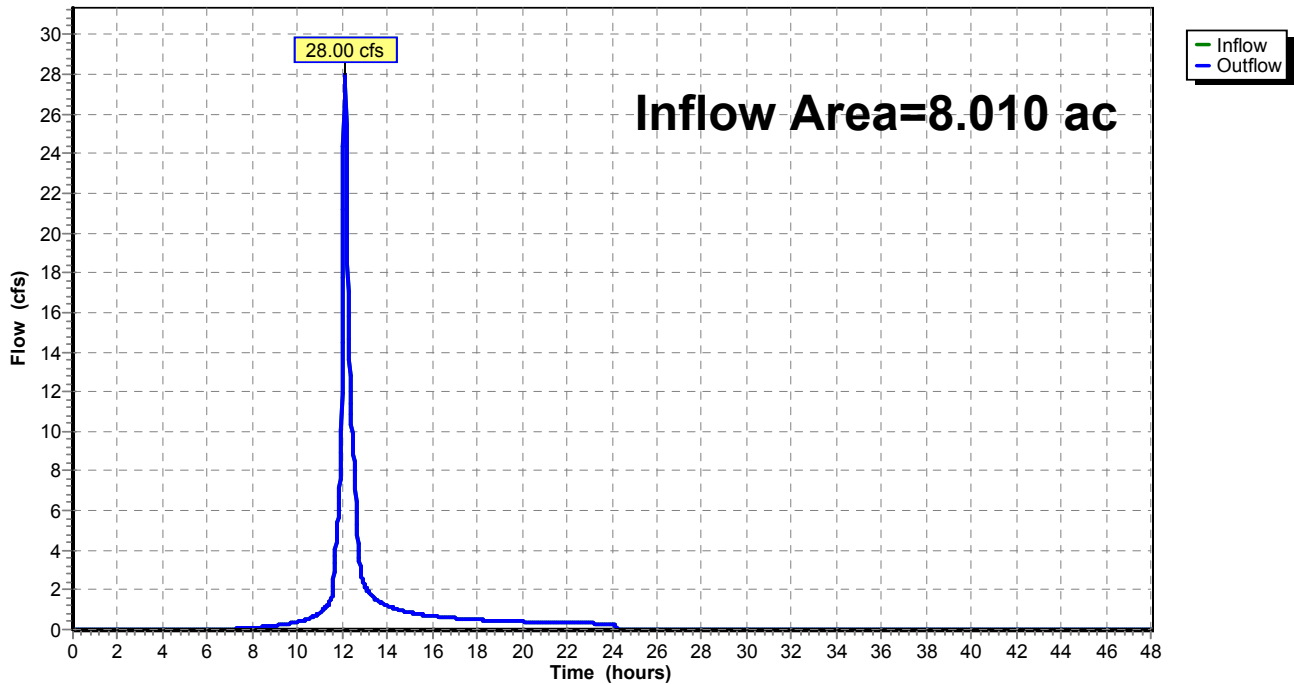
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 8.010 ac, 0.00% Impervious, Inflow Depth = 2.75" for 10-Year event
Inflow = 28.00 cfs @ 12.11 hrs, Volume= 1.833 af
Outflow = 28.00 cfs @ 12.11 hrs, Volume= 1.833 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 52R: Outfall of SB 17

Hydrograph



Summary for Reach 53R: Outfall of SB 18

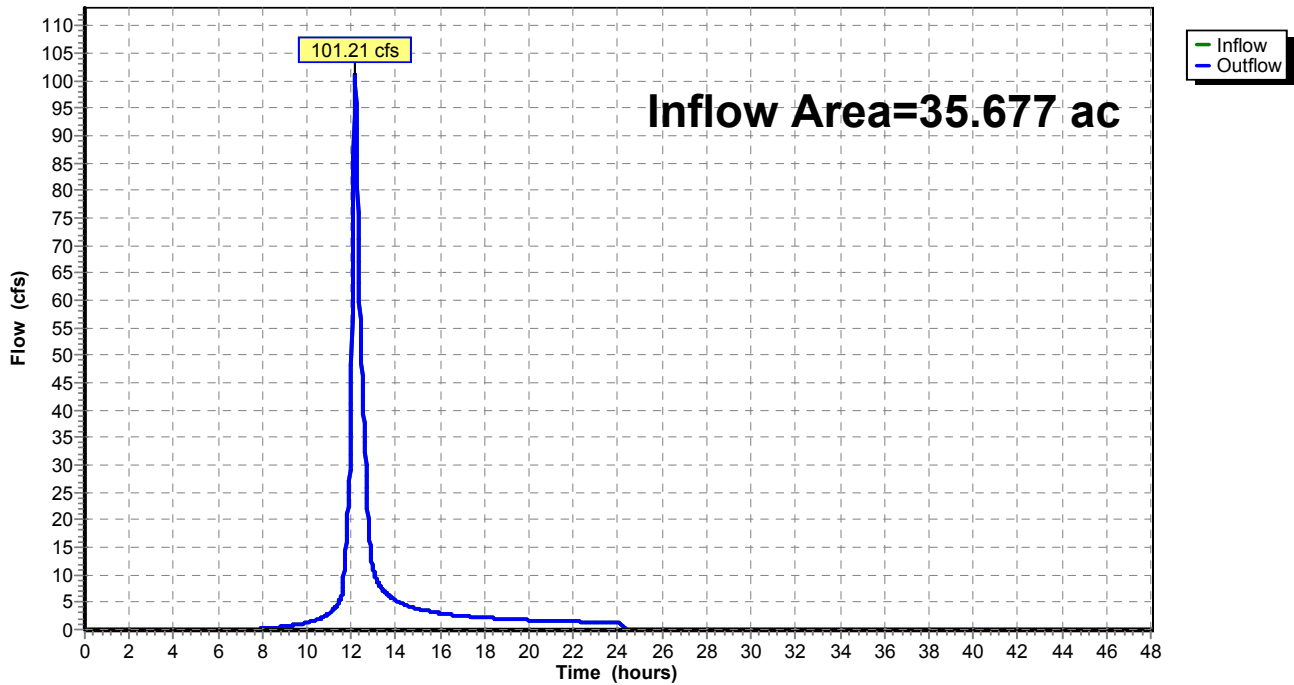
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 35.677 ac, 0.00% Impervious, Inflow Depth = 2.57" for 10-Year event
Inflow = 101.21 cfs @ 12.17 hrs, Volume= 7.627 af
Outflow = 101.21 cfs @ 12.17 hrs, Volume= 7.627 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 53R: Outfall of SB 18

Hydrograph



Summary for Reach 54R: Outfall of SB 25

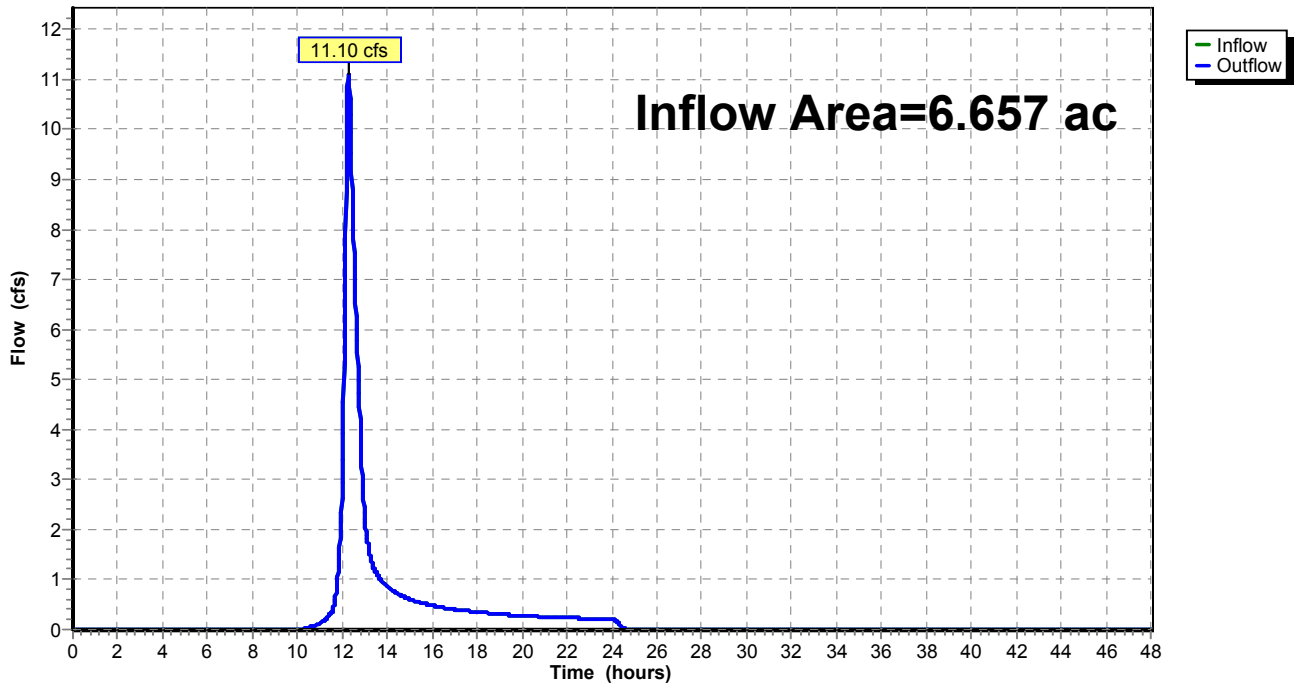
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 6.657 ac, 0.00% Impervious, Inflow Depth = 1.83" for 10-Year event
Inflow = 11.10 cfs @ 12.28 hrs, Volume= 1.017 af
Outflow = 11.10 cfs @ 12.28 hrs, Volume= 1.017 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 54R: Outfall of SB 25

Hydrograph



Summary for Reach 55R: Outfall of SB 26

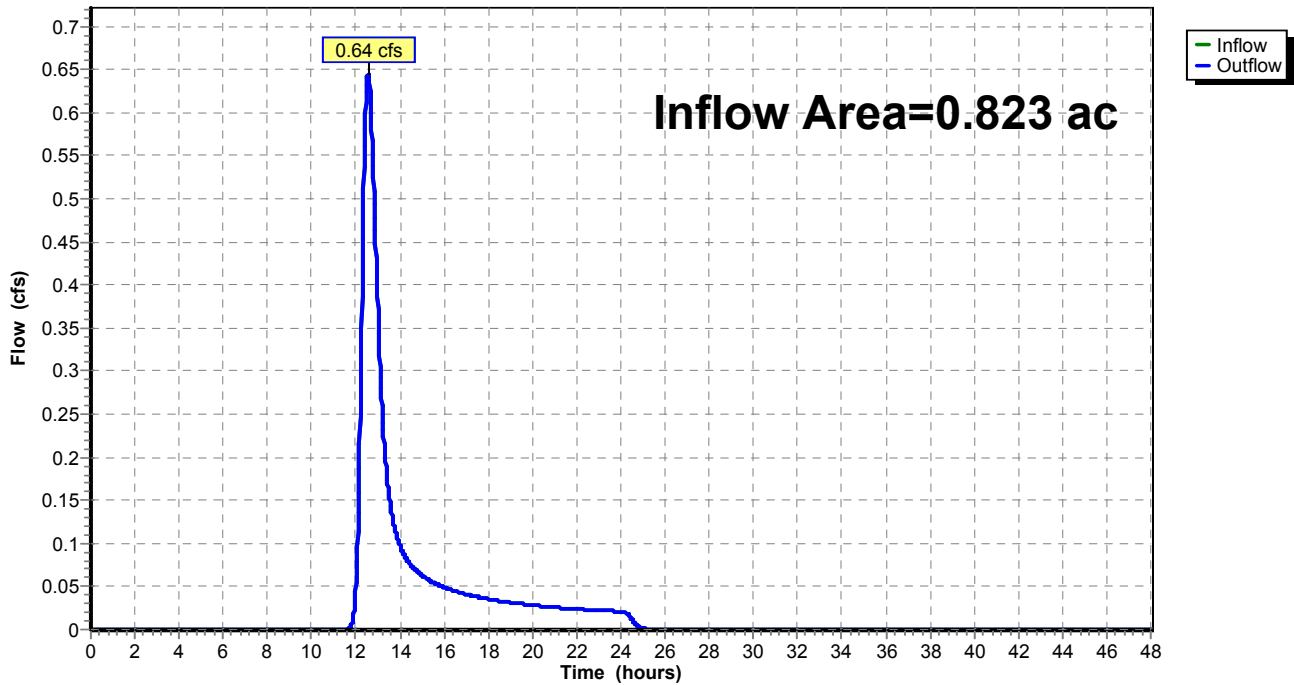
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.823 ac, 0.00% Impervious, Inflow Depth = 1.22" for 10-Year event
Inflow = 0.64 cfs @ 12.56 hrs, Volume= 0.084 af
Outflow = 0.64 cfs @ 12.56 hrs, Volume= 0.084 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 55R: Outfall of SB 26

Hydrograph



Summary for Reach 56R: Outfall of SB 23, 24

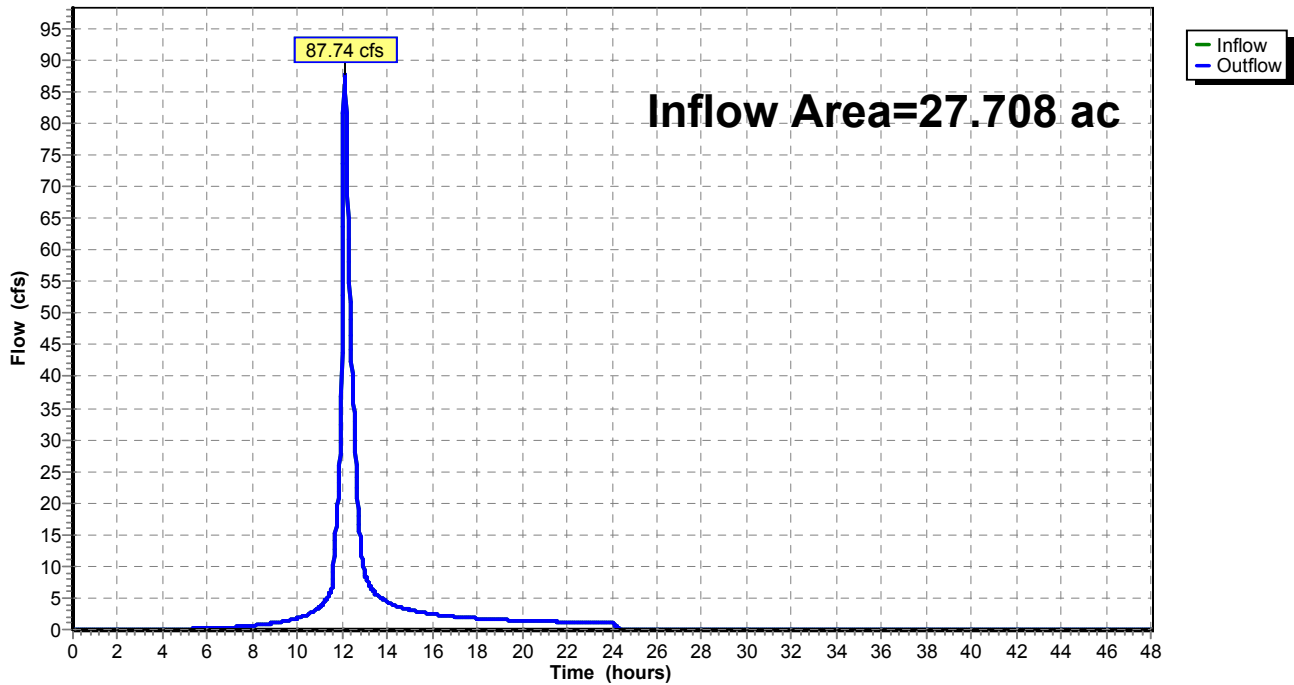
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 27.708 ac, 0.00% Impervious, Inflow Depth = 2.99" for 10-Year event
Inflow = 87.74 cfs @ 12.10 hrs, Volume= 6.907 af
Outflow = 87.74 cfs @ 12.10 hrs, Volume= 6.907 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 56R: Outfall of SB 23, 24

Hydrograph



Summary for Reach 59R: Outfall of SB 20, 22

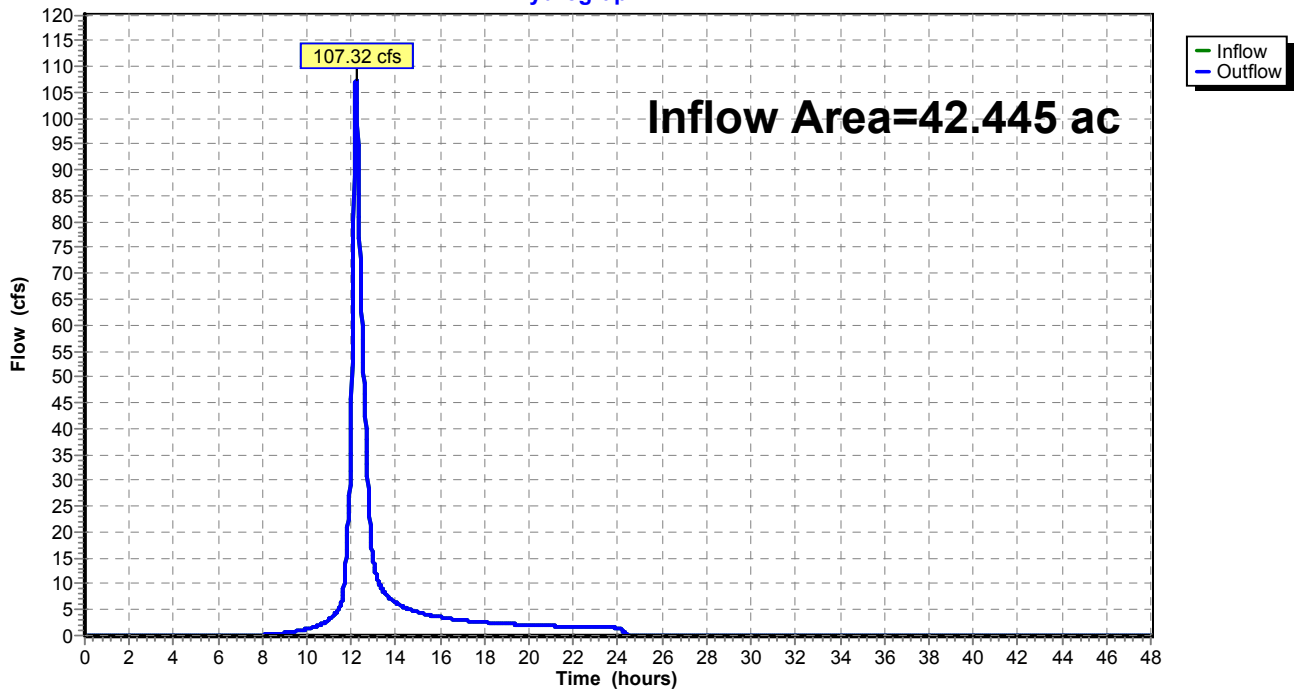
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 42.445 ac, 0.00% Impervious, Inflow Depth = 2.49" for 10-Year event
Inflow = 107.32 cfs @ 12.21 hrs, Volume= 8.807 af
Outflow = 107.32 cfs @ 12.21 hrs, Volume= 8.807 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 59R: Outfall of SB 20, 22

Hydrograph



Summary for Reach 61R: Outfall of SB 15, 16, 21

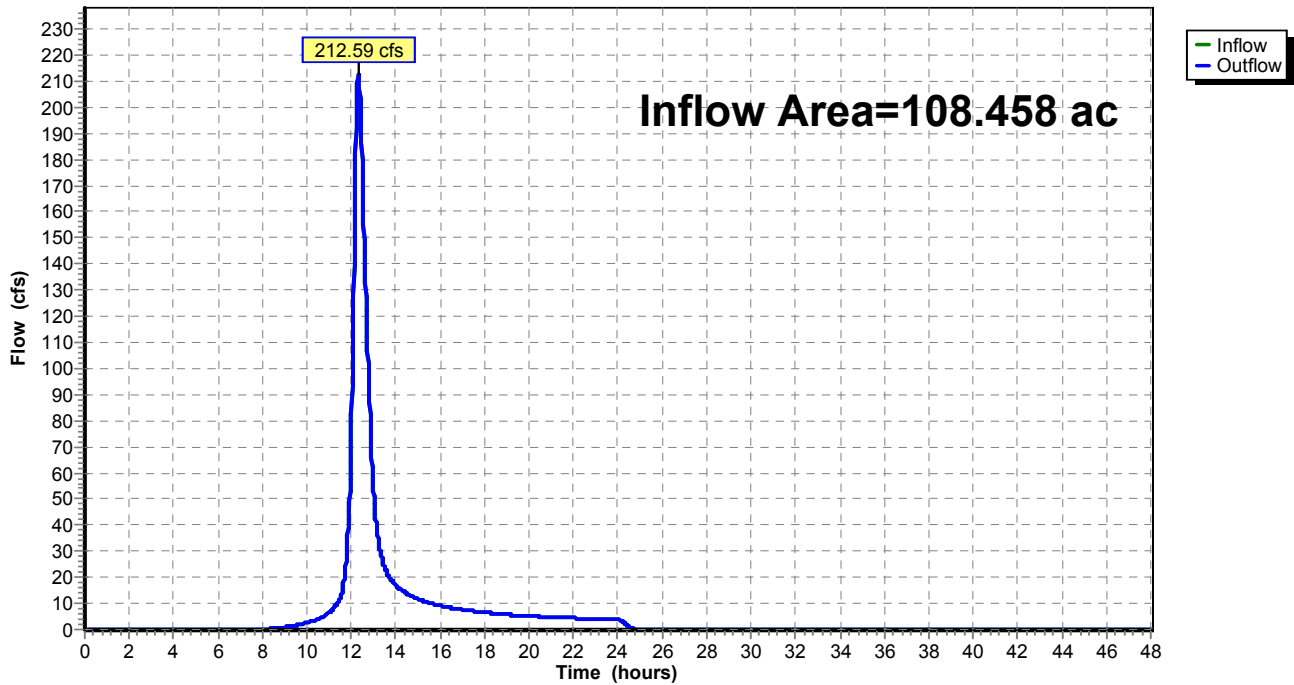
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 108.458 ac, 0.00% Impervious, Inflow Depth = 2.41" for 10-Year event
Inflow = 212.59 cfs @ 12.32 hrs, Volume= 21.738 af
Outflow = 212.59 cfs @ 12.32 hrs, Volume= 21.738 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 61R: Outfall of SB 15, 16, 21

Hydrograph



Summary for Reach 67R: Outfall of SB 28

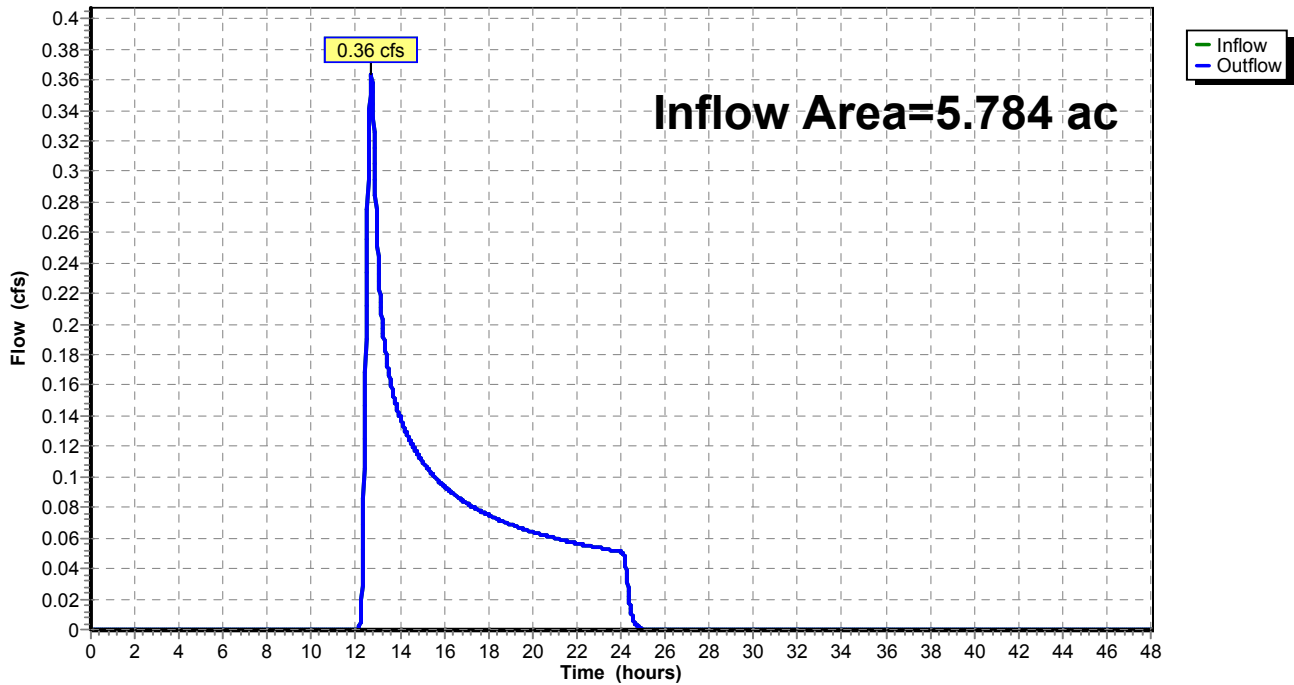
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 5.784 ac, 0.00% Impervious, Inflow Depth = 0.19" for 10-Year event
Inflow = 0.36 cfs @ 12.70 hrs, Volume= 0.094 af
Outflow = 0.36 cfs @ 12.70 hrs, Volume= 0.094 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 67R: Outfall of SB 28

Hydrograph



Summary for Reach 68R: Outfall of SB 29

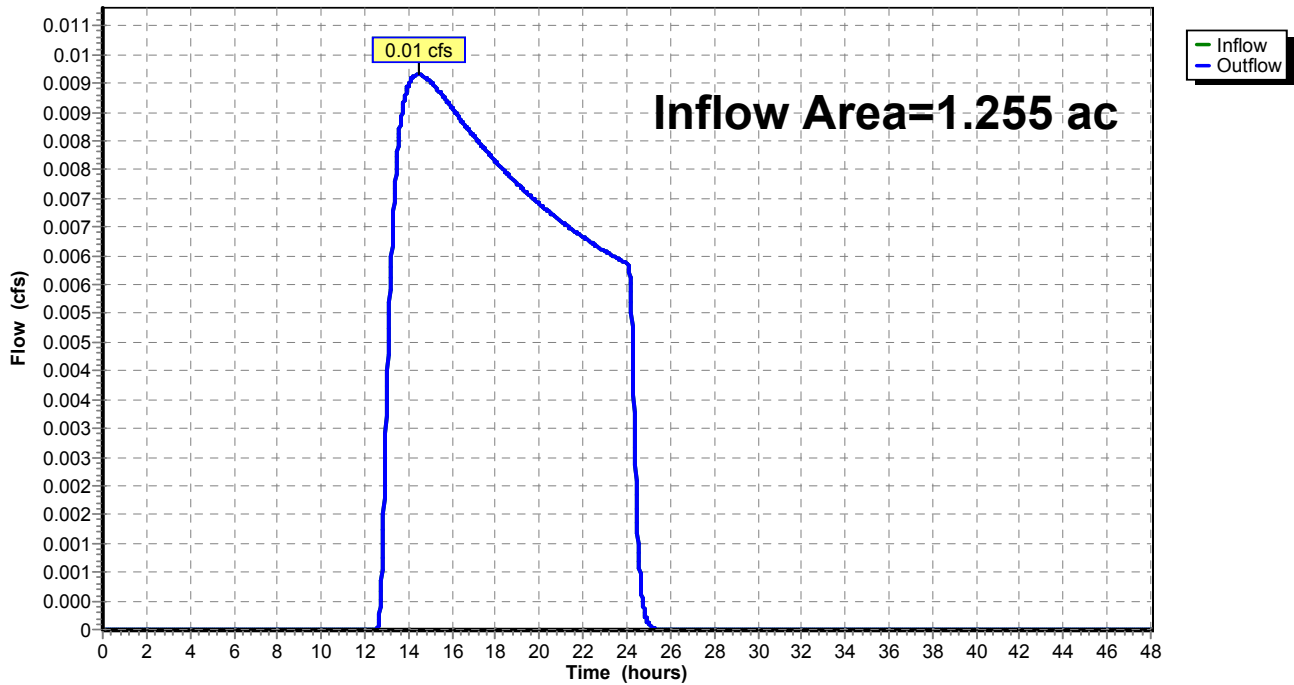
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.255 ac, 0.00% Impervious, Inflow Depth = 0.07" for 10-Year event
Inflow = 0.01 cfs @ 14.44 hrs, Volume= 0.007 af
Outflow = 0.01 cfs @ 14.44 hrs, Volume= 0.007 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 68R: Outfall of SB 29

Hydrograph



Summary for Reach 69R: Outfall of SB 30

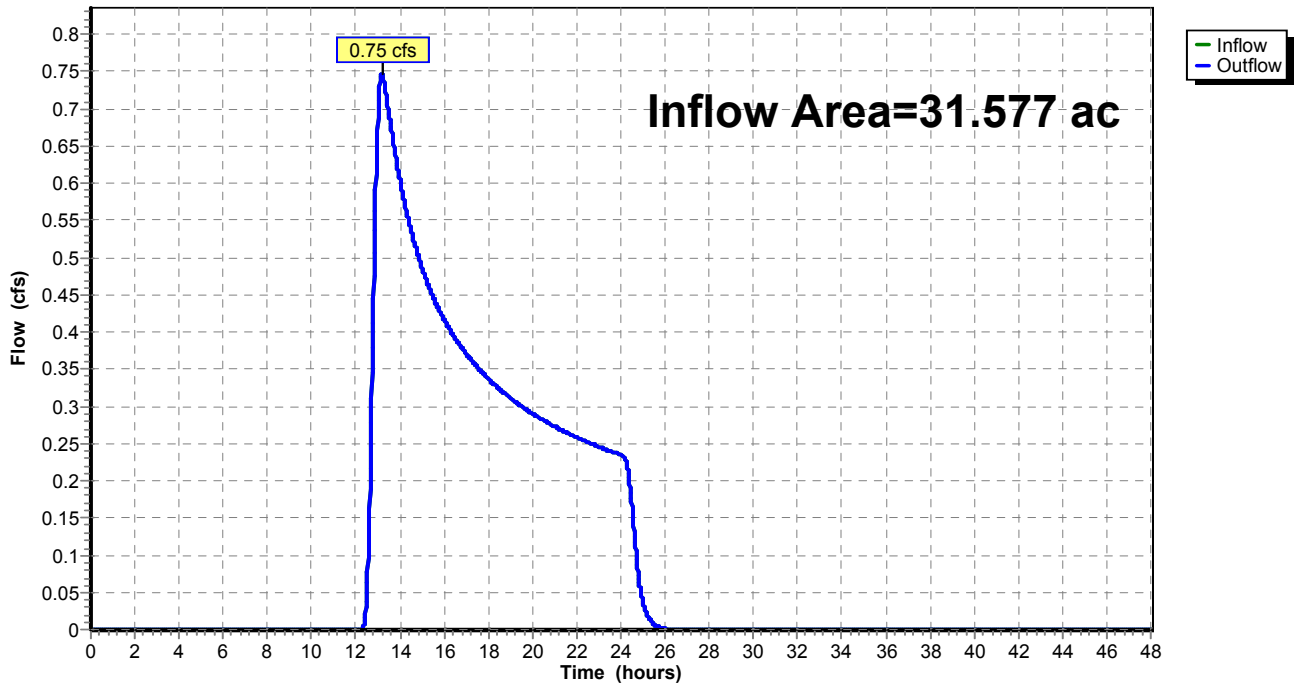
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 31.577 ac, 0.00% Impervious, Inflow Depth = 0.14" for 10-Year event
Inflow = 0.75 cfs @ 13.19 hrs, Volume= 0.366 af
Outflow = 0.75 cfs @ 13.19 hrs, Volume= 0.366 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 69R: Outfall of SB 30

Hydrograph



Summary for Reach 70R: Outfall of SB 31

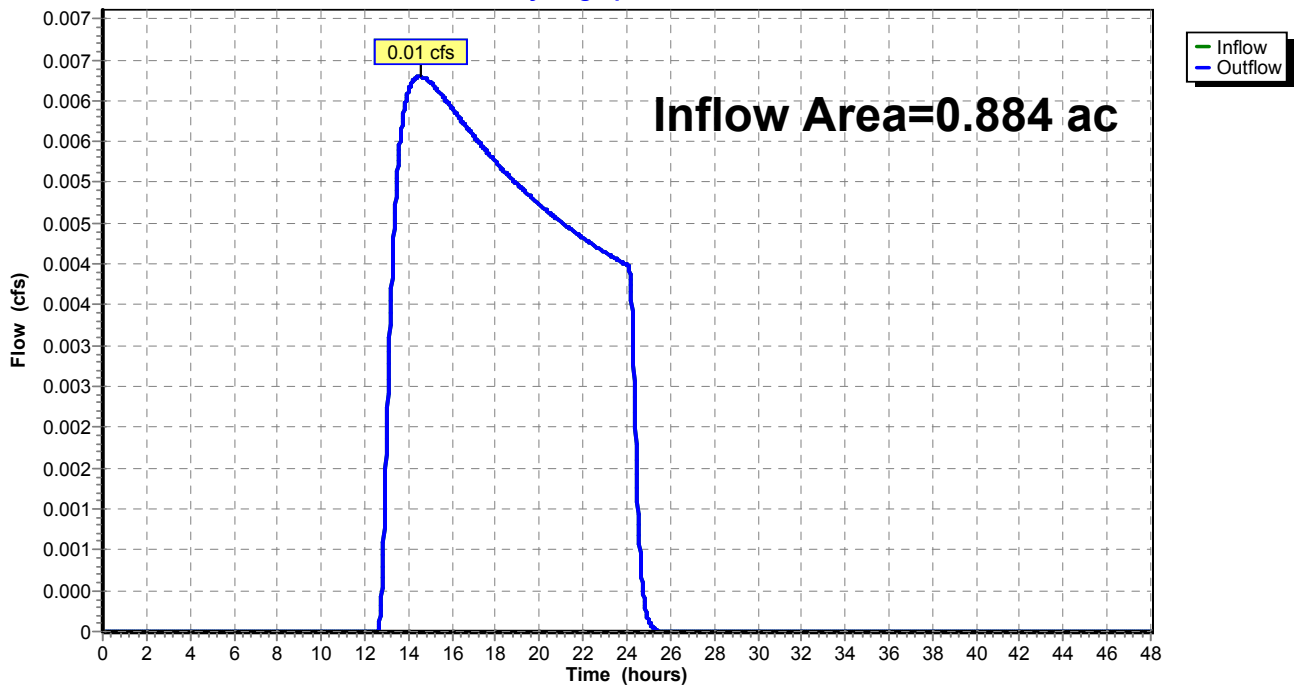
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.884 ac, 0.00% Impervious, Inflow Depth = 0.07" for 10-Year event
Inflow = 0.01 cfs @ 14.53 hrs, Volume= 0.005 af
Outflow = 0.01 cfs @ 14.53 hrs, Volume= 0.005 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 70R: Outfall of SB 31

Hydrograph



Summary for Reach 71R: Outfall of SB 32

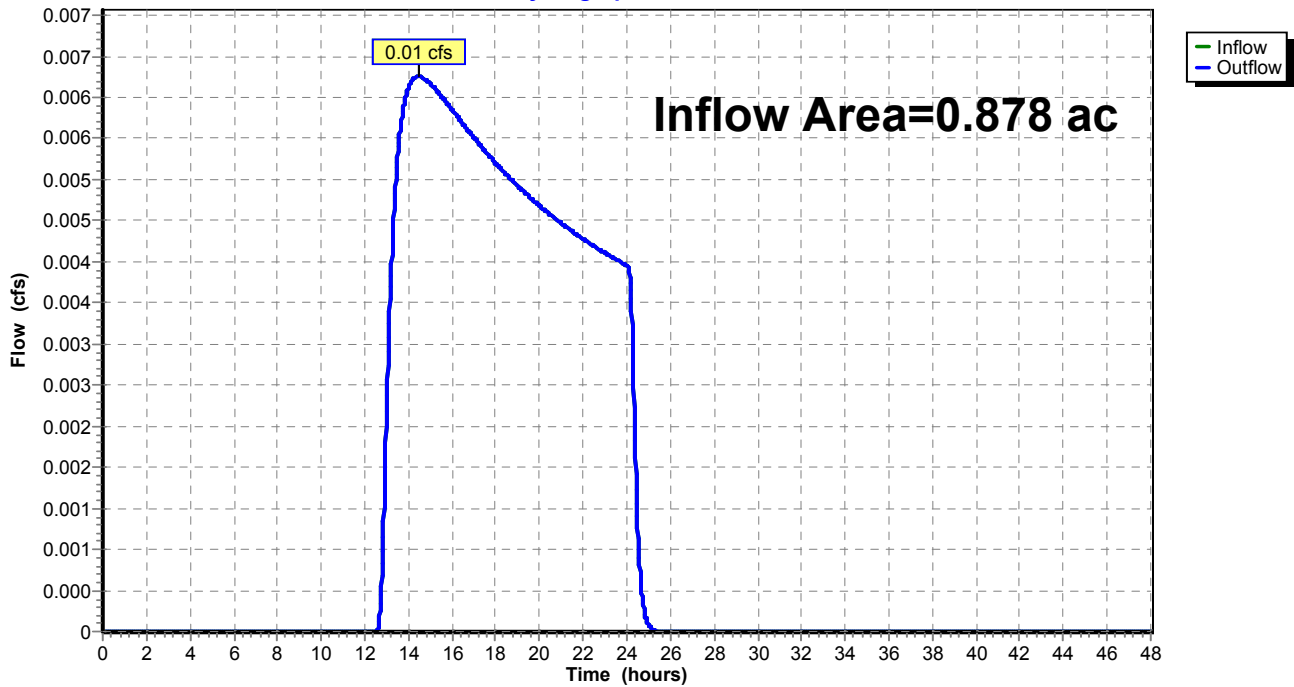
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.878 ac, 0.00% Impervious, Inflow Depth = 0.07" for 10-Year event
Inflow = 0.01 cfs @ 14.44 hrs, Volume= 0.005 af
Outflow = 0.01 cfs @ 14.44 hrs, Volume= 0.005 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 71R: Outfall of SB 32

Hydrograph



Summary for Reach 72R: Outfall of SB 33

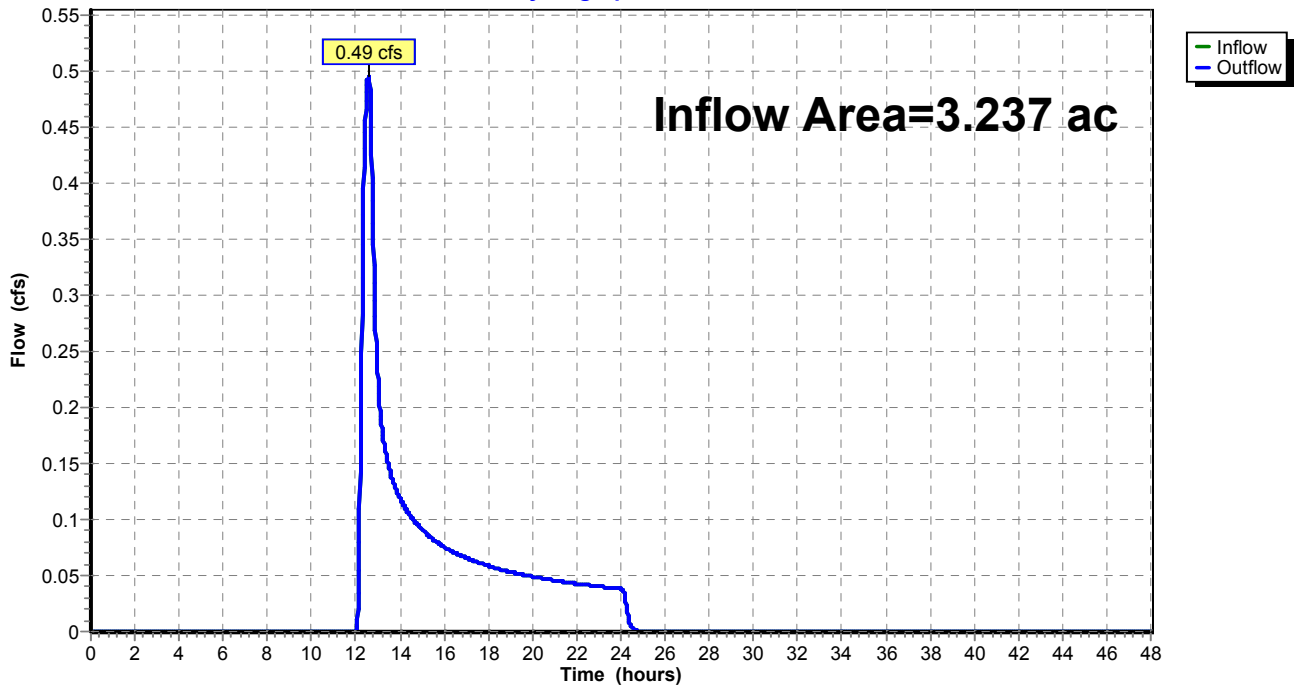
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 3.237 ac, 0.00% Impervious, Inflow Depth = 0.33" for 10-Year event
Inflow = 0.49 cfs @ 12.58 hrs, Volume= 0.088 af
Outflow = 0.49 cfs @ 12.58 hrs, Volume= 0.088 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 72R: Outfall of SB 33

Hydrograph



Summary for Reach 73R: Outfall of SB 34

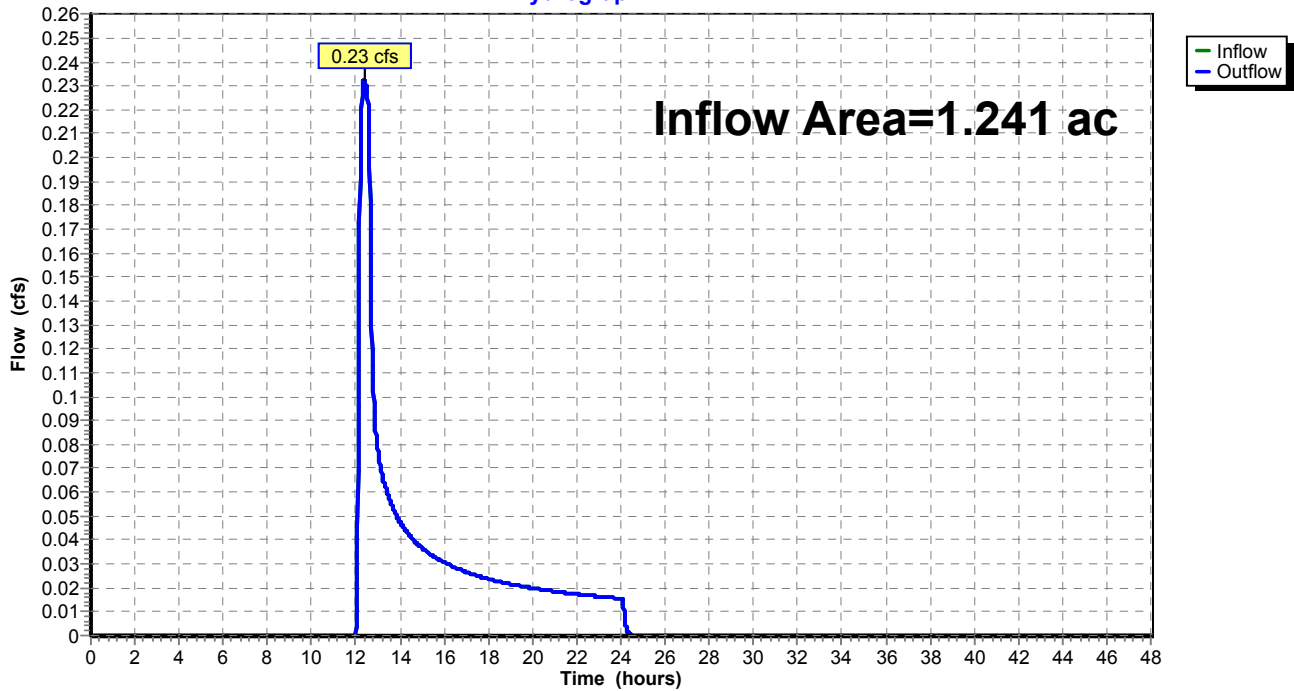
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.241 ac, 0.00% Impervious, Inflow Depth = 0.36" for 10-Year event
Inflow = 0.23 cfs @ 12.38 hrs, Volume= 0.038 af
Outflow = 0.23 cfs @ 12.38 hrs, Volume= 0.038 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 73R: Outfall of SB 34

Hydrograph



Summary for Reach 74R: Outfall of SB 35

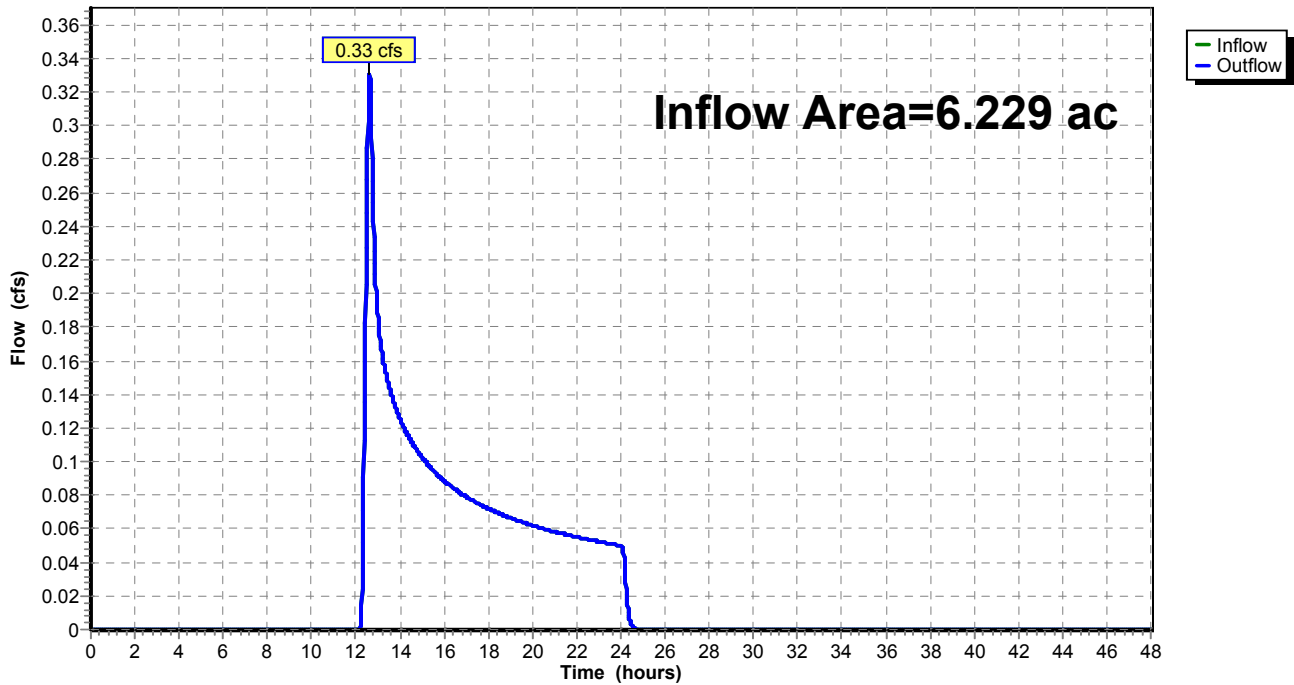
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 6.229 ac, 0.00% Impervious, Inflow Depth = 0.17" for 10-Year event
Inflow = 0.33 cfs @ 12.63 hrs, Volume= 0.086 af
Outflow = 0.33 cfs @ 12.63 hrs, Volume= 0.086 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 74R: Outfall of SB 35

Hydrograph



Summary for Reach 75R: Outfall of SB 19

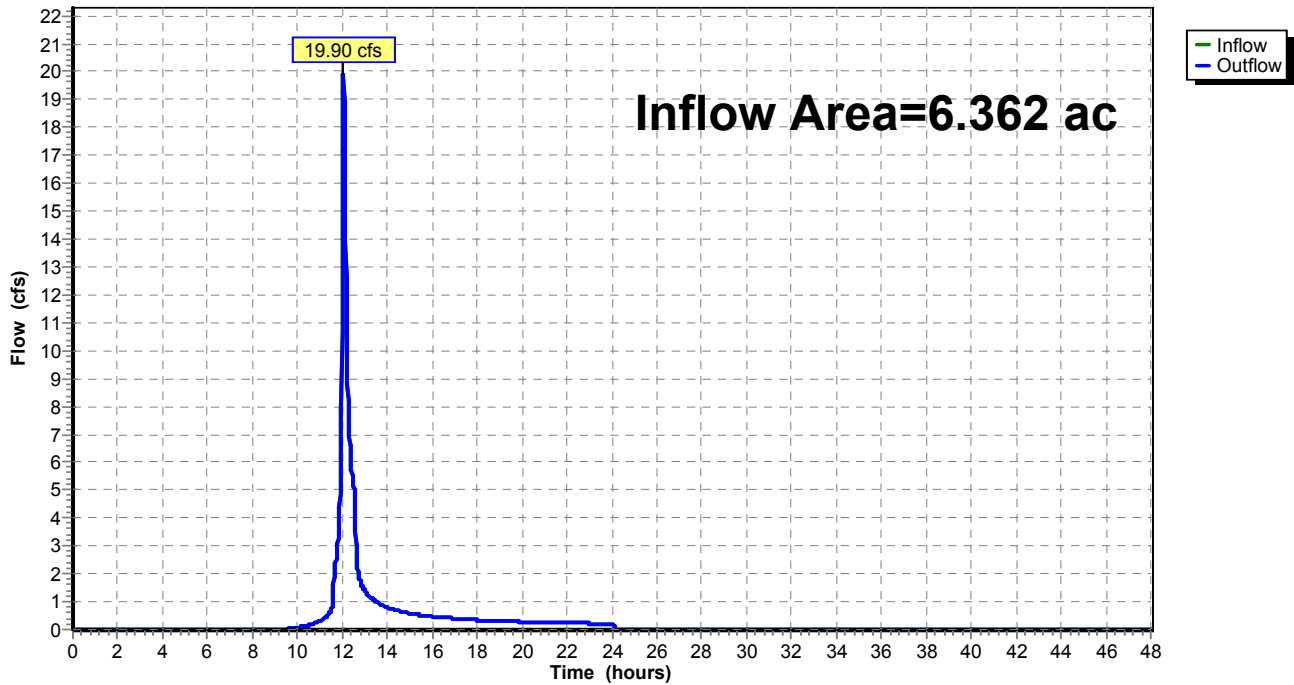
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 6.362 ac, 0.00% Impervious, Inflow Depth = 2.06" for 10-Year event
Inflow = 19.90 cfs @ 12.06 hrs, Volume= 1.094 af
Outflow = 19.90 cfs @ 12.06 hrs, Volume= 1.094 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 75R: Outfall of SB 19

Hydrograph



Summary for Reach 82R: Outfall of SB 27

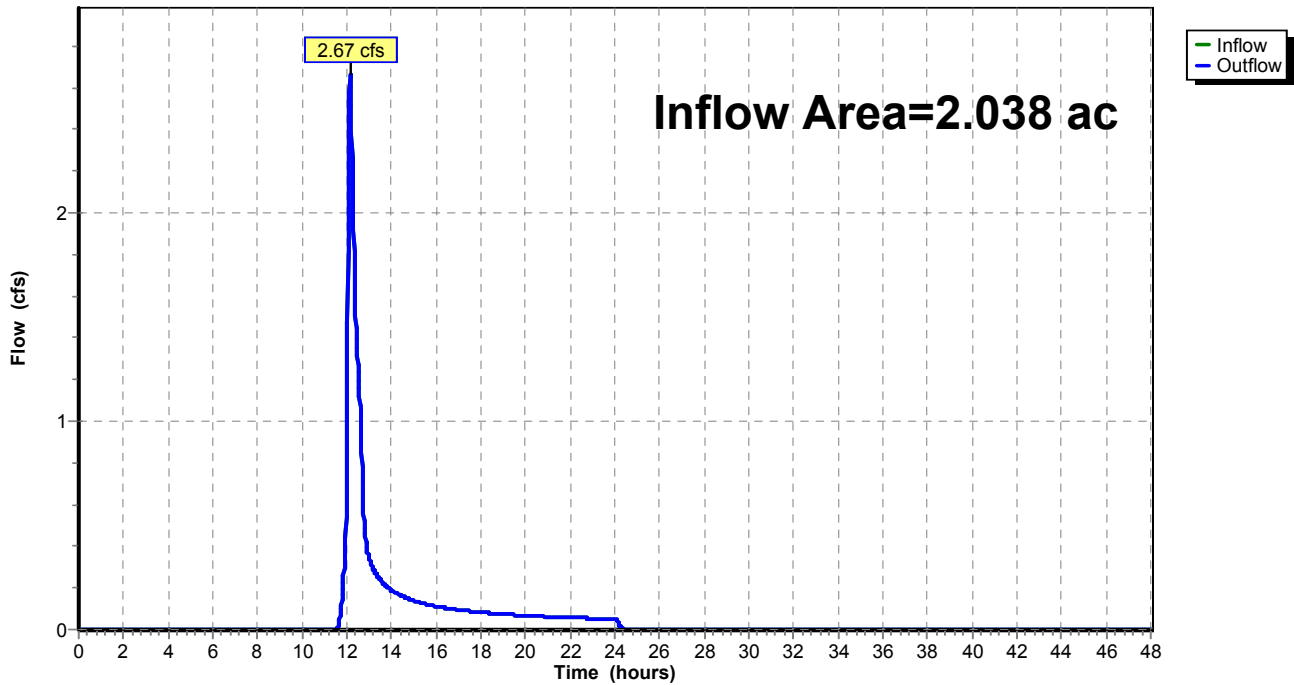
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 2.038 ac, 0.00% Impervious, Inflow Depth = 1.22" for 10-Year event
Inflow = 2.67 cfs @ 12.15 hrs, Volume= 0.207 af
Outflow = 2.67 cfs @ 12.15 hrs, Volume= 0.207 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 82R: Outfall of SB 27

Hydrograph



Summary for Reach 84R: Outfall of Future County Road H Subbasin

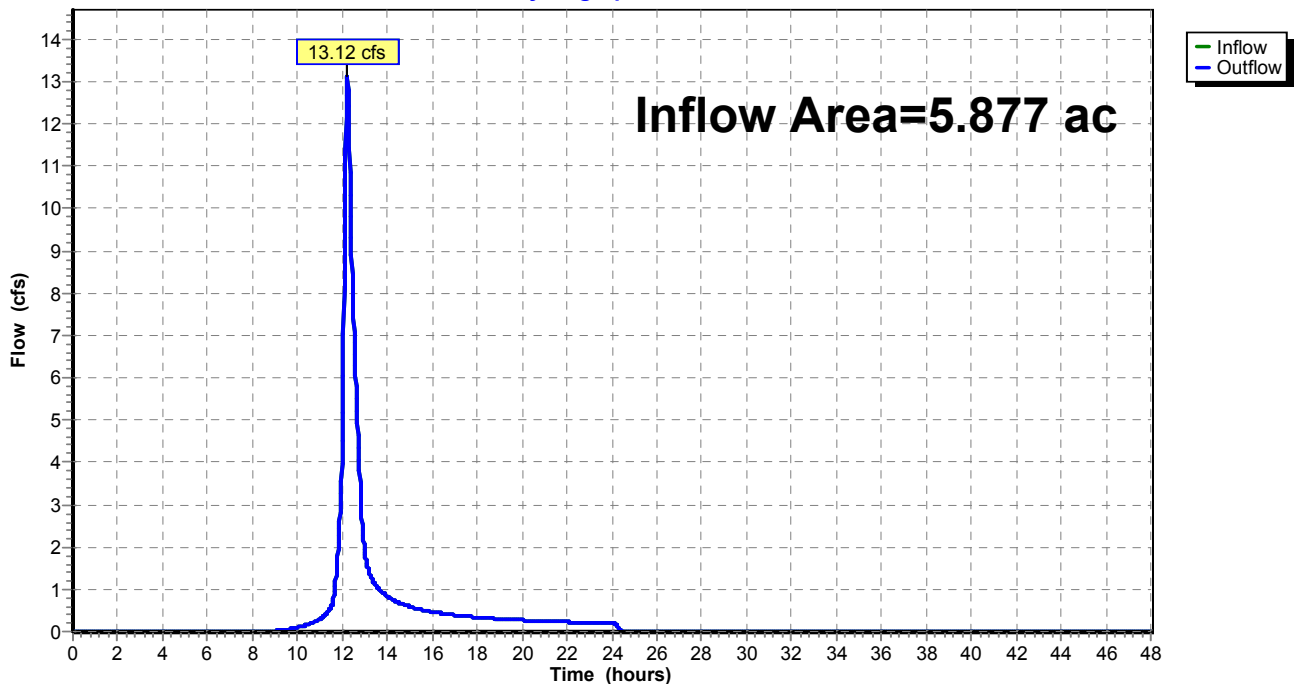
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 5.877 ac, 0.00% Impervious, Inflow Depth = 2.22" for 10-Year event
Inflow = 13.12 cfs @ 12.22 hrs, Volume= 1.089 af
Outflow = 13.12 cfs @ 12.22 hrs, Volume= 1.089 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 84R: Outfall of Future County Road H Subbasin

Hydrograph



Time span=0.00-48.00 hrs, dt=0.02 hrs, 2401 points

Runoff by SCS TR-20 method, UH=SCS

Reach routing by Muskingum-Cunge method - Pond routing by Stor-Ind method

Subcatchment1: Sub-basin1	Runoff Area=15.328 ac 0.00% Impervious Runoff Depth=5.78" Tc=16.3 min CN=87 Runoff=86.42 cfs 7.383 af
Subcatchment2: Sub-basin 2	Runoff Area=4.913 ac 0.00% Impervious Runoff Depth=4.42" Tc=12.2 min CN=75 Runoff=24.74 cfs 1.811 af
Subcatchment3: Sub-basin 3	Runoff Area=15.522 ac 0.00% Impervious Runoff Depth=4.42" Tc=32.8 min CN=75 Runoff=49.25 cfs 5.723 af
Subcatchment4S: Sub-basin 4	Runoff Area=23.961 ac 0.00% Impervious Runoff Depth=5.55" Tc=11.3 min CN=85 Runoff=153.95 cfs 11.082 af
Subcatchment5S: Sub-basin 5	Runoff Area=27.171 ac 0.00% Impervious Runoff Depth=5.32" Tc=40.5 min CN=83 Runoff=91.61 cfs 12.049 af
Subcatchment6S: Sub-basin 6	Runoff Area=22.467 ac 0.00% Impervious Runoff Depth=4.76" Tc=46.4 min CN=78 Runoff=63.49 cfs 8.907 af
Subcatchment7S: Sub-basin 7	Runoff Area=9.790 ac 0.00% Impervious Runoff Depth=3.03" Tc=27.0 min CN=62 Runoff=22.73 cfs 2.473 af
Subcatchment8S: Sub-basin 8	Runoff Area=21.017 ac 0.00% Impervious Runoff Depth=5.09" Tc=9.5 min CN=81 Runoff=134.02 cfs 8.923 af
Subcatchment9S: Sub-basin 9	Runoff Area=9.296 ac 0.00% Impervious Runoff Depth=5.21" Tc=12.7 min CN=82 Runoff=53.64 cfs 4.034 af
Subcatchment10S: Sub-basin 10	Runoff Area=30.014 ac 0.00% Impervious Runoff Depth=5.09" Tc=37.7 min CN=81 Runoff=101.31 cfs 12.742 af
Subcatchment11S: Sub-basin 11	Runoff Area=4.343 ac 0.00% Impervious Runoff Depth=4.53" Tc=32.9 min CN=76 Runoff=14.07 cfs 1.641 af
Subcatchment12S: Sub-basin 12	Runoff Area=3.310 ac 0.00% Impervious Runoff Depth=4.98" Tc=14.0 min CN=80 Runoff=17.60 cfs 1.374 af
Subcatchment13S: Sub-basin 13	Runoff Area=2.279 ac 0.00% Impervious Runoff Depth=5.21" Tc=36.2 min CN=82 Runoff=8.02 cfs 0.989 af
Subcatchment14S: Sub-basin 14	Runoff Area=2.518 ac 0.00% Impervious Runoff Depth=5.44" Tc=8.9 min CN=84 Runoff=17.43 cfs 1.141 af
Subcatchment15S: Sub-basin 15	Runoff Area=56.506 ac 0.00% Impervious Runoff Depth=5.21" Tc=28.0 min CN=82 Runoff=226.35 cfs 24.522 af
Subcatchment16S: Sub-basin 16	Runoff Area=44.796 ac 0.00% Impervious Runoff Depth=5.09" Tc=26.3 min CN=81 Runoff=181.67 cfs 19.018 af

Subcatchment17S: Sub-basin 17	Runoff Area=8.010 ac 0.00% Impervious Runoff Depth=5.66" Tc=11.5 min CN=86 Runoff=51.71 cfs 3.781 af
Subcatchment18S: Sub-basin 18	Runoff Area=35.677 ac 0.00% Impervious Runoff Depth=5.44" Tc=15.8 min CN=84 Runoff=194.75 cfs 16.160 af
Subcatchment19S: Sub-basin 19	Runoff Area=6.362 ac 0.00% Impervious Runoff Depth=4.76" Tc=7.3 min CN=78 Runoff=41.96 cfs 2.522 af
Subcatchment20S: Sub-basin 20	Runoff Area=15.897 ac 0.00% Impervious Runoff Depth=5.55" Tc=17.1 min CN=85 Runoff=85.17 cfs 7.352 af
Subcatchment21S: Sub-basin 21	Runoff Area=7.156 ac 0.00% Impervious Runoff Depth=6.13" Tc=10.8 min CN=90 Runoff=50.44 cfs 3.654 af
Subcatchment22S: Sub-basin 22	Runoff Area=26.548 ac 0.00% Impervious Runoff Depth=5.21" Tc=19.6 min CN=82 Runoff=126.46 cfs 11.521 af
Subcatchment23S: Sub-basin 23	Runoff Area=13.825 ac 0.00% Impervious Runoff Depth=6.36" Tc=9.4 min CN=92 Runoff=104.89 cfs 7.329 af
Subcatchment24S: Sub-basin 24	Runoff Area=13.883 ac 0.00% Impervious Runoff Depth=5.55" Tc=19.0 min CN=85 Runoff=70.75 cfs 6.421 af
Subcatchment25S: Sub-basin 25	Runoff Area=6.657 ac 0.00% Impervious Runoff Depth=4.42" Tc=22.6 min CN=75 Runoff=25.47 cfs 2.454 af
Subcatchment26S: Sub-basin 26	Runoff Area=0.823 ac 0.00% Impervious Runoff Depth=3.45" Tc=38.2 min CN=66 Runoff=1.86 cfs 0.237 af
Subcatchment27S: Sub-basin 27	Runoff Area=2.038 ac 0.00% Impervious Runoff Depth=3.45" Tc=13.0 min CN=66 Runoff=7.67 cfs 0.586 af
Subcatchment28S: Sub-basin 28	Runoff Area=5.784 ac 0.00% Impervious Runoff Depth=1.30" Tc=23.9 min CN=44 Runoff=4.65 cfs 0.626 af
Subcatchment29S: Sub-basin 29	Runoff Area=1.255 ac 0.00% Impervious Runoff Depth=0.88" Tc=26.9 min CN=39 Runoff=0.54 cfs 0.092 af
Subcatchment30S: Sub-basin 30	Runoff Area=31.577 ac 0.00% Impervious Runoff Depth=1.13" Tc=45.9 min CN=42 Runoff=15.95 cfs 2.966 af
Subcatchment31S: Sub-basin 31	Runoff Area=0.884 ac 0.00% Impervious Runoff Depth=0.88" Tc=30.2 min CN=39 Runoff=0.37 cfs 0.065 af
Subcatchment32S: Sub-basin 32	Runoff Area=0.878 ac 0.00% Impervious Runoff Depth=0.88" Tc=27.6 min CN=39 Runoff=0.38 cfs 0.065 af
Subcatchment33S: Sub-basin 33	Runoff Area=3.237 ac 0.00% Impervious Runoff Depth=1.66" Tc=19.9 min CN=48 Runoff=4.03 cfs 0.447 af

Subcatchment34S: Sub-basin 34	Runoff Area=1.241 ac 0.00% Impervious Runoff Depth=1.75" Tc=12.1 min CN=49 Runoff=2.05 cfs 0.181 af
Subcatchment35S: Sub-basin 35	Runoff Area=6.229 ac 0.00% Impervious Runoff Depth=1.21" Tc=16.7 min CN=43 Runoff=5.16 cfs 0.629 af
Subcatchment36S: Sub-basin 36	Runoff Area=11.210 ac 0.00% Impervious Runoff Depth=4.87" Tc=52.2 min CN=79 Runoff=30.42 cfs 4.549 af
Subcatchment83S: County Road H	Runoff Area=5.877 ac 0.00% Impervious Runoff Depth=4.98" Tc=19.1 min CN=80 Runoff=27.19 cfs 2.440 af
Reach 37R: Outfall of SB 2, 3, 7	Inflow=83.23 cfs 10.007 af Outflow=83.23 cfs 10.007 af
Reach 39R: Outfall of SB 1, 4, 5, 6, 9, 10, 11, 36	Inflow=438.22 cfs 62.386 af Outflow=438.22 cfs 62.386 af
Reach 40R: 60 in SB 4	Avg. Flow Depth=3.01' Max Vel=35.97 fps Inflow=438.10 cfs 62.386 af 60.0" Round Pipe n=0.013 L=718.0' S=0.0330 '/' Capacity=473.08 cfs Outflow=438.22 cfs 62.386 af
Reach 41R: Channel in SB 9, 10	Avg. Flow Depth=0.87' Max Vel=10.60 fps Inflow=53.64 cfs 4.034 af n=0.050 L=1,660.0' S=0.0048 '/' Capacity=280.23 cfs Outflow=49.75 cfs 4.034 af
Reach 46R: Channel SB1	Avg. Flow Depth=1.24' Max Vel=6.91 fps Inflow=86.42 cfs 7.383 af n=0.050 L=841.0' S=0.0071 '/' Capacity=296.86 cfs Outflow=85.24 cfs 7.383 af
Reach 48R: Outfall of SB 8, 13	Inflow=135.34 cfs 9.912 af Outflow=135.34 cfs 9.912 af
Reach 49R: Channel SB8	Avg. Flow Depth=1.27' Max Vel=6.95 fps Inflow=134.02 cfs 8.923 af n=0.050 L=521.0' S=0.0077 '/' Capacity=706.58 cfs Outflow=131.67 cfs 8.923 af
Reach 50R: Outfall of SB 12	Inflow=17.60 cfs 1.374 af Outflow=17.60 cfs 1.374 af
Reach 51R: Outfall of SB 14	Inflow=17.43 cfs 1.141 af Outflow=17.43 cfs 1.141 af
Reach 52R: Outfall of SB 17	Inflow=51.71 cfs 3.781 af Outflow=51.71 cfs 3.781 af
Reach 53R: Outfall of SB 18	Inflow=194.75 cfs 16.160 af Outflow=194.75 cfs 16.160 af
Reach 54R: Outfall of SB 25	Inflow=25.47 cfs 2.454 af Outflow=25.47 cfs 2.454 af
Reach 55R: Outfall of SB 26	Inflow=1.86 cfs 0.237 af Outflow=1.86 cfs 0.237 af

Reach 56R: Outfall of SB 23, 24	Inflow=155.89 cfs 13.749 af Outflow=155.89 cfs 13.749 af
Reach 59R: Outfall of SB 20, 22	Inflow=210.05 cfs 18.873 af Outflow=210.05 cfs 18.873 af
Reach 61R: Outfall of SB 15, 16, 21	Inflow=429.83 cfs 47.194 af Outflow=429.83 cfs 47.194 af
Reach 67R: Outfall of SB 28	Inflow=4.65 cfs 0.626 af Outflow=4.65 cfs 0.626 af
Reach 68R: Outfall of SB 29	Inflow=0.54 cfs 0.092 af Outflow=0.54 cfs 0.092 af
Reach 69R: Outfall of SB 30	Inflow=15.95 cfs 2.966 af Outflow=15.95 cfs 2.966 af
Reach 70R: Outfall of SB 31	Inflow=0.37 cfs 0.065 af Outflow=0.37 cfs 0.065 af
Reach 71R: Outfall of SB 32	Inflow=0.38 cfs 0.065 af Outflow=0.38 cfs 0.065 af
Reach 72R: Outfall of SB 33	Inflow=4.03 cfs 0.447 af Outflow=4.03 cfs 0.447 af
Reach 73R: Outfall of SB 34	Inflow=2.05 cfs 0.181 af Outflow=2.05 cfs 0.181 af
Reach 74R: Outfall of SB 35	Inflow=5.16 cfs 0.629 af Outflow=5.16 cfs 0.629 af
Reach 75R: Outfall of SB 19	Inflow=41.96 cfs 2.522 af Outflow=41.96 cfs 2.522 af
Reach 82R: Outfall of SB 27	Inflow=7.67 cfs 0.586 af Outflow=7.67 cfs 0.586 af
Reach 84R: Outfall of Future County Road H Subbasin	Inflow=27.19 cfs 2.440 af Outflow=27.19 cfs 2.440 af

Total Runoff Area = 498.279 ac Runoff Volume = 197.886 af Average Runoff Depth = 4.77"
100.00% Pervious = 498.279 ac 0.00% Impervious = 0.000 ac

Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 1: Sub-basin1

Runoff = 86.42 cfs @ 12.17 hrs, Volume= 7.383 af, Depth= 5.78"

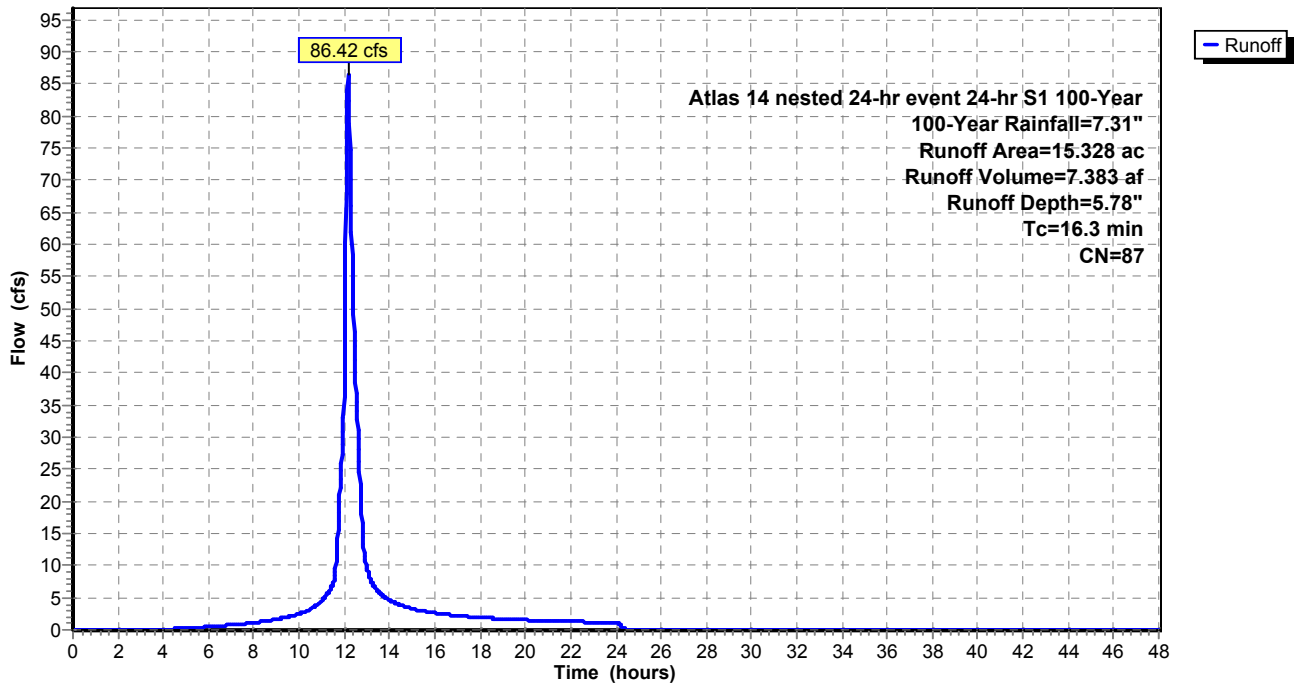
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 15.328	87	
15.328		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.3					Direct Entry,

Subcatchment 1: Sub-basin1

Hydrograph



Summary for Subcatchment 2: Sub-basin 2

Runoff = 24.74 cfs @ 12.12 hrs, Volume= 1.811 af, Depth= 4.42"

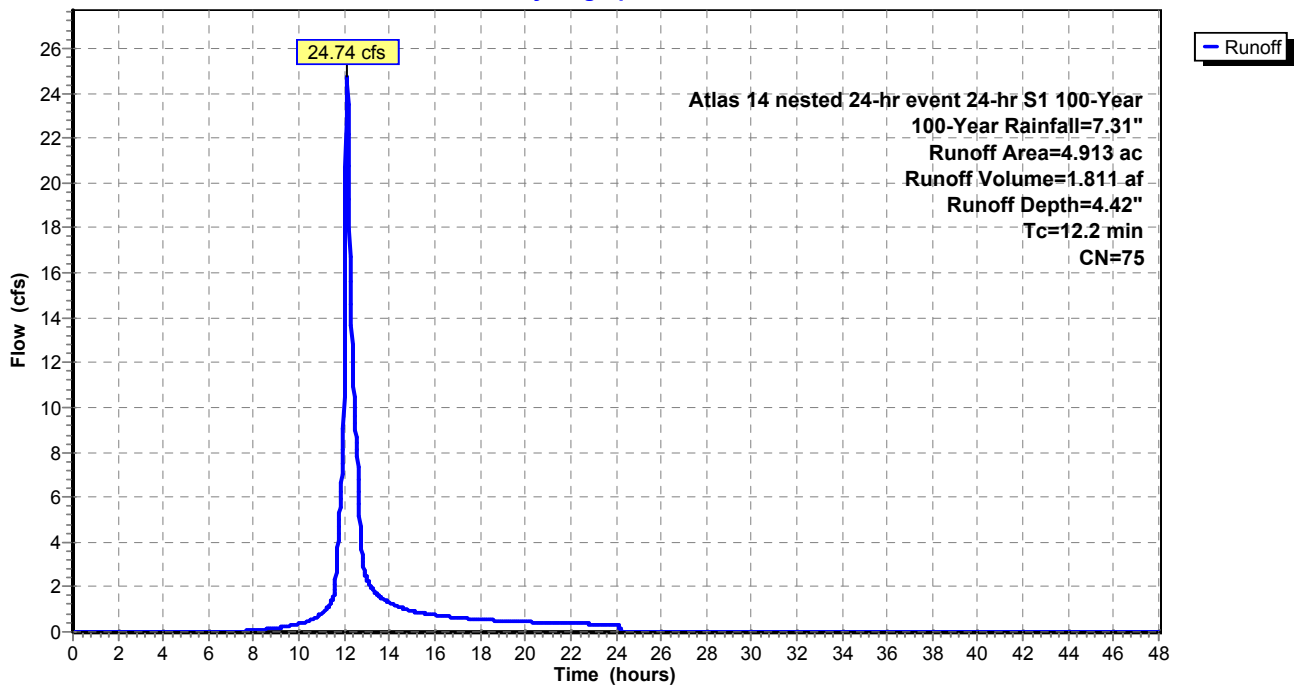
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 4.913	75	
4.913		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.2					Direct Entry,

Subcatchment 2: Sub-basin 2

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 3: Sub-basin 3

Runoff = 49.25 cfs @ 12.42 hrs, Volume= 5.723 af, Depth= 4.42"

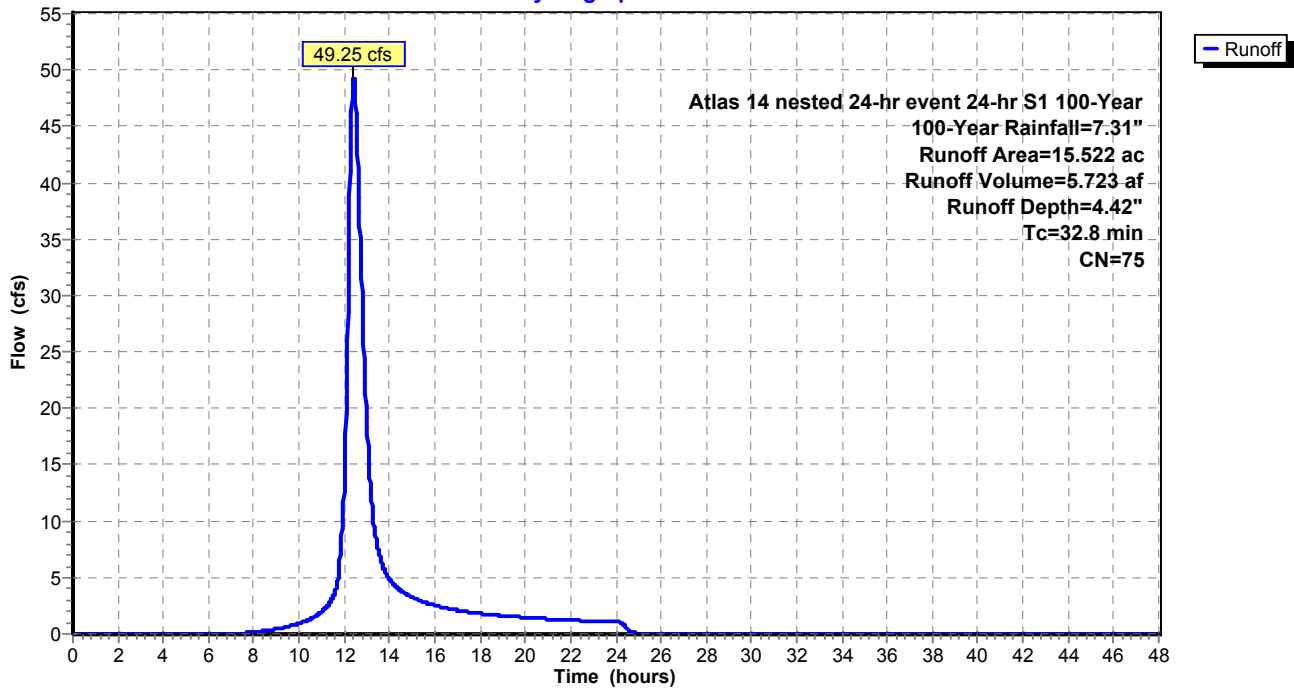
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 15.522	75	
15.522		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.8					Direct Entry,

Subcatchment 3: Sub-basin 3

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 4S: Sub-basin 4

Runoff = 153.95 cfs @ 12.10 hrs, Volume= 11.082 af, Depth= 5.55"

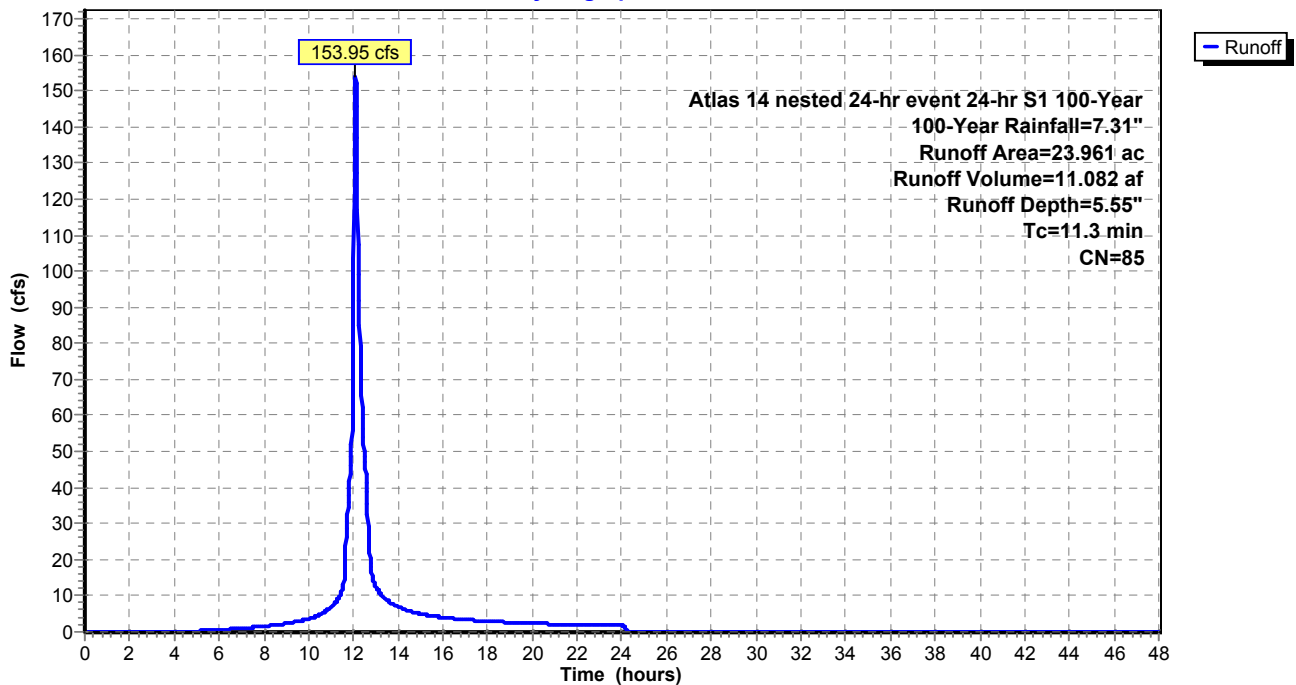
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 23.961	85	
23.961		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.3					Direct Entry,

Subcatchment 4S: Sub-basin 4

Hydrograph



Summary for Subcatchment 5S: Sub-basin 5

Runoff = 91.61 cfs @ 12.51 hrs, Volume= 12.049 af, Depth= 5.32"

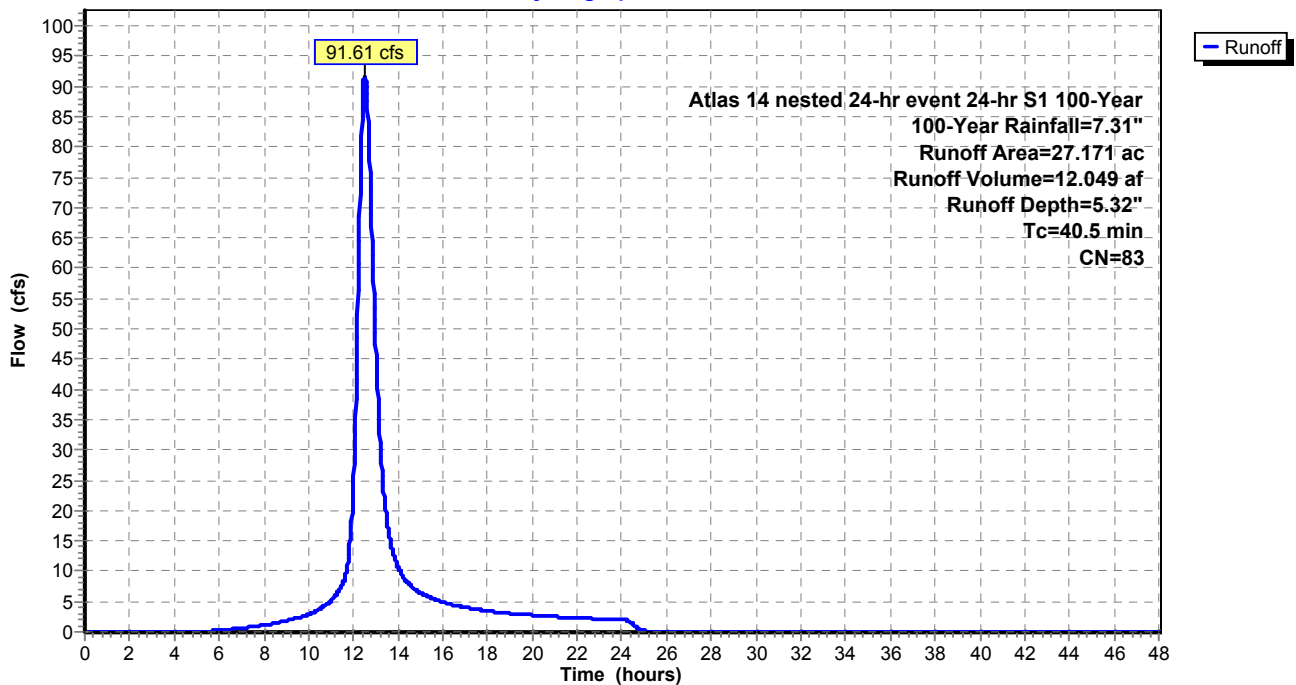
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 27.171	83	
27.171		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
40.5					Direct Entry,

Subcatchment 5S: Sub-basin 5

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 6S: Sub-basin 6

Runoff = 63.49 cfs @ 12.61 hrs, Volume= 8.907 af, Depth= 4.76"

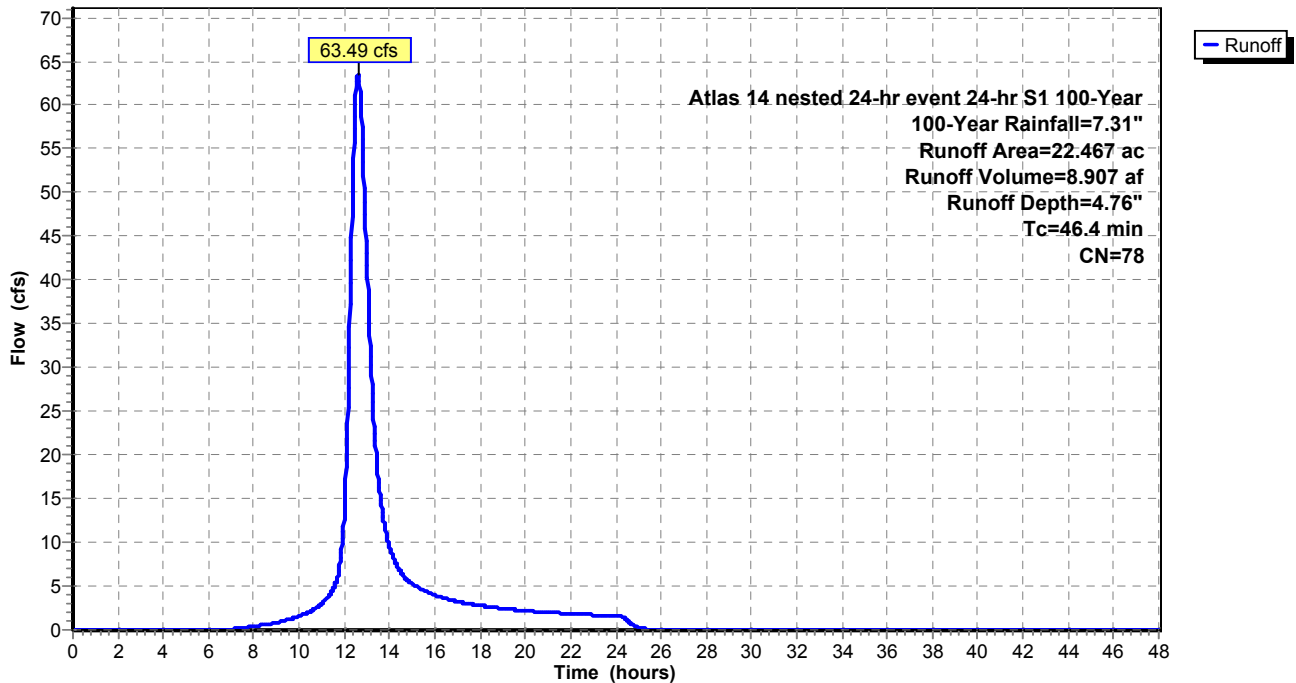
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 22.467	78	
22.467		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
46.4					Direct Entry,

Subcatchment 6S: Sub-basin 6

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 7S: Sub-basin 7

Runoff = 22.73 cfs @ 12.35 hrs, Volume= 2.473 af, Depth= 3.03"

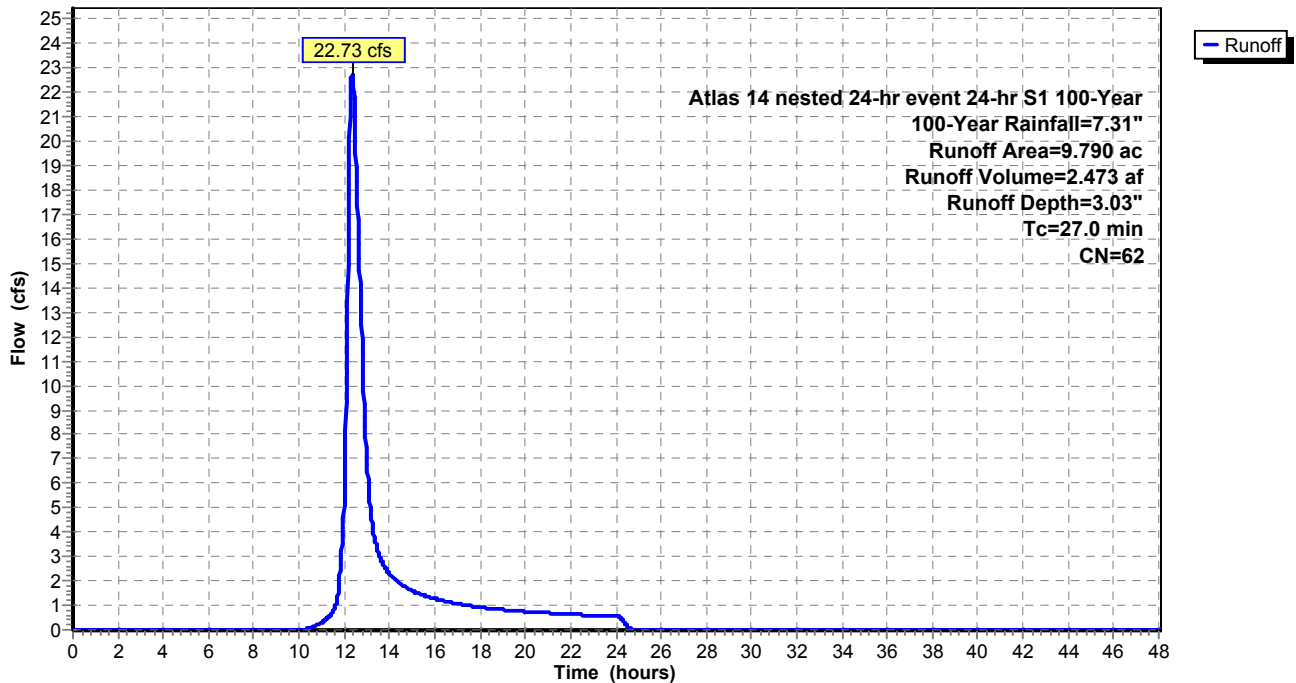
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 9.790	62	
9.790		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.0					Direct Entry,

Subcatchment 7S: Sub-basin 7

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 8S: Sub-basin 8

Runoff = 134.02 cfs @ 12.08 hrs, Volume= 8.923 af, Depth= 5.09"

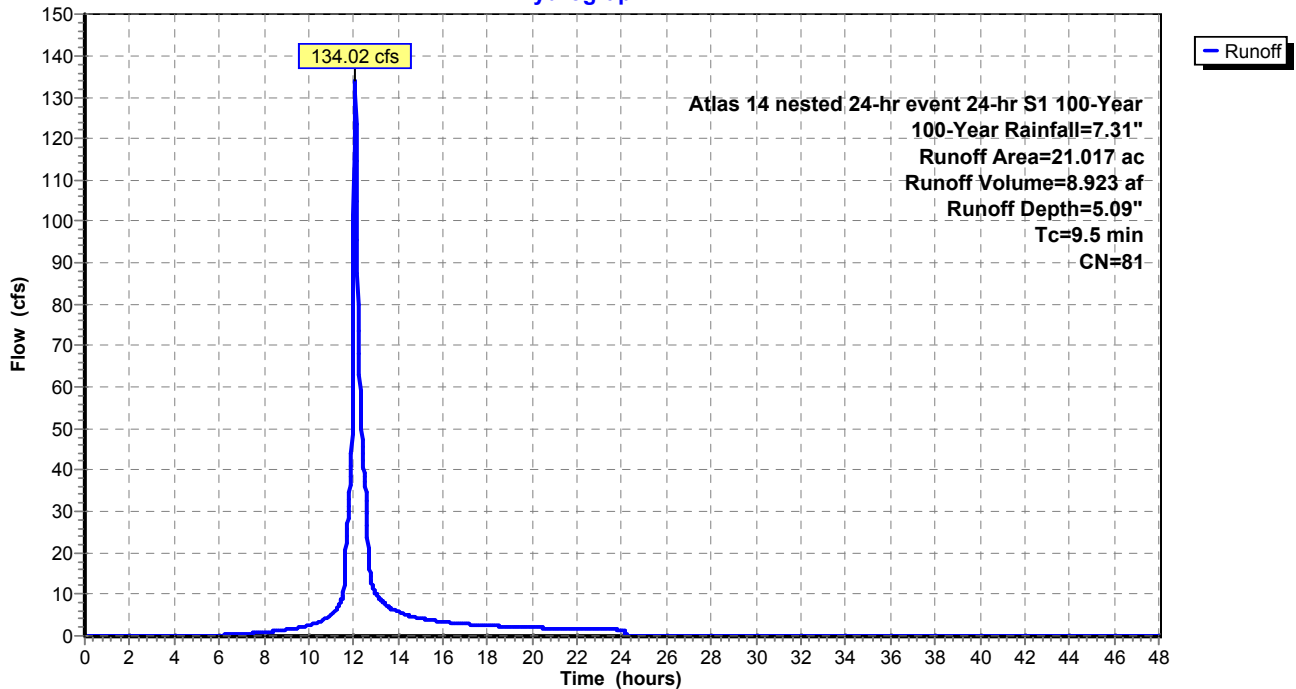
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 21.017	81	
21.017		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.5					Direct Entry,

Subcatchment 8S: Sub-basin 8

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 9S: Sub-basin 9

Runoff = 53.64 cfs @ 12.13 hrs, Volume= 4.034 af, Depth= 5.21"

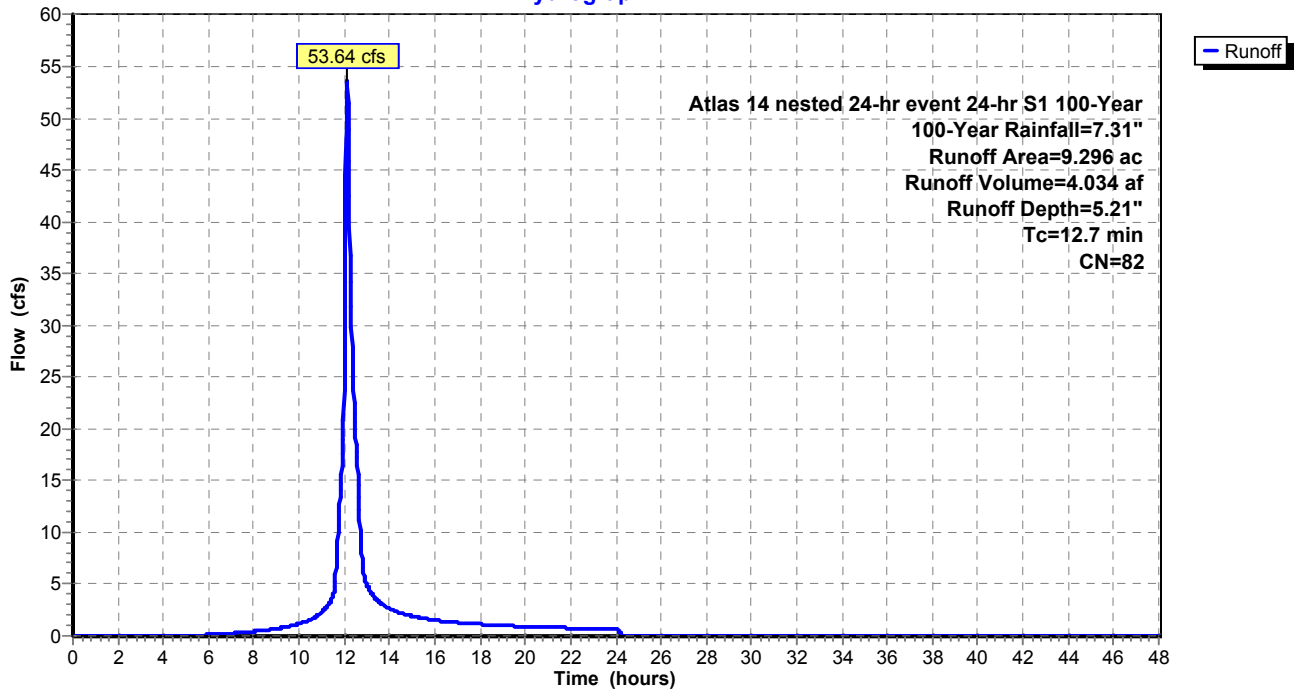
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 9.296	82	
9.296		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.7					Direct Entry,

Subcatchment 9S: Sub-basin 9

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 10S: Sub-basin 10

Runoff = 101.31 cfs @ 12.47 hrs, Volume= 12.742 af, Depth= 5.09"

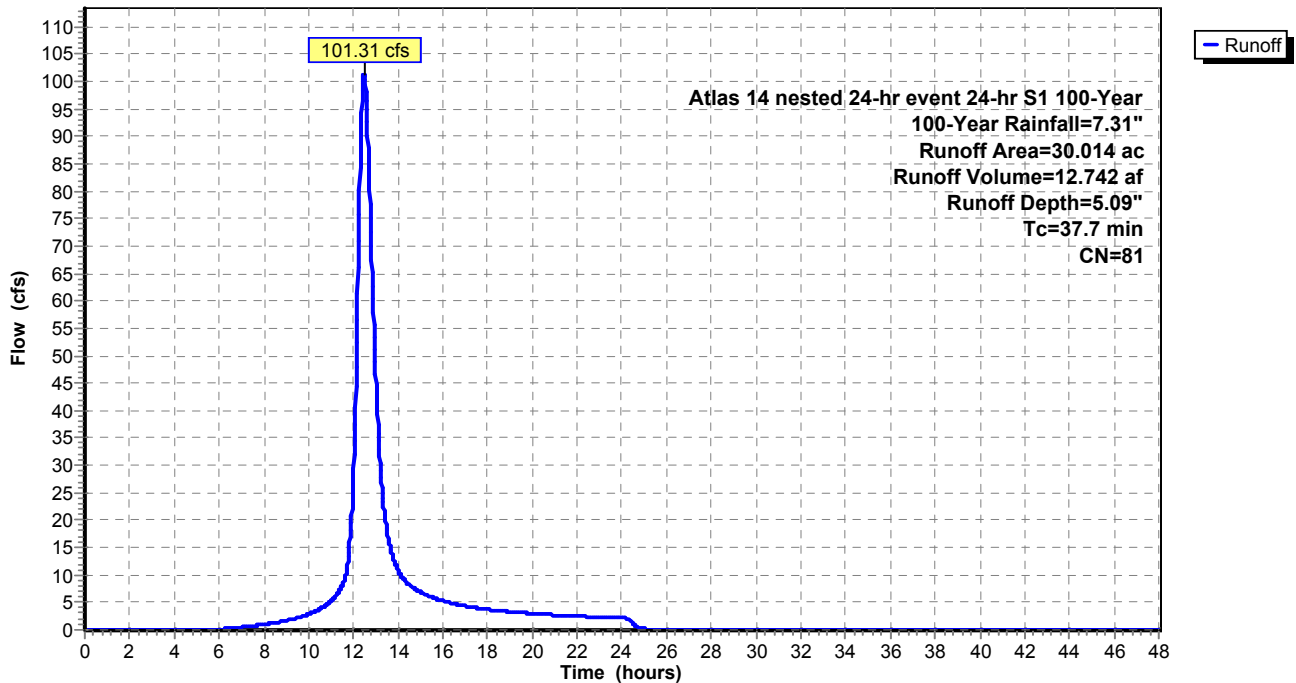
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 30.014	81	
30.014		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
37.7					Direct Entry,

Subcatchment 10S: Sub-basin 10

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 11S: Sub-basin 11

Runoff = 14.07 cfs @ 12.41 hrs, Volume= 1.641 af, Depth= 4.53"

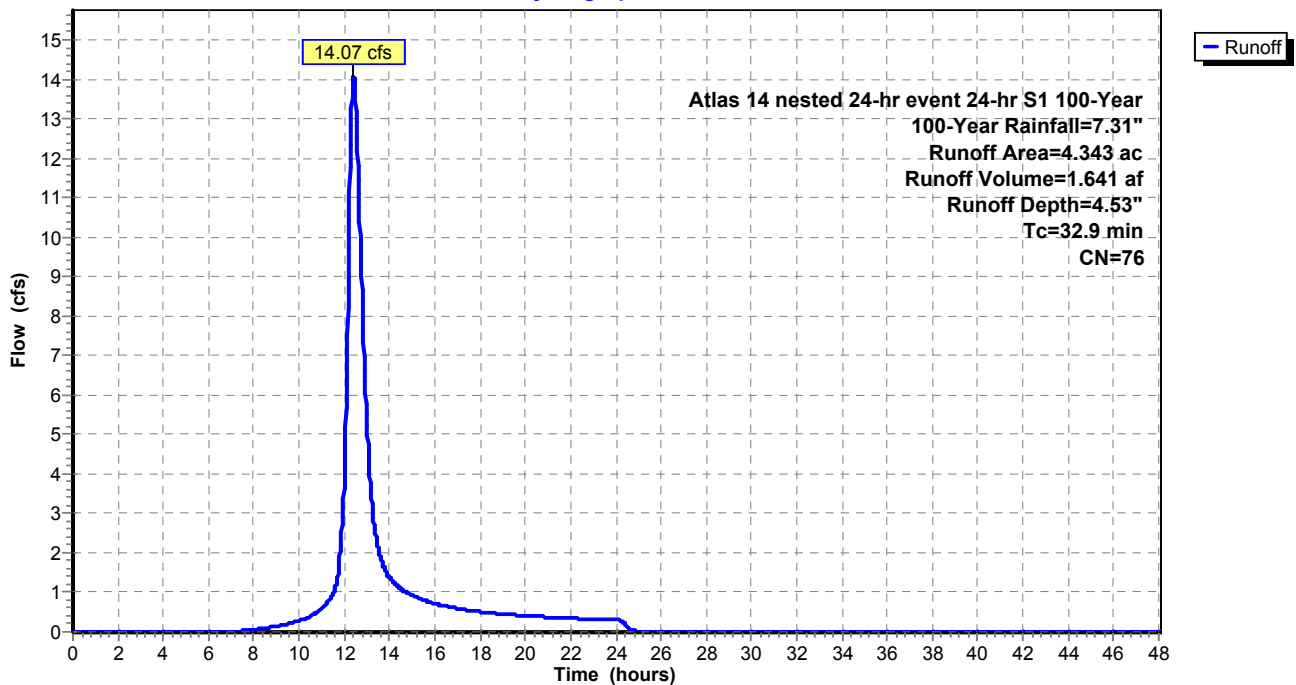
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 4.343	76	
4.343		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.9					Direct Entry,

Subcatchment 11S: Sub-basin 11

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 12S: Sub-basin 12

Runoff = 17.60 cfs @ 12.14 hrs, Volume= 1.374 af, Depth= 4.98"

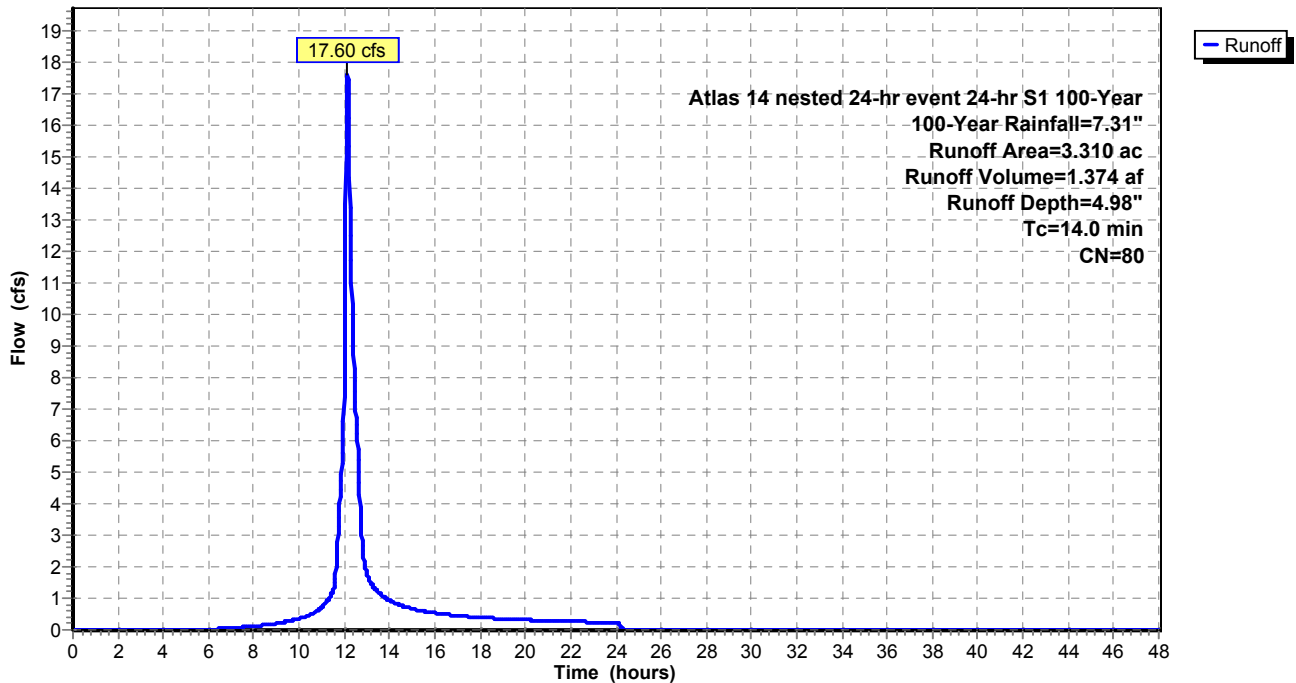
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 3.310	80	
3.310		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.0					Direct Entry,

Subcatchment 12S: Sub-basin 12

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 13S: Sub-basin 13

Runoff = 8.02 cfs @ 12.44 hrs, Volume= 0.989 af, Depth= 5.21"

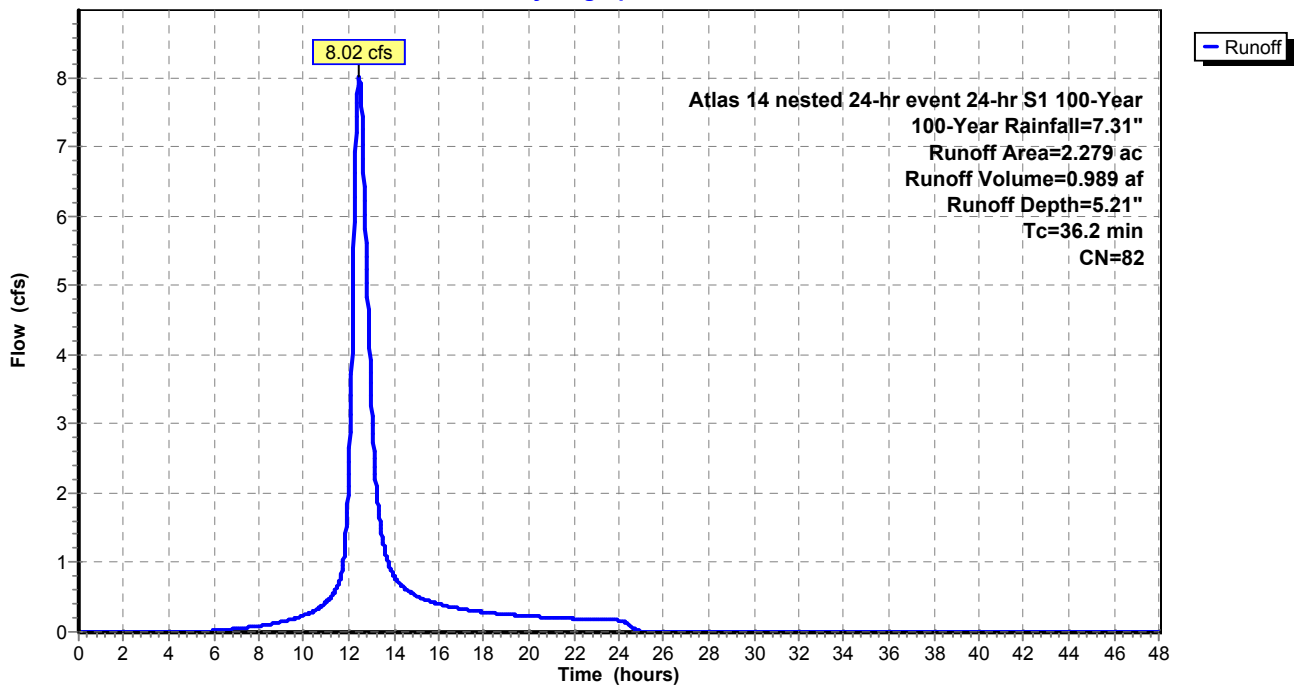
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 2.279	82	
2.279		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
36.2					Direct Entry,

Subcatchment 13S: Sub-basin 13

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 14S: Sub-basin 14

Runoff = 17.43 cfs @ 12.07 hrs, Volume= 1.141 af, Depth= 5.44"

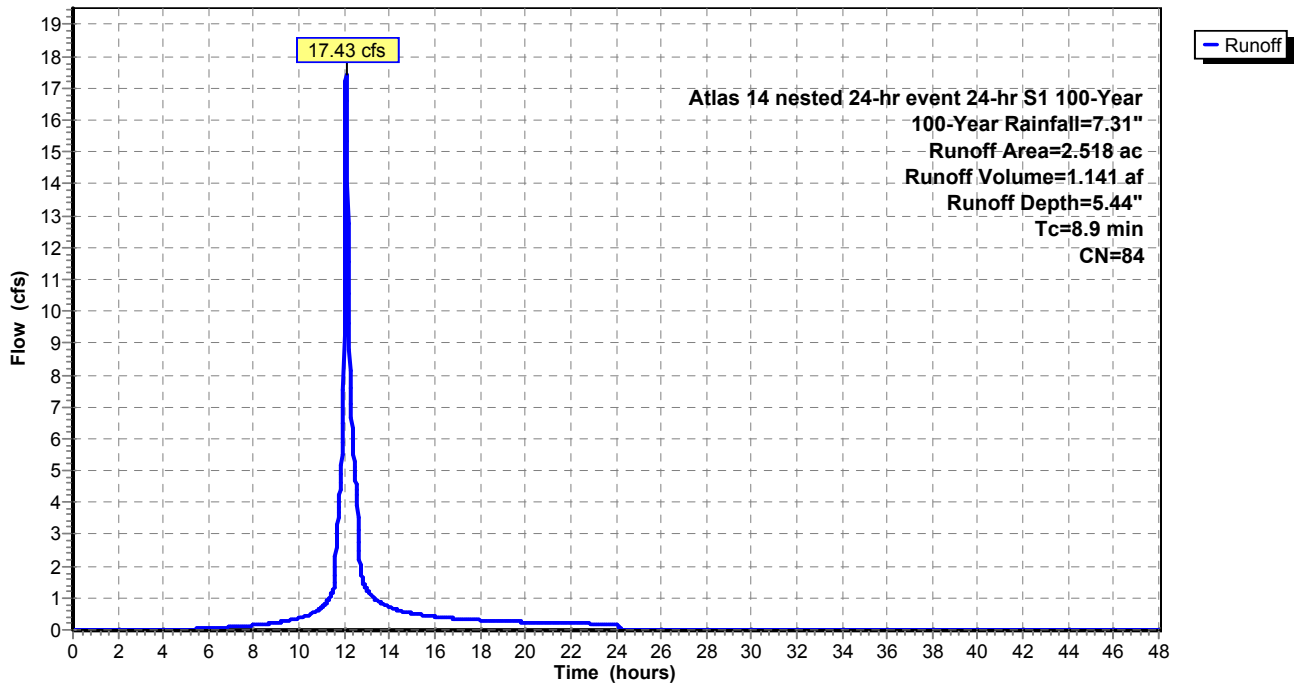
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 2.518	84	
2.518		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.9					Direct Entry,

Subcatchment 14S: Sub-basin 14

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 15S: Sub-basin 15

Runoff = 226.35 cfs @ 12.34 hrs, Volume= 24.522 af, Depth= 5.21"

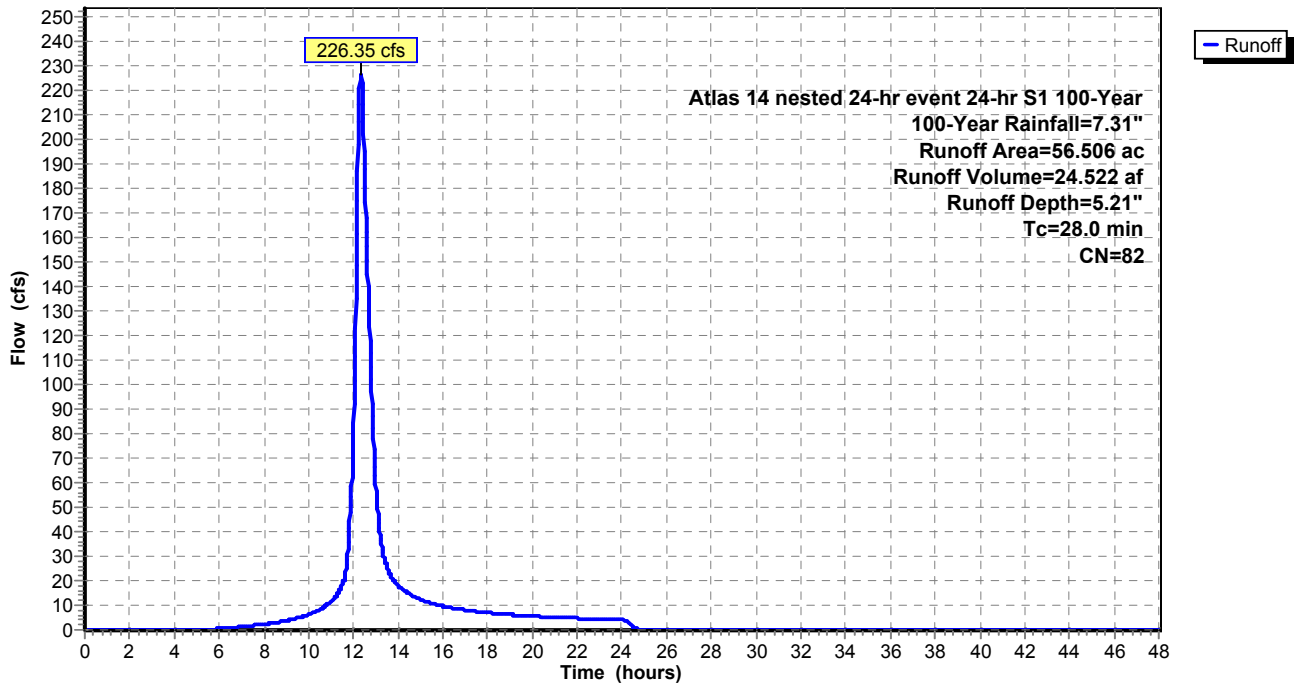
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 56.506	82	
56.506		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.0					Direct Entry,

Subcatchment 15S: Sub-basin 15

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 16S: Sub-basin 16

Runoff = 181.67 cfs @ 12.31 hrs, Volume= 19.018 af, Depth= 5.09"

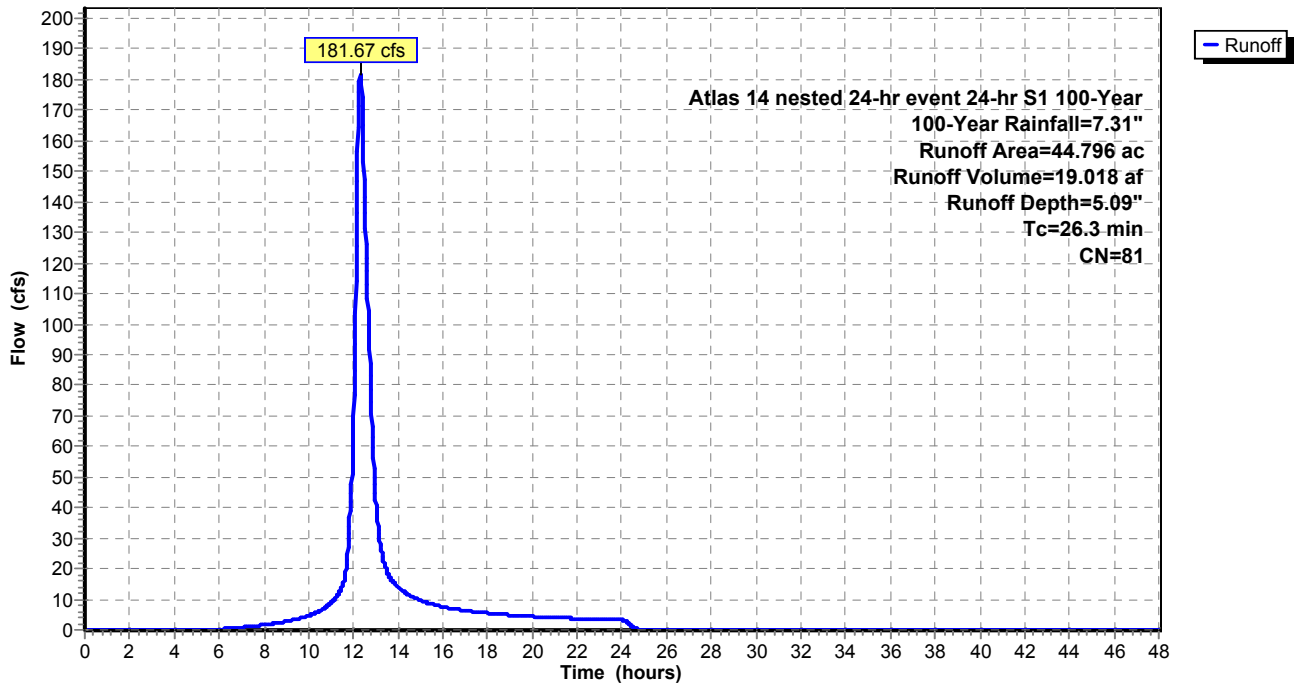
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 44.796	81	
44.796		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
26.3					Direct Entry,

Subcatchment 16S: Sub-basin 16

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 17S: Sub-basin 17

Runoff = 51.71 cfs @ 12.11 hrs, Volume= 3.781 af, Depth= 5.66"

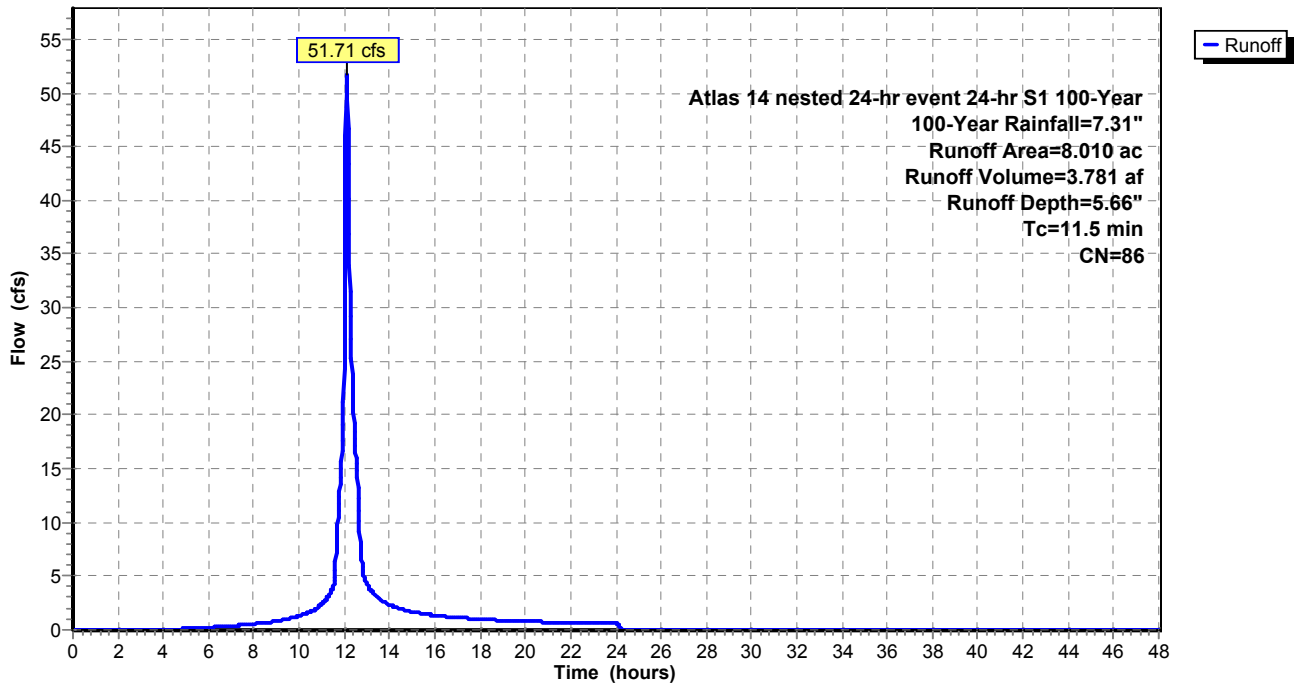
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 8.010	86	
8.010		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.5					Direct Entry,

Subcatchment 17S: Sub-basin 17

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 18S: Sub-basin 18

Runoff = 194.75 cfs @ 12.17 hrs, Volume= 16.160 af, Depth= 5.44"

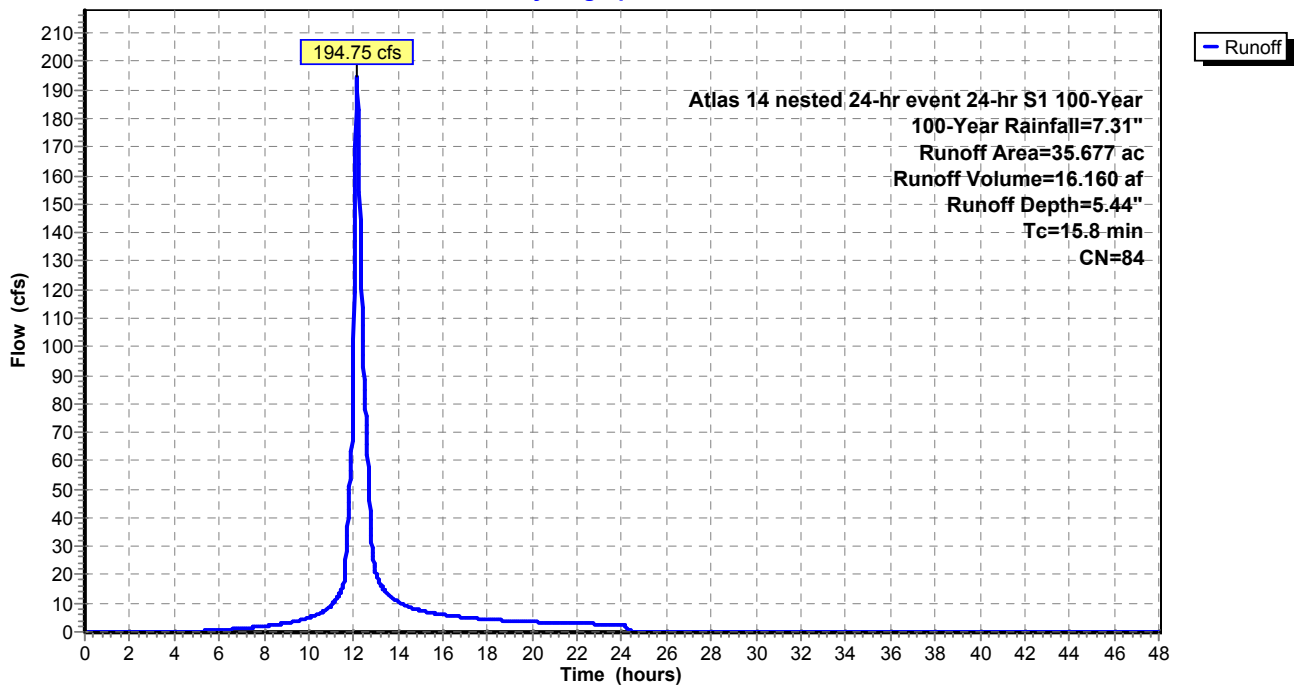
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 35.677	84	
35.677		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.8					Direct Entry,

Subcatchment 18S: Sub-basin 18

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 19S: Sub-basin 19

Runoff = 41.96 cfs @ 12.05 hrs, Volume= 2.522 af, Depth= 4.76"

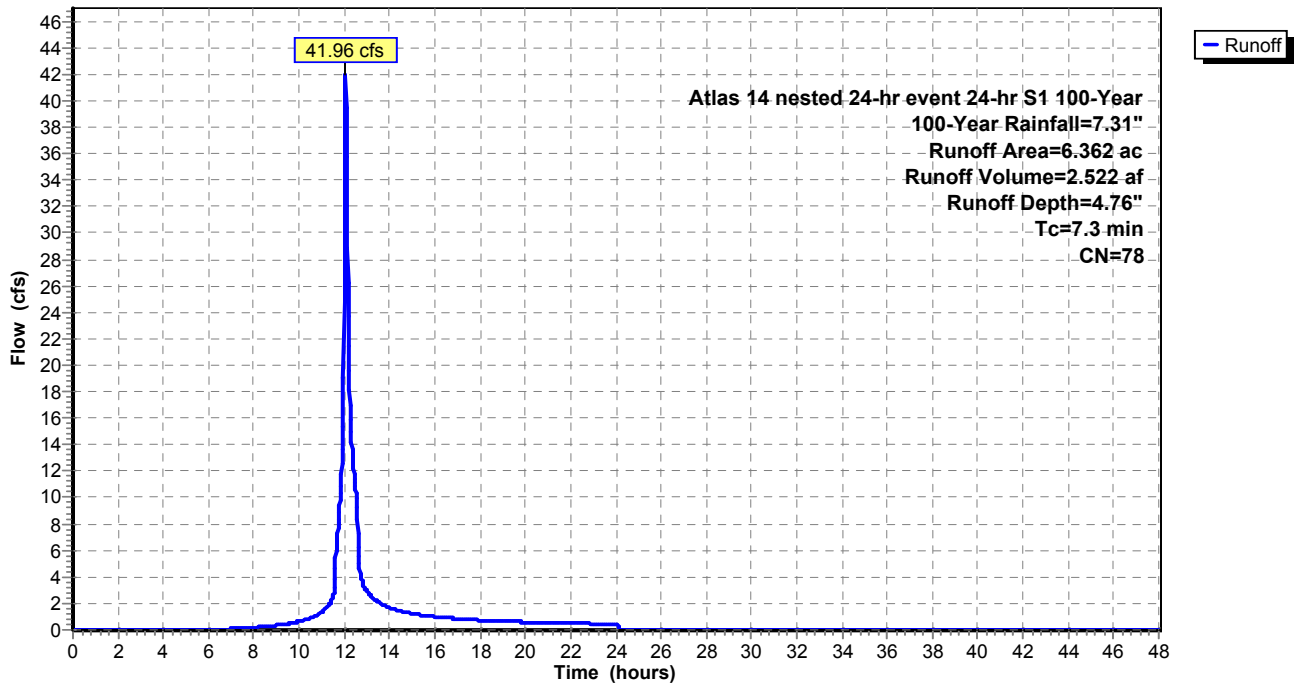
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 6.362	78	
6.362		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.3					Direct Entry,

Subcatchment 19S: Sub-basin 19

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 20S: Sub-basin 20

Runoff = 85.17 cfs @ 12.18 hrs, Volume= 7.352 af, Depth= 5.55"

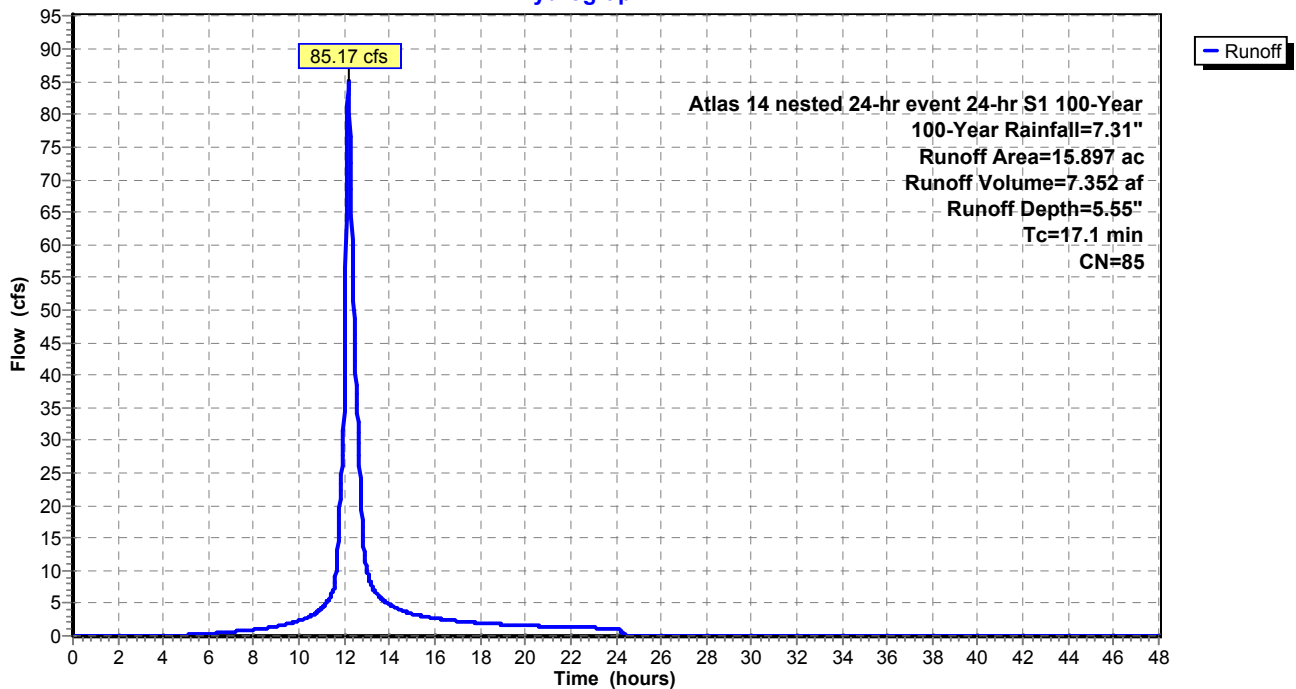
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 15.897	85	
15.897		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
17.1					Direct Entry,

Subcatchment 20S: Sub-basin 20

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 21S: Sub-basin 21

Runoff = 50.44 cfs @ 12.10 hrs, Volume= 3.654 af, Depth= 6.13"

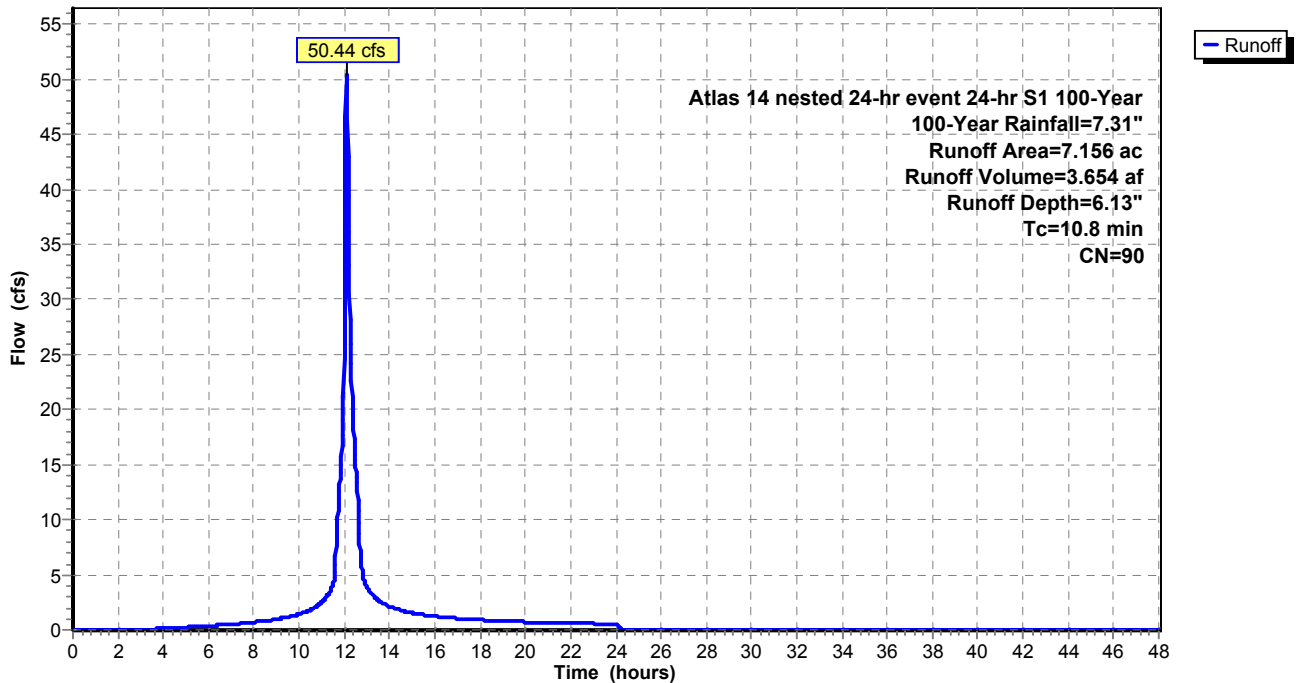
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 7.156	90	
7.156		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.8					Direct Entry,

Subcatchment 21S: Sub-basin 21

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 22S: Sub-basin 22

Runoff = 126.46 cfs @ 12.22 hrs, Volume= 11.521 af, Depth= 5.21"

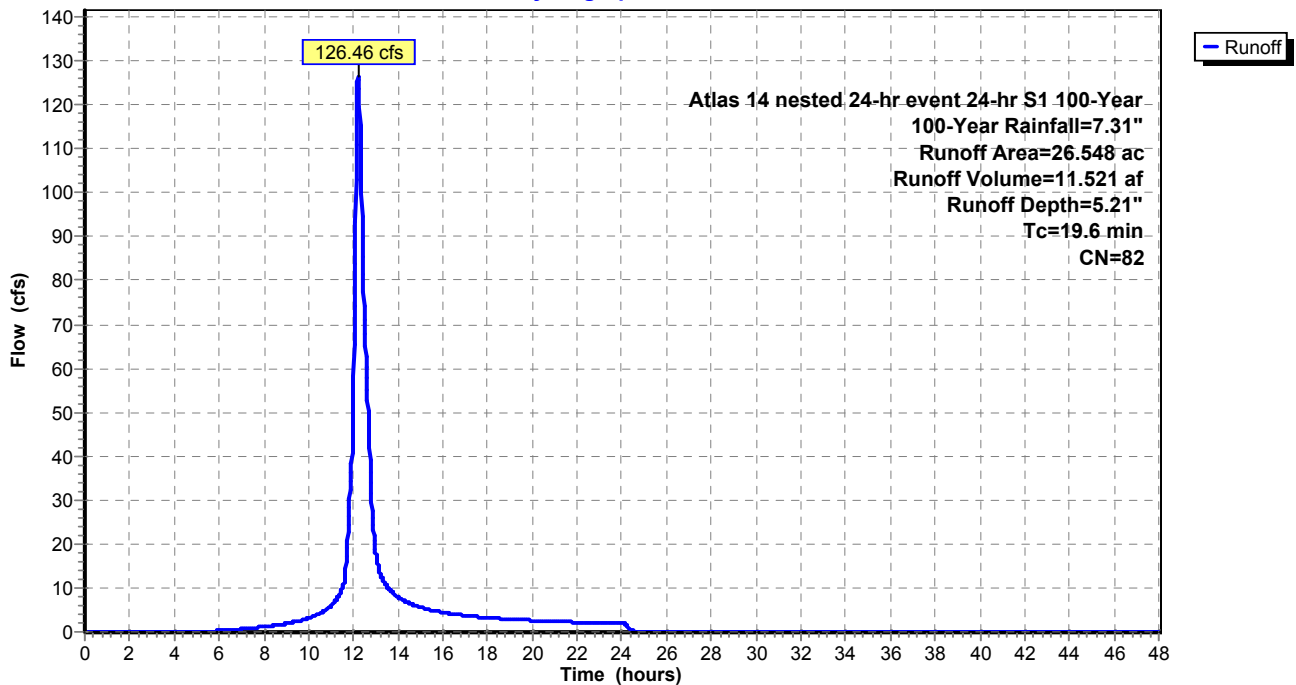
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 26.548	82	
26.548		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.6					Direct Entry,

Subcatchment 22S: Sub-basin 22

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 23S: Sub-basin 23

Runoff = 104.89 cfs @ 12.08 hrs, Volume= 7.329 af, Depth= 6.36"

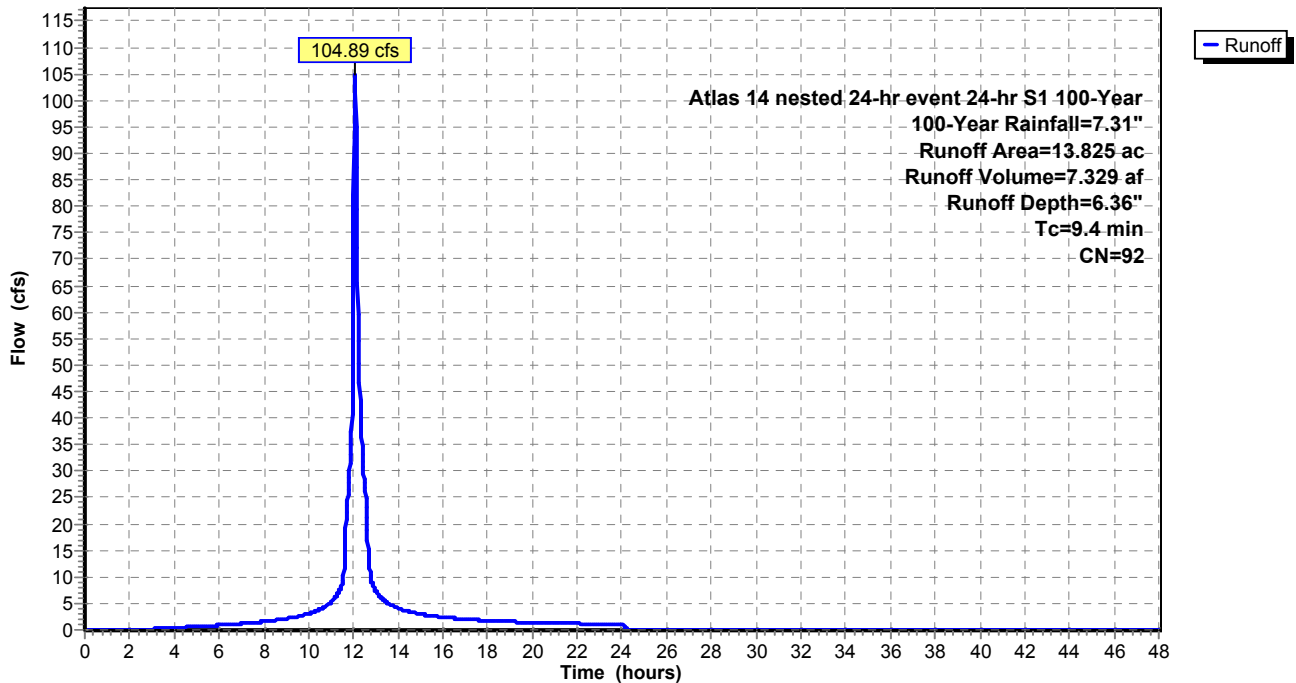
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 13.825	92	
13.825		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.4					Direct Entry,

Subcatchment 23S: Sub-basin 23

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 24S: Sub-basin 24

Runoff = 70.75 cfs @ 12.21 hrs, Volume= 6.421 af, Depth= 5.55"

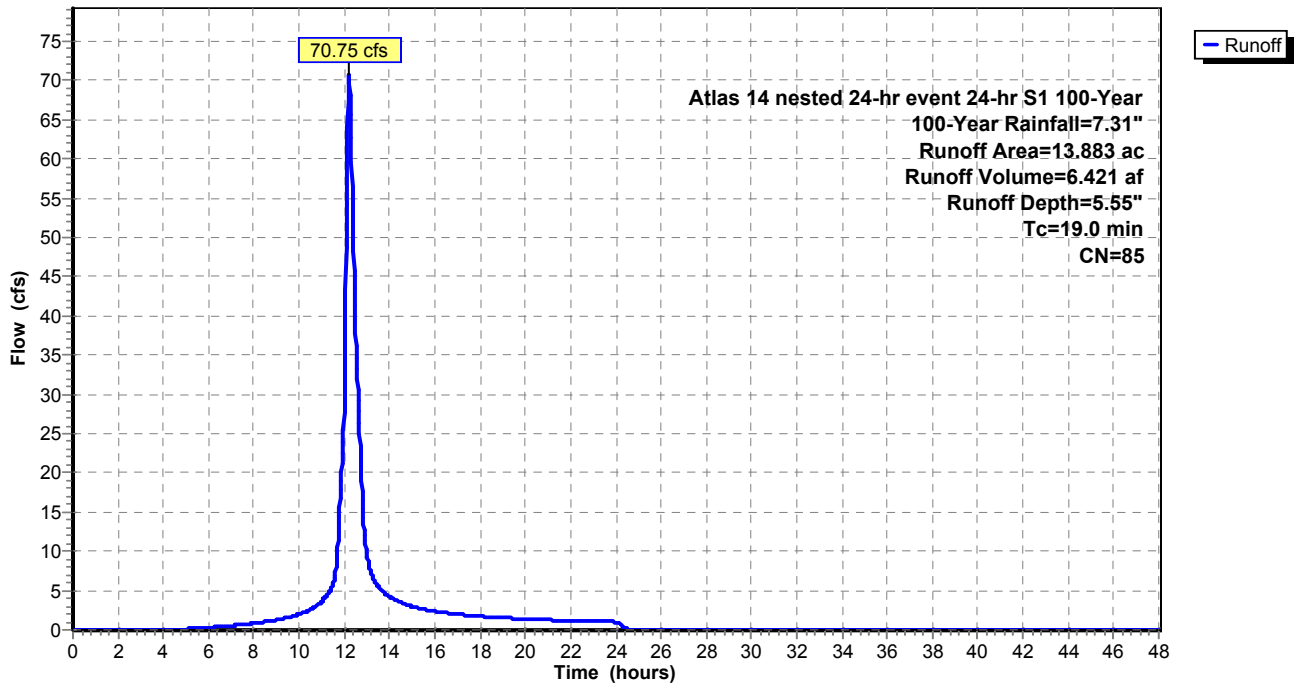
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 13.883	85	
13.883		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.0					Direct Entry,

Subcatchment 24S: Sub-basin 24

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 25S: Sub-basin 25

Runoff = 25.47 cfs @ 12.27 hrs, Volume= 2.454 af, Depth= 4.42"

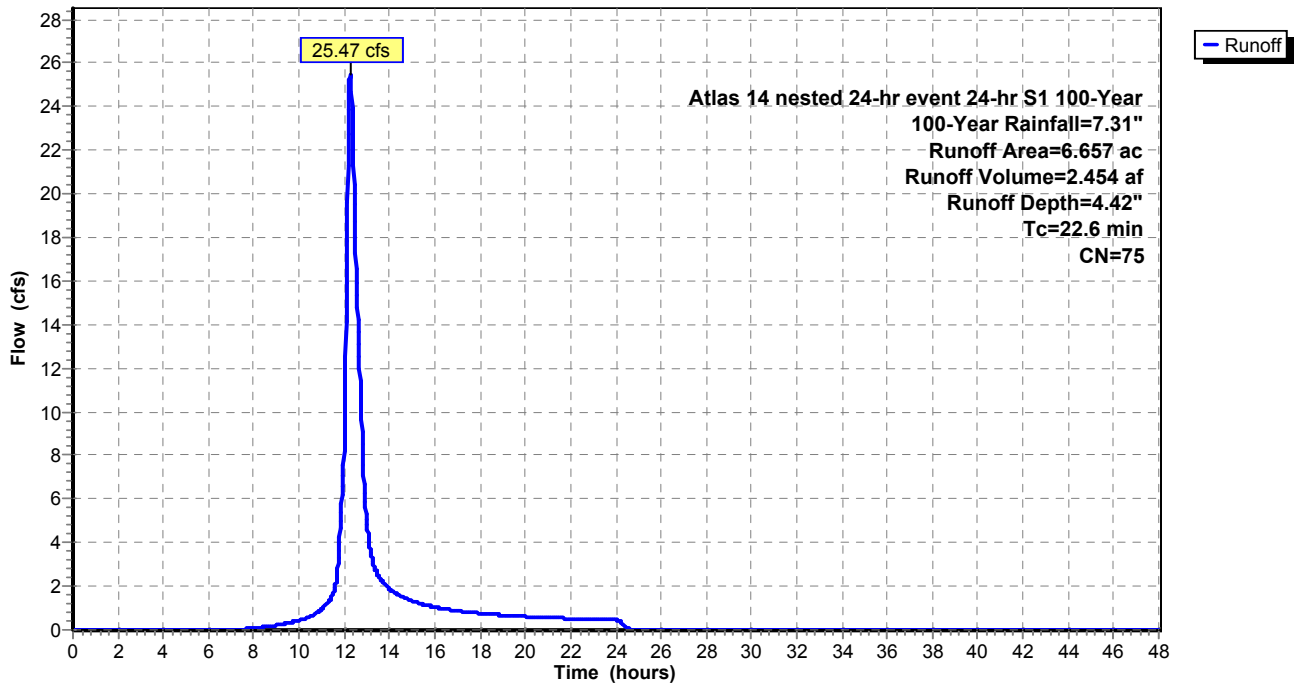
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 6.657	75	
6.657		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
22.6					Direct Entry,

Subcatchment 25S: Sub-basin 25

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 26S: Sub-basin 26

Runoff = 1.86 cfs @ 12.52 hrs, Volume= 0.237 af, Depth= 3.45"

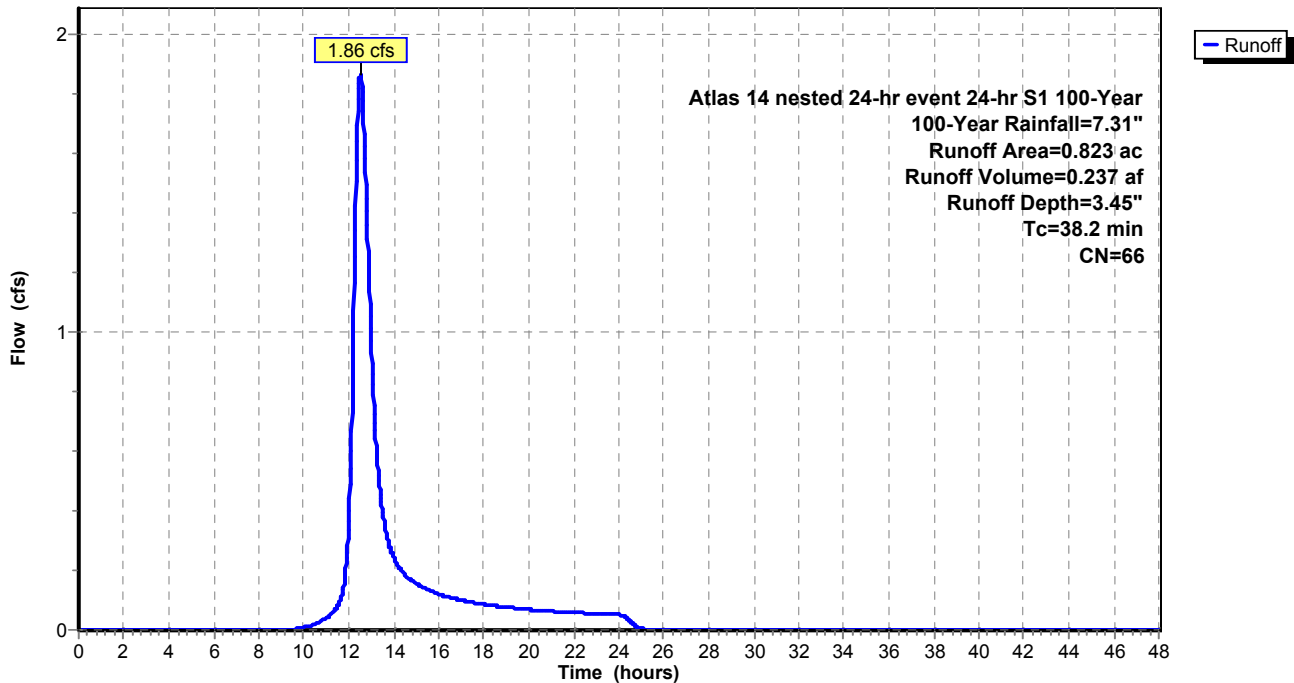
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 0.823	66	
0.823		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
38.2					Direct Entry,

Subcatchment 26S: Sub-basin 26

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 27S: Sub-basin 27

Runoff = 7.67 cfs @ 12.14 hrs, Volume= 0.586 af, Depth= 3.45"

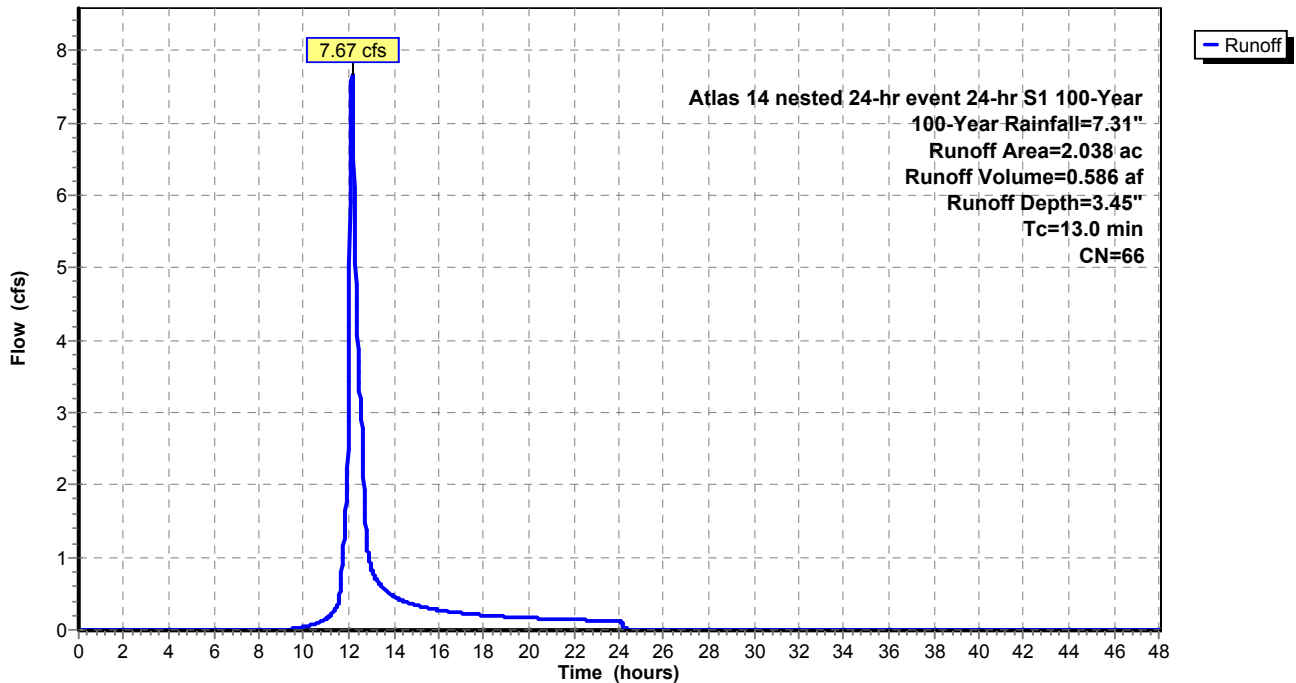
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 2.038	66	
2.038		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.0					Direct Entry,

Subcatchment 27S: Sub-basin 27

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 28S: Sub-basin 28

Runoff = 4.65 cfs @ 12.38 hrs, Volume= 0.626 af, Depth= 1.30"

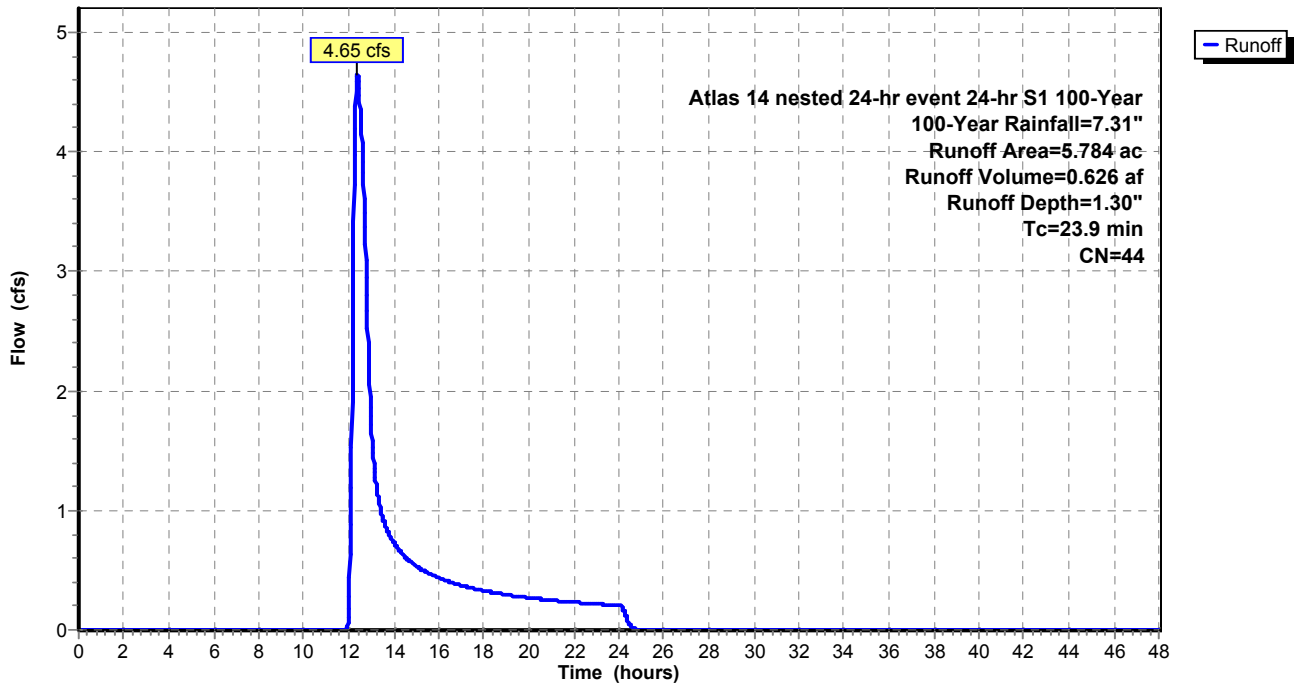
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 5.784	44	
5.784		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.9					Direct Entry,

Subcatchment 28S: Sub-basin 28

Hydrograph



Summary for Subcatchment 29S: Sub-basin 29

Runoff = 0.54 cfs @ 12.57 hrs, Volume= 0.092 af, Depth= 0.88"

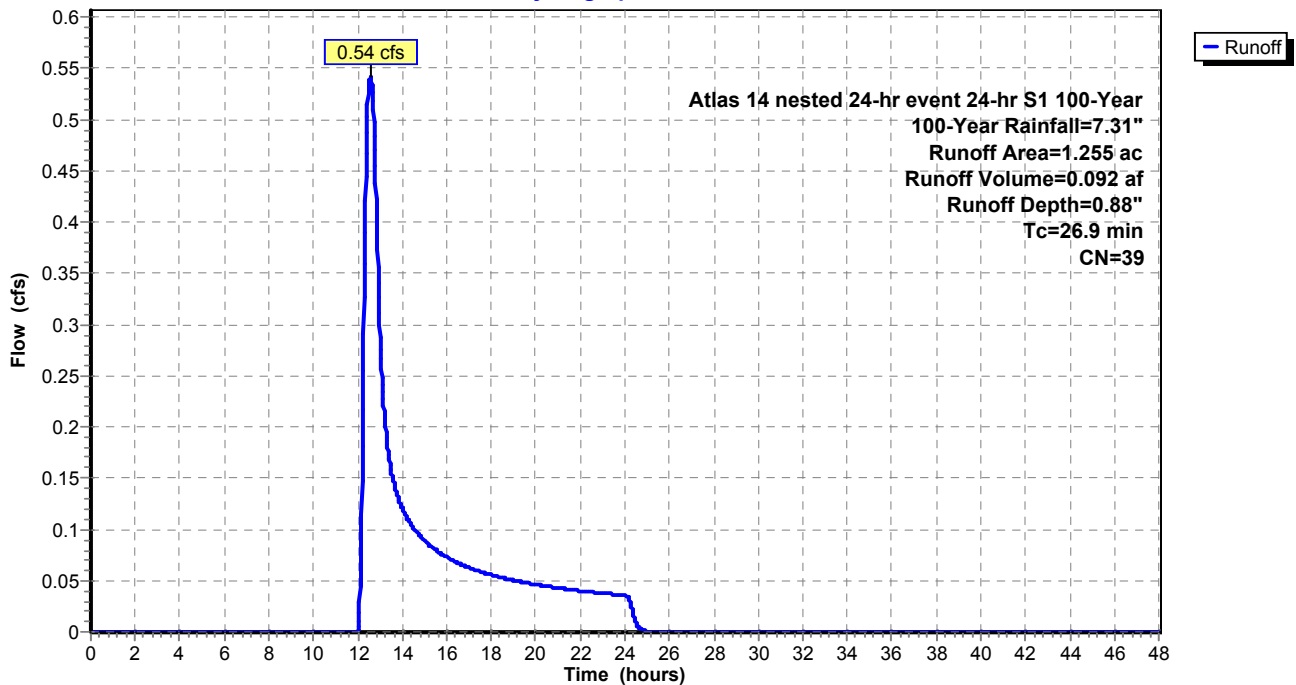
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 1.255	39	
1.255		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
26.9					Direct Entry,

Subcatchment 29S: Sub-basin 29

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 30S: Sub-basin 30

Runoff = 15.95 cfs @ 12.79 hrs, Volume= 2.966 af, Depth= 1.13"

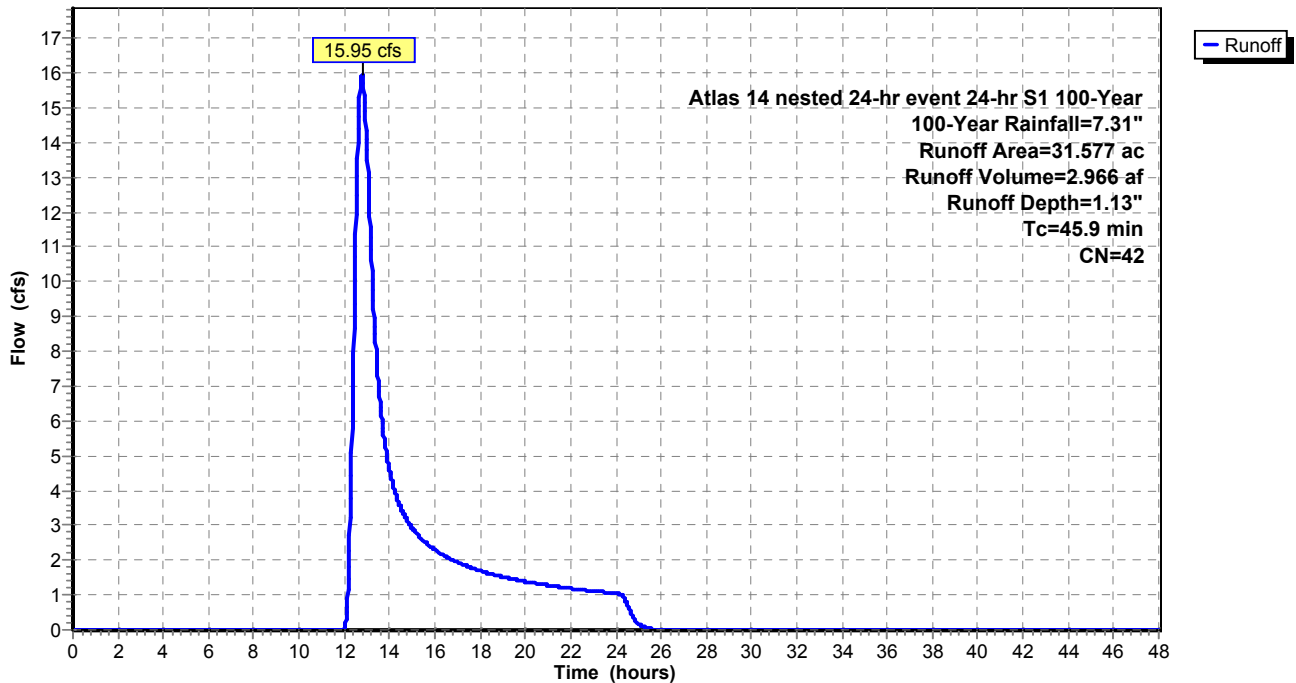
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 31.577	42	
31.577		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
45.9					Direct Entry,

Subcatchment 30S: Sub-basin 30

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 31S: Sub-basin 31

Runoff = 0.37 cfs @ 12.61 hrs, Volume= 0.065 af, Depth= 0.88"

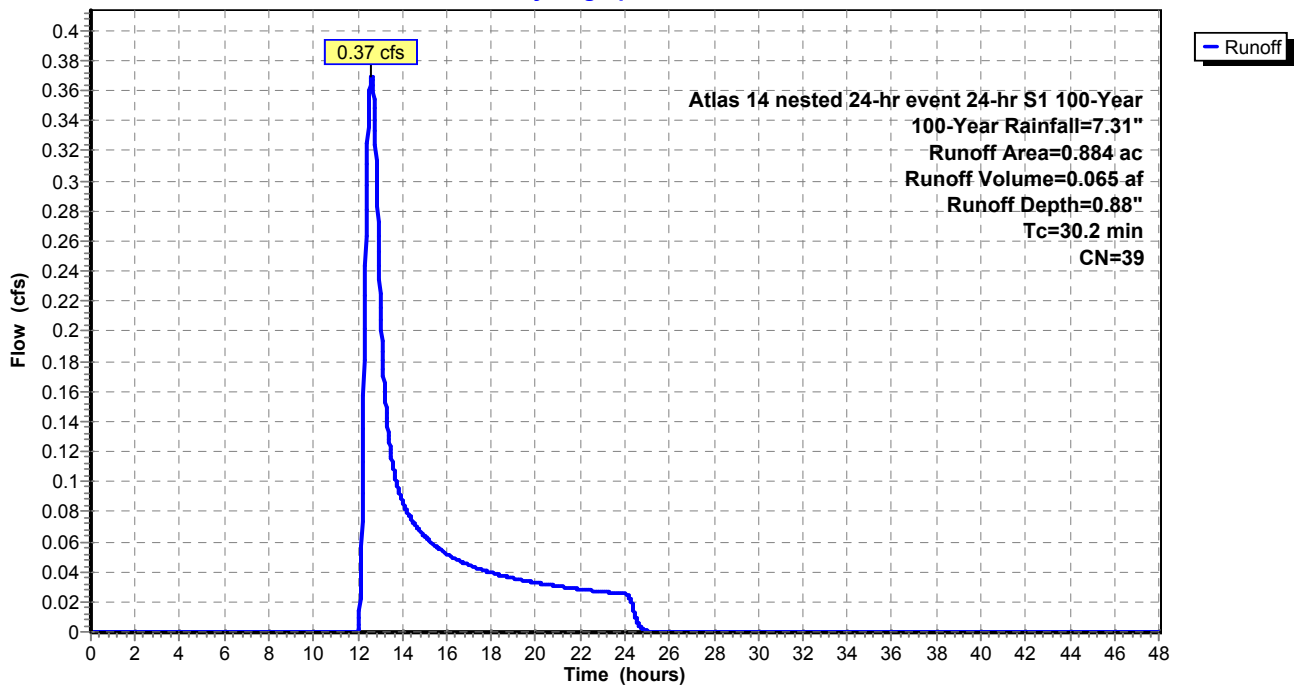
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 0.884	39	
0.884		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.2					Direct Entry,

Subcatchment 31S: Sub-basin 31

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 32S: Sub-basin 32

Runoff = 0.38 cfs @ 12.57 hrs, Volume= 0.065 af, Depth= 0.88"

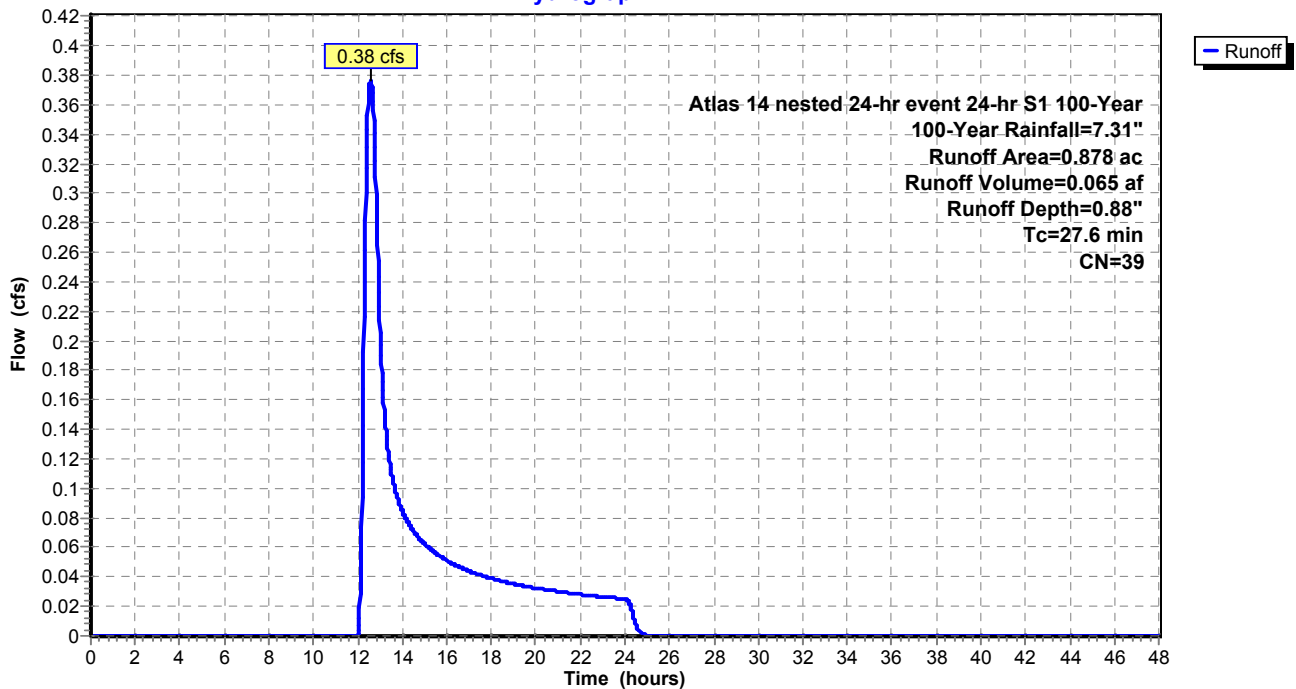
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 0.878	39	
0.878		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.6					Direct Entry,

Subcatchment 32S: Sub-basin 32

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 33S: Sub-basin 33

Runoff = 4.03 cfs @ 12.28 hrs, Volume= 0.447 af, Depth= 1.66"

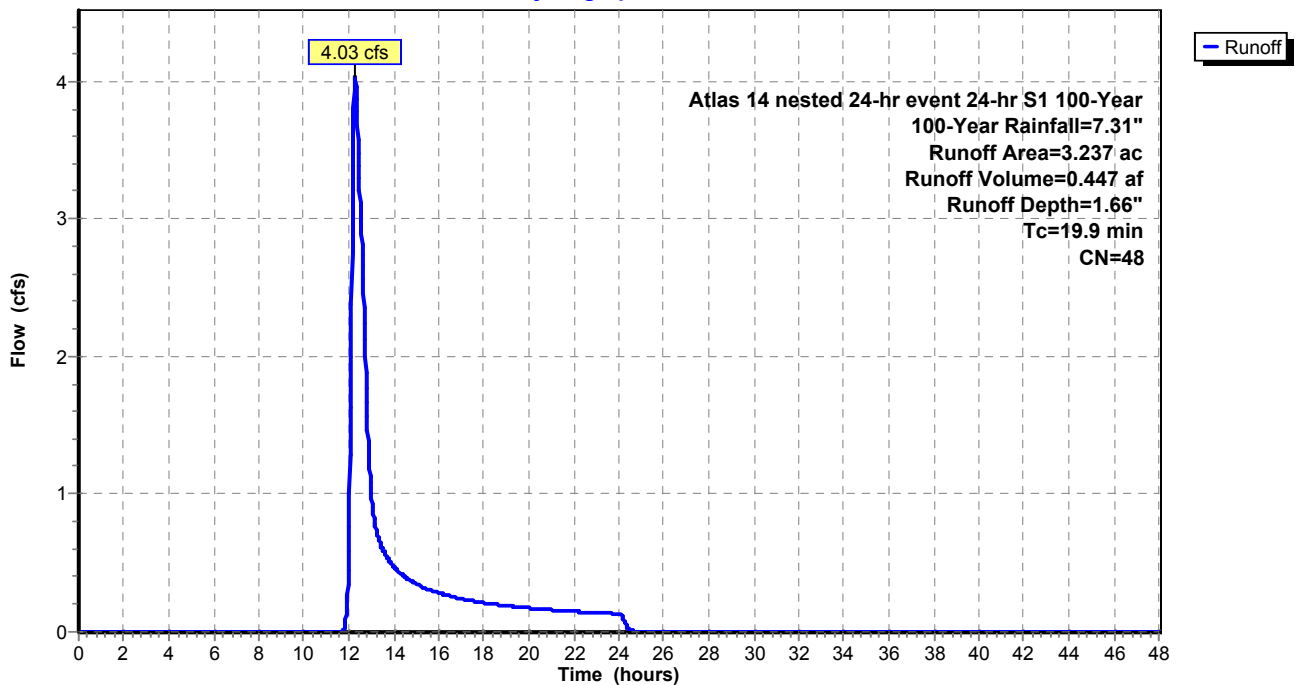
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 3.237	48	
3.237		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.9					Direct Entry,

Subcatchment 33S: Sub-basin 33

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 34S: Sub-basin 34

Runoff = 2.05 cfs @ 12.15 hrs, Volume= 0.181 af, Depth= 1.75"

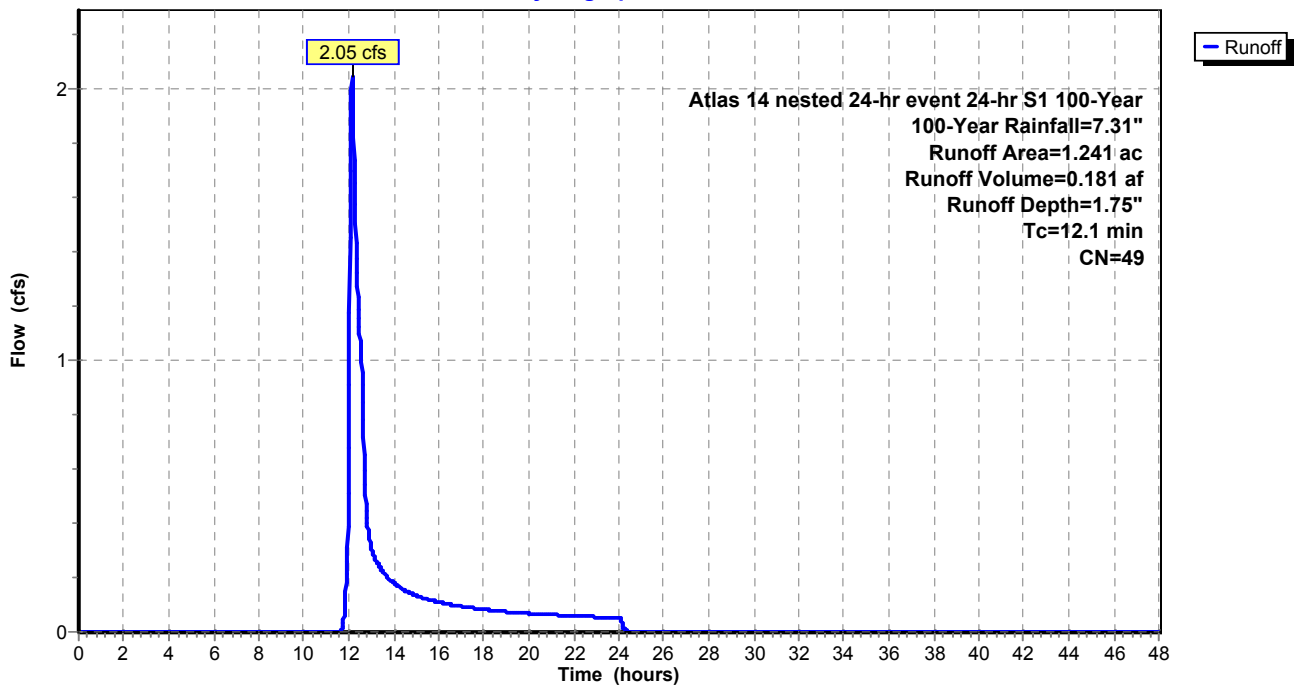
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 1.241	49	
1.241		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.1					Direct Entry,

Subcatchment 34S: Sub-basin 34

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 35S: Sub-basin 35

Runoff = 5.16 cfs @ 12.26 hrs, Volume= 0.629 af, Depth= 1.21"

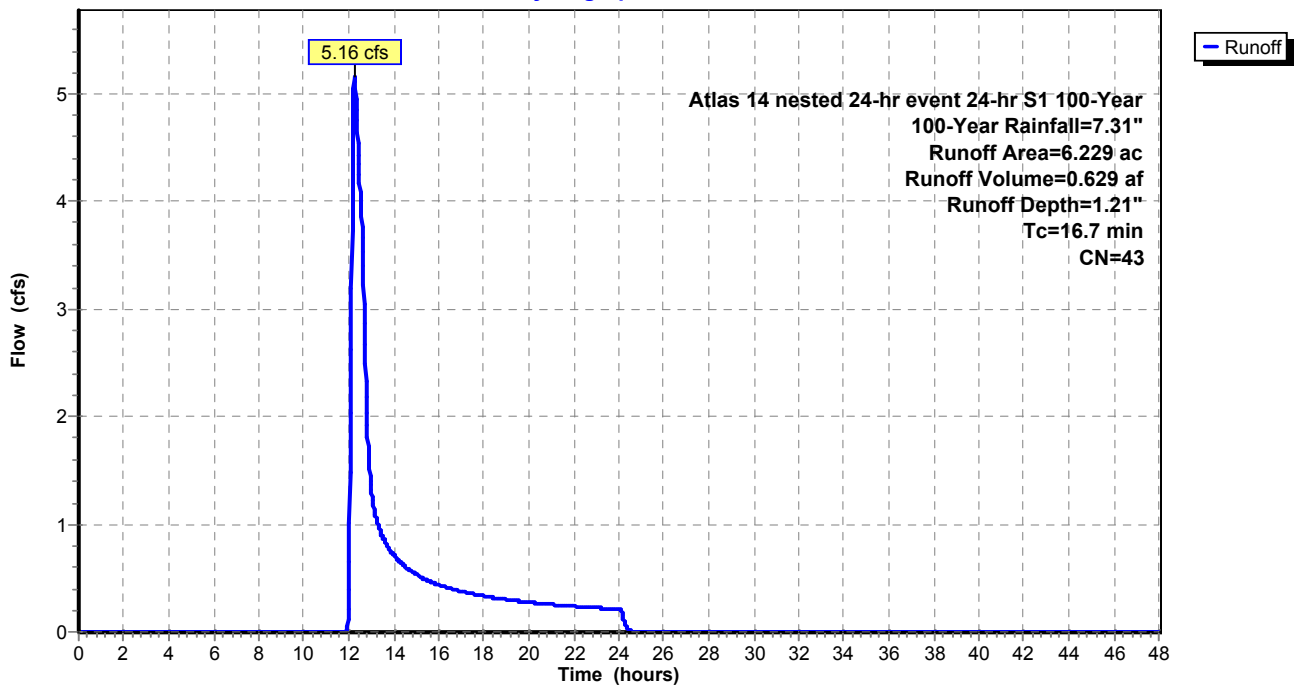
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 6.229	43	
6.229		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.7					Direct Entry,

Subcatchment 35S: Sub-basin 35

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 36S: Sub-basin 36

Runoff = 30.42 cfs @ 12.69 hrs, Volume= 4.549 af, Depth= 4.87"

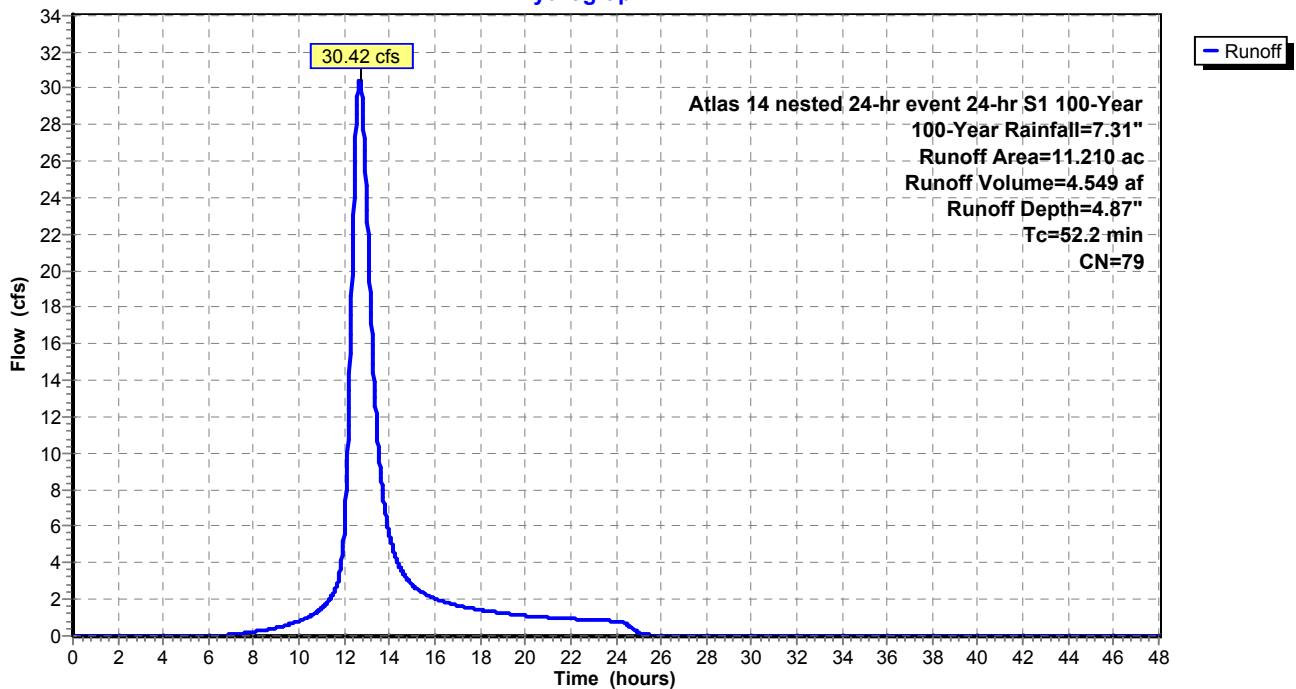
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 11.210	79	
11.210		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
52.2					Direct Entry,

Subcatchment 36S: Sub-basin 36

Hydrograph



Existing Conditions_HyAtlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

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Summary for Subcatchment 83S: County Road H Subbasin Redirected After Regrading

Runoff = 27.19 cfs @ 12.21 hrs, Volume= 2.440 af, Depth= 4.98"

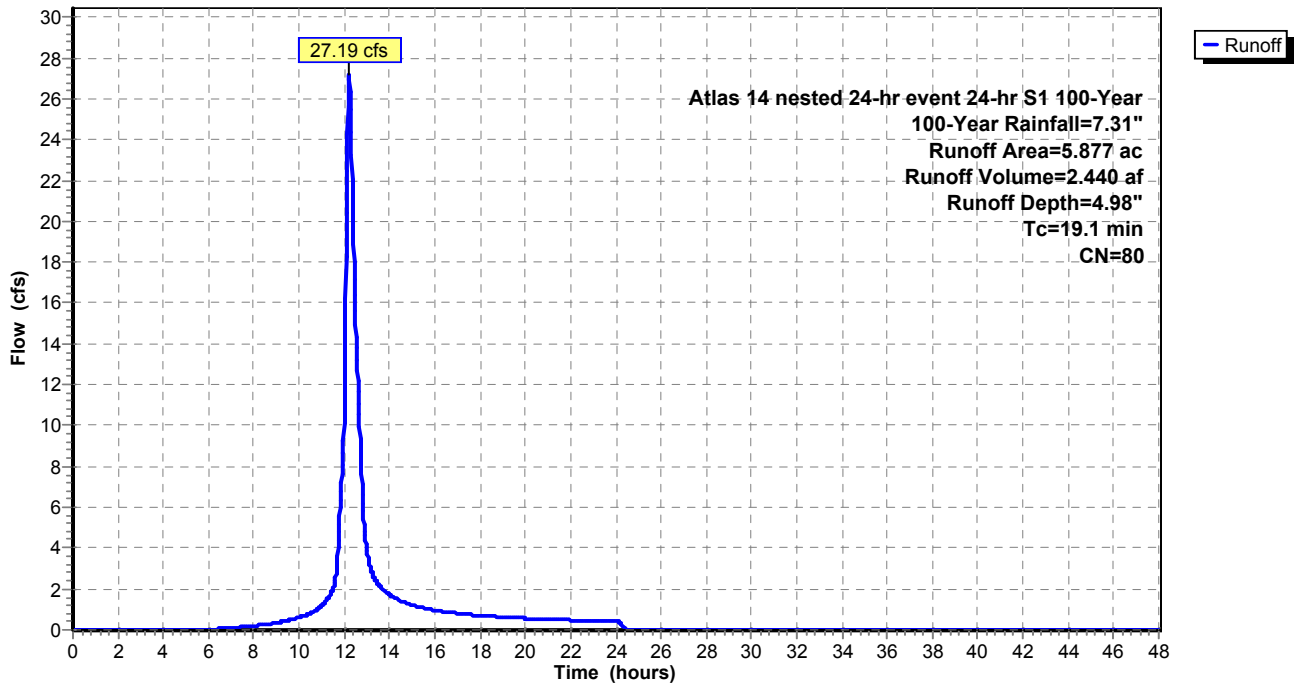
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 5.877	80	
5.877		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.1					Direct Entry,

Subcatchment 83S: County Road H Subbasin Redirected After Regrading

Hydrograph



Summary for Reach 37R: Outfall of SB 2, 3, 7

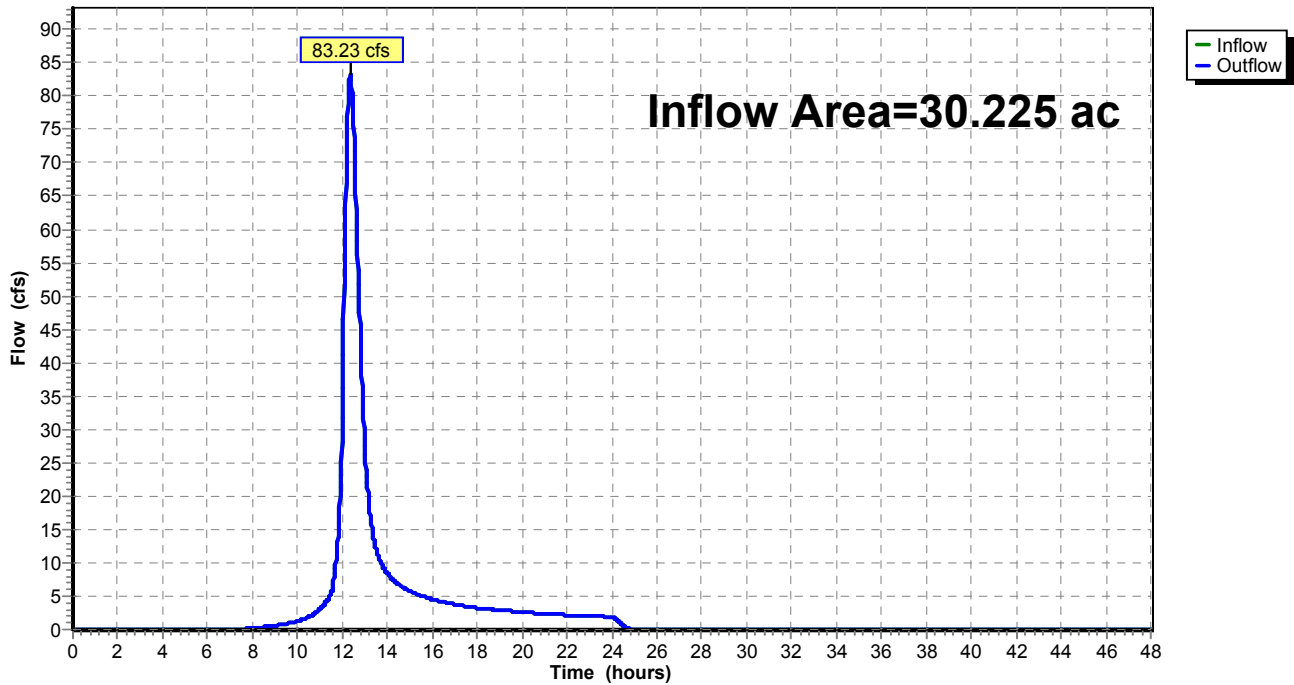
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 30.225 ac, 0.00% Impervious, Inflow Depth = 3.97" for 100-Year event
Inflow = 83.23 cfs @ 12.36 hrs, Volume= 10.007 af
Outflow = 83.23 cfs @ 12.36 hrs, Volume= 10.007 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 37R: Outfall of SB 2, 3, 7

Hydrograph



Summary for Reach 39R: Outfall of SB 1, 4, 5, 6, 9, 10, 11, 36

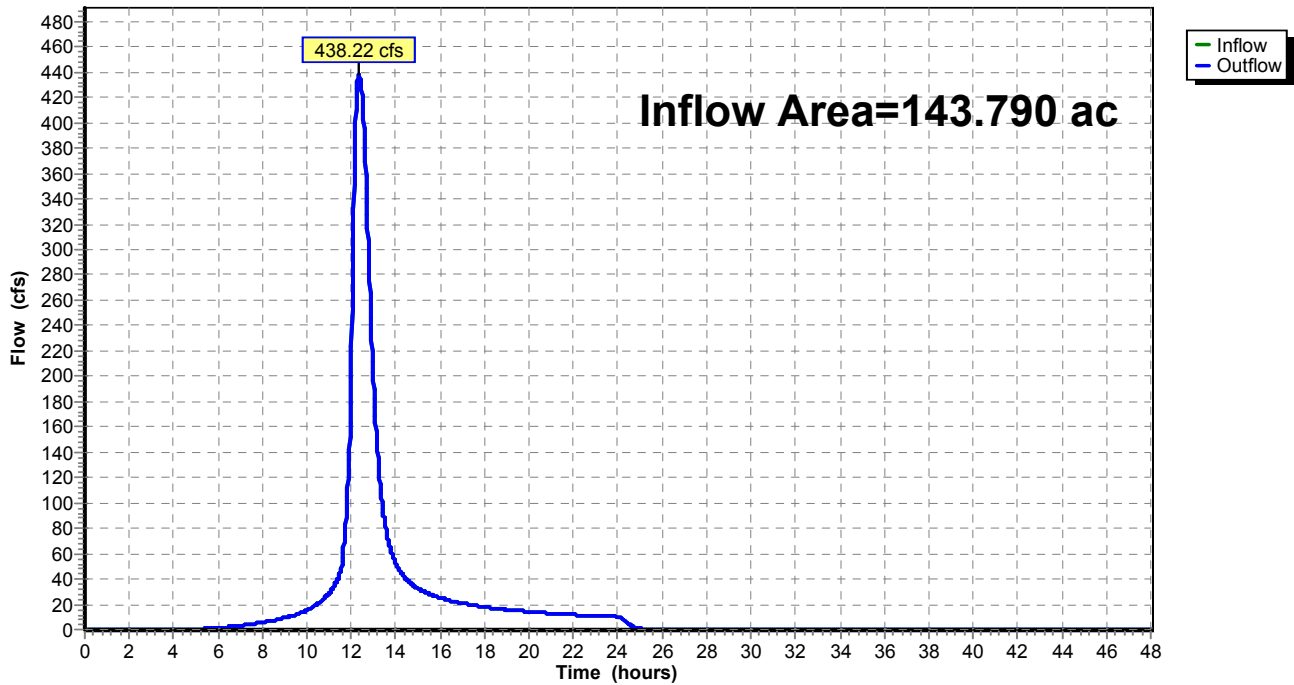
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 143.790 ac, 0.00% Impervious, Inflow Depth = 5.21" for 100-Year event
Inflow = 438.22 cfs @ 12.34 hrs, Volume= 62.386 af
Outflow = 438.22 cfs @ 12.34 hrs, Volume= 62.386 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 39R: Outfall of SB 1, 4, 5, 6, 9, 10, 11, 36

Hydrograph



Summary for Reach 40R: 60 in SB 4

[52] Hint: Inlet/Outlet conditions not evaluated

[65] Warning: Inlet elevation not specified

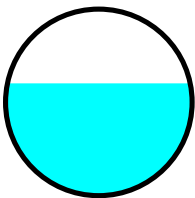
[88] Warning: Qout>Qin may require Finer Routing>1

Inflow Area = 143.790 ac, 0.00% Impervious, Inflow Depth = 5.21" for 100-Year event
Inflow = 438.10 cfs @ 12.33 hrs, Volume= 62.386 af
Outflow = 438.22 cfs @ 12.34 hrs, Volume= 62.386 af, Atten= 0%, Lag= 0.5 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
Reference Flow= 328.58 cfs Estimated Depth= 3.07' Velocity= 26.02 fps
m= 1.363, c= 35.46 fps, dt= 1.2 min, dx= 718.0' / 1 = 718.0', K= 0.3 min, X= 0.453
Max. Velocity= 35.97 fps, Min. Travel Time= 0.3 min
Avg. Velocity = 35.46 fps, Avg. Travel Time= 0.3 min

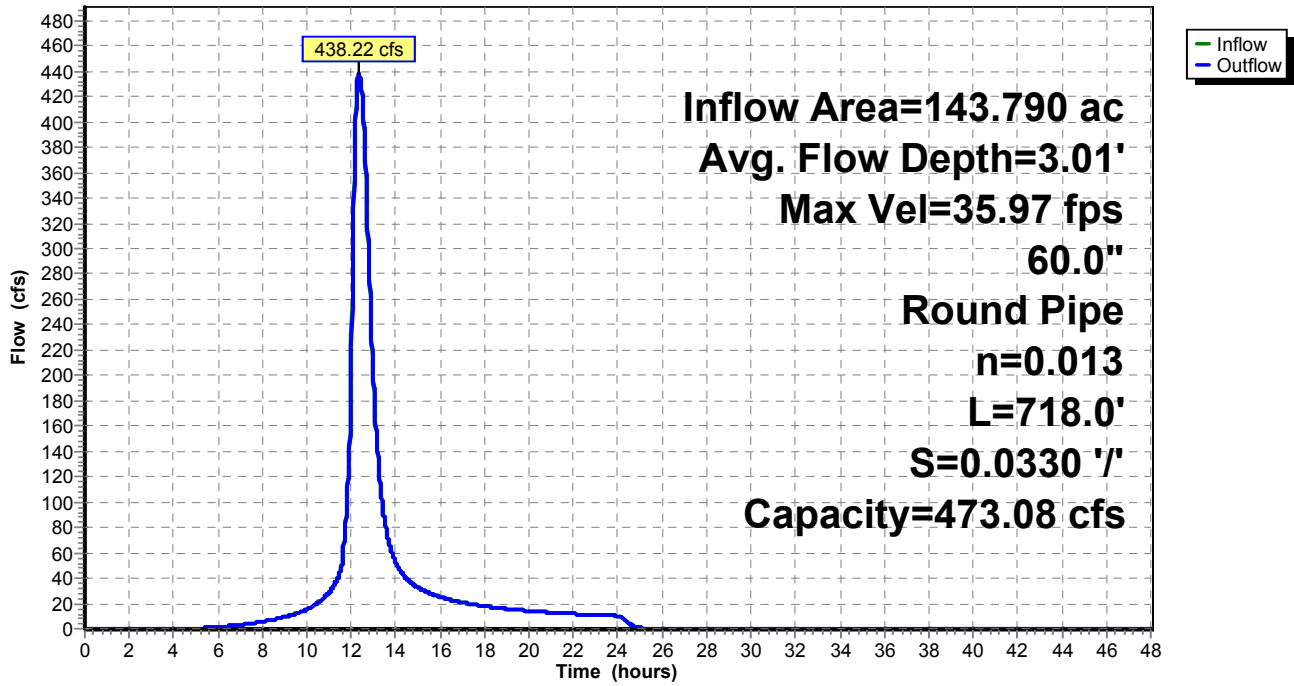
Peak Storage= 8,871 cf @ 12.34 hrs
Average Depth at Peak Storage= 3.01'
Bank-Full Depth= 5.00' Flow Area= 19.6 sf, Capacity= 473.08 cfs

60.0" Round Pipe
n= 0.013
Length= 718.0' Slope= 0.0330 '/'
Inlet Invert= 0.00', Outlet Invert= -23.69'



Reach 40R: 60 in SB 4

Hydrograph



Summary for Reach 41R: Channel in SB 9, 10

[65] Warning: Inlet elevation not specified

Inflow Area = 9.296 ac, 0.00% Impervious, Inflow Depth = 5.21" for 100-Year event
Inflow = 53.64 cfs @ 12.13 hrs, Volume= 4.034 af
Outflow = 49.75 cfs @ 12.29 hrs, Volume= 4.034 af, Atten= 7%, Lag= 10.1 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
Reference Flow= 40.23 cfs Estimated Depth= 1.06' Velocity= 1.87 fps
m= 1.523, c= 2.85 fps, dt= 1.2 min, dx= 1,660.0' / 8 = 207.5', K= 1.2 min, X= 0.150
Max. Velocity= 10.60 fps, Min. Travel Time= 2.6 min
Avg. Velocity = 2.90 fps, Avg. Travel Time= 9.5 min

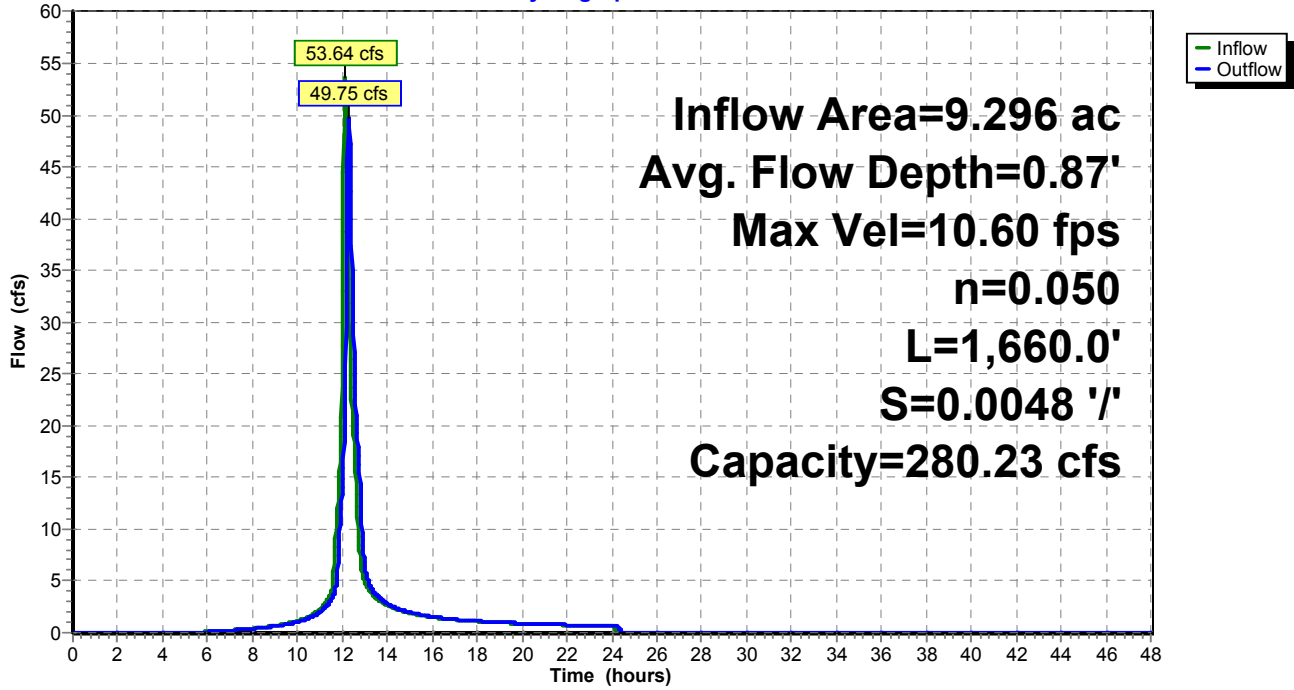
Peak Storage= 27,957 cf @ 12.22 hrs
Average Depth at Peak Storage= 0.87'
Bank-Full Depth= 3.00' Flow Area= 84.0 sf, Capacity= 280.23 cfs

16.00' x 3.00' deep channel, n= 0.050
Side Slope Z-value= 4.0 ' ' Top Width= 40.00'
Length= 1,660.0' Slope= 0.0048 ' '
Inlet Invert= 0.00', Outlet Invert= -7.97'



Reach 41R: Channel in SB 9, 10

Hydrograph



Summary for Reach 46R: Channel SB1

[65] Warning: Inlet elevation not specified

Inflow Area = 15.328 ac, 0.00% Impervious, Inflow Depth = 5.78" for 100-Year event
Inflow = 86.42 cfs @ 12.17 hrs, Volume= 7.383 af
Outflow = 85.24 cfs @ 12.24 hrs, Volume= 7.383 af, Atten= 1%, Lag= 3.8 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
Reference Flow= 64.81 cfs Estimated Depth= 1.37' Velocity= 2.57 fps
m= 1.479, c= 3.80 fps, dt= 1.2 min, dx= 841.0' / 3 = 280.3', K= 1.2 min, X= 0.268
Max. Velocity= 6.91 fps, Min. Travel Time= 2.0 min
Avg. Velocity = 3.81 fps, Avg. Travel Time= 3.7 min

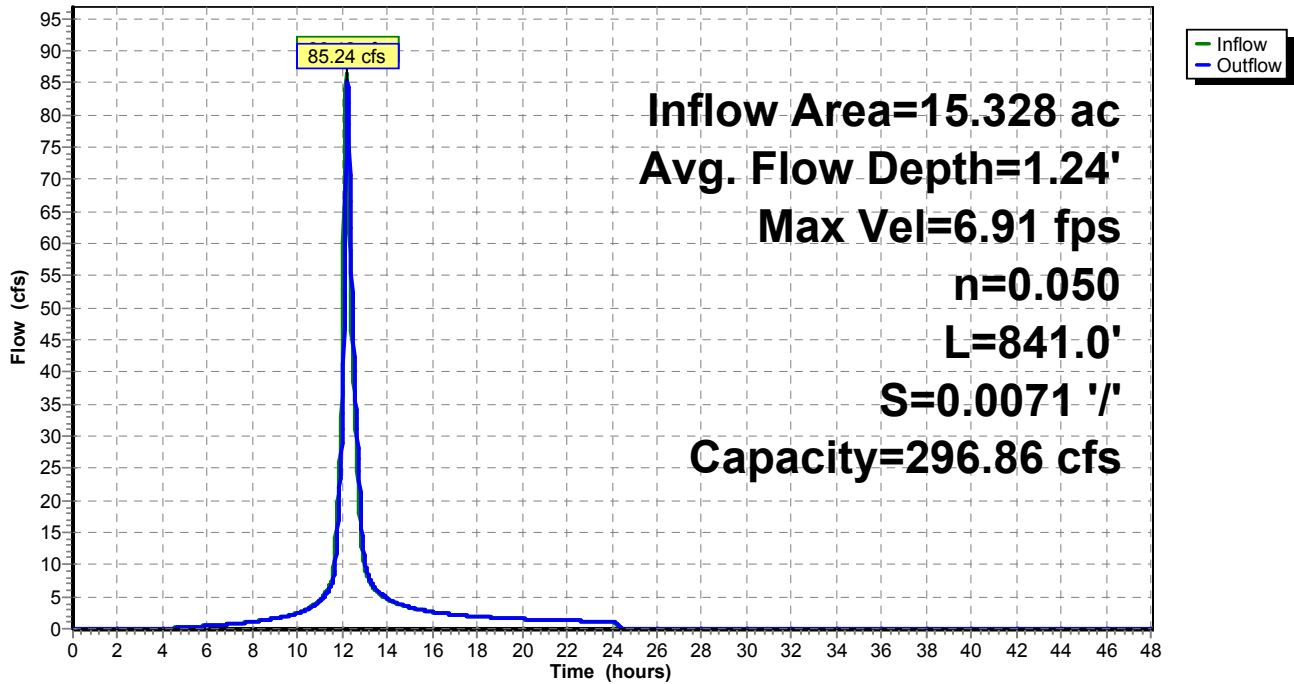
Peak Storage= 18,802 cf @ 12.21 hrs
Average Depth at Peak Storage= 1.24'
Bank-Full Depth= 3.00' Flow Area= 75.0 sf, Capacity= 296.86 cfs

13.00' x 3.00' deep channel, n= 0.050
Side Slope Z-value= 4.0 ' ' Top Width= 37.00'
Length= 841.0' Slope= 0.0071 ' '
Inlet Invert= 0.00', Outlet Invert= -5.97'



Reach 46R: Channel SB1

Hydrograph



Summary for Reach 48R: Outfall of SB 8, 13

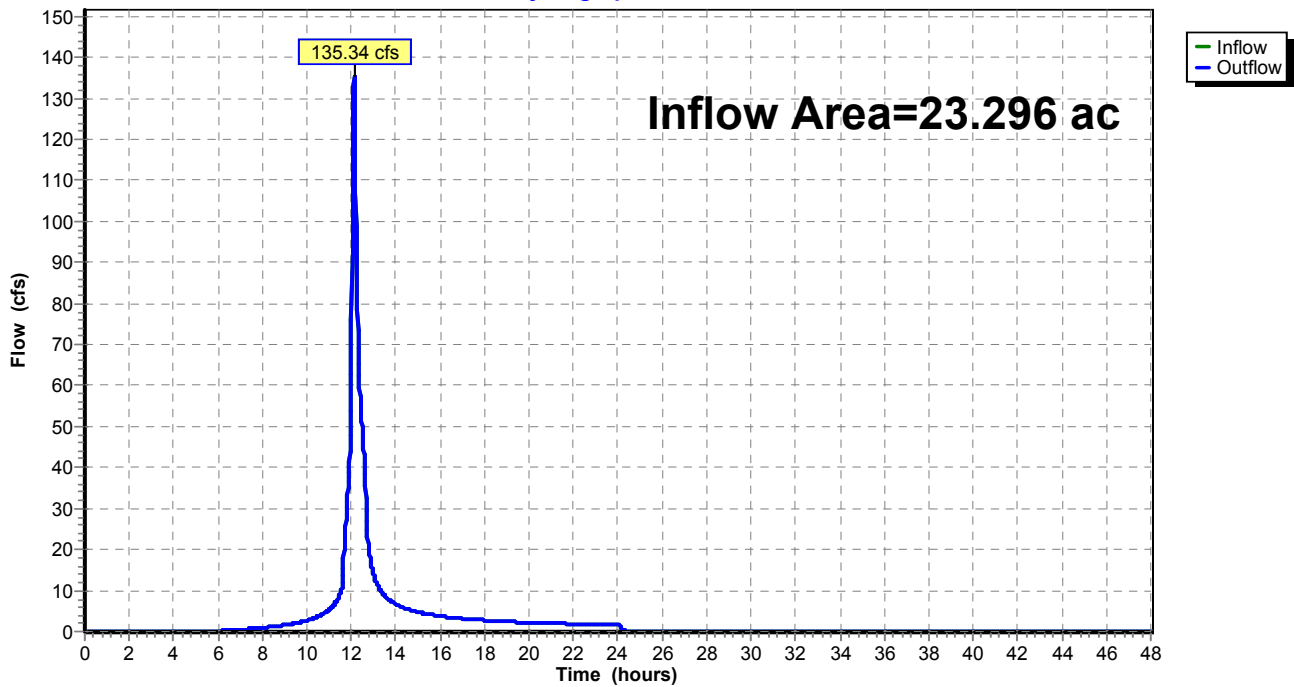
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 23.296 ac, 0.00% Impervious, Inflow Depth = 5.11" for 100-Year event
Inflow = 135.34 cfs @ 12.12 hrs, Volume= 9.912 af
Outflow = 135.34 cfs @ 12.12 hrs, Volume= 9.912 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 48R: Outfall of SB 8, 13

Hydrograph



Summary for Reach 49R: Channel SB8

[65] Warning: Inlet elevation not specified

Inflow Area = 21.017 ac, 0.00% Impervious, Inflow Depth = 5.09" for 100-Year event
Inflow = 134.02 cfs @ 12.08 hrs, Volume= 8.923 af
Outflow = 131.67 cfs @ 12.12 hrs, Volume= 8.923 af, Atten= 2%, Lag= 2.1 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs
Reference Flow= 100.52 cfs Estimated Depth= 1.43' Velocity= 2.85 fps
m= 1.511, c= 4.31 fps, dt= 1.2 min, dx= 521.0' / 2 = 260.5', K= 1.0 min, X= 0.265
Max. Velocity= 6.95 fps, Min. Travel Time= 1.3 min
Avg. Velocity = 4.32 fps, Avg. Travel Time= 2.0 min

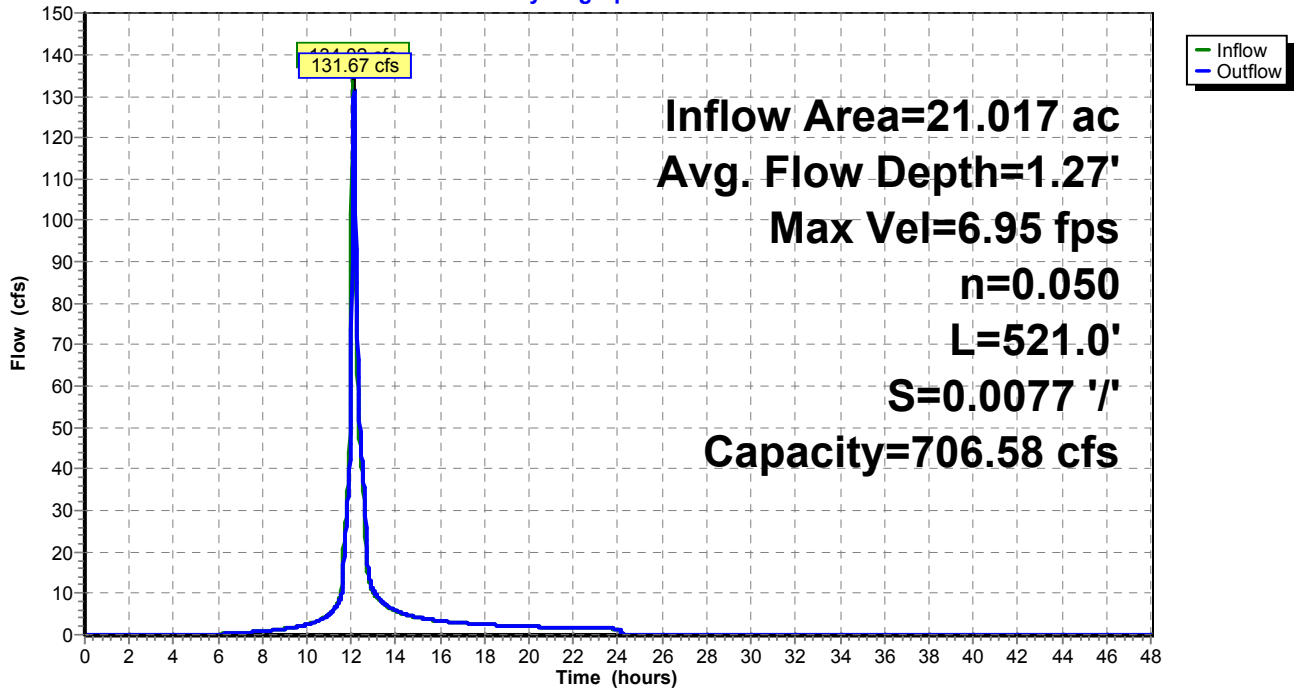
Peak Storage= 15,883 cf @ 12.10 hrs
Average Depth at Peak Storage= 1.27'
Bank-Full Depth= 4.00' Flow Area= 140.0 sf, Capacity= 706.58 cfs

19.00' x 4.00' deep channel, n= 0.050
Side Slope Z-value= 4.0 ' ' Top Width= 51.00'
Length= 521.0' Slope= 0.0077 ' '
Inlet Invert= 0.00', Outlet Invert= -4.01'



Reach 49R: Channel SB8

Hydrograph



Summary for Reach 50R: Outfall of SB 12

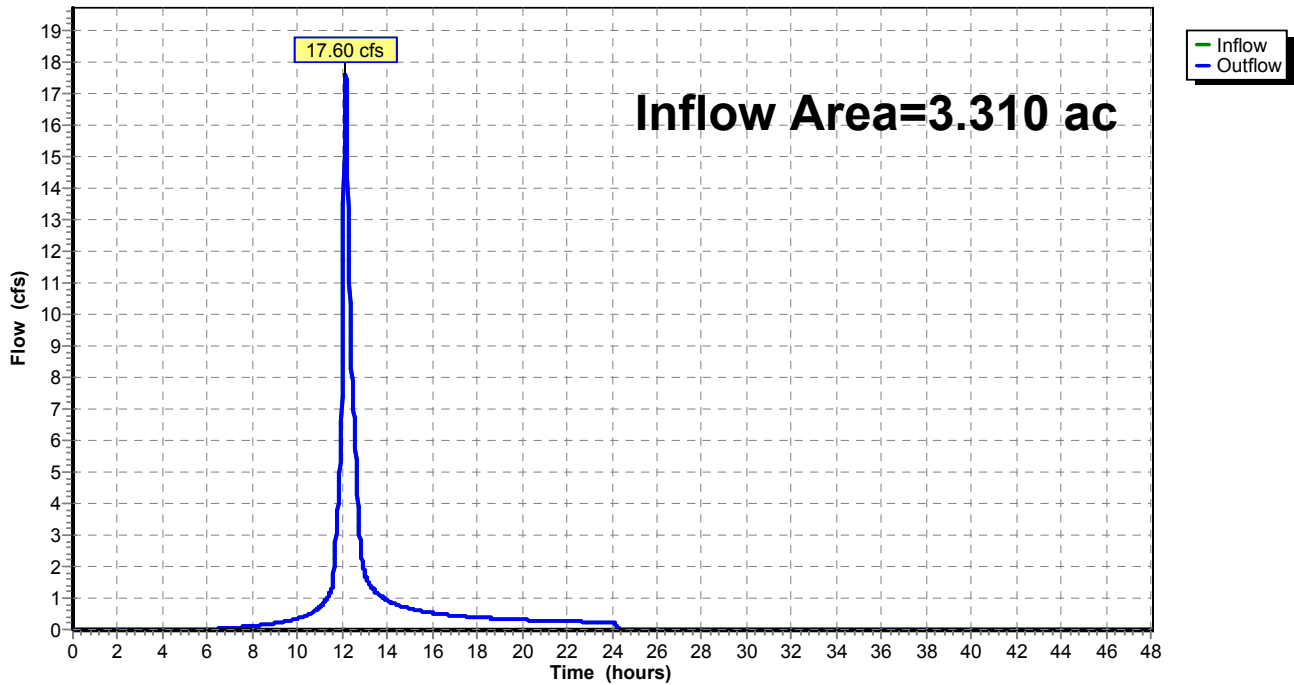
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 3.310 ac, 0.00% Impervious, Inflow Depth = 4.98" for 100-Year event
Inflow = 17.60 cfs @ 12.14 hrs, Volume= 1.374 af
Outflow = 17.60 cfs @ 12.14 hrs, Volume= 1.374 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 50R: Outfall of SB 12

Hydrograph



Summary for Reach 51R: Outfall of SB 14

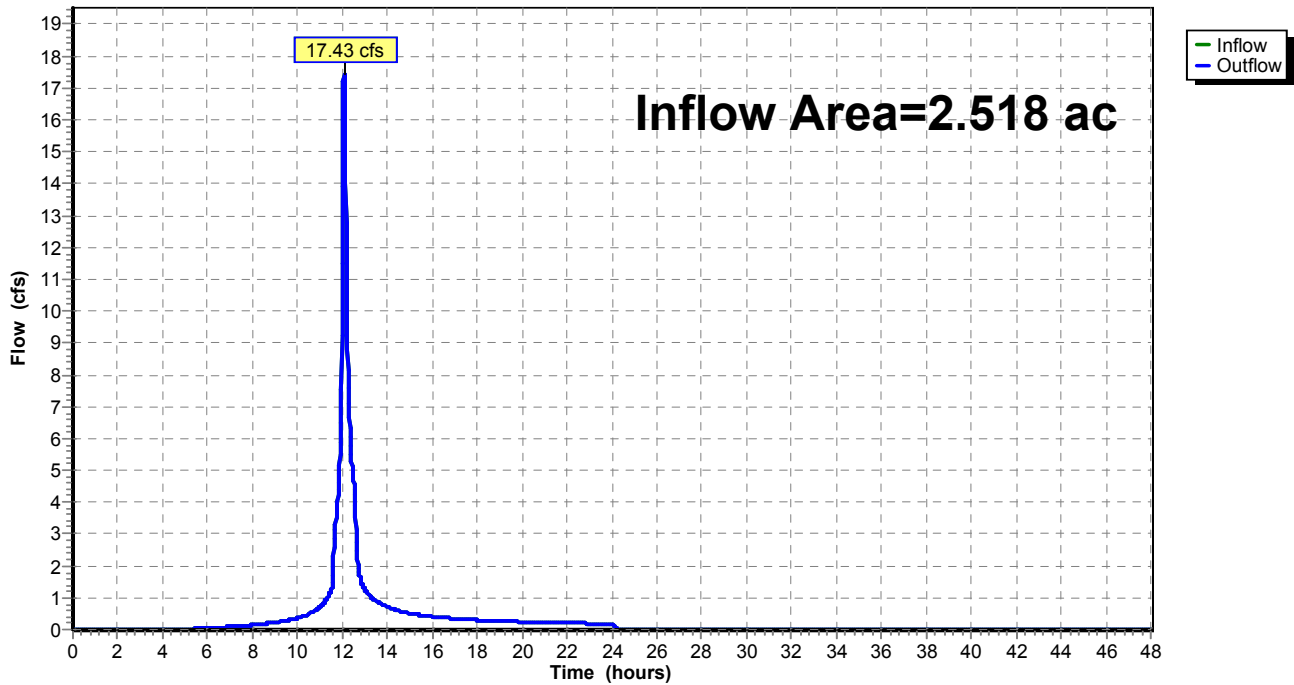
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 2.518 ac, 0.00% Impervious, Inflow Depth = 5.44" for 100-Year event
Inflow = 17.43 cfs @ 12.07 hrs, Volume= 1.141 af
Outflow = 17.43 cfs @ 12.07 hrs, Volume= 1.141 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 51R: Outfall of SB 14

Hydrograph



Summary for Reach 52R: Outfall of SB 17

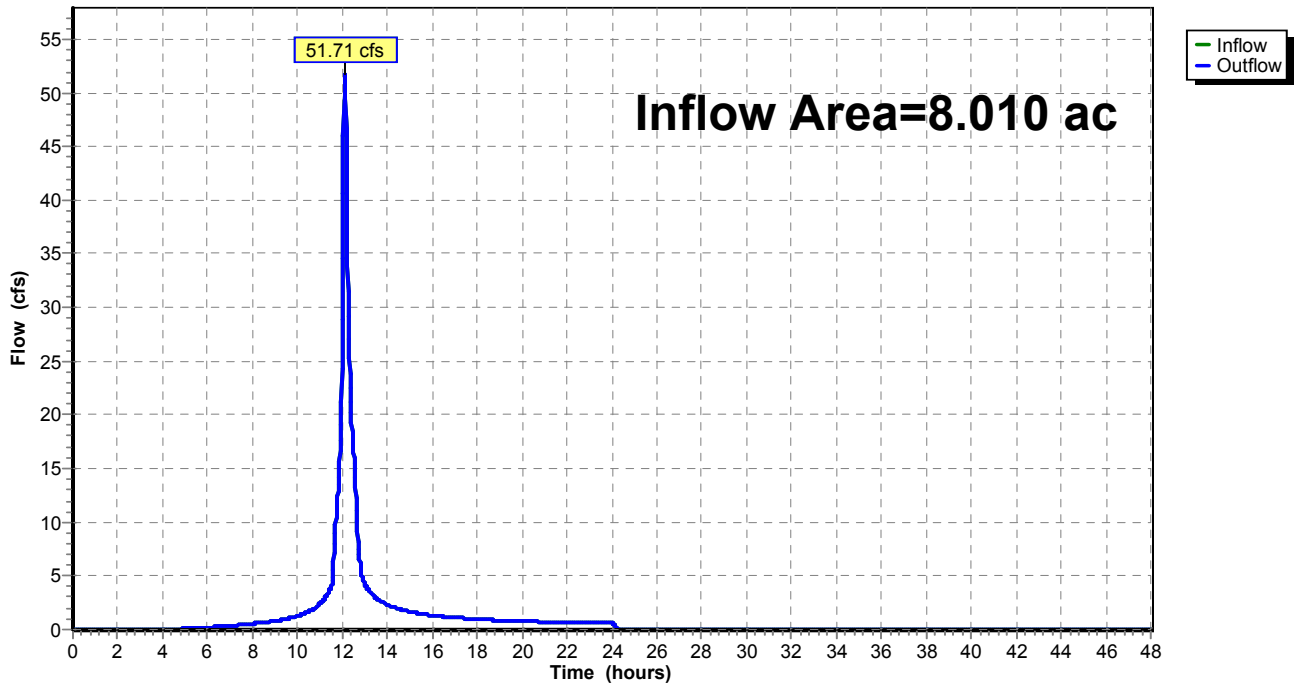
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 8.010 ac, 0.00% Impervious, Inflow Depth = 5.66" for 100-Year event
Inflow = 51.71 cfs @ 12.11 hrs, Volume= 3.781 af
Outflow = 51.71 cfs @ 12.11 hrs, Volume= 3.781 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 52R: Outfall of SB 17

Hydrograph



Summary for Reach 53R: Outfall of SB 18

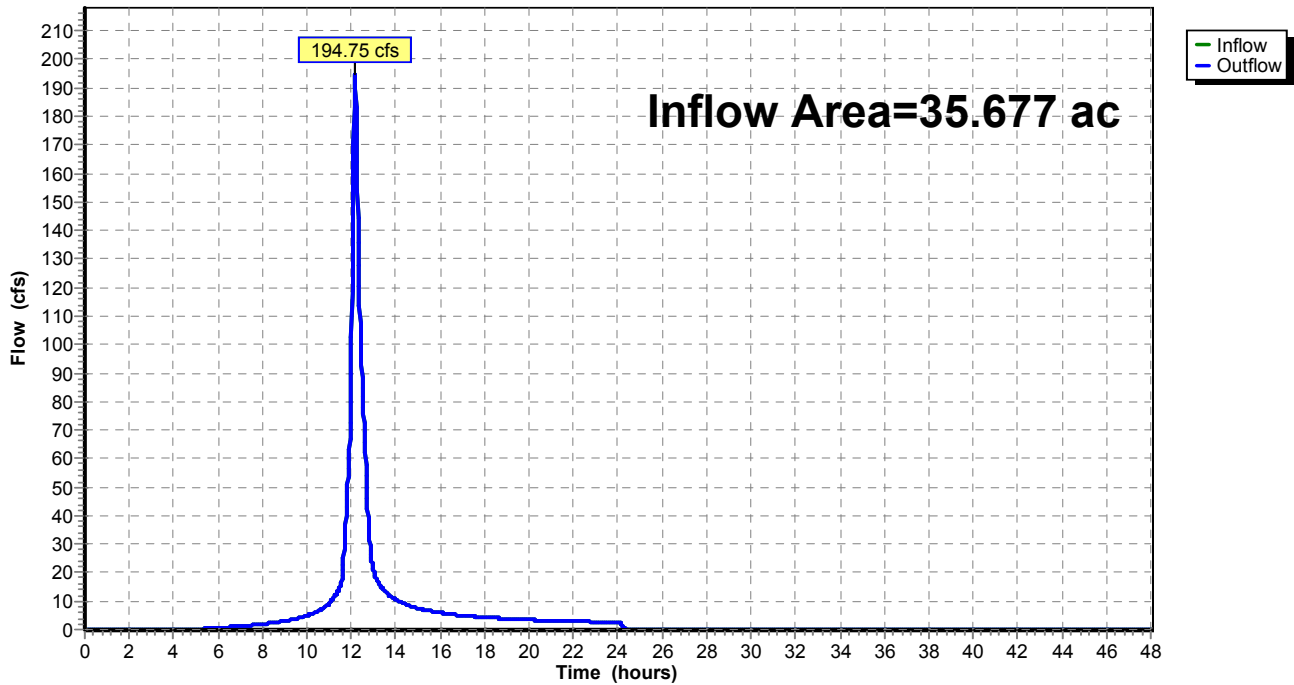
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 35.677 ac, 0.00% Impervious, Inflow Depth = 5.44" for 100-Year event
Inflow = 194.75 cfs @ 12.17 hrs, Volume= 16.160 af
Outflow = 194.75 cfs @ 12.17 hrs, Volume= 16.160 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 53R: Outfall of SB 18

Hydrograph



Summary for Reach 54R: Outfall of SB 25

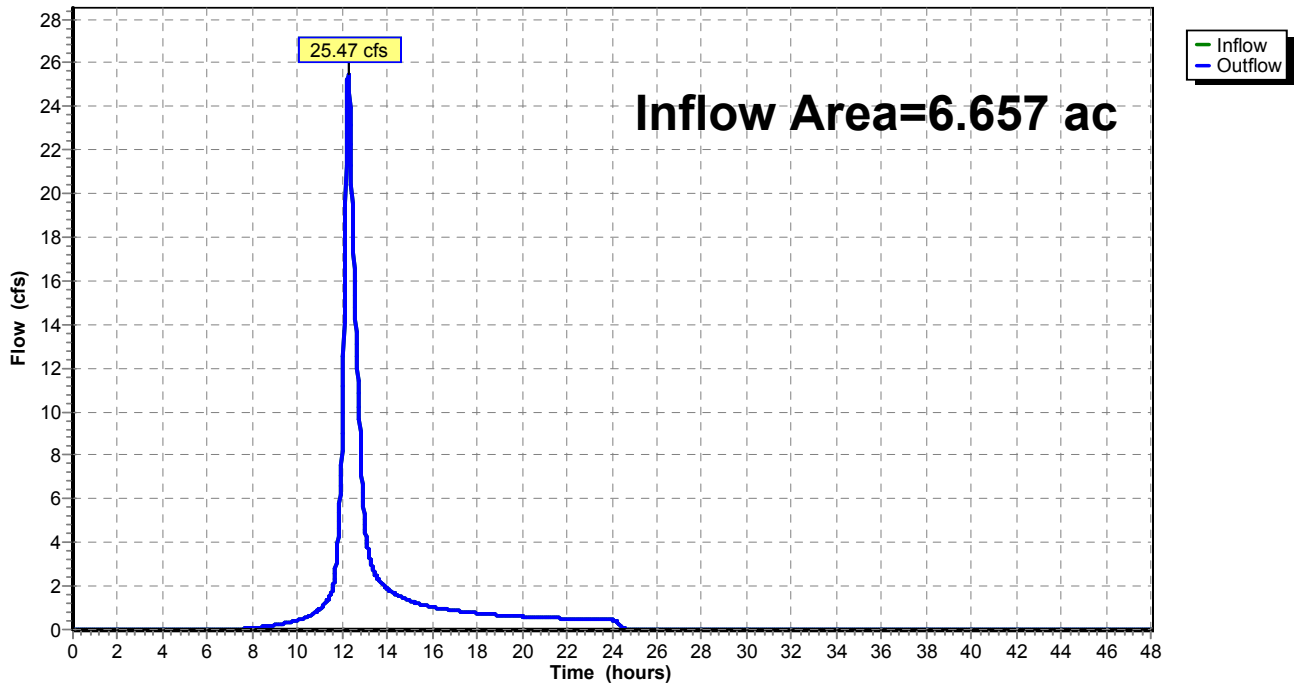
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 6.657 ac, 0.00% Impervious, Inflow Depth = 4.42" for 100-Year event
Inflow = 25.47 cfs @ 12.27 hrs, Volume= 2.454 af
Outflow = 25.47 cfs @ 12.27 hrs, Volume= 2.454 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 54R: Outfall of SB 25

Hydrograph



Summary for Reach 55R: Outfall of SB 26

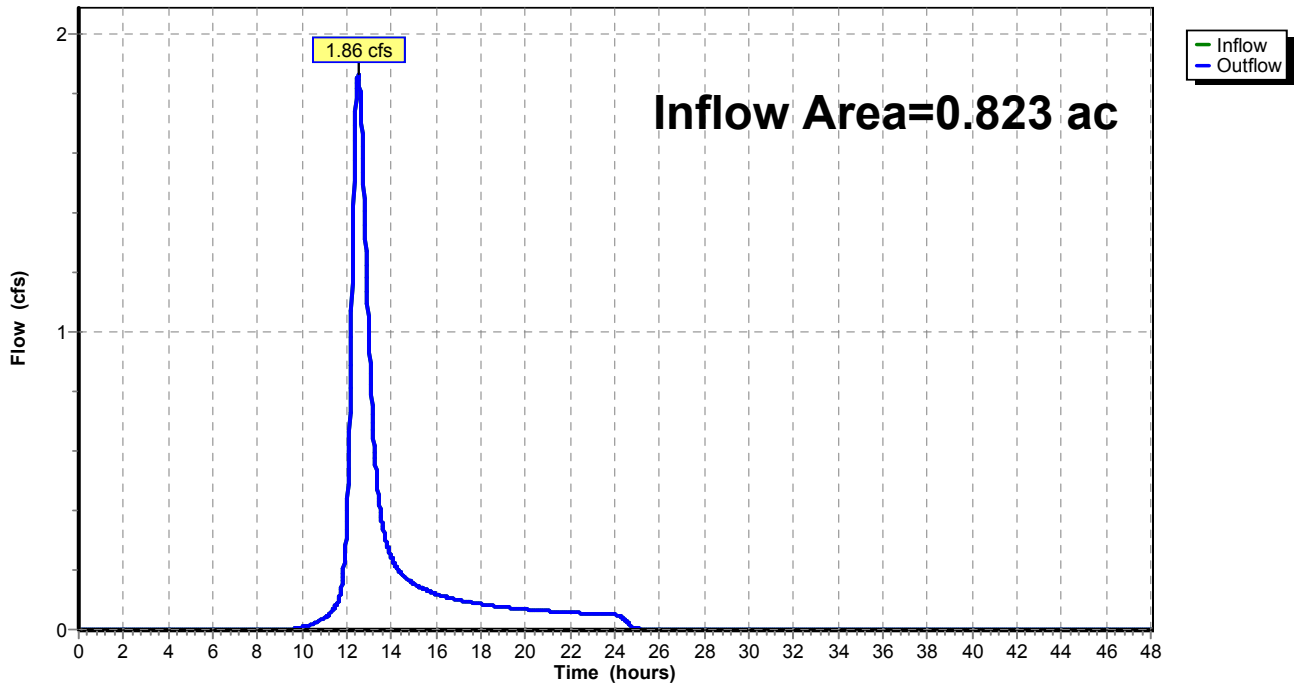
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.823 ac, 0.00% Impervious, Inflow Depth = 3.45" for 100-Year event
Inflow = 1.86 cfs @ 12.52 hrs, Volume= 0.237 af
Outflow = 1.86 cfs @ 12.52 hrs, Volume= 0.237 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 55R: Outfall of SB 26

Hydrograph



Summary for Reach 56R: Outfall of SB 23, 24

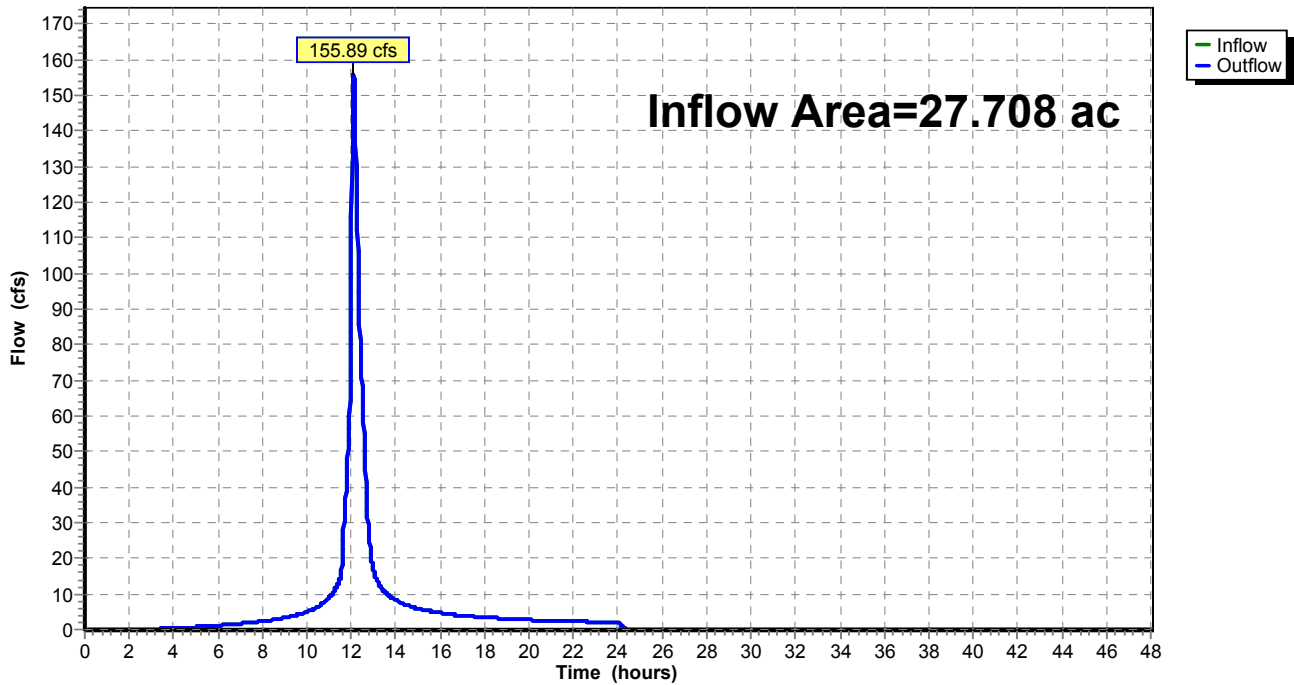
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 27.708 ac, 0.00% Impervious, Inflow Depth = 5.95" for 100-Year event
Inflow = 155.89 cfs @ 12.10 hrs, Volume= 13.749 af
Outflow = 155.89 cfs @ 12.10 hrs, Volume= 13.749 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 56R: Outfall of SB 23, 24

Hydrograph



Summary for Reach 59R: Outfall of SB 20, 22

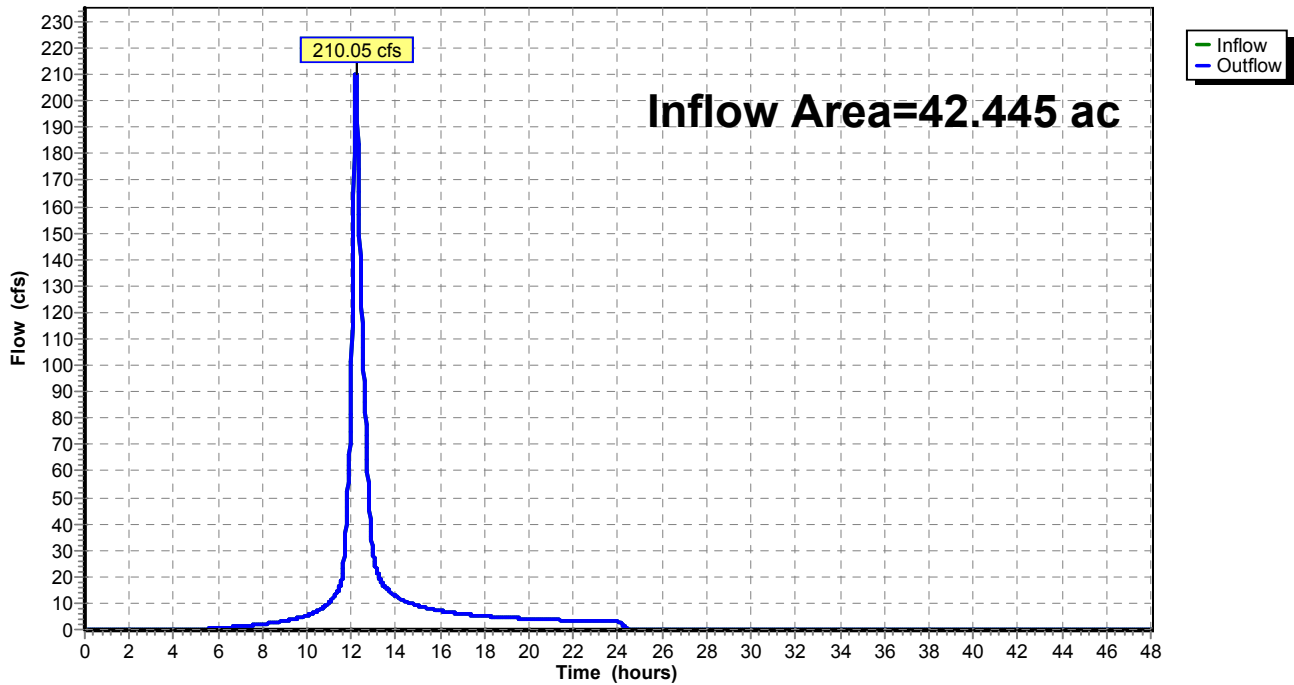
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 42.445 ac, 0.00% Impervious, Inflow Depth = 5.34" for 100-Year event
Inflow = 210.05 cfs @ 12.21 hrs, Volume= 18.873 af
Outflow = 210.05 cfs @ 12.21 hrs, Volume= 18.873 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 59R: Outfall of SB 20, 22

Hydrograph



Summary for Reach 61R: Outfall of SB 15, 16, 21

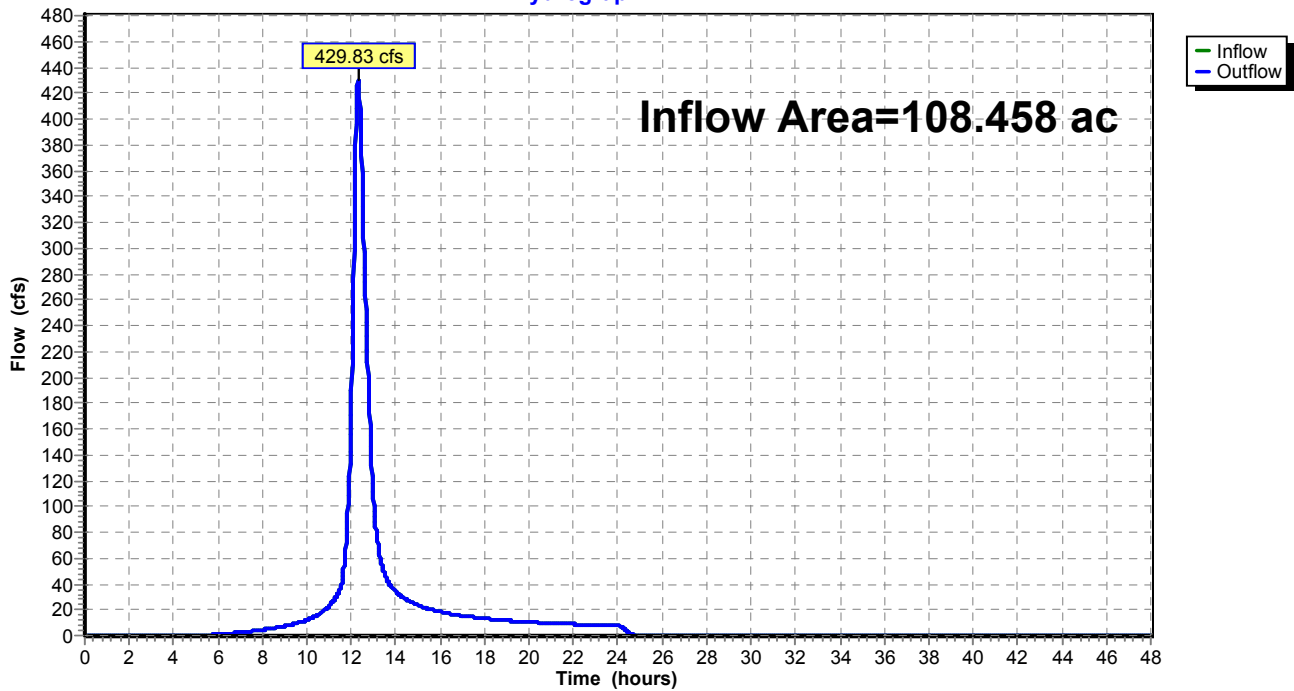
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 108.458 ac, 0.00% Impervious, Inflow Depth = 5.22" for 100-Year event
Inflow = 429.83 cfs @ 12.31 hrs, Volume= 47.194 af
Outflow = 429.83 cfs @ 12.31 hrs, Volume= 47.194 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 61R: Outfall of SB 15, 16, 21

Hydrograph



Summary for Reach 67R: Outfall of SB 28

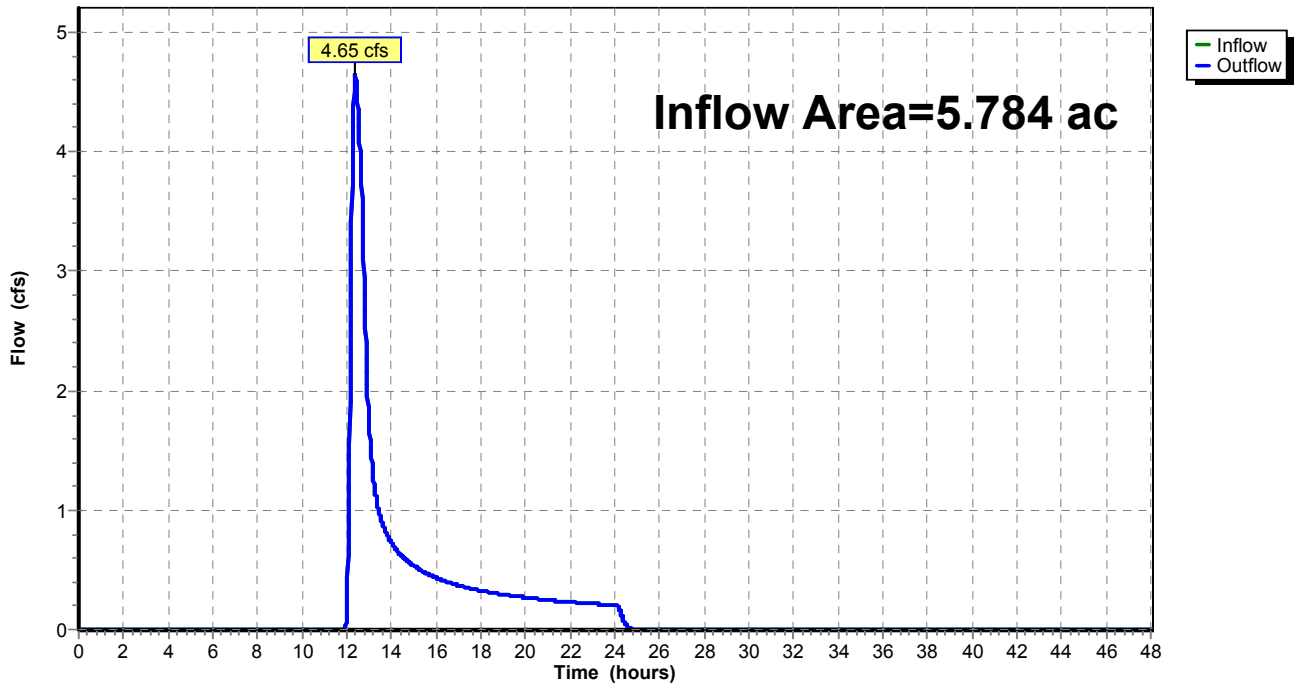
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 5.784 ac, 0.00% Impervious, Inflow Depth = 1.30" for 100-Year event
Inflow = 4.65 cfs @ 12.38 hrs, Volume= 0.626 af
Outflow = 4.65 cfs @ 12.38 hrs, Volume= 0.626 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 67R: Outfall of SB 28

Hydrograph



Summary for Reach 68R: Outfall of SB 29

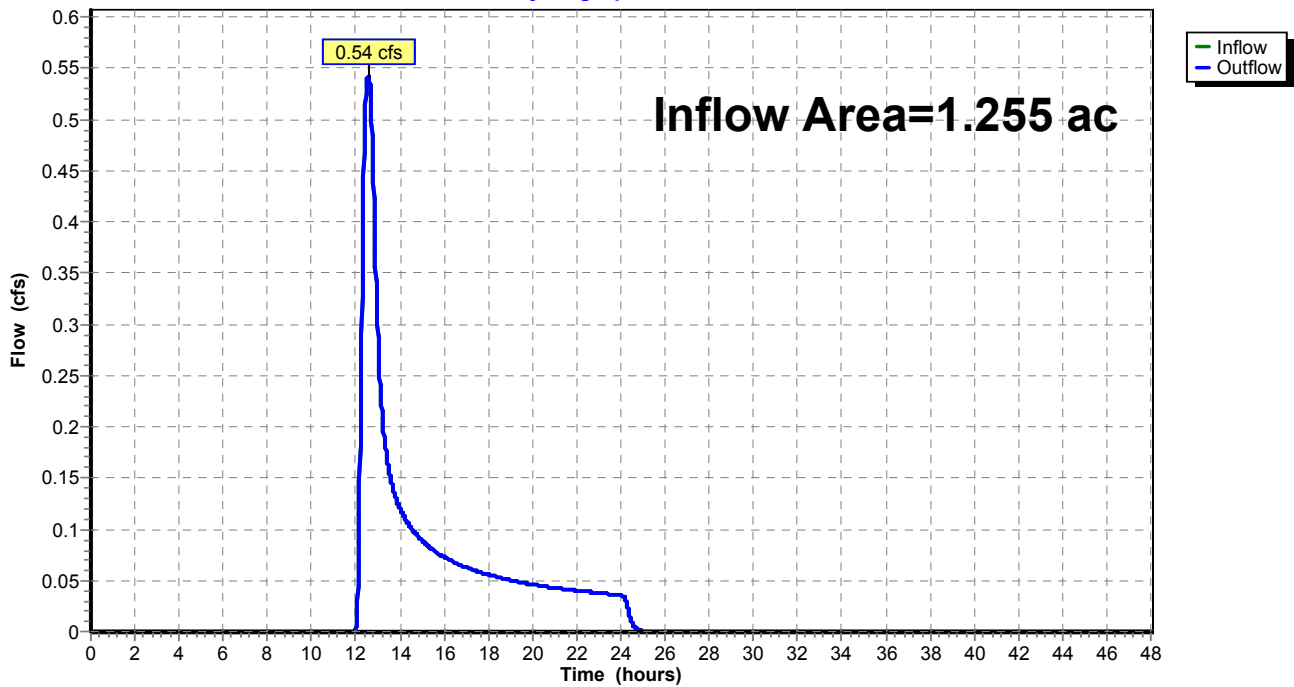
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.255 ac, 0.00% Impervious, Inflow Depth = 0.88" for 100-Year event
Inflow = 0.54 cfs @ 12.57 hrs, Volume= 0.092 af
Outflow = 0.54 cfs @ 12.57 hrs, Volume= 0.092 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 68R: Outfall of SB 29

Hydrograph



Summary for Reach 69R: Outfall of SB 30

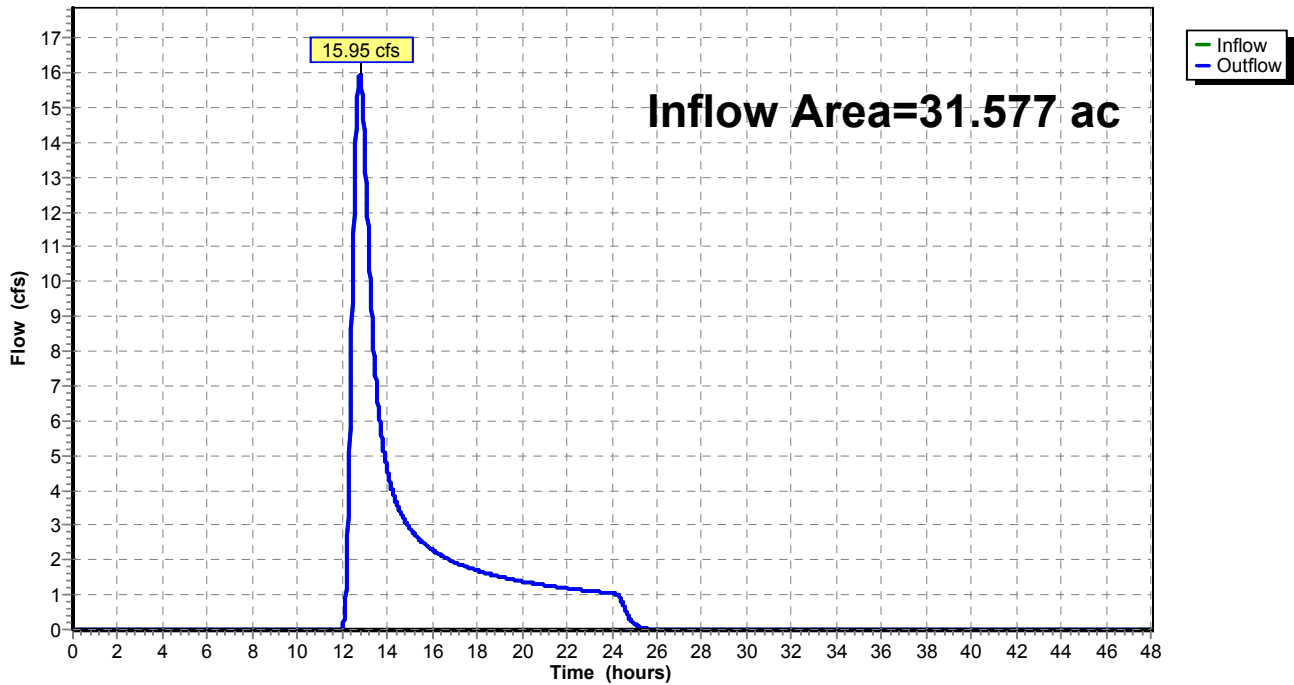
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 31.577 ac, 0.00% Impervious, Inflow Depth = 1.13" for 100-Year event
Inflow = 15.95 cfs @ 12.79 hrs, Volume= 2.966 af
Outflow = 15.95 cfs @ 12.79 hrs, Volume= 2.966 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 69R: Outfall of SB 30

Hydrograph



Summary for Reach 70R: Outfall of SB 31

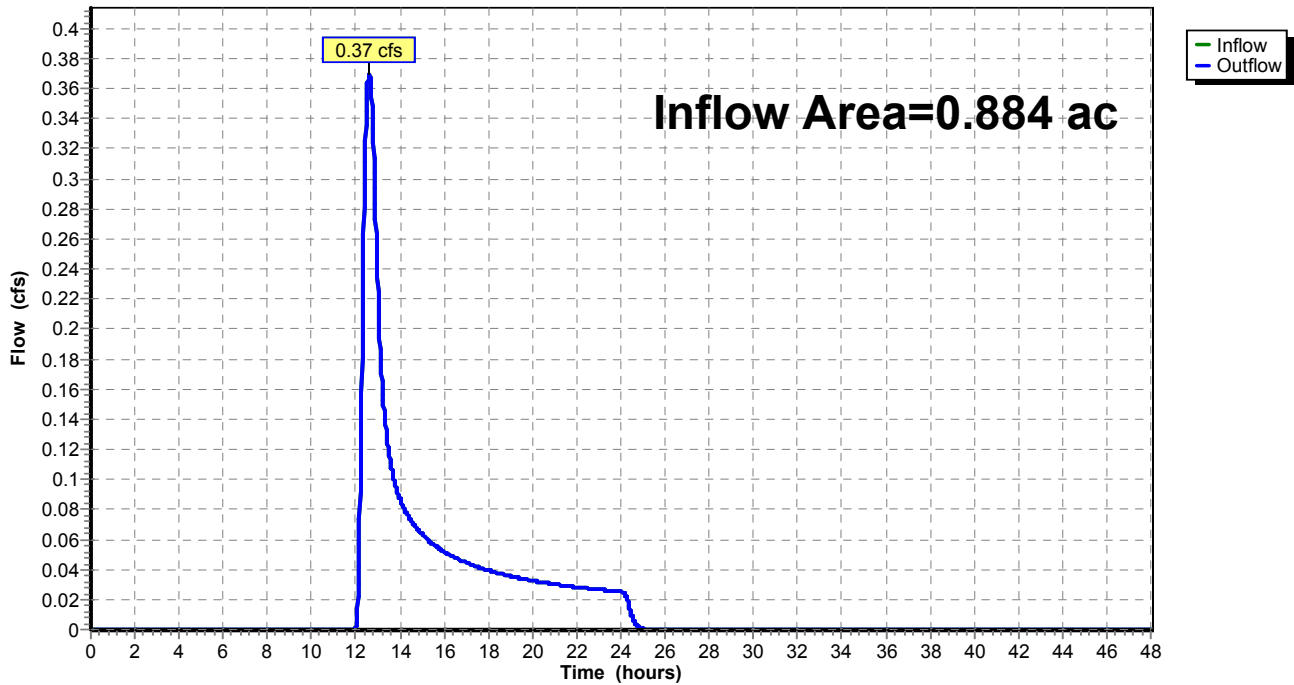
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.884 ac, 0.00% Impervious, Inflow Depth = 0.88" for 100-Year event
Inflow = 0.37 cfs @ 12.61 hrs, Volume= 0.065 af
Outflow = 0.37 cfs @ 12.61 hrs, Volume= 0.065 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 70R: Outfall of SB 31

Hydrograph



Summary for Reach 71R: Outfall of SB 32

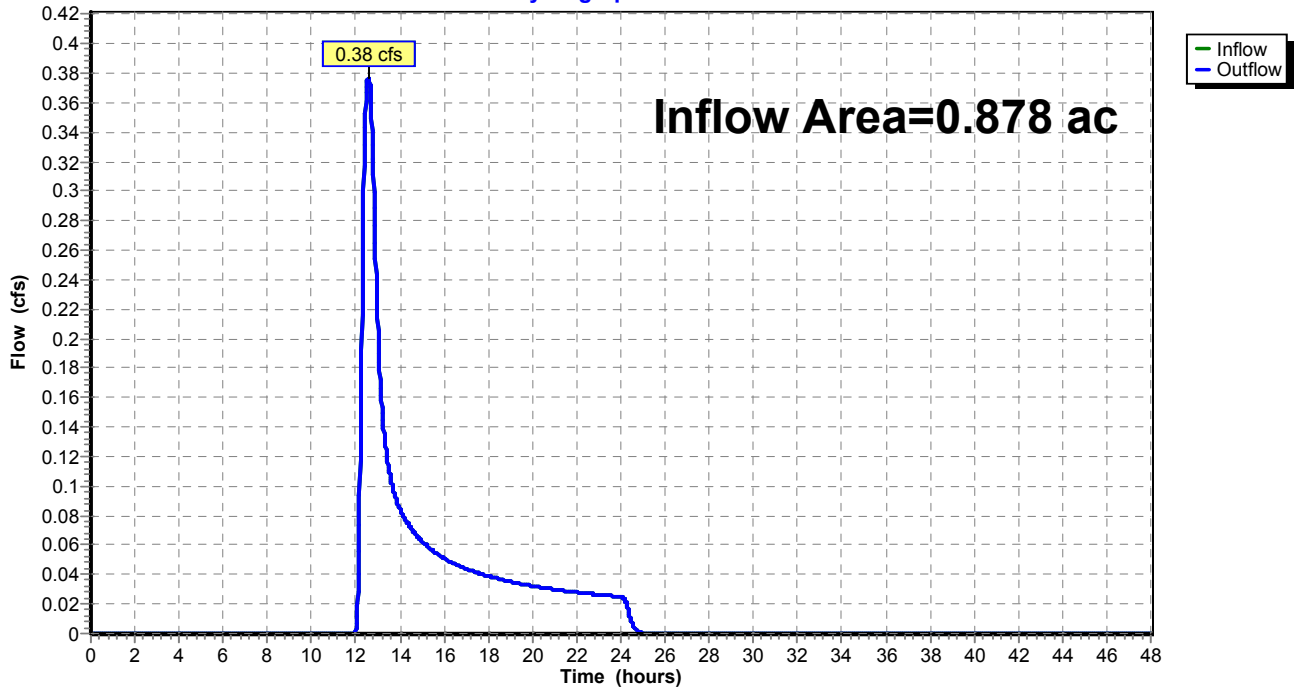
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.878 ac, 0.00% Impervious, Inflow Depth = 0.88" for 100-Year event
Inflow = 0.38 cfs @ 12.57 hrs, Volume= 0.065 af
Outflow = 0.38 cfs @ 12.57 hrs, Volume= 0.065 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 71R: Outfall of SB 32

Hydrograph



Summary for Reach 72R: Outfall of SB 33

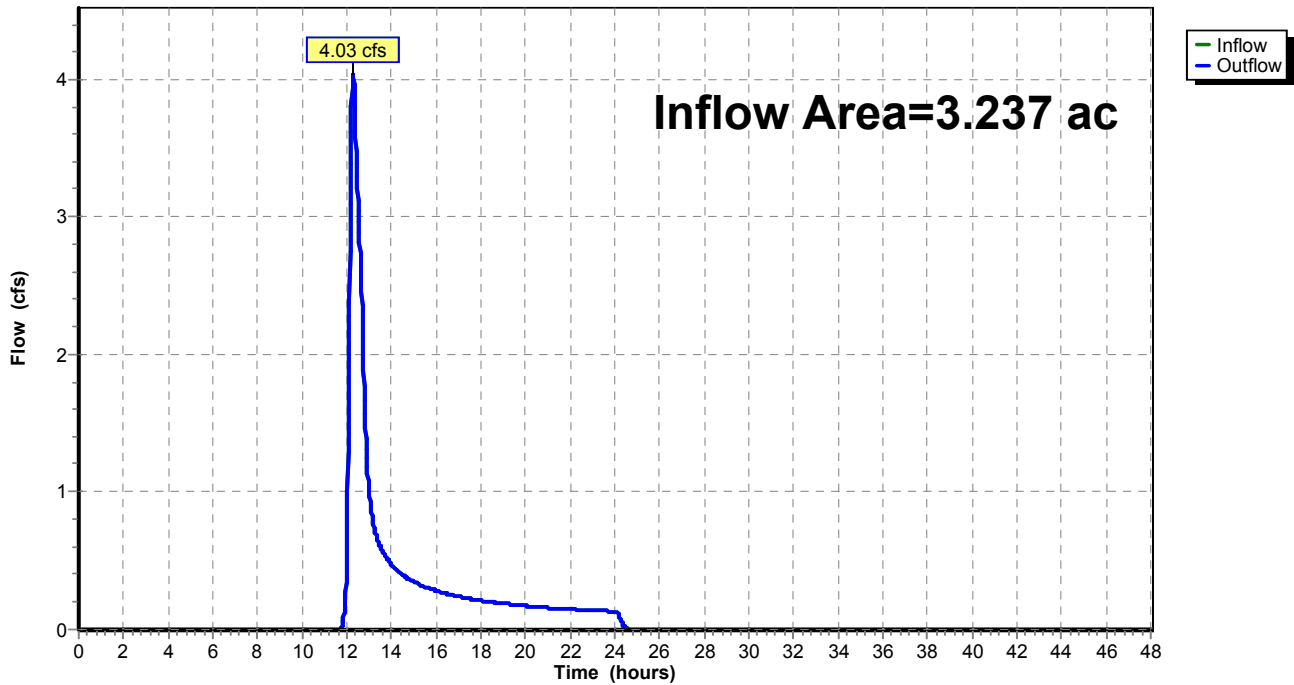
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 3.237 ac, 0.00% Impervious, Inflow Depth = 1.66" for 100-Year event
Inflow = 4.03 cfs @ 12.28 hrs, Volume= 0.447 af
Outflow = 4.03 cfs @ 12.28 hrs, Volume= 0.447 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 72R: Outfall of SB 33

Hydrograph



Summary for Reach 73R: Outfall of SB 34

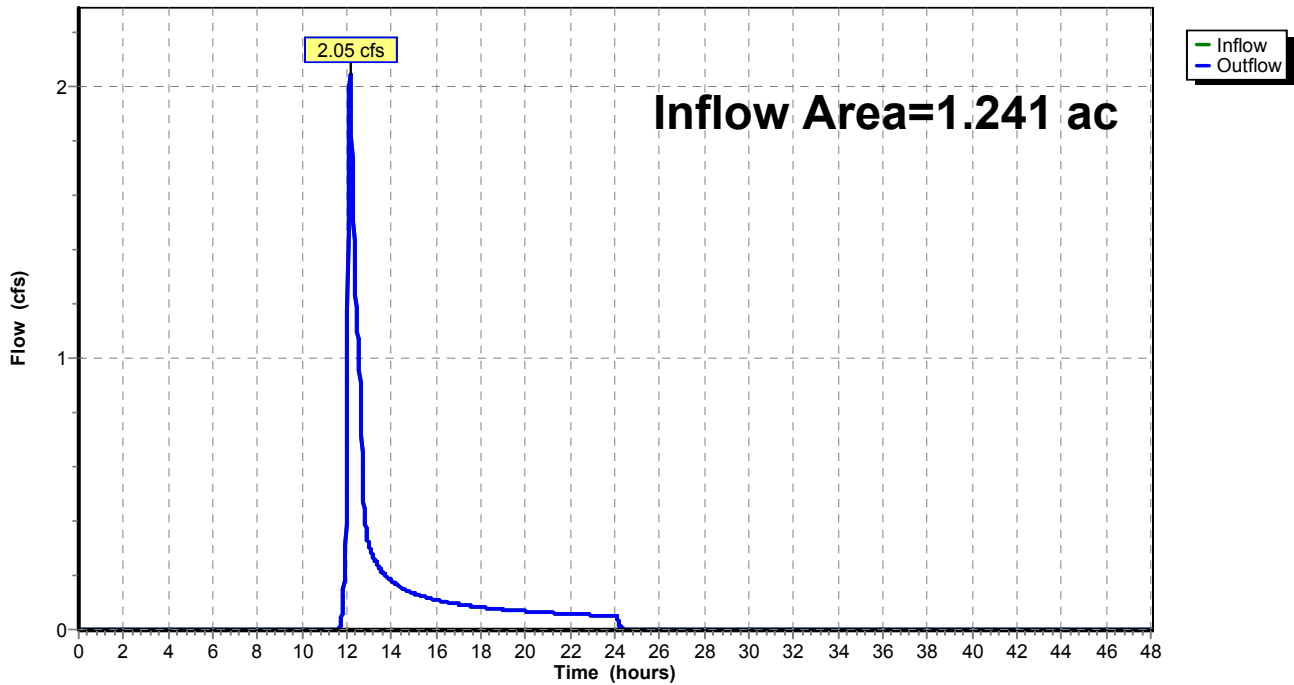
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.241 ac, 0.00% Impervious, Inflow Depth = 1.75" for 100-Year event
Inflow = 2.05 cfs @ 12.15 hrs, Volume= 0.181 af
Outflow = 2.05 cfs @ 12.15 hrs, Volume= 0.181 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 73R: Outfall of SB 34

Hydrograph



Summary for Reach 74R: Outfall of SB 35

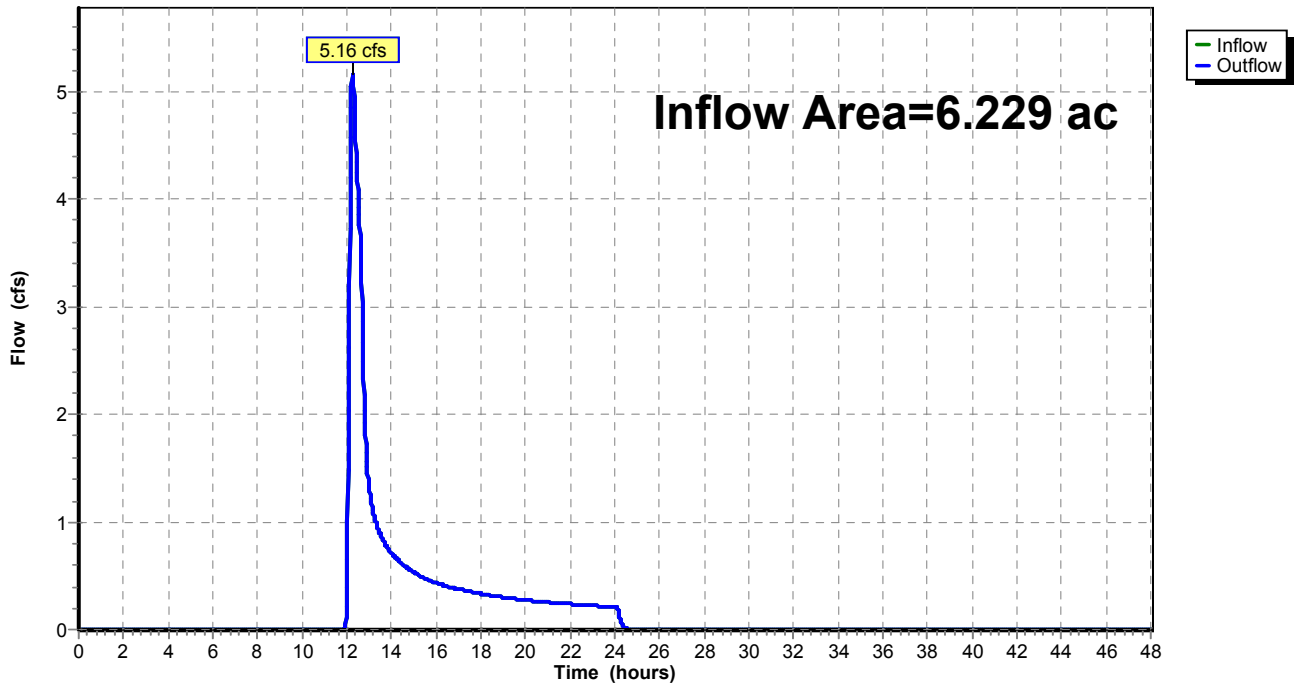
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 6.229 ac, 0.00% Impervious, Inflow Depth = 1.21" for 100-Year event
Inflow = 5.16 cfs @ 12.26 hrs, Volume= 0.629 af
Outflow = 5.16 cfs @ 12.26 hrs, Volume= 0.629 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 74R: Outfall of SB 35

Hydrograph



Summary for Reach 75R: Outfall of SB 19

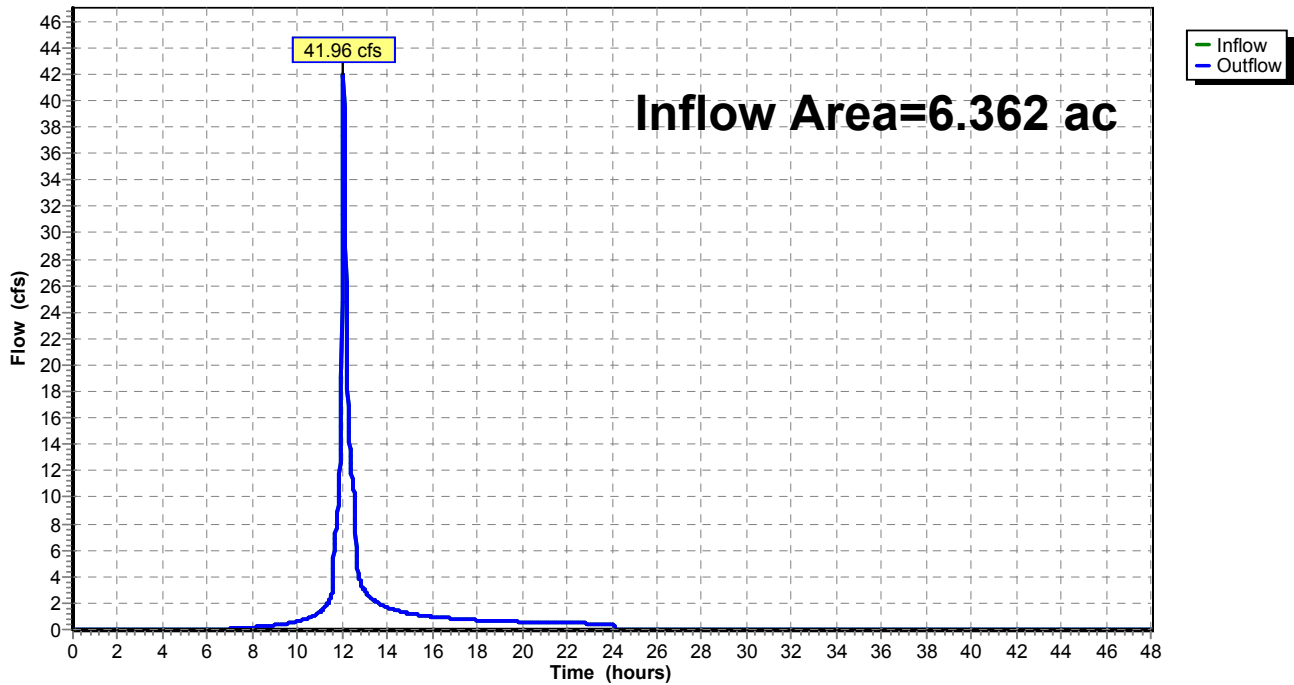
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 6.362 ac, 0.00% Impervious, Inflow Depth = 4.76" for 100-Year event
Inflow = 41.96 cfs @ 12.05 hrs, Volume= 2.522 af
Outflow = 41.96 cfs @ 12.05 hrs, Volume= 2.522 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 75R: Outfall of SB 19

Hydrograph



Summary for Reach 82R: Outfall of SB 27

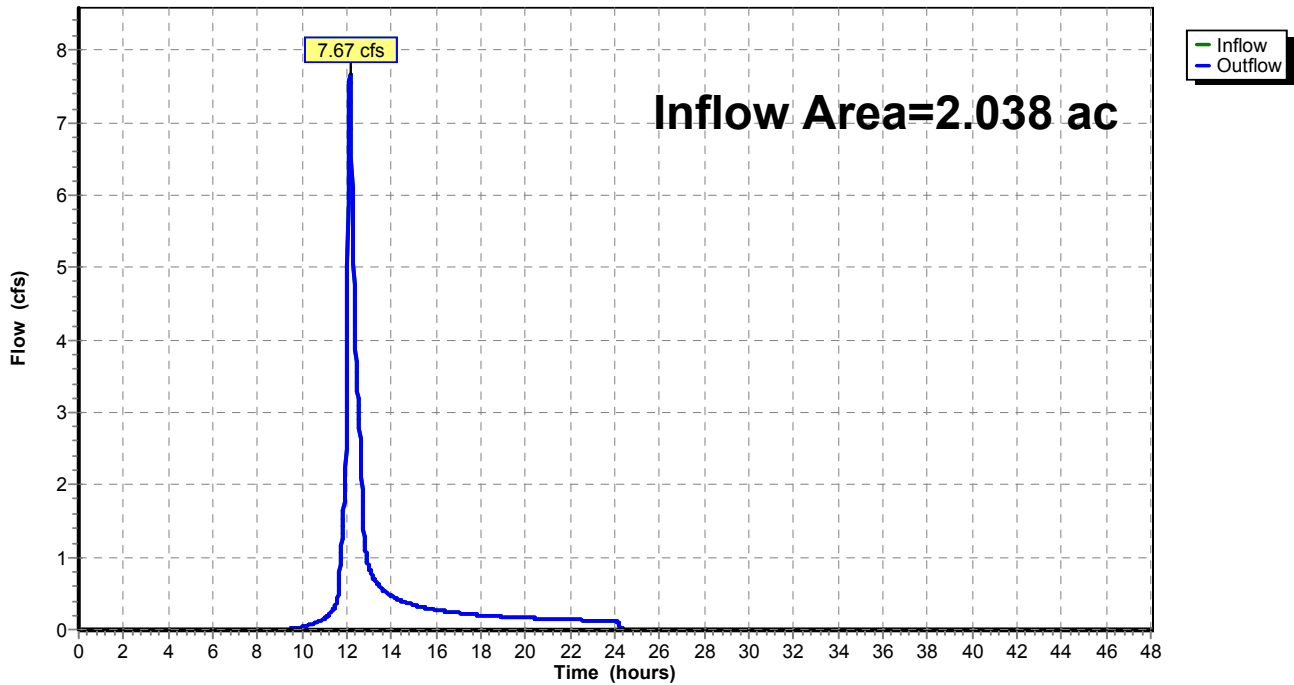
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 2.038 ac, 0.00% Impervious, Inflow Depth = 3.45" for 100-Year event
Inflow = 7.67 cfs @ 12.14 hrs, Volume= 0.586 af
Outflow = 7.67 cfs @ 12.14 hrs, Volume= 0.586 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

Reach 82R: Outfall of SB 27

Hydrograph



Summary for Reach 84R: Outfall of Future County Road H Subbasin

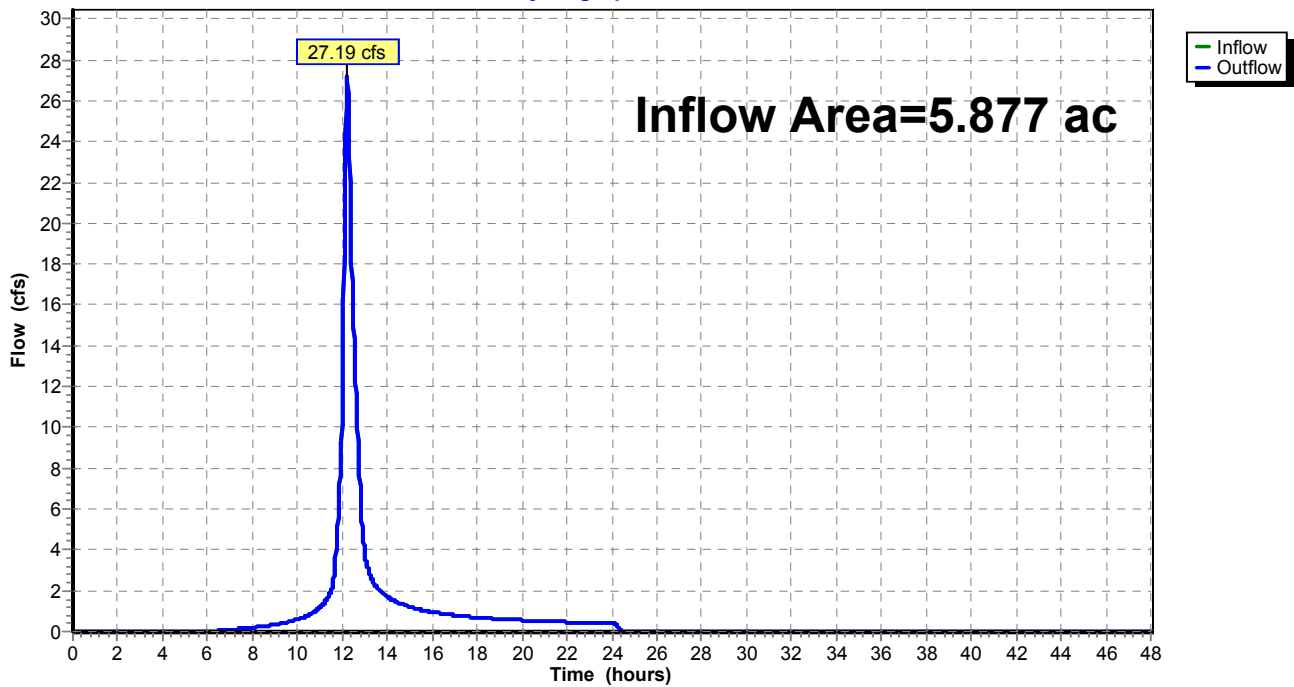
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 5.877 ac, 0.00% Impervious, Inflow Depth = 4.98" for 100-Year event
Inflow = 27.19 cfs @ 12.21 hrs, Volume= 2.440 af
Outflow = 27.19 cfs @ 12.21 hrs, Volume= 2.440 af, Atten= 0%, Lag= 0.0 min

Routing by Muskingum-Cunge method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs

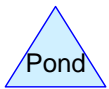
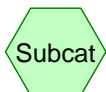
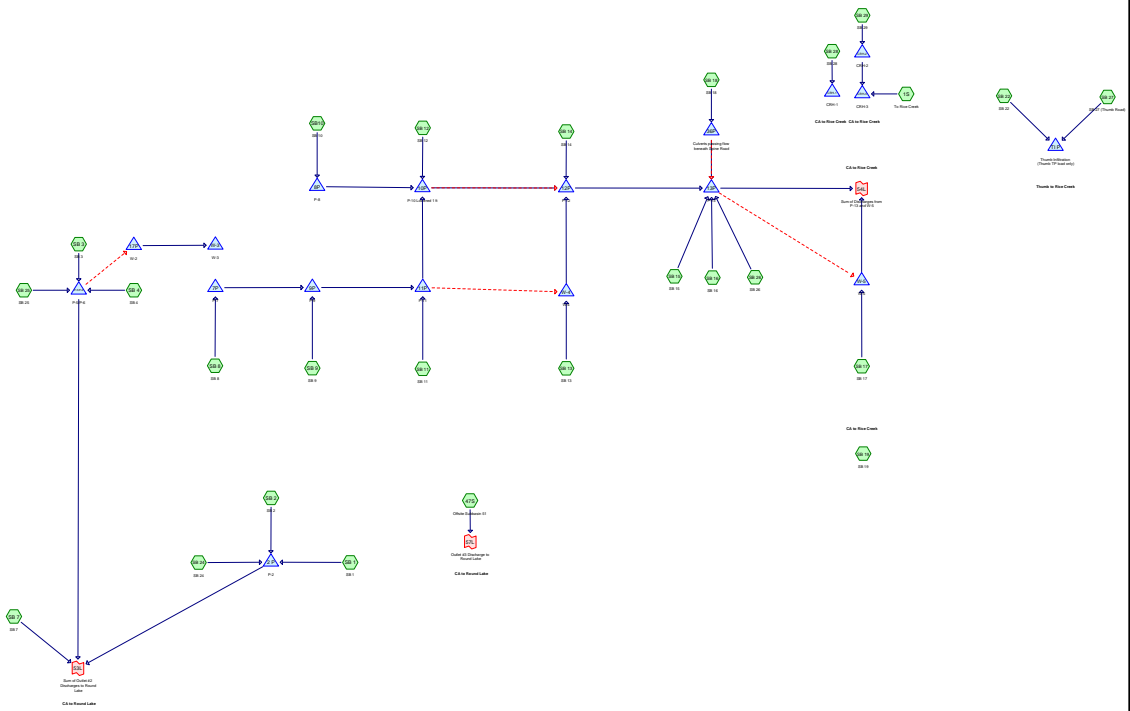
Reach 84R: Outfall of Future County Road H Subbasin

Hydrograph



Appendix B

Interim Conditions (Public Infrastructure) Hydrology and Hydraulics Modeling (HydroCAD)



Routing Diagram for Interim Spine Road_HydroCAD Model_20150830

Prepared by {enter your company name here}, Printed 1/28/2016
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Interim Spine Road_HydroCAD Model_20150830

Prepared by {enter your company name here}

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Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
5.038	98	(47S)
8.414	98	Impervious (SB 27, SB 3)
20.200	65	Offsite subbasin 51 (47S)
41.910	49	Pervious (SB 22)
35.825	74	Pervious (SB 27, SB 3)
4.950	98	impermiabile (SB 24, SB 9)
32.898	98	impervious (1S, SB 12, SB 14, SB 15, SB 16, SB 2, SB 25, SB 26, SB 28, SB 29, SB 5, SB 8, SB10)
6.033	100	impervious (SB 11, SB 13, SB 17, SB 4, SB 6)
25.873	74	permiabile (SB 24, SB 9)
319.995	74	pervious (1S, SB 1, SB 11, SB 12, SB 13, SB 14, SB 15, SB 16, SB 17, SB 18, SB 19, SB 2, SB 25, SB 26, SB 28, SB 29, SB 4, SB 5, SB 6, SB 7, SB 8, SB10)
501.136	74	TOTAL AREA

Interim Spine Road_HydroCAD Model_20150830

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Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
501.136	Other	1S, 47S, SB 1, SB 11, SB 12, SB 13, SB 14, SB 15, SB 16, SB 17, SB 18, SB 19, SB 2, SB 22, SB 24, SB 25, SB 26, SB 27, SB 28, SB 29, SB 3, SB 4, SB 5, SB 6, SB 7, SB 8, SB 9, SB10
501.136		TOTAL AREA

Interim Spine Road_HydroCAD Model_20150830

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Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	0.000	0.000	5.038	5.038		47S
0.000	0.000	0.000	0.000	8.414	8.414	Impervious	SB 27, SB 3
0.000	0.000	0.000	0.000	20.200	20.200	Offsite subbasin 51	47S
0.000	0.000	0.000	0.000	77.735	77.735	Pervious	SB 22, SB 27, SB 3
0.000	0.000	0.000	0.000	4.950	4.950	impermiabile	SB 24, SB 9
0.000	0.000	0.000	0.000	38.931	38.931	impervious	1S, SB 11, SB 12, SB 13, SB 14, SB 15, SB 16, SB 17, SB 2, SB 25, SB 26, SB 28, SB 29, SB 4, SB 5, SB 6, SB 8, SB10
0.000	0.000	0.000	0.000	25.873	25.873	permiabile	SB 24, SB 9
0.000	0.000	0.000	0.000	319.995	319.995	pervious	1S, SB 1, SB 11, SB 12, SB 13, SB 14, SB 15, SB 16, SB 17, SB 18, SB 19, SB 2, SB 25, SB 26, SB 28, SB 29, SB 4, SB 5, SB 6, SB 7, SB 8, SB10
0.000	0.000	0.000	0.000	501.136	501.136	TOTAL AREA	

Interim Spine Road_HydroCAD Model_20150830

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Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	4P	915.80	915.95	50.0	-0.0030	0.013	24.0	0.0	0.0
2	8P	897.00	895.94	380.0	0.0028	0.013	24.0	0.0	0.0
3	11P	910.00	909.00	200.0	0.0050	0.013	24.0	0.0	0.0
4	11P	910.00	909.00	200.0	0.0050	0.013	24.0	0.0	0.0
5	11P	909.00	908.00	150.0	0.0067	0.013	12.0	0.0	0.0
6	12P	893.50	893.35	30.0	0.0050	0.013	43.8	26.6	0.0
7	12P	893.50	893.35	30.0	0.0050	0.013	43.8	26.6	0.0
8	12P	893.50	893.35	30.0	0.0050	0.013	43.8	26.6	0.0
9	12P	893.50	893.35	30.0	0.0050	0.013	43.8	26.6	0.0
10	13P	883.00	882.75	100.0	0.0025	0.013	12.0	0.0	0.0
11	13P	883.00	882.75	100.0	0.0025	0.013	12.0	0.0	0.0
12	13P	883.00	882.75	100.0	0.0025	0.013	12.0	0.0	0.0
13	13P	883.00	882.75	100.0	0.0025	0.013	12.0	0.0	0.0
14	13P	883.00	882.75	100.0	0.0025	0.013	12.0	0.0	0.0
15	17P	929.10	916.00	300.0	0.0437	0.013	12.0	0.0	0.0
16	36P	887.50	886.50	100.0	0.0100	0.013	18.0	0.0	0.0
17	36P	887.50	886.50	100.0	0.0100	0.013	18.0	0.0	0.0
18	36P	887.50	886.50	100.0	0.0100	0.013	18.0	0.0	0.0
19	36P	887.50	886.50	100.0	0.0100	0.013	18.0	0.0	0.0
20	36P	887.50	886.50	100.0	0.0100	0.013	18.0	0.0	0.0
21	36P	887.50	886.50	100.0	0.0100	0.013	18.0	0.0	0.0
22	36P	887.50	886.50	100.0	0.0100	0.013	18.0	0.0	0.0
23	36P	887.50	886.50	100.0	0.0100	0.013	18.0	0.0	0.0
24	CRH-1	877.00	876.00	155.0	0.0065	0.013	24.0	0.0	0.0
25	CRH-1	877.00	876.00	155.0	0.0065	0.013	24.0	0.0	0.0
26	CRH-2	881.50	881.00	155.0	0.0032	0.013	24.0	0.0	0.0
27	CRH-2	881.50	881.00	155.0	0.0032	0.013	24.0	0.0	0.0
28	CRH-3	878.00	877.00	155.0	0.0065	0.013	24.0	0.0	0.0
29	CRH-3	878.00	877.00	155.0	0.0065	0.013	24.0	0.0	0.0
30	W-4	908.00	904.00	170.0	0.0235	0.013	12.0	0.0	0.0

Time span=0.00-96.00 hrs, dt=0.01 hrs, 9601 points
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: To Rice Creek	Runoff Area=1.601 ac 31.98% Impervious Runoff Depth=1.37" Tc=5.7 min CN=74/98 Runoff=2.97 cfs 0.183 af
Subcatchment 47S: Offsite Subbasin 51	Runoff Area=25.238 ac 19.96% Impervious Runoff Depth=0.86" Tc=32.8 min CN=65/98 Runoff=12.44 cfs 1.805 af
Subcatchment SB 1: SB 1	Runoff Area=52.150 ac 0.00% Impervious Runoff Depth=0.80" Tc=53.1 min CN=74/0 Runoff=21.06 cfs 3.460 af
Subcatchment SB 11: SB 11	Runoff Area=3.290 ac 36.78% Impervious Runoff Depth=1.54" Tc=11.7 min CN=74/100 Runoff=5.05 cfs 0.422 af
Subcatchment SB 12: SB 12	Runoff Area=1.390 ac 20.86% Impervious Runoff Depth=1.17" Tc=9.5 min CN=74/98 Runoff=1.83 cfs 0.136 af
Subcatchment SB 13: SB 13	Runoff Area=2.980 ac 26.17% Impervious Runoff Depth=1.33" Tc=9.4 min CN=74/100 Runoff=4.32 cfs 0.329 af
Subcatchment SB 14: SB 14	Runoff Area=10.230 ac 16.03% Impervious Runoff Depth=1.08" Tc=4.3 min CN=74/98 Runoff=16.11 cfs 0.924 af
Subcatchment SB 15: SB 15	Runoff Area=58.570 ac 0.05% Impervious Runoff Depth=0.80" Tc=31.3 min CN=74/98 Runoff=30.89 cfs 3.890 af
Subcatchment SB 16: SB 16	Runoff Area=32.440 ac 5.76% Impervious Runoff Depth=0.90" Tc=12.1 min CN=74/98 Runoff=29.45 cfs 2.432 af
Subcatchment SB 17: SB 17	Runoff Area=7.608 ac 48.41% Impervious Runoff Depth=1.78" Tc=4.3 min CN=74/100 Runoff=18.70 cfs 1.126 af
Subcatchment SB 18: SB 18	Runoff Area=52.790 ac 0.00% Impervious Runoff Depth=0.80" Tc=33.5 min CN=74/0 Runoff=26.87 cfs 3.502 af
Subcatchment SB 19: SB 19	Runoff Area=21.190 ac 0.00% Impervious Runoff Depth=0.80" Tc=24.7 min CN=74/0 Runoff=12.45 cfs 1.406 af
Subcatchment SB 2: SB 2	Runoff Area=11.067 ac 0.33% Impervious Runoff Depth=0.80" Tc=16.6 min CN=74/98 Runoff=7.83 cfs 0.740 af
Subcatchment SB 22: SB 22	Runoff Area=41.910 ac 0.00% Impervious Runoff Depth=0.05" Tc=41.0 min CN=49/0 Runoff=0.21 cfs 0.171 af
Subcatchment SB 24: SB 24	Runoff Area=5.043 ac 97.56% Impervious Runoff Depth=2.55" Tc=7.5 min CN=74/98 Runoff=16.33 cfs 1.070 af
Subcatchment SB 25: SB 25	Runoff Area=5.136 ac 95.72% Impervious Runoff Depth=2.51" Tc=10.7 min CN=74/98 Runoff=14.24 cfs 1.075 af

Subcatchment SB 26: SB 26	Runoff Area=14.335 ac 98.27% Impervious Runoff Depth=2.56" Tc=25.4 min CN=74/98 Runoff=27.53 cfs 3.056 af
Subcatchment SB 27: SB 27 (Thumb Road)	Runoff Area=6.629 ac 83.33% Impervious Runoff Depth=2.29" Tc=27.6 min CN=74/98 Runoff=10.90 cfs 1.265 af
Subcatchment SB 28: SB 28	Runoff Area=6.955 ac 46.76% Impervious Runoff Depth=1.63" Tc=14.6 min CN=74/98 Runoff=10.87 cfs 0.947 af
Subcatchment SB 29: SB 29	Runoff Area=10.214 ac 37.73% Impervious Runoff Depth=1.47" Tc=19.1 min CN=74/98 Runoff=12.67 cfs 1.253 af
Subcatchment SB 3: SB 3	Runoff Area=37.610 ac 7.68% Impervious Runoff Depth=0.93" Tc=15.3 min CN=74/98 Runoff=32.32 cfs 2.927 af
Subcatchment SB 4: SB 4	Runoff Area=0.600 ac 43.33% Impervious Runoff Depth=1.67" Tc=5.9 min CN=74/100 Runoff=1.29 cfs 0.084 af
Subcatchment SB 5: SB 5	Runoff Area=7.860 ac 5.98% Impervious Runoff Depth=0.90" Tc=59.3 min CN=74/98 Runoff=3.34 cfs 0.592 af
Subcatchment SB 6: SB 6	Runoff Area=1.000 ac 10.00% Impervious Runoff Depth=1.00" Tc=20.3 min CN=74/100 Runoff=0.79 cfs 0.083 af
Subcatchment SB 7: SB 7	Runoff Area=21.550 ac 0.00% Impervious Runoff Depth=0.80" Tc=5.7 min CN=74/0 Runoff=22.95 cfs 1.430 af
Subcatchment SB 8: SB 8	Runoff Area=29.580 ac 5.51% Impervious Runoff Depth=0.89" Tc=47.1 min CN=74/98 Runoff=14.20 cfs 2.206 af
Subcatchment SB 9: SB 9	Runoff Area=25.780 ac 0.12% Impervious Runoff Depth=0.80" Tc=30.0 min CN=74/98 Runoff=13.82 cfs 1.715 af
Subcatchment SB10: SB 10	Runoff Area=6.390 ac 4.85% Impervious Runoff Depth=0.88" Tc=7.3 min CN=74/98 Runoff=6.95 cfs 0.470 af
Pond 2 P: P-2	Peak Elev=924.73' Storage=1.016 af Inflow=25.48 cfs 5.269 af Outflow=25.29 cfs 5.269 af
Pond 4P: P-4	Peak Elev=915.44' Storage=0.773 af Inflow=3.34 cfs 0.592 af Primary=0.63 cfs 0.211 af Secondary=1.41 cfs 0.381 af Outflow=2.04 cfs 0.592 af
Pond 7P: P-7	Peak Elev=915.29' Storage=1.196 af Inflow=14.20 cfs 2.206 af Outflow=14.39 cfs 2.206 af
Pond 8P: P-8	Peak Elev=897.58' Storage=0.694 af Inflow=6.95 cfs 0.470 af 24.0" Round Culvert n=0.013 L=380.0' S=0.0028 '/' Outflow=1.19 cfs 0.465 af
Pond 9P: P-9	Peak Elev=915.26' Storage=0.414 af Inflow=25.40 cfs 3.921 af Outflow=25.36 cfs 3.921 af
Pond 10P: P-10 Lowered 1 ft	Peak Elev=896.83' Storage=0.954 af Inflow=6.10 cfs 3.683 af Primary=5.76 cfs 3.682 af Secondary=0.00 cfs 0.000 af Outflow=5.76 cfs 3.682 af

Pond 11P: P-11 Peak Elev=910.30' Storage=5.310 af Inflow=26.75 cfs 4.343 af
Primary=5.18 cfs 3.083 af Secondary=3.14 cfs 1.259 af Outflow=8.30 cfs 4.342 af

Pond 12P: P-12 Peak Elev=893.62' Storage=6.109 af Inflow=16.56 cfs 6.175 af
Outflow=6.46 cfs 6.168 af

Pond 13P: P-13 Peak Elev=883.70' Storage=5.659 af Inflow=99.54 cfs 19.048 af
Primary=86.58 cfs 18.259 af Secondary=5.32 cfs 0.787 af Outflow=91.90 cfs 19.046 af

Pond 17P: W-2 Peak Elev=929.35' Storage=0.390 af Inflow=1.51 cfs 0.492 af
12.0" Round Culvert n=0.013 L=300.0' S=0.0437 '/' Outflow=0.32 cfs 0.351 af

Pond 36P: Culverts passing flow beneath Peak Elev=887.11' Storage=0.000 af Inflow=26.87 cfs 3.502 af
Primary=26.87 cfs 3.502 af Secondary=0.00 cfs 0.000 af Outflow=26.87 cfs 3.502 af

Pond CRH-1: CRH-1 Peak Elev=877.67' Storage=0.354 af Inflow=10.87 cfs 0.947 af
Discarded=0.22 cfs 0.471 af Primary=4.58 cfs 0.476 af Outflow=4.80 cfs 0.947 af

Pond CRH-2: CRH-2 Peak Elev=882.04' Storage=0.617 af Inflow=12.67 cfs 1.253 af
Discarded=0.33 cfs 0.833 af Primary=2.29 cfs 0.420 af Outflow=2.62 cfs 1.254 af

Pond CRH-3: CRH-3 Peak Elev=878.30' Storage=0.259 af Inflow=2.97 cfs 0.603 af
Discarded=0.20 cfs 0.381 af Primary=0.97 cfs 0.222 af Outflow=1.17 cfs 0.603 af

Pond P-5/P-6: P-5/P-6 Peak Elev=929.88' Storage=6.897 af Inflow=44.72 cfs 4.086 af
Primary=8.78 cfs 3.587 af Secondary=1.51 cfs 0.492 af Outflow=10.29 cfs 4.079 af

Pond TI P: Thumb Infiltration (Thumb TP Peak Elev=901.44' Storage=1.436 af Inflow=10.90 cfs 1.436 af
Outflow=0.00 cfs 0.000 af

Pond W-1: W-1 Peak Elev=914.97' Storage=0.148 af Inflow=1.52 cfs 0.464 af
Outflow=1.05 cfs 0.464 af

Pond W-3: W-3 Peak Elev=914.92' Storage=0.351 af Inflow=0.32 cfs 0.351 af
Outflow=0.00 cfs 0.000 af

Pond W-4: W-4 Peak Elev=908.71' Storage=0.649 af Inflow=4.39 cfs 1.588 af
12.0" Round Culvert n=0.013 L=170.0' S=0.0235 '/' Outflow=2.16 cfs 1.570 af

Pond W-5: W-5 Peak Elev=882.93' Storage=4.298 af Inflow=19.24 cfs 1.913 af
Outflow=2.91 cfs 1.906 af

Link 53L: Sum of Outlet #2 Discharges to Round Lake Inflow=40.20 cfs 10.286 af
Primary=40.20 cfs 10.286 af

Link 54L: Sum of Discharges from P-13 and W-5 Inflow=88.88 cfs 20.165 af
Primary=88.88 cfs 20.165 af

Link 55L: Sum of Outlet #1 Discharges to Round Lake Inflow=1.55 cfs 0.675 af
Primary=1.55 cfs 0.675 af

Link 57L: Outlet #3 Discharge to Round Lake

Inflow=12.44 cfs 1.805 af

Primary=12.44 cfs 1.805 af

Total Runoff Area = 501.136 ac Runoff Volume = 38.698 af Average Runoff Depth = 0.93"
88.56% Pervious = 443.803 ac 11.44% Impervious = 57.333 ac

Summary for Subcatchment 1S: To Rice Creek

Runoff = 2.97 cfs @ 12.04 hrs, Volume= 0.183 af, Depth= 1.37"

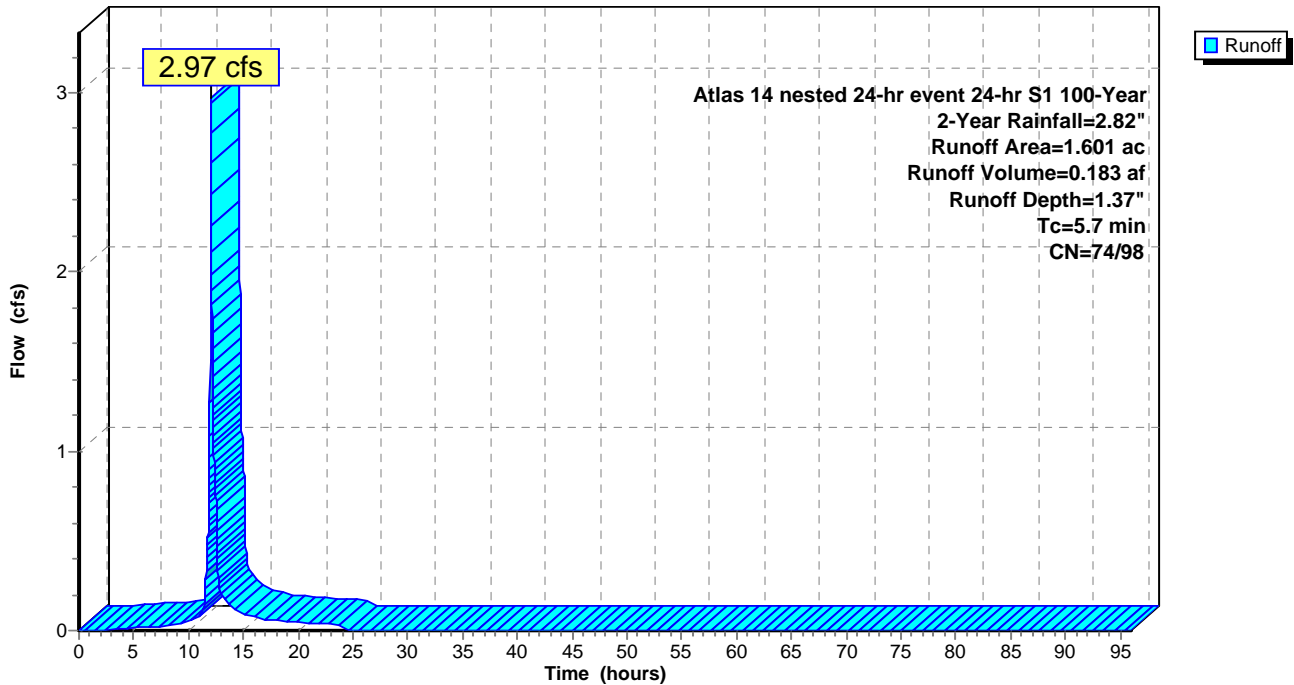
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 0.512	98	impervious
* 1.089	74	pervious
1.601	82	Weighted Average
1.089		68.02% Pervious Area
0.512		31.98% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.7					Direct Entry,

Subcatchment 1S: To Rice Creek

Hydrograph



Summary for Subcatchment 47S: Offsite Subbasin 51

Runoff = 12.44 cfs @ 12.43 hrs, Volume= 1.805 af, Depth= 0.86"

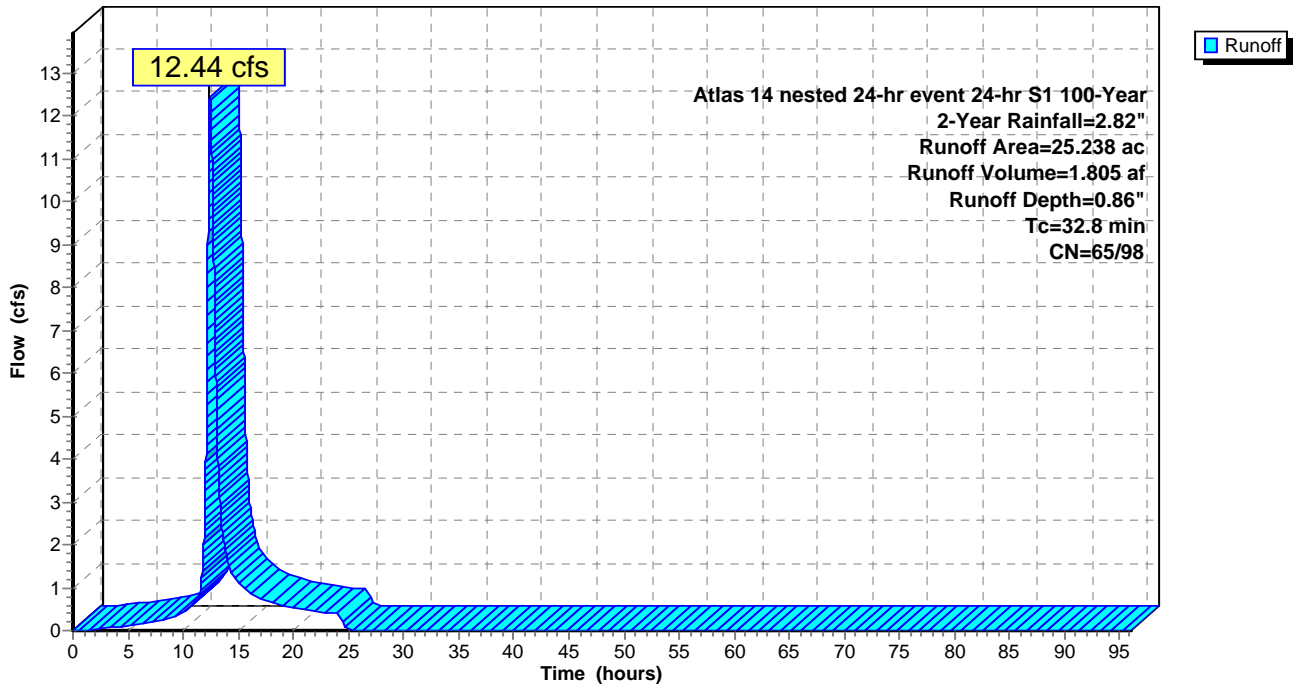
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 20.200	65	Offsite subbasin 51
* 5.038	98	
25.238	72	Weighted Average
20.200		80.04% Pervious Area
5.038		19.96% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.8					Direct Entry,

Subcatchment 47S: Offsite Subbasin 51

Hydrograph



Summary for Subcatchment SB 1: SB 1

Runoff = 21.06 cfs @ 12.80 hrs, Volume= 3.460 af, Depth= 0.80"

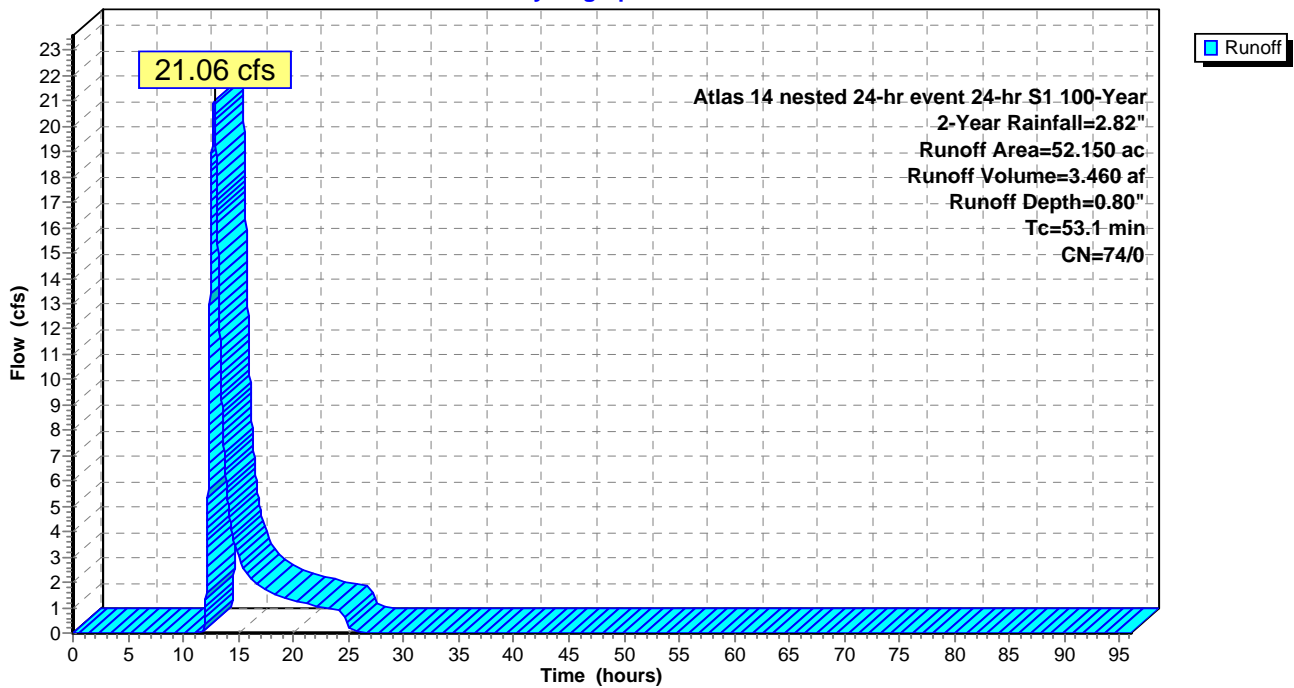
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 52.150	74	pervious
* 0.000	98	impervious
52.150	74	Weighted Average
52.150		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
53.1					Direct Entry,

Subcatchment SB 1: SB 1

Hydrograph



Summary for Subcatchment SB 11: SB 11

Runoff = 5.05 cfs @ 12.11 hrs, Volume= 0.422 af, Depth= 1.54"

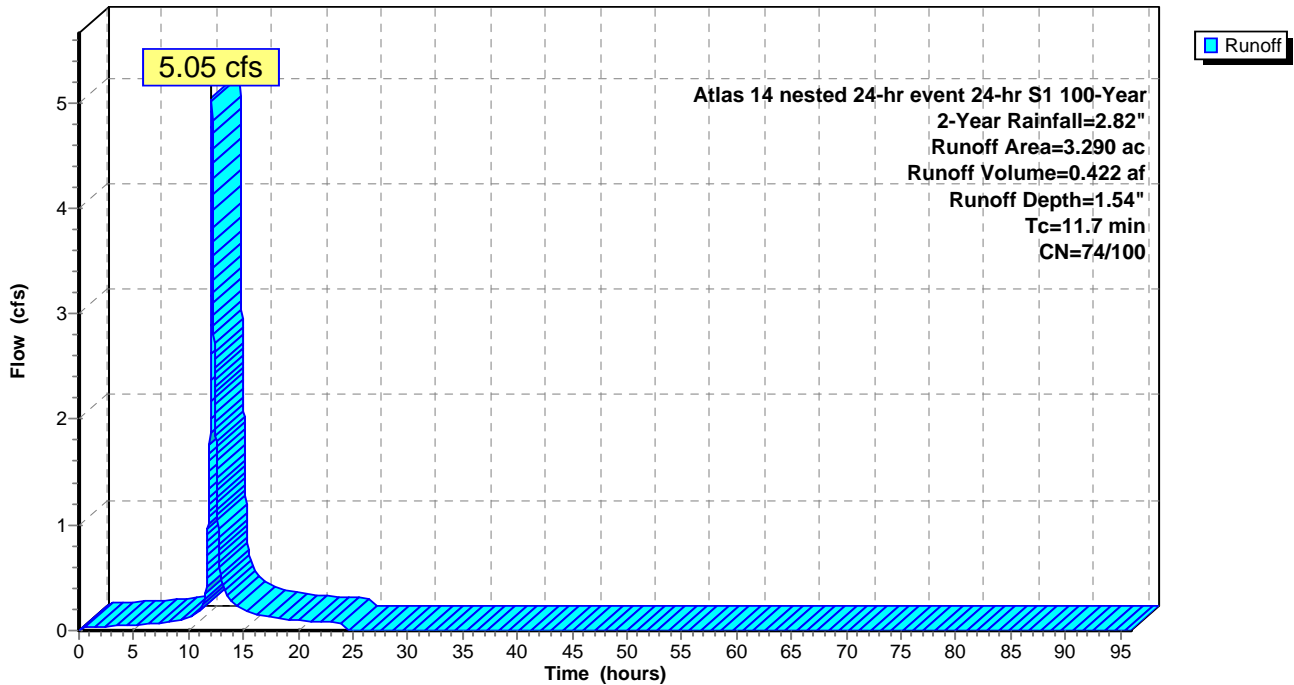
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 2.080	74	pervious
* 1.210	100	impervious
3.290	84	Weighted Average
2.080		63.22% Pervious Area
1.210		36.78% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.7					Direct Entry,

Subcatchment SB 11: SB 11

Hydrograph



Summary for Subcatchment SB 12: SB 12

Runoff = 1.83 cfs @ 12.09 hrs, Volume= 0.136 af, Depth= 1.17"

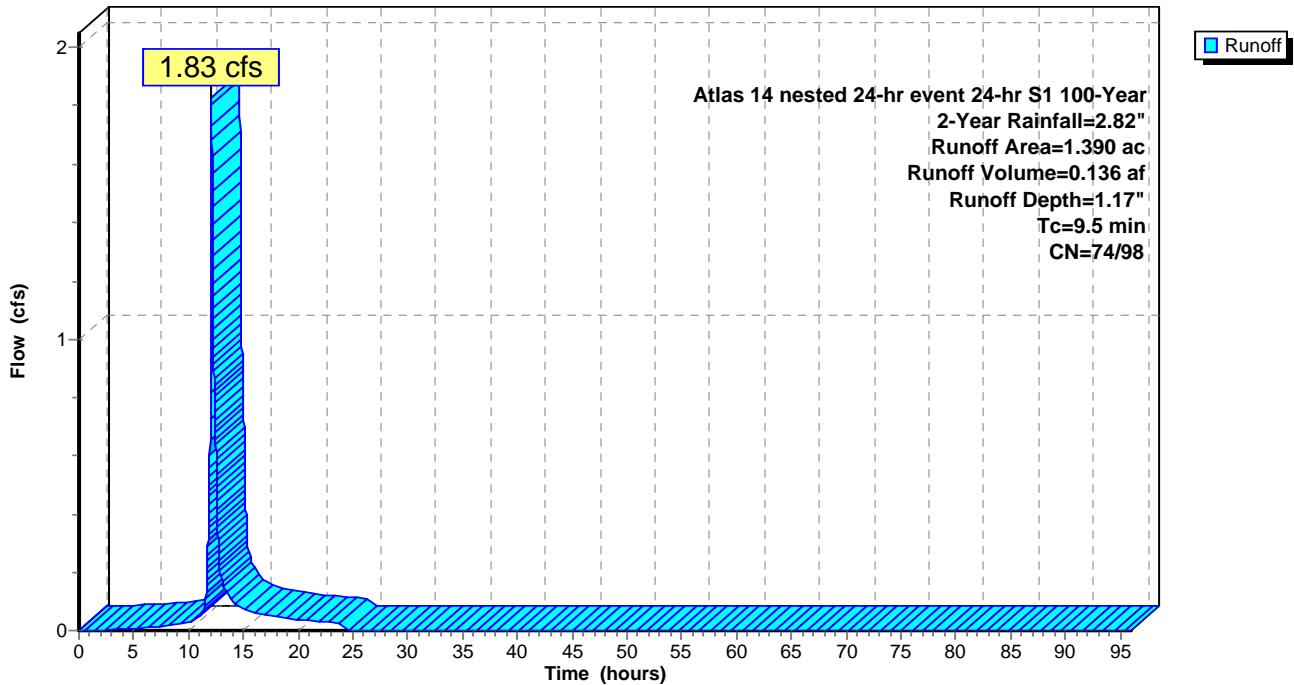
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 1.100	74	pervious
* 0.290	98	impervious
1.390	79	Weighted Average
1.100		79.14% Pervious Area
0.290		20.86% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.5					Direct Entry,

Subcatchment SB 12: SB 12

Hydrograph



Summary for Subcatchment SB 13: SB 13

Runoff = 4.32 cfs @ 12.08 hrs, Volume= 0.329 af, Depth= 1.33"

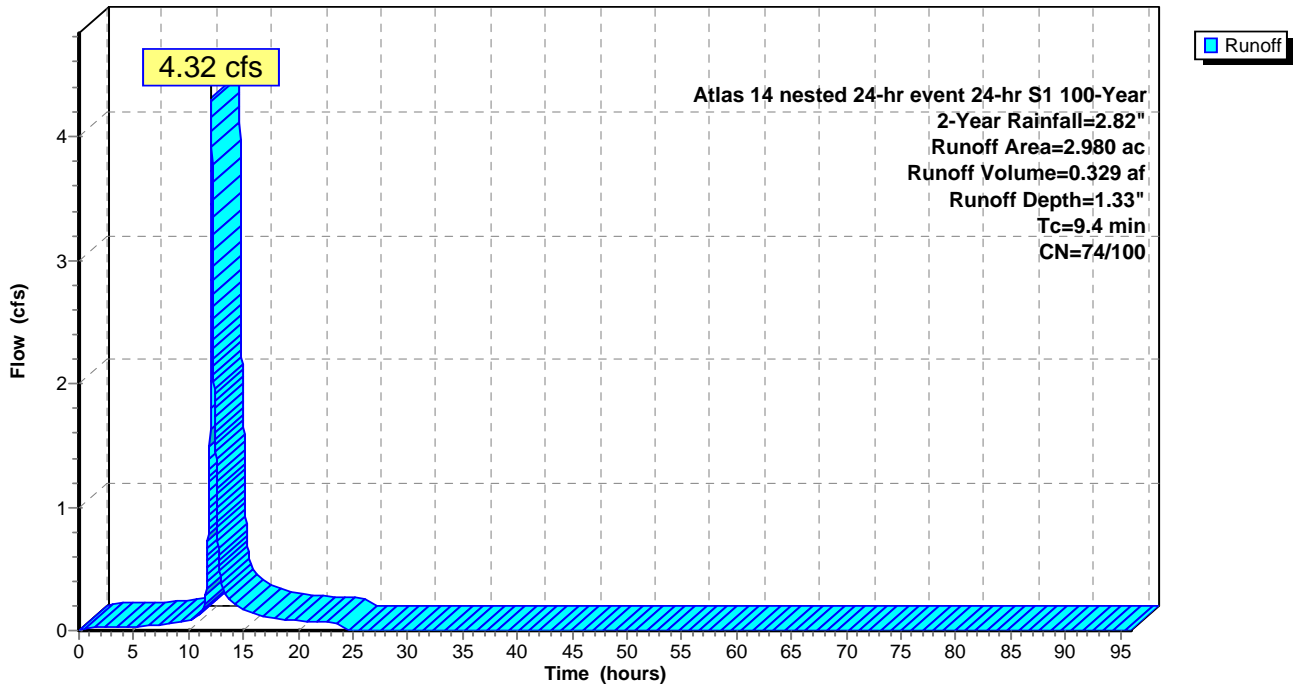
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 2.200	74	pervious
* 0.780	100	impervious
2.980	81	Weighted Average
2.200		73.83% Pervious Area
0.780		26.17% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.4					Direct Entry,

Subcatchment SB 13: SB 13

Hydrograph



Summary for Subcatchment SB 14: SB 14

Runoff = 16.11 cfs @ 12.03 hrs, Volume= 0.924 af, Depth= 1.08"

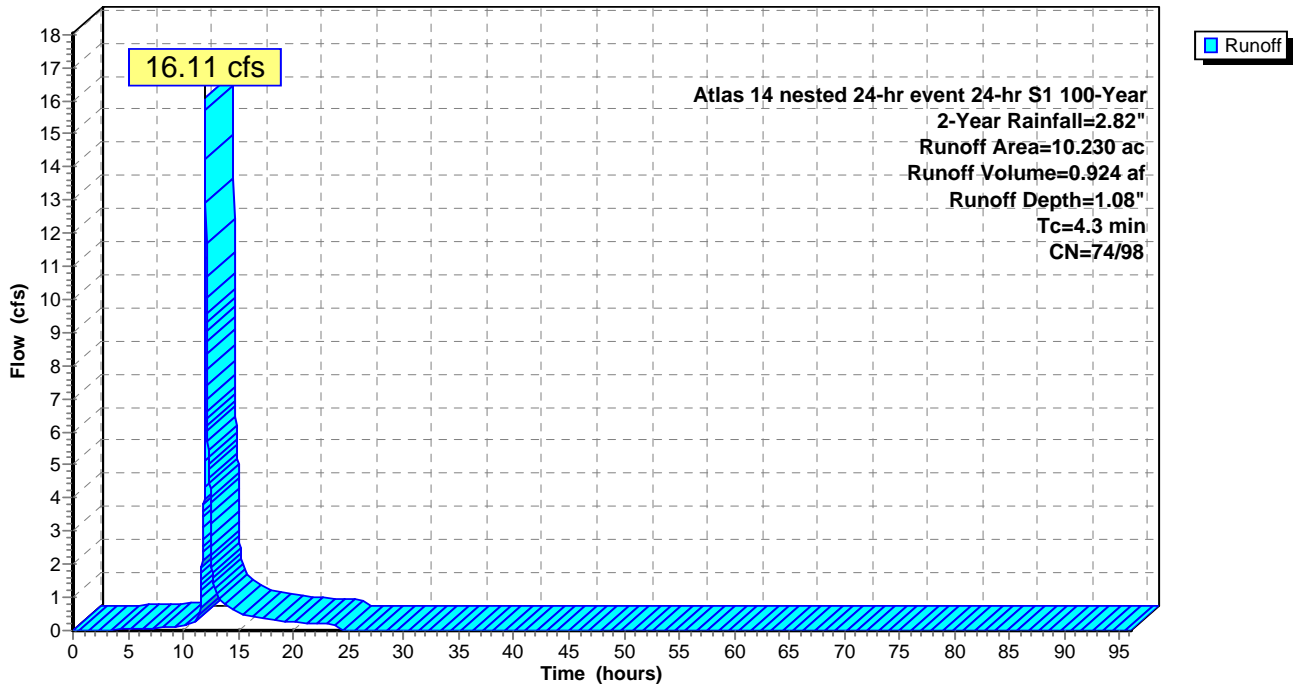
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 8.590	74	pervious
* 1.640	98	impervious
10.230	78	Weighted Average
8.590		83.97% Pervious Area
1.640		16.03% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.3					Direct Entry,

Subcatchment SB 14: SB 14

Hydrograph



Summary for Subcatchment SB 15: SB 15

Runoff = 30.89 cfs @ 12.45 hrs, Volume= 3.890 af, Depth= 0.80"

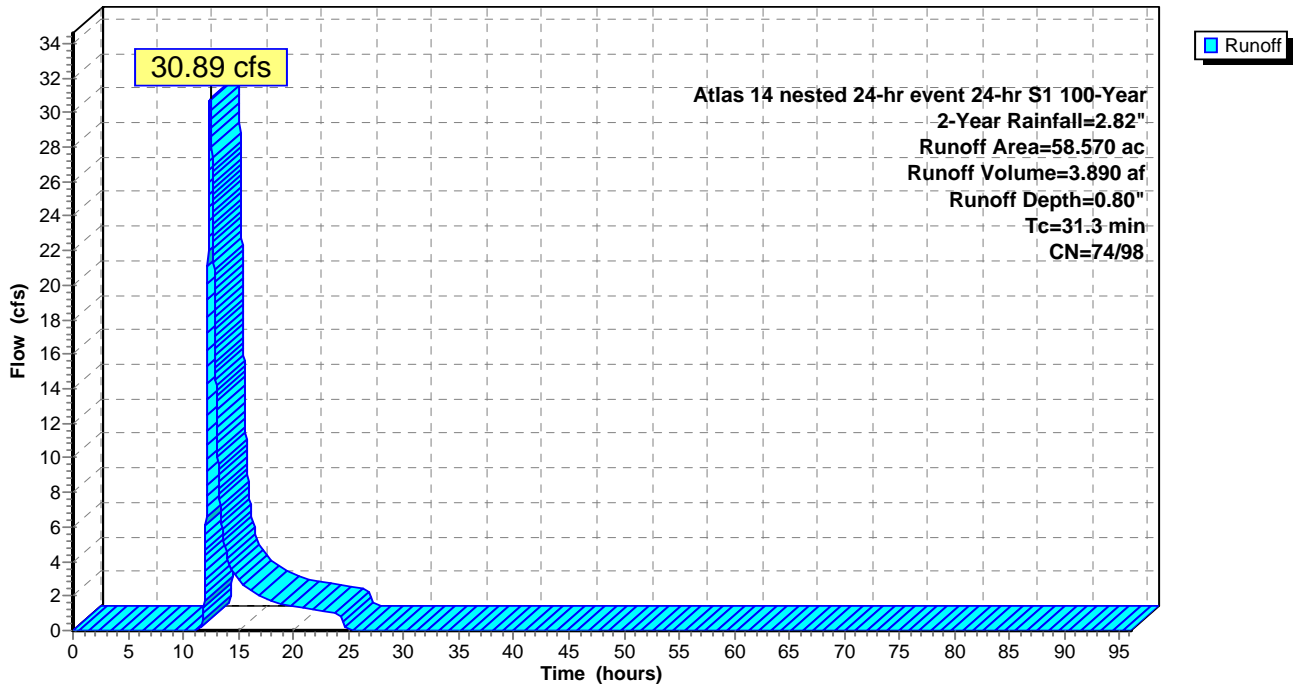
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 58.540	74	pervious
* 0.030	98	impervious
58.570	74	Weighted Average
58.540		99.95% Pervious Area
0.030		0.05% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
31.3					Direct Entry,

Subcatchment SB 15: SB 15

Hydrograph



Summary for Subcatchment SB 16: SB 16

Runoff = 29.45 cfs @ 12.14 hrs, Volume= 2.432 af, Depth= 0.90"

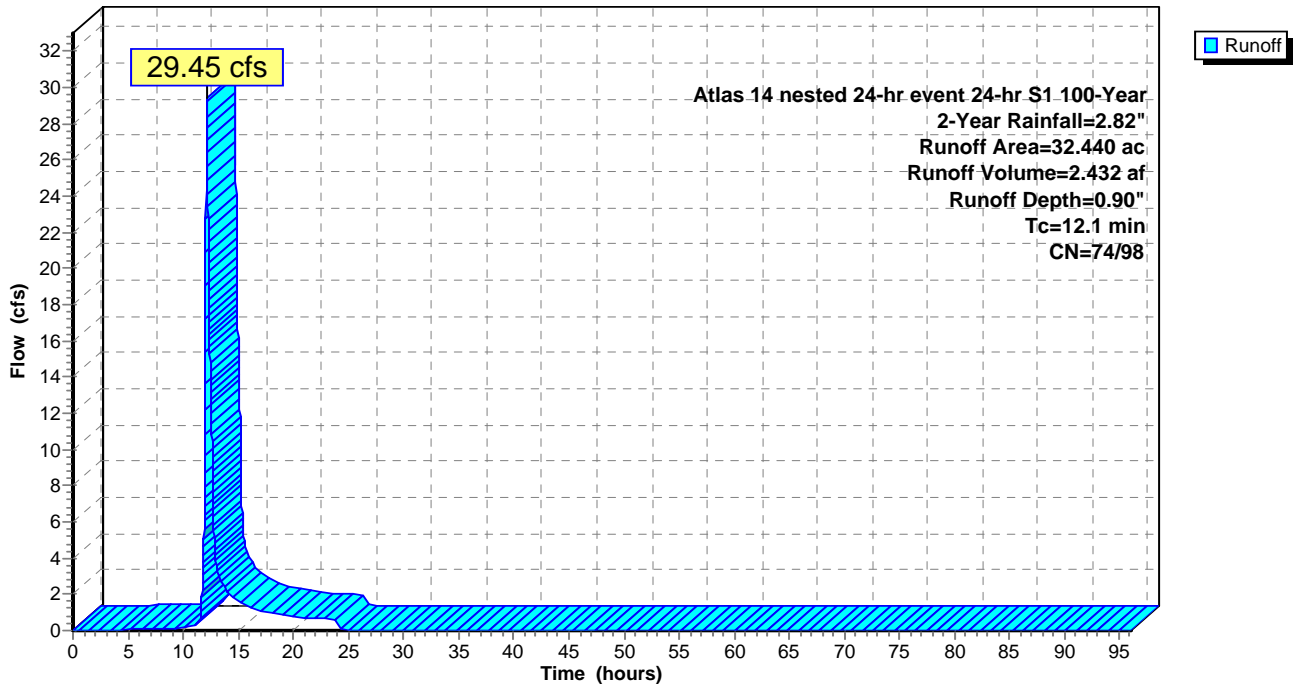
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 30.570	74	pervious
* 1.870	98	impervious
32.440	75	Weighted Average
30.570		94.24% Pervious Area
1.870		5.76% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.1					Direct Entry,

Subcatchment SB 16: SB 16

Hydrograph



Summary for Subcatchment SB 17: SB 17

Runoff = 18.70 cfs @ 12.02 hrs, Volume= 1.126 af, Depth= 1.78"

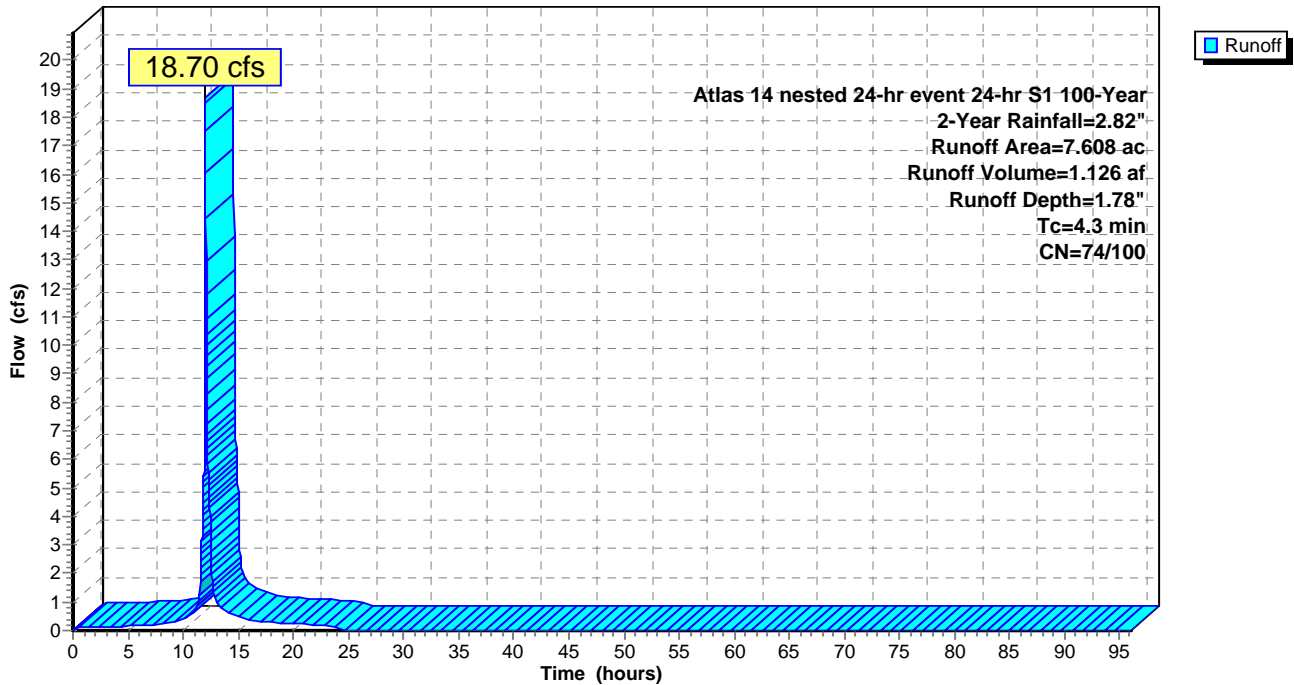
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 3.925	74	pervious
* 3.683	100	impervious
7.608	87	Weighted Average
3.925		51.59% Pervious Area
3.683		48.41% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.3					Direct Entry,

Subcatchment SB 17: SB 17

Hydrograph



Summary for Subcatchment SB 18: SB 18

Runoff = 26.87 cfs @ 12.48 hrs, Volume= 3.502 af, Depth= 0.80"

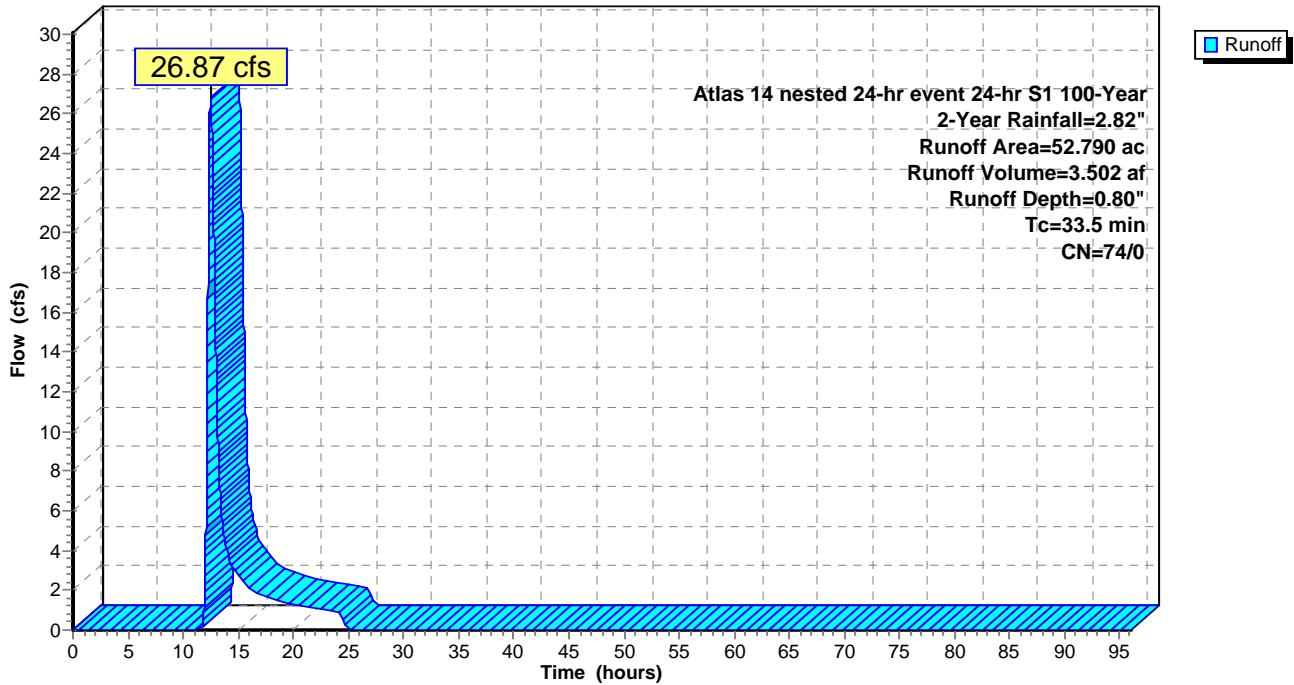
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 52.790	74	pervious
* 0.000	98	impervious
52.790	74	Weighted Average
52.790		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
33.5					Direct Entry,

Subcatchment SB 18: SB 18

Hydrograph



Summary for Subcatchment SB 19: SB 19

Runoff = 12.45 cfs @ 12.34 hrs, Volume= 1.406 af, Depth= 0.80"

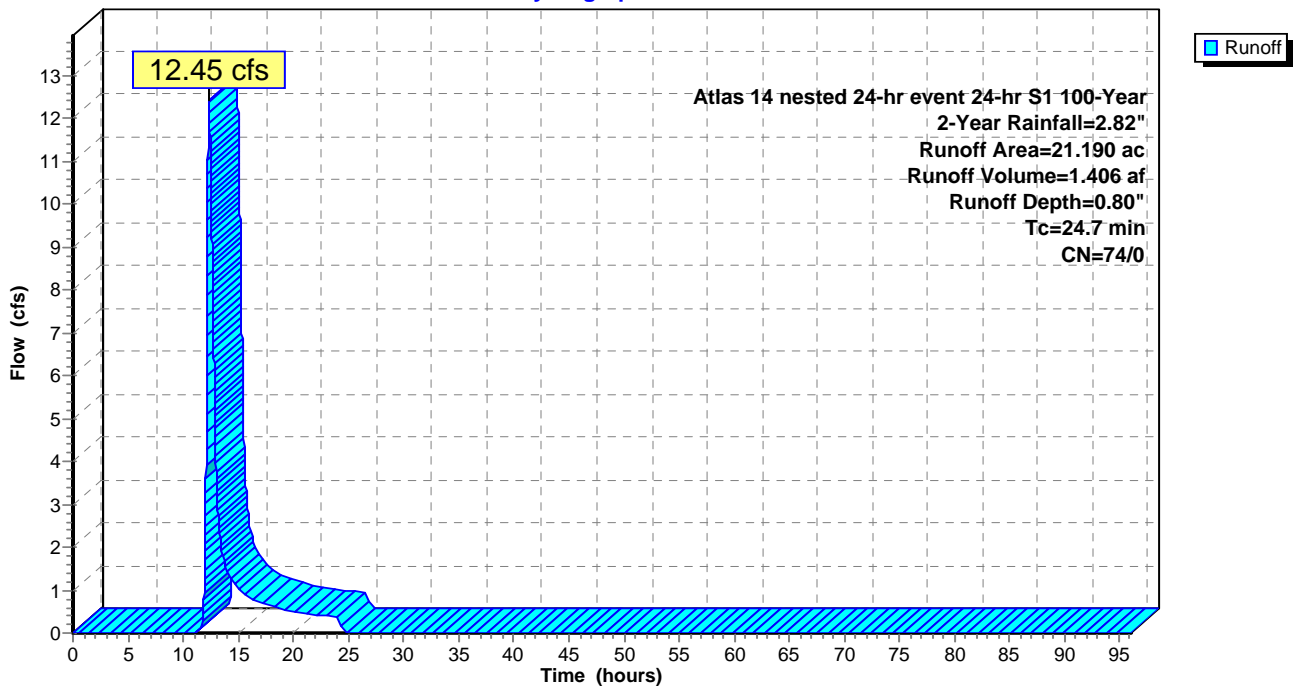
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 21.190	74	pervious
* 0.000	98	impervious
21.190	74	Weighted Average
21.190		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.7					Direct Entry,

Subcatchment SB 19: SB 19

Hydrograph



Summary for Subcatchment SB 2: SB 2

Runoff = 7.83 cfs @ 12.21 hrs, Volume= 0.740 af, Depth= 0.80"

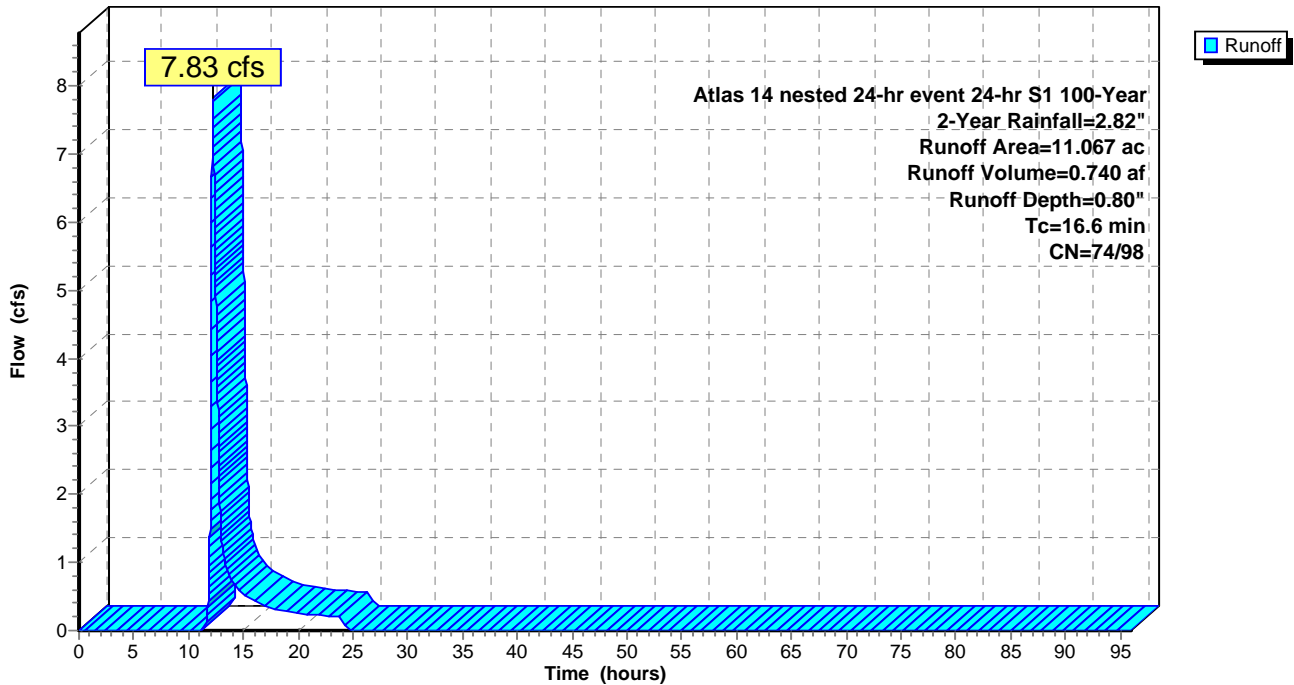
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

	Area (ac)	CN	Description
*	11.030	74	pervious
*	0.037	98	impervious
	11.067	74	Weighted Average
	11.030		99.67% Pervious Area
	0.037		0.33% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.6					Direct Entry,

Subcatchment SB 2: SB 2

Hydrograph



Summary for Subcatchment SB 22: SB 22

Runoff = 0.21 cfs @ 15.17 hrs, Volume= 0.171 af, Depth= 0.05"

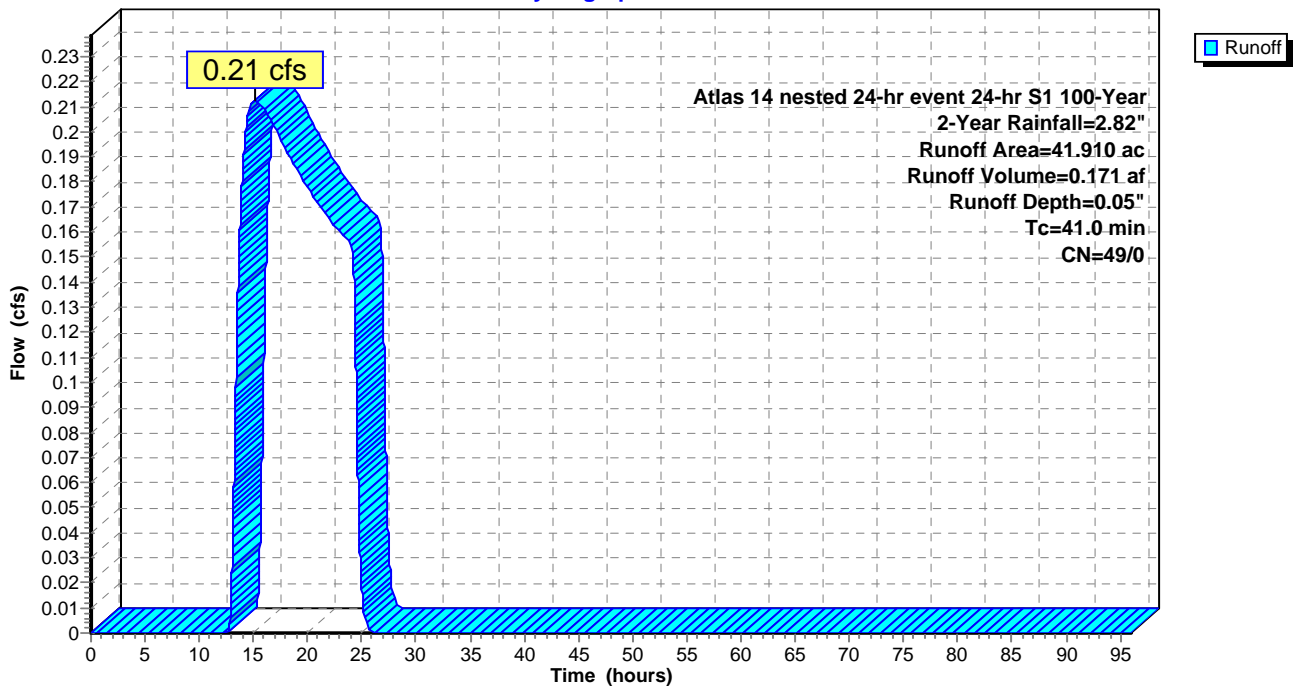
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 41.910	49	Pervious
* 0.000	98	Impervious
41.910	49	Weighted Average
41.910		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
41.0					Direct Entry,

Subcatchment SB 22: SB 22

Hydrograph



Summary for Subcatchment SB 24: SB 24

Runoff = 16.33 cfs @ 12.05 hrs, Volume= 1.070 af, Depth= 2.55"

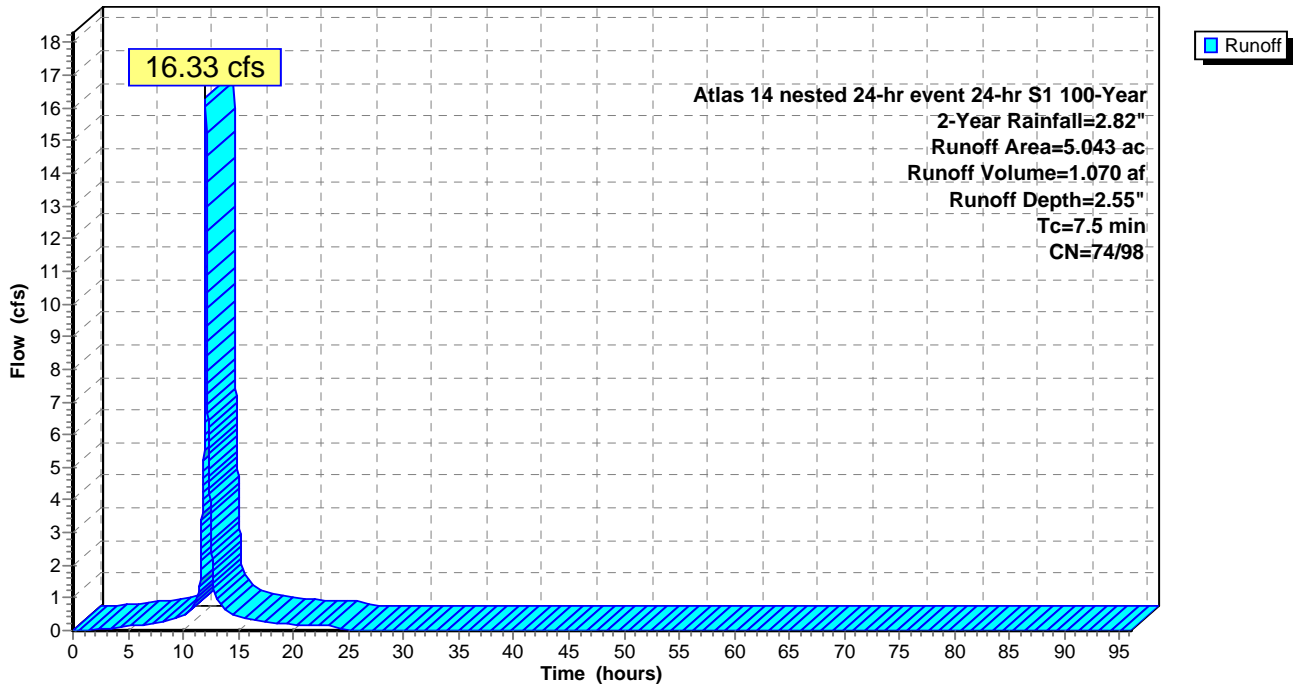
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 0.123	74	permiabile
* 4.920	98	impermiabile
5.043	97	Weighted Average
0.123		2.44% Pervious Area
4.920		97.56% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.5					Direct Entry,

Subcatchment SB 24: SB 24

Hydrograph



Summary for Subcatchment SB 25: SB 25

Runoff = 14.24 cfs @ 12.09 hrs, Volume= 1.075 af, Depth= 2.51"

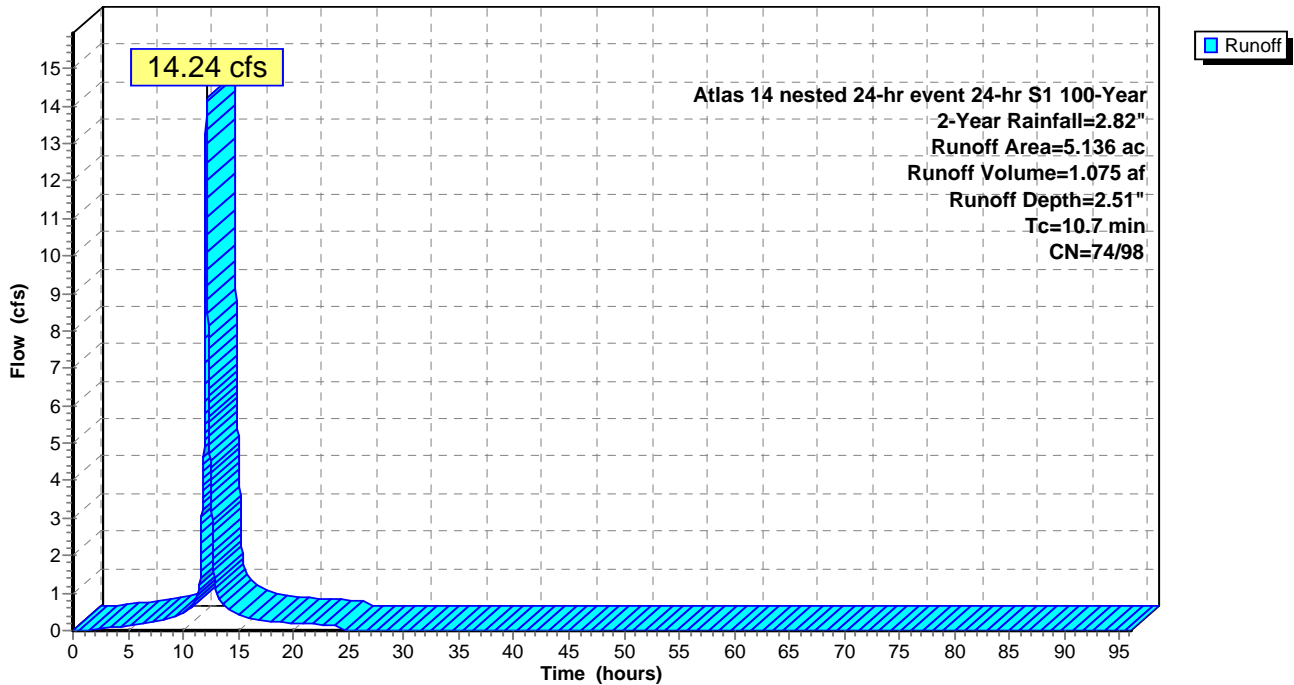
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 0.220	74	pervious
* 4.916	98	impervious
5.136	97	Weighted Average
0.220		4.28% Pervious Area
4.916		95.72% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7					Direct Entry,

Subcatchment SB 25: SB 25

Hydrograph



Summary for Subcatchment SB 26: SB 26

Runoff = 27.53 cfs @ 12.28 hrs, Volume= 3.056 af, Depth= 2.56"

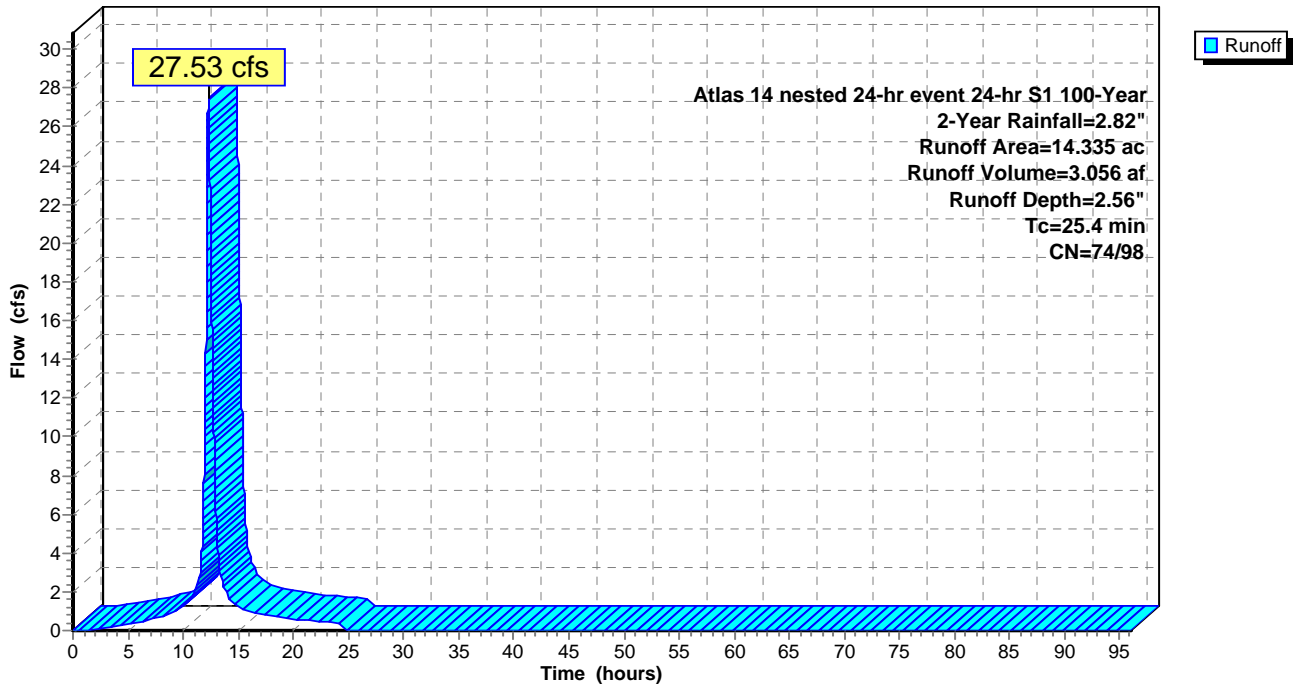
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 0.248	74	pervious
* 14.087	98	impervious
14.335	98	Weighted Average
0.248		1.73% Pervious Area
14.087		98.27% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
25.4					Direct Entry,

Subcatchment SB 26: SB 26

Hydrograph



Summary for Subcatchment SB 27: SB 27 (Thumb Road)

Runoff = 10.90 cfs @ 12.32 hrs, Volume= 1.265 af, Depth= 2.29"

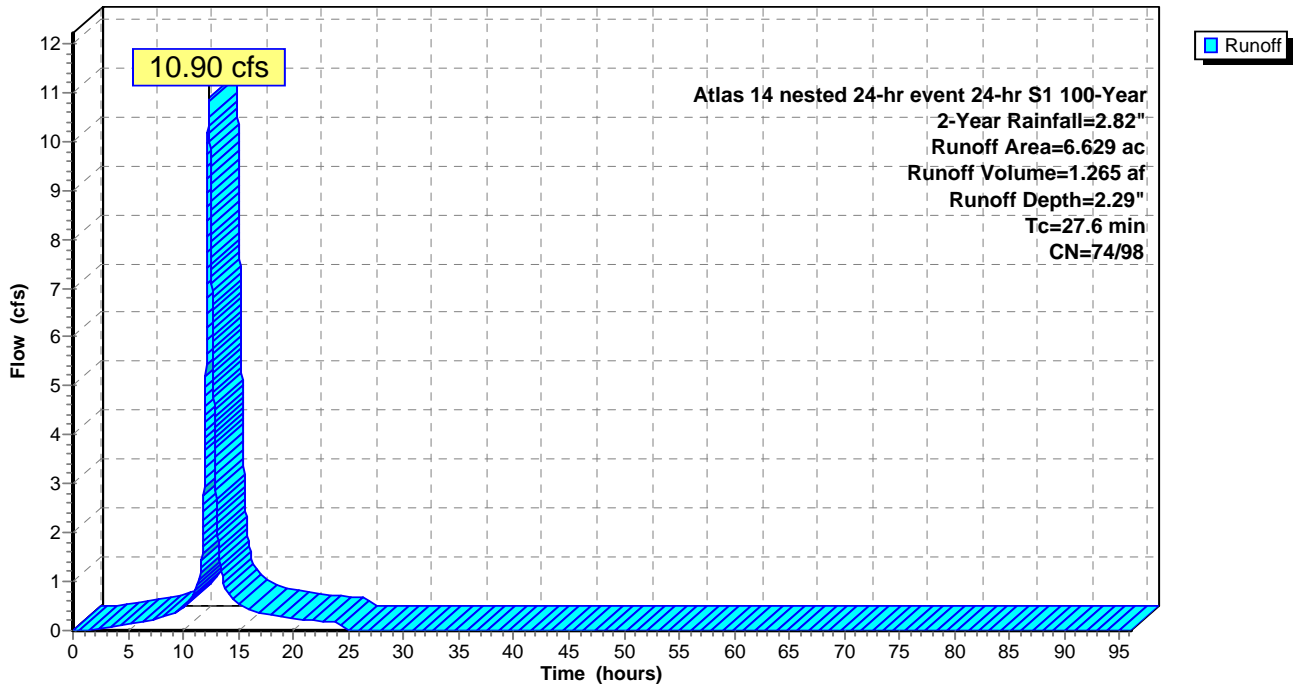
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 1.105	74	Pervious
* 5.524	98	Impervious
6.629	94	Weighted Average
1.105		16.67% Pervious Area
5.524		83.33% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.6					Direct Entry,

Subcatchment SB 27: SB 27 (Thumb Road)

Hydrograph



Summary for Subcatchment SB 28: SB 28

Runoff = 10.87 cfs @ 12.15 hrs, Volume= 0.947 af, Depth= 1.63"

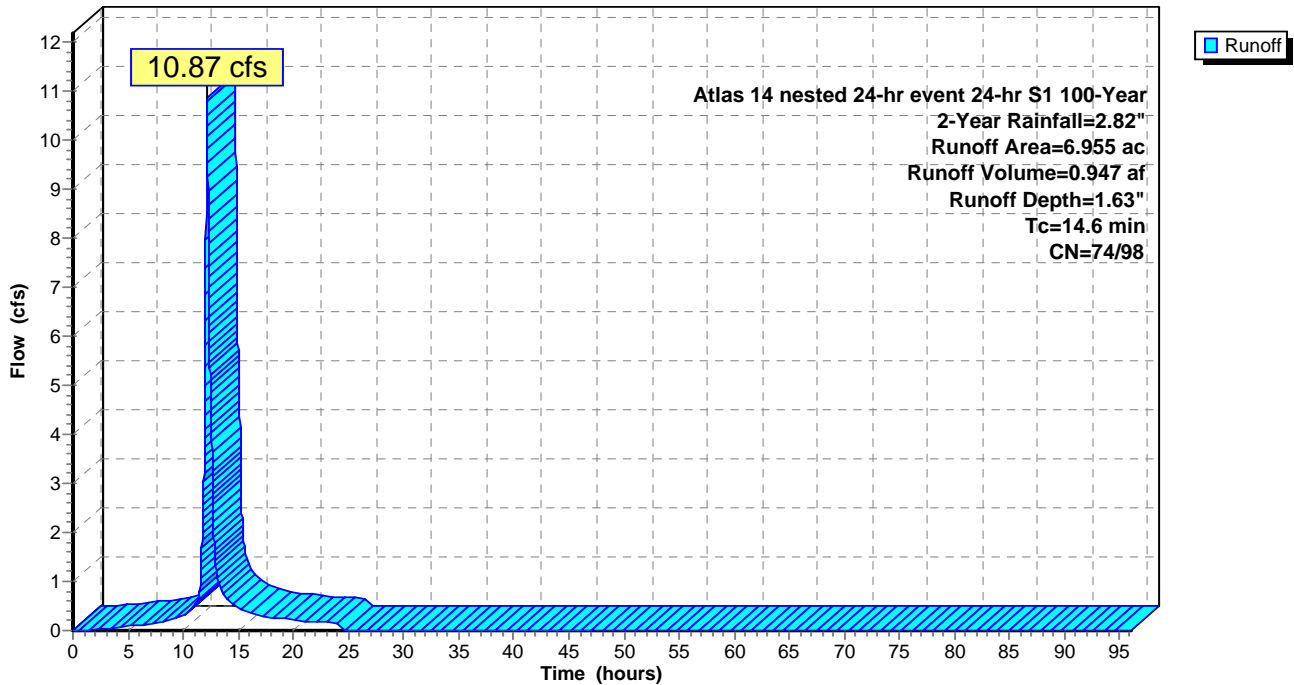
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 3.703	74	pervious
* 3.252	98	impervious
6.955	85	Weighted Average
3.703		53.24% Pervious Area
3.252		46.76% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.6					Direct Entry,

Subcatchment SB 28: SB 28

Hydrograph



Summary for Subcatchment SB 29: SB 29

Runoff = 12.67 cfs @ 12.22 hrs, Volume= 1.253 af, Depth= 1.47"

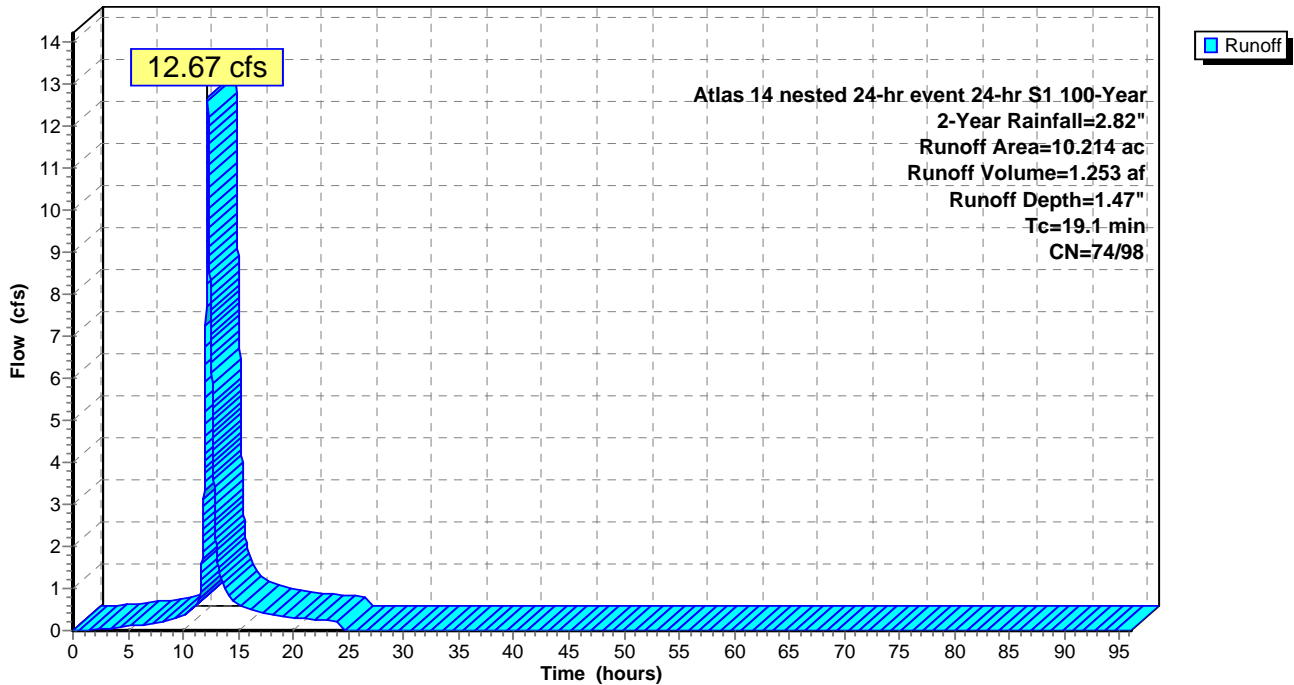
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 6.360	74	pervious
* 3.854	98	impervious
10.214	83	Weighted Average
6.360		62.27% Pervious Area
3.854		37.73% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.1					Direct Entry,

Subcatchment SB 29: SB 29

Hydrograph



Summary for Subcatchment SB 3: SB 3

Runoff = 32.32 cfs @ 12.19 hrs, Volume= 2.927 af, Depth= 0.93"

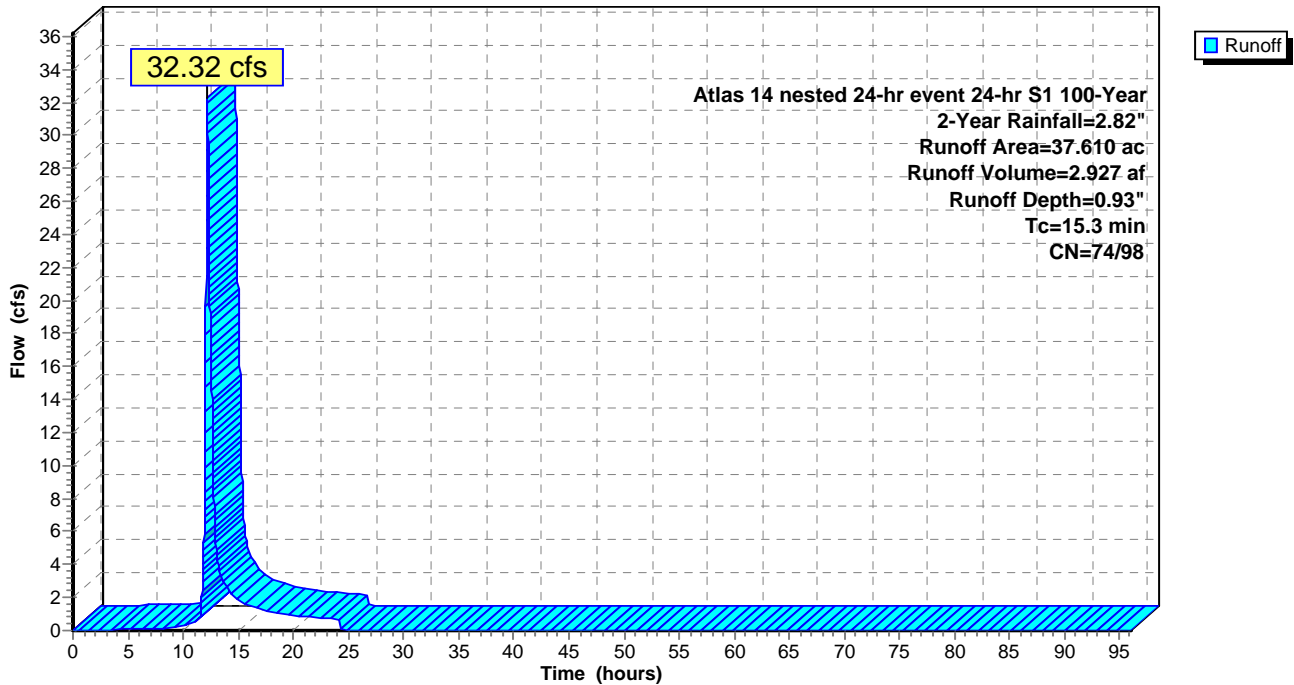
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 34.720	74	Pervious
* 2.890	98	Impervious
37.610	76	Weighted Average
34.720		92.32% Pervious Area
2.890		7.68% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.3					Direct Entry,

Subcatchment SB 3: SB 3

Hydrograph



Summary for Subcatchment SB 4: SB 4

Runoff = 1.29 cfs @ 12.04 hrs, Volume= 0.084 af, Depth= 1.67"

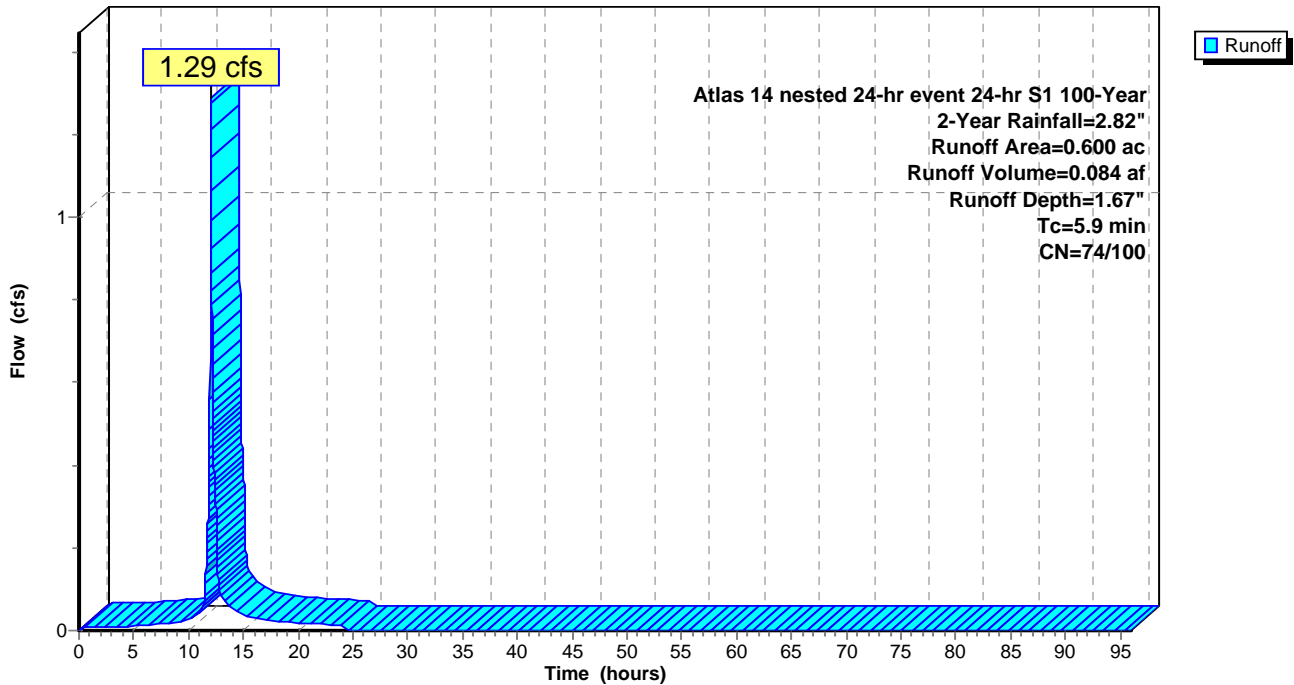
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 0.340	74	pervious
* 0.260	100	impervious
0.600	85	Weighted Average
0.340		56.67% Pervious Area
0.260		43.33% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.9					Direct Entry,

Subcatchment SB 4: SB 4

Hydrograph



Summary for Subcatchment SB 5: SB 5

Runoff = 3.34 cfs @ 12.85 hrs, Volume= 0.592 af, Depth= 0.90"

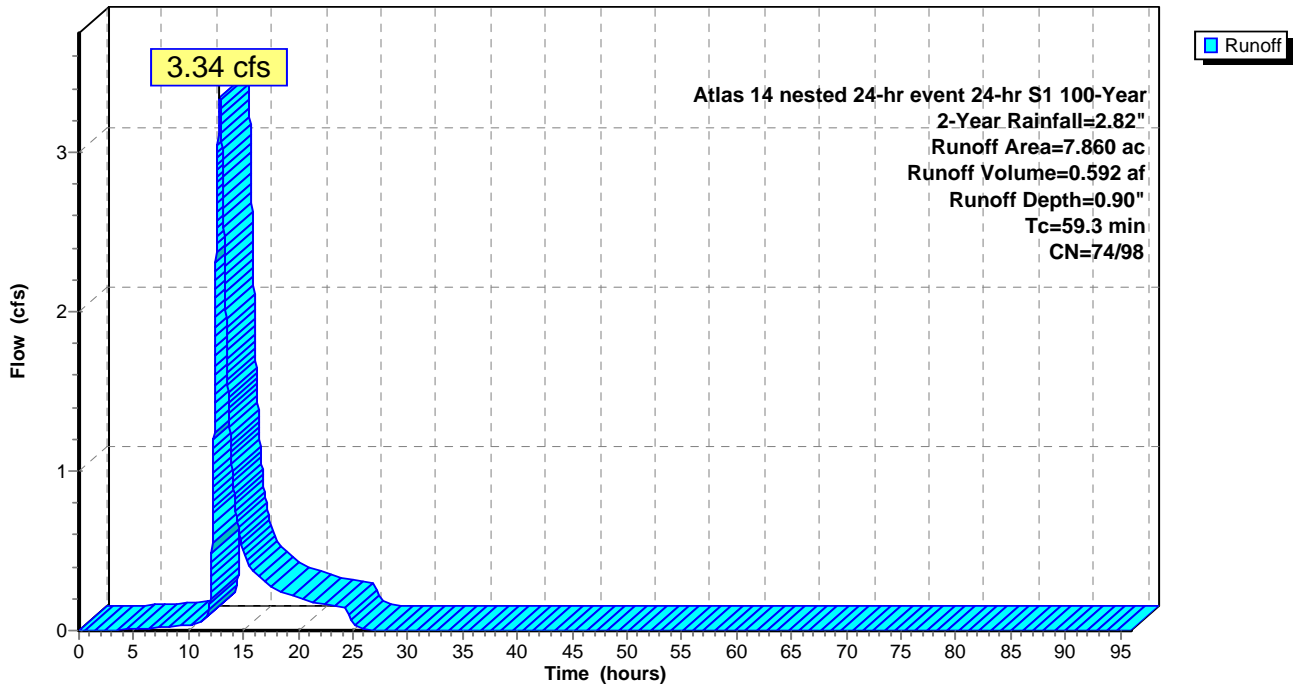
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 7.390	74	pervious
* 0.470	98	impervious
7.860	75	Weighted Average
7.390		94.02% Pervious Area
0.470		5.98% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
59.3					Direct Entry,

Subcatchment SB 5: SB 5

Hydrograph



Summary for Subcatchment SB 6: SB 6

Runoff = 0.79 cfs @ 12.25 hrs, Volume= 0.083 af, Depth= 1.00"

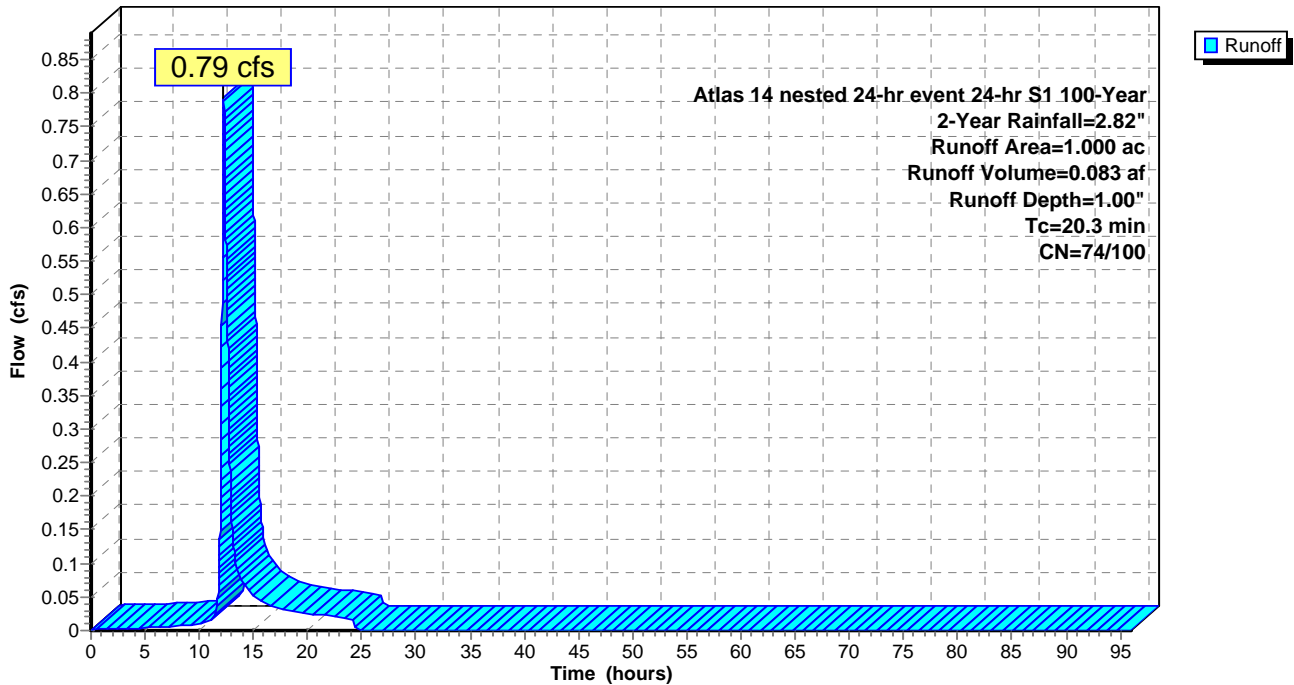
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 0.900	74	pervious
* 0.100	100	impervious
1.000	77	Weighted Average
0.900		90.00% Pervious Area
0.100		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.3					Direct Entry,

Subcatchment SB 6: SB 6

Hydrograph



Summary for Subcatchment SB 7: SB 7

Runoff = 22.95 cfs @ 12.04 hrs, Volume= 1.430 af, Depth= 0.80"

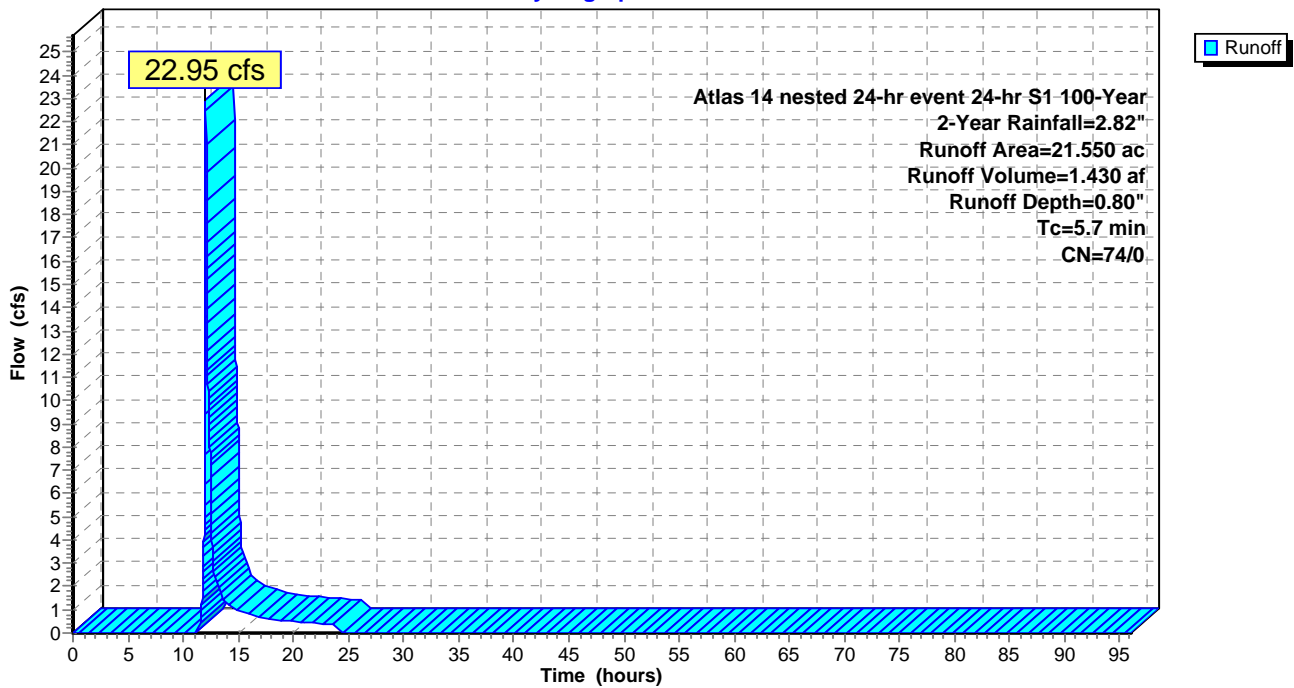
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 21.550	74	pervious
* 0.000	98	impervious
21.550	74	Weighted Average
21.550		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.7					Direct Entry,

Subcatchment SB 7: SB 7

Hydrograph



Summary for Subcatchment SB 8: SB 8

Runoff = 14.20 cfs @ 12.67 hrs, Volume= 2.206 af, Depth= 0.89"

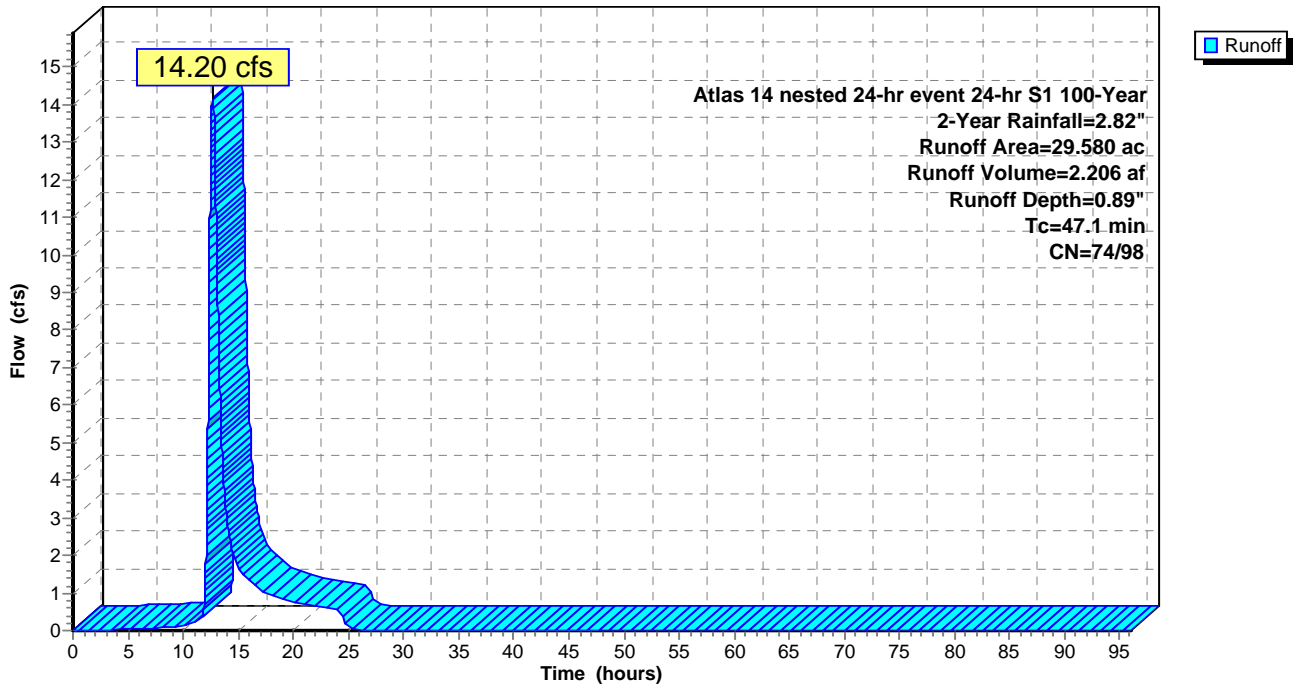
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 27.950	74	pervious
* 1.630	98	impervious
29.580	75	Weighted Average
27.950		94.49% Pervious Area
1.630		5.51% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
47.1					Direct Entry,

Subcatchment SB 8: SB 8

Hydrograph



Summary for Subcatchment SB 9: SB 9

Runoff = 13.82 cfs @ 12.43 hrs, Volume= 1.715 af, Depth= 0.80"

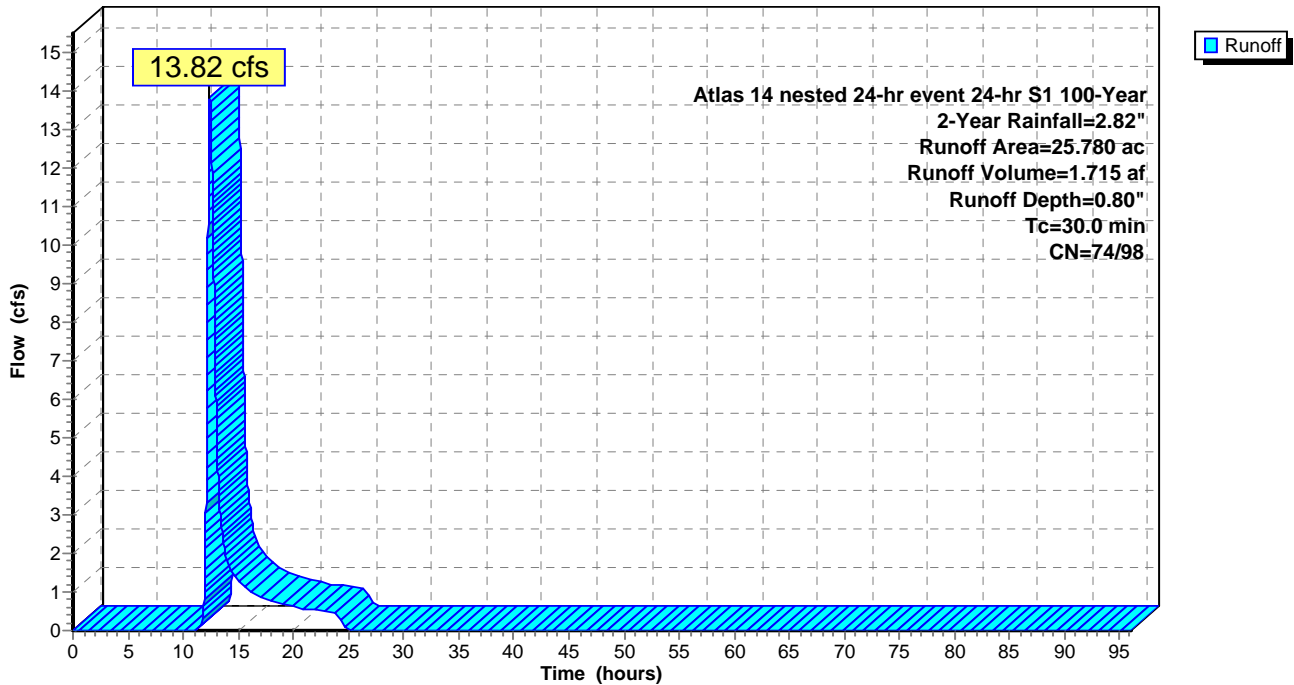
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 25.750	74	permiabile
* 0.030	98	impermiabile
25.780	74	Weighted Average
25.750		99.88% Pervious Area
0.030		0.12% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.0					Direct Entry,

Subcatchment SB 9: SB 9

Hydrograph



Summary for Subcatchment SB10: SB 10

Runoff = 6.95 cfs @ 12.06 hrs, Volume= 0.470 af, Depth= 0.88"

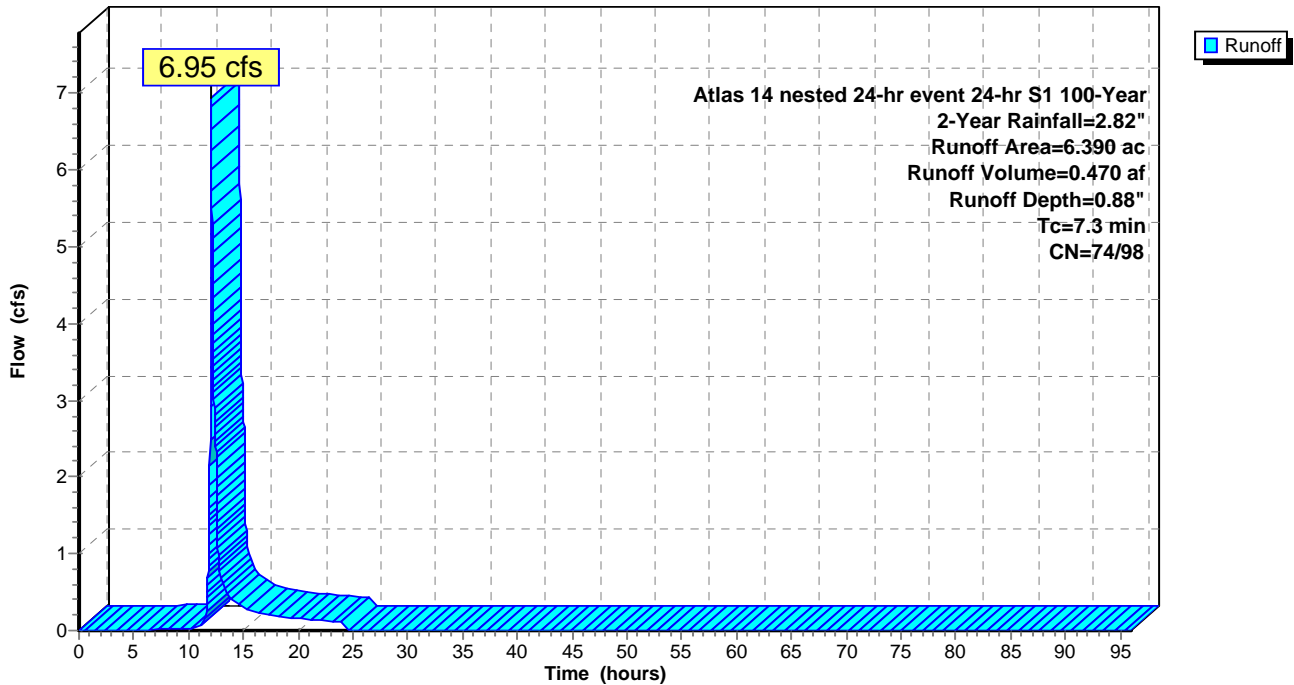
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 6.080	74	pervious
* 0.310	98	impervious
6.390	75	Weighted Average
6.080		95.15% Pervious Area
0.310		4.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.3					Direct Entry,

Subcatchment SB10: SB 10

Hydrograph



Summary for Pond 2 P: P-2

Inflow Area = 68.260 ac, 7.26% Impervious, Inflow Depth = 0.93" for 2-Year event
 Inflow = 25.48 cfs @ 12.57 hrs, Volume= 5.269 af
 Outflow = 25.29 cfs @ 12.69 hrs, Volume= 5.269 af, Atten= 1%, Lag= 6.8 min
 Primary = 25.29 cfs @ 12.69 hrs, Volume= 5.269 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 924.00' Surf.Area= 0.370 ac Storage= 0.730 af
 Peak Elev= 924.73' @ 12.69 hrs Surf.Area= 0.417 ac Storage= 1.016 af (0.286 af above start)

Plug-Flow detention time= 138.8 min calculated for 4.539 af (86% of inflow)
 Center-of-Mass det. time= 37.3 min (912.7 - 875.4)

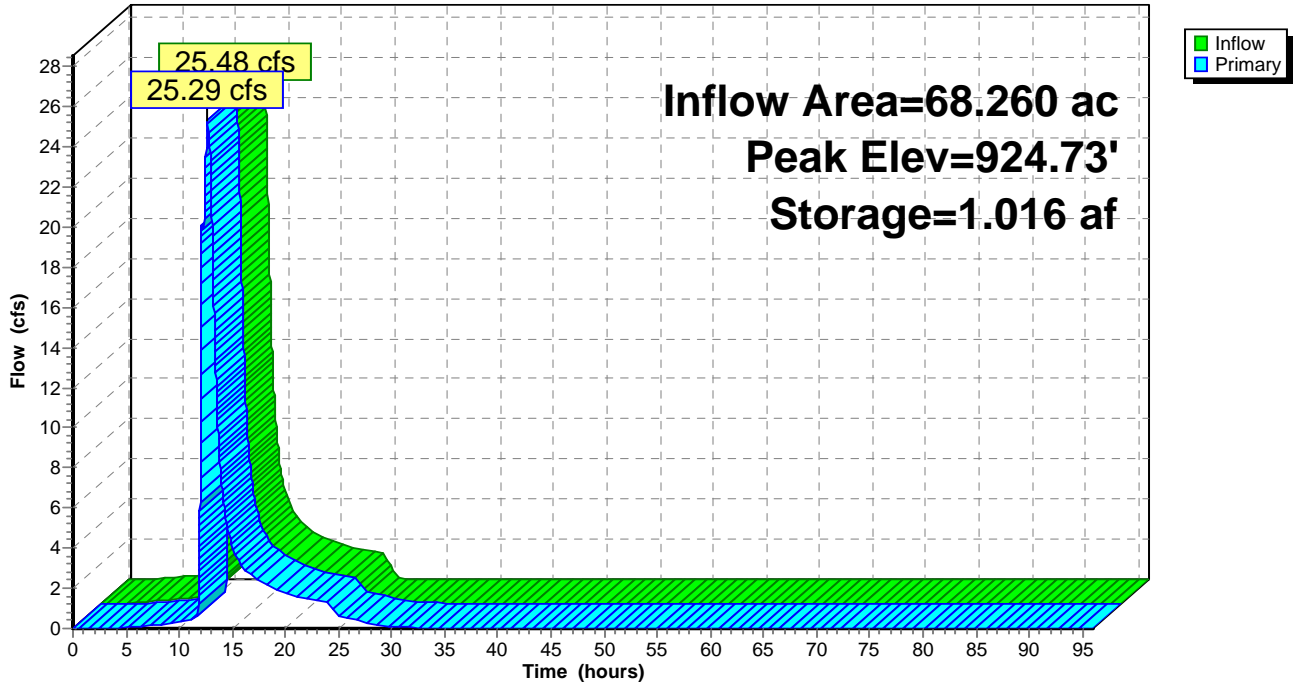
Volume	Invert	Avail.Storage	Storage Description
#1	920.00'	1.600 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
920.00	0.100	0.000	0.000
922.00	0.130	0.230	0.230
924.00	0.370	0.500	0.730
926.00	0.500	0.870	1.600

Device	Routing	Invert	Outlet Devices
#1	Primary	924.40'	40.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Primary	924.00'	6.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=25.29 cfs @ 12.69 hrs HW=924.73' TW=0.00' (Dynamic Tailwater)
 1=Sharp-Crested Rectangular Weir (Weir Controls 24.48 cfs @ 1.87 fps)
 2=Orifice/Grate (Orifice Controls 0.81 cfs @ 4.11 fps)

Pond 2 P: P-2

Hydrograph



Summary for Pond 4P: P-4

Inflow Area = 7.860 ac, 5.98% Impervious, Inflow Depth = 0.90" for 2-Year event
 Inflow = 3.34 cfs @ 12.85 hrs, Volume= 0.592 af
 Outflow = 2.04 cfs @ 13.38 hrs, Volume= 0.592 af, Atten= 39%, Lag= 31.9 min
 Primary = 0.63 cfs @ 13.38 hrs, Volume= 0.211 af
 Secondary = 1.41 cfs @ 13.38 hrs, Volume= 0.381 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 915.00' Surf.Area= 0.275 ac Storage= 0.646 af
 Peak Elev= 915.44' @ 13.38 hrs Surf.Area= 0.299 ac Storage= 0.773 af (0.127 af above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= 50.2 min (948.0 - 897.8)

Volume	Invert	Avail.Storage	Storage Description
#1	910.90'	1.728 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
910.90	0.070	0.000	0.000
912.00	0.090	0.088	0.088
914.00	0.220	0.310	0.398
916.00	0.330	0.550	0.948
918.00	0.450	0.780	1.728

Device	Routing	Invert	Outlet Devices
#1	Primary	915.00'	6.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Secondary	915.00'	9.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	915.95'	24.0" Round RCP_Round 24" L= 50.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 915.80' / 915.95' S= -0.0030 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Primary OutFlow Max=0.63 cfs @ 13.38 hrs HW=915.44' TW=0.00' (Dynamic Tailwater)

↑1=Orifice/Grate (Orifice Controls 0.63 cfs @ 3.20 fps)

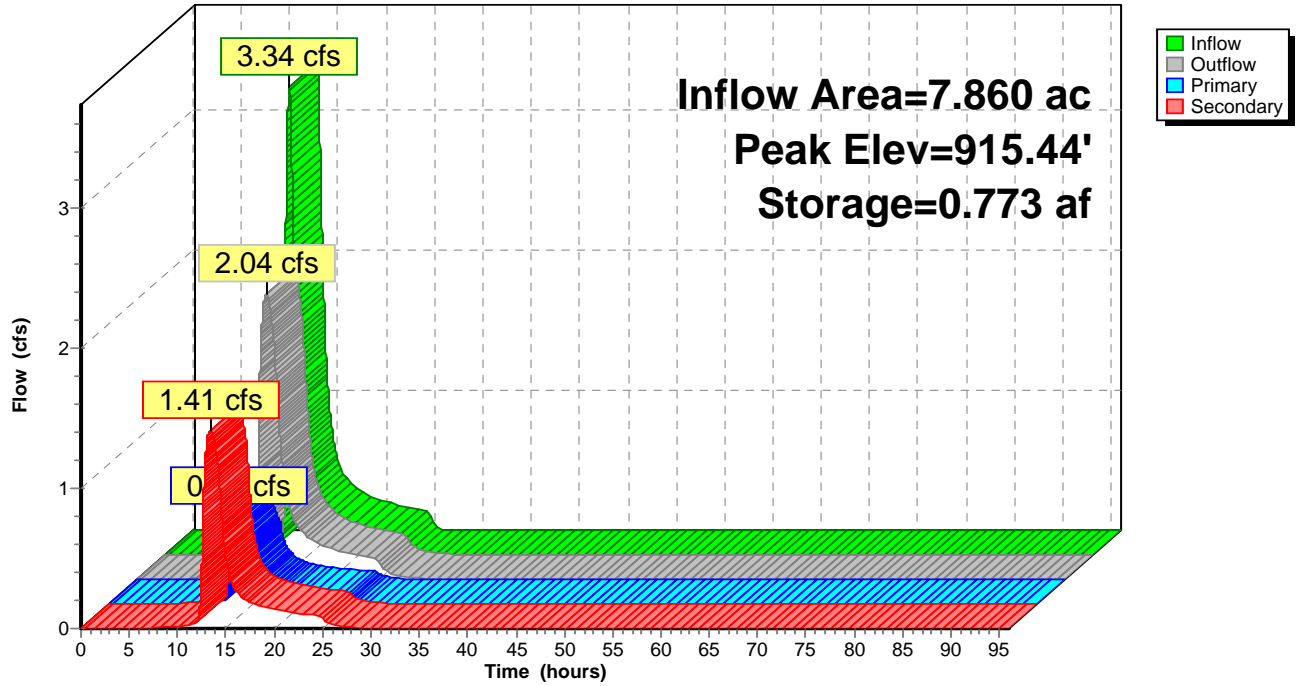
└3=RCP_Round 24" (Controls 0.00 cfs)

Secondary OutFlow Max=1.41 cfs @ 13.38 hrs HW=915.44' TW=914.92' (Dynamic Tailwater)

↑2=Orifice/Grate (Orifice Controls 1.41 cfs @ 3.20 fps)

Pond 4P: P-4

Hydrograph



Summary for Pond 7P: P-7

[90] Warning: Qout>Qin may require smaller dt or Finer Routing

Inflow Area = 29.580 ac, 5.51% Impervious, Inflow Depth = 0.89" for 2-Year event
 Inflow = 14.20 cfs @ 12.67 hrs, Volume= 2.206 af
 Outflow = 14.39 cfs @ 12.74 hrs, Volume= 2.206 af, Atten= 0%, Lag= 4.4 min
 Primary = 14.39 cfs @ 12.74 hrs, Volume= 2.206 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 915.00' Surf.Area= 0.440 ac Storage= 1.062 af
 Peak Elev= 915.29' @ 12.69 hrs Surf.Area= 0.475 ac Storage= 1.196 af (0.134 af above start)

Plug-Flow detention time= 279.4 min calculated for 1.144 af (52% of inflow)
 Center-of-Mass det. time= 12.1 min (900.0 - 887.9)

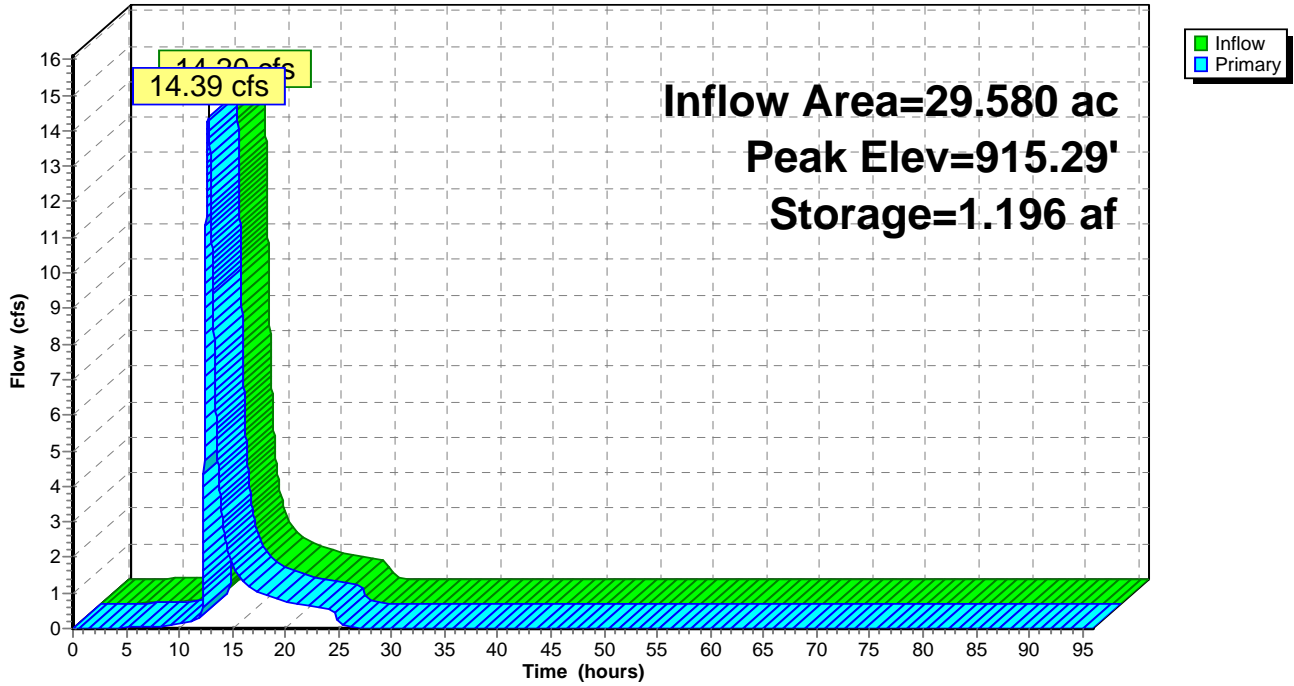
Volume	Invert	Avail.Storage	Storage Description
#1	910.95'	1.562 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
910.95	0.110	0.000	0.000
912.00	0.180	0.152	0.152
914.00	0.340	0.520	0.672
915.00	0.440	0.390	1.062
916.00	0.560	0.500	1.562

Device	Routing	Invert	Outlet Devices
#1	Primary	915.00'	75.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=14.52 cfs @ 12.74 hrs HW=915.29' TW=915.26' (Dynamic Tailwater)
 ↑1=Broad-Crested Rectangular Weir (Weir Controls 14.52 cfs @ 0.67 fps)

Pond 7P: P-7

Hydrograph



Summary for Pond 8P: P-8

Inflow Area = 6.390 ac, 4.85% Impervious, Inflow Depth = 0.88" for 2-Year event
 Inflow = 6.95 cfs @ 12.06 hrs, Volume= 0.470 af
 Outflow = 1.19 cfs @ 12.61 hrs, Volume= 0.465 af, Atten= 83%, Lag= 32.9 min
 Primary = 1.19 cfs @ 12.61 hrs, Volume= 0.465 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 897.00' Surf.Area= 0.300 ac Storage= 0.495 af
 Peak Elev= 897.58' @ 12.65 hrs Surf.Area= 0.387 ac Storage= 0.694 af (0.199 af above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= 260.3 min (1,113.1 - 852.8)

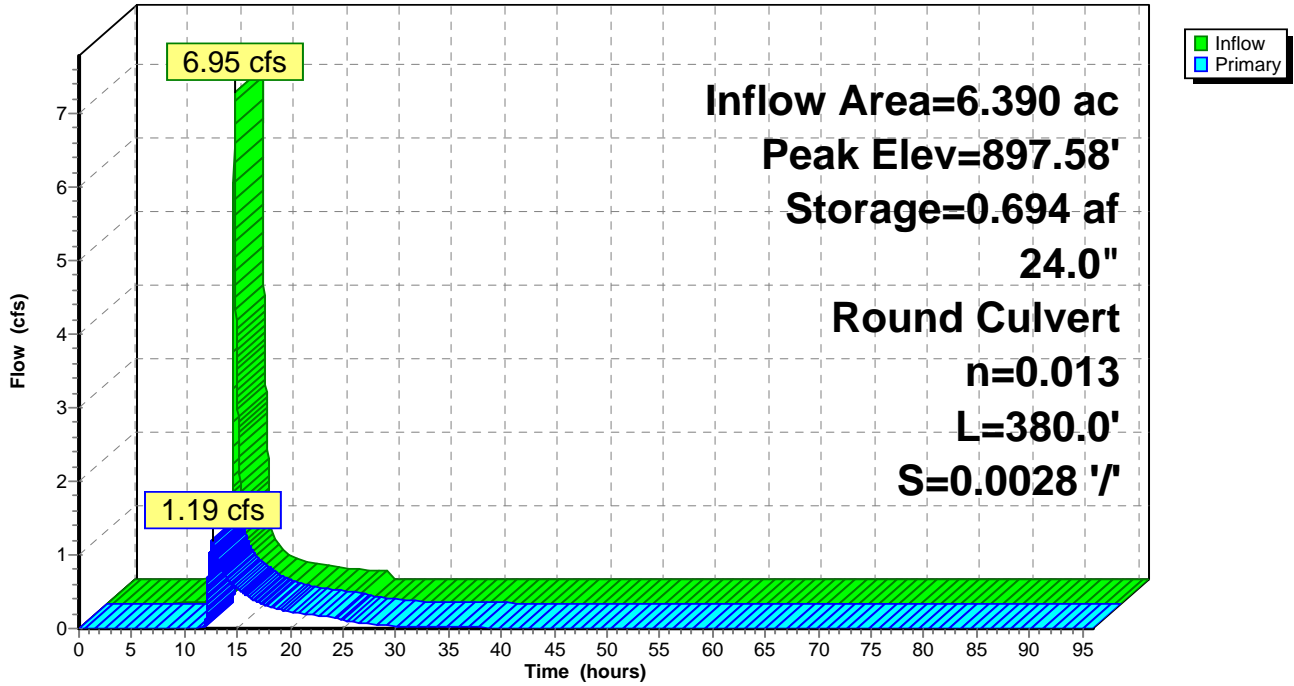
Volume	Invert	Avail.Storage	Storage Description
#1	893.00'	1.850 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
893.00	0.030	0.000	0.000
894.00	0.070	0.050	0.050
896.00	0.150	0.220	0.270
897.00	0.300	0.225	0.495
898.00	0.450	0.375	0.870
900.00	0.530	0.980	1.850

Device	Routing	Invert	Outlet Devices
#1	Primary	897.00'	24.0" Round RCP_Round 24" L= 380.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 897.00' / 895.94' S= 0.0028 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Primary OutFlow Max=1.19 cfs @ 12.61 hrs HW=897.58' TW=896.49' (Dynamic Tailwater)
 ↑1=RCP_Round 24" (Outlet Controls 1.19 cfs @ 2.36 fps)

Pond 8P: P-8

Hydrograph



Summary for Pond 9P: P-9

[80] Warning: Exceeded Pond 7P by 0.01' @ 12.19 hrs (2.42 cfs 0.050 af)

Inflow Area = 55.360 ac, 3.00% Impervious, Inflow Depth = 0.85" for 2-Year event
 Inflow = 25.40 cfs @ 12.62 hrs, Volume= 3.921 af
 Outflow = 25.36 cfs @ 12.64 hrs, Volume= 3.921 af, Atten= 0%, Lag= 1.0 min
 Primary = 25.36 cfs @ 12.64 hrs, Volume= 3.921 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 915.00' Surf.Area= 0.210 ac Storage= 0.353 af
 Peak Elev= 915.26' @ 12.64 hrs Surf.Area= 0.262 ac Storage= 0.414 af (0.061 af above start)

Plug-Flow detention time= 68.6 min calculated for 3.568 af (91% of inflow)
 Center-of-Mass det. time= 2.9 min (898.1 - 895.1)

Volume	Invert	Avail.Storage	Storage Description
#1	910.50'	1.673 af	Custom Stage Data (Prismatic) Listed below (Recalc)

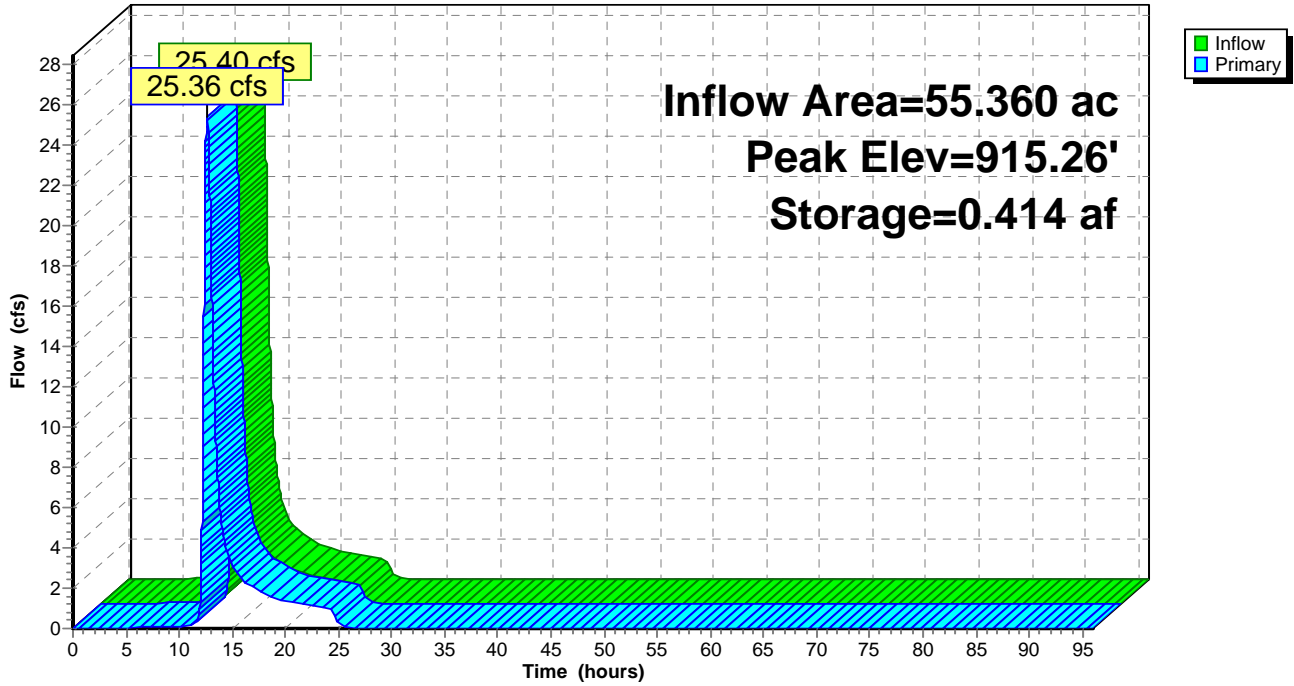
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
910.50	0.020	0.000	0.000
912.00	0.050	0.052	0.052
913.00	0.070	0.060	0.112
914.00	0.100	0.085	0.198
915.00	0.210	0.155	0.353
916.00	0.410	0.310	0.662
918.00	0.600	1.010	1.673

Device	Routing	Invert	Outlet Devices
#1	Primary	915.00'	80.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=25.36 cfs @ 12.64 hrs HW=915.26' TW=909.74' (Dynamic Tailwater)
 ↑1=**Broad-Crested Rectangular Weir** (Weir Controls 25.36 cfs @ 1.22 fps)

Pond 9P: P-9

Hydrograph



Summary for Pond 10P: P-10 Lowered 1 ft

Inflow Area = 66.430 ac, 5.22% Impervious, Inflow Depth > 0.67" for 2-Year event
 Inflow = 6.10 cfs @ 13.48 hrs, Volume= 3.683 af
 Outflow = 5.76 cfs @ 13.93 hrs, Volume= 3.682 af, Atten= 6%, Lag= 26.8 min
 Primary = 5.76 cfs @ 13.93 hrs, Volume= 3.682 af
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 896.00' Surf.Area= 0.290 ac Storage= 0.700 af
 Peak Elev= 896.83' @ 13.93 hrs Surf.Area= 0.323 ac Storage= 0.954 af (0.254 af above start)

Plug-Flow detention time= 239.7 min calculated for 2.982 af (81% of inflow)
 Center-of-Mass det. time= 42.5 min (1,113.2 - 1,070.7)

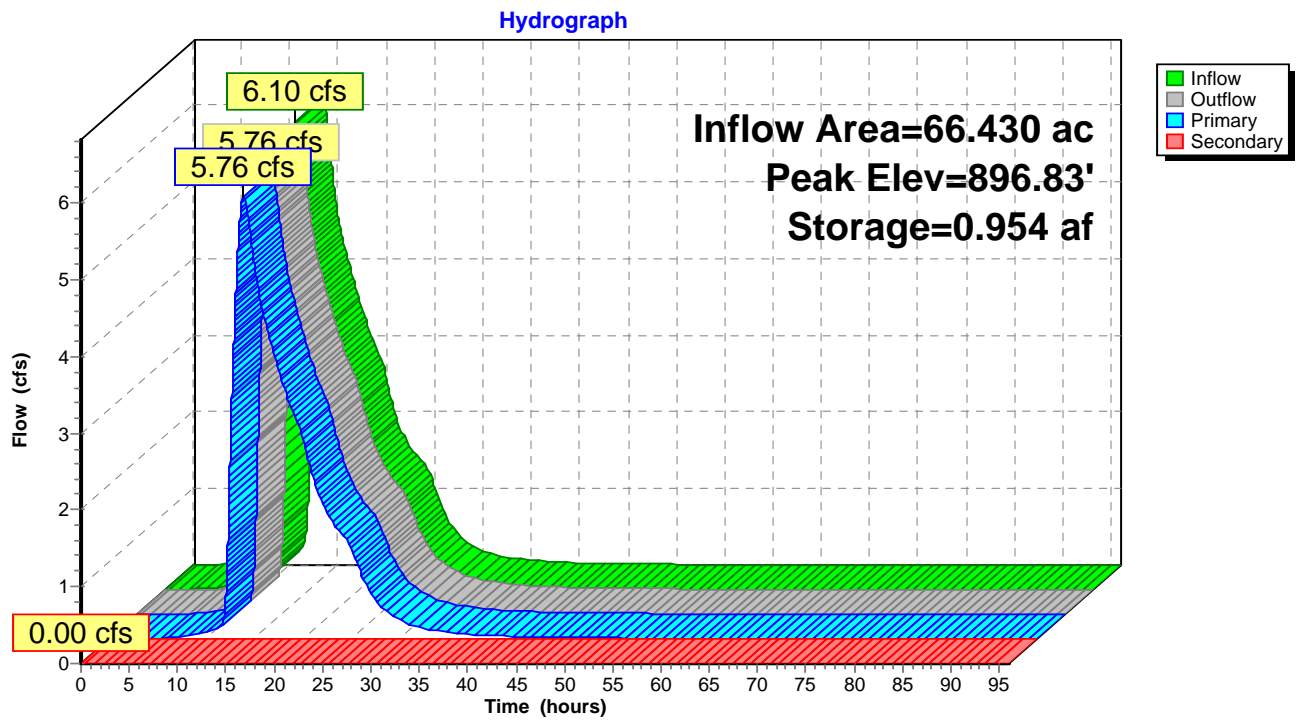
Volume	Invert	Avail.Storage	Storage Description
#1	892.00'	1.760 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
892.00	0.120	0.000	0.000
893.00	0.140	0.130	0.130
895.00	0.190	0.330	0.460
896.00	0.290	0.240	0.700
897.00	0.330	0.310	1.010
899.00	0.420	0.750	1.760

Device	Routing	Invert	Outlet Devices
#1	Primary	896.00'	2.5' long x 1.00' rise Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Secondary	897.40'	50.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=5.76 cfs @ 13.93 hrs HW=896.83' TW=893.44' (Dynamic Tailwater)
 ↑1=Sharp-Crested Rectangular Weir (Weir Controls 5.76 cfs @ 2.98 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=896.00' TW=893.00' (Dynamic Tailwater)
 ↑2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 10P: P-10 Lowered 1 ft



Summary for Pond 11P: P-11

Inflow Area = 58.650 ac, 4.89% Impervious, Inflow Depth = 0.89" for 2-Year event
 Inflow = 26.75 cfs @ 12.60 hrs, Volume= 4.343 af
 Outflow = 8.30 cfs @ 13.57 hrs, Volume= 4.342 af, Atten= 69%, Lag= 58.0 min
 Primary = 5.18 cfs @ 13.57 hrs, Volume= 3.083 af
 Secondary = 3.14 cfs @ 13.91 hrs, Volume= 1.259 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 909.00' Surf.Area= 1.210 ac Storage= 3.640 af
 Peak Elev= 910.30' @ 13.57 hrs Surf.Area= 1.356 ac Storage= 5.310 af (1.670 af above start)

Plug-Flow detention time= 838.1 min calculated for 0.702 af (16% of inflow)
 Center-of-Mass det. time= 153.5 min (1,039.7 - 886.2)

Volume	Invert	Avail.Storage	Storage Description
#1	905.00'	9.405 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
905.00	0.760	0.000	0.000
906.00	0.820	0.790	0.790
908.00	0.950	1.770	2.560
909.00	1.210	1.080	3.640
910.00	1.320	1.265	4.905
912.00	1.560	2.880	7.785
913.00	1.680	1.620	9.405

Device	Routing	Invert	Outlet Devices
#1	Primary	909.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	910.00'	24.0" Round RCP_Round 24" L= 200.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 910.00' / 909.00' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Primary	910.00'	24.0" Round RCP_Round 24" L= 200.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 910.00' / 909.00' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#4	Primary	912.00'	60.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88
#5	Secondary	909.00'	12.0" Round RCP_Round 12" L= 150.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 909.00' / 908.00' S= 0.0067 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=5.18 cfs @ 13.57 hrs HW=910.30' TW=896.80' (Dynamic Tailwater)

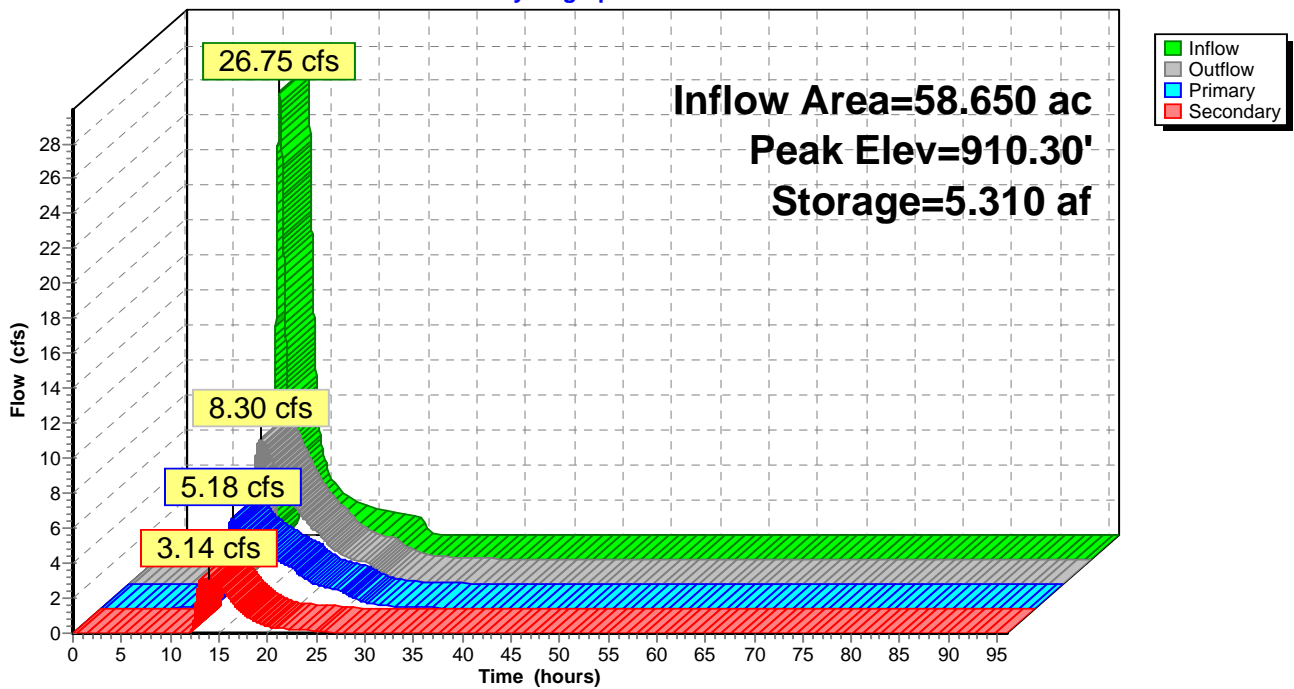
- 1=Orifice/Grate (Orifice Controls 4.32 cfs @ 5.50 fps)
- 2=RCP_Round 24" (Barrel Controls 0.43 cfs @ 2.19 fps)
- 3=RCP_Round 24" (Barrel Controls 0.43 cfs @ 2.19 fps)
- 4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Secondary OutFlow Max=3.14 cfs @ 13.91 hrs HW=910.28' TW=908.54' (Dynamic Tailwater)

- 5=RCP_Round 12" (Barrel Controls 3.14 cfs @ 4.05 fps)

Pond 11P: P-11

Hydrograph



Summary for Pond 12P: P-12

Inflow Area = 79.640 ac, 7.40% Impervious, Inflow Depth > 0.93" for 2-Year event
 Inflow = 16.56 cfs @ 12.03 hrs, Volume= 6.175 af
 Outflow = 6.46 cfs @ 16.56 hrs, Volume= 6.168 af, Atten= 61%, Lag= 271.9 min
 Primary = 6.46 cfs @ 16.56 hrs, Volume= 6.168 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 893.00' Surf.Area= 1.640 ac Storage= 5.075 af
 Peak Elev= 893.62' @ 16.56 hrs Surf.Area= 1.720 ac Storage= 6.109 af (1.034 af above start)

Plug-Flow detention time= 1,198.1 min calculated for 1.093 af (18% of inflow)
 Center-of-Mass det. time= 124.6 min (1,225.3 - 1,100.7)

Volume	Invert	Avail.Storage	Storage Description
#1	889.00'	10.590 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
889.00	1.070	0.000	0.000
890.00	1.150	1.110	1.110
892.00	1.330	2.480	3.590
893.00	1.640	1.485	5.075
894.00	1.770	1.705	6.780
896.00	2.040	3.810	10.590

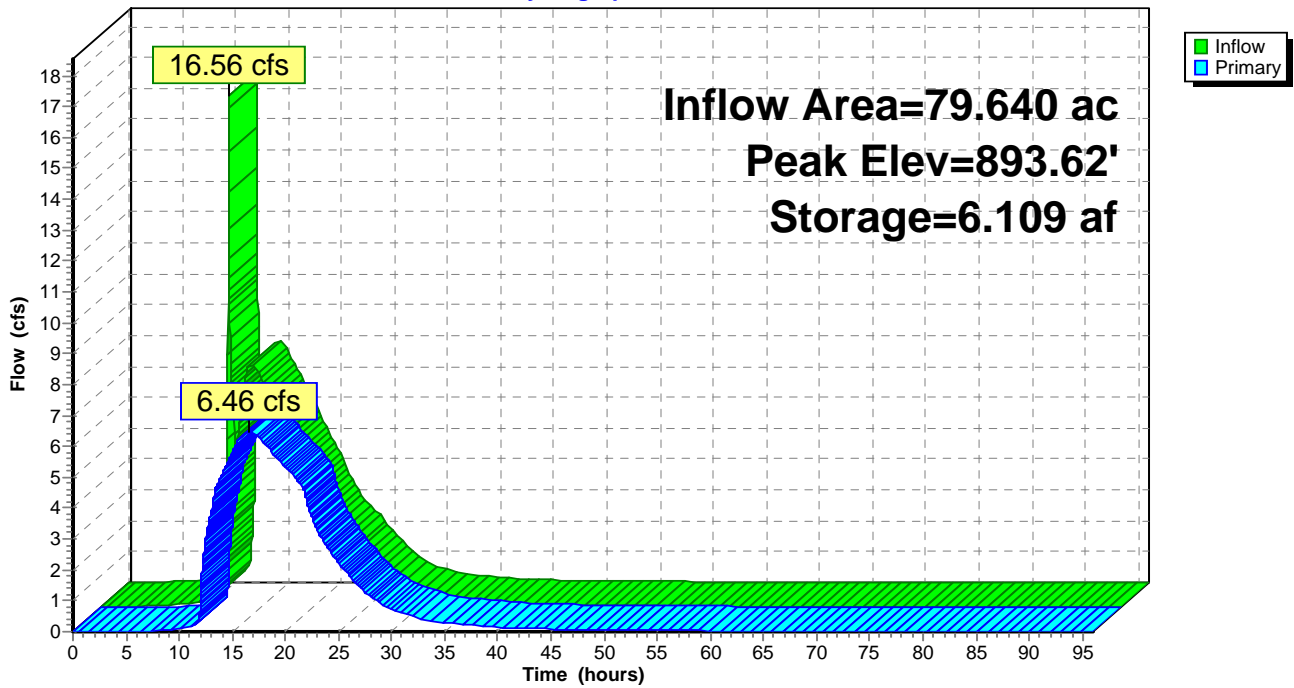
Device	Routing	Invert	Outlet Devices
#1	Primary	893.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	893.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf
#4	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf
#5	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf
#6	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf

Primary OutFlow Max=6.46 cfs @ 16.56 hrs HW=893.62' TW=883.21' (Dynamic Tailwater)

- 1=Orifice/Grate (Orifice Controls 2.97 cfs @ 3.78 fps)
- 2=Orifice/Grate (Orifice Controls 2.97 cfs @ 3.78 fps)
- 3=RCP_Arch 44x27 (Barrel Controls 0.13 cfs @ 1.21 fps)
- 4=RCP_Arch 44x27 (Barrel Controls 0.13 cfs @ 1.21 fps)
- 5=RCP_Arch 44x27 (Barrel Controls 0.13 cfs @ 1.21 fps)
- 6=RCP_Arch 44x27 (Barrel Controls 0.13 cfs @ 1.21 fps)

Pond 12P: P-12

Hydrograph



Summary for Pond 13P: P-13

Inflow Area = 237.775 ac, 9.20% Impervious, Inflow Depth = 0.96" for 2-Year event
 Inflow = 99.54 cfs @ 12.38 hrs, Volume= 19.048 af
 Outflow = 91.90 cfs @ 12.52 hrs, Volume= 19.046 af, Atten= 8%, Lag= 8.4 min
 Primary = 86.58 cfs @ 12.52 hrs, Volume= 18.259 af
 Secondary = 5.32 cfs @ 12.52 hrs, Volume= 0.787 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 883.00' Surf.Area= 1.870 ac Storage= 4.265 af
 Peak Elev= 883.70' @ 12.52 hrs Surf.Area= 2.115 ac Storage= 5.659 af (1.394 af above start)

Plug-Flow detention time= 243.6 min calculated for 14.781 af (78% of inflow)
 Center-of-Mass det. time= 22.5 min (999.0 - 976.5)

Volume	Invert	Avail.Storage	Storage Description
#1	878.00'	11.490 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
878.00	0.000	0.000	0.000
879.00	0.630	0.315	0.315
880.00	0.730	0.680	0.995
882.00	1.070	1.800	2.795
883.00	1.870	1.470	4.265
884.00	2.220	2.045	6.310
886.00	2.960	5.180	11.490

Device	Routing	Invert	Outlet Devices
#1	Primary	883.00'	55.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88
#2	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#3	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#4	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#5	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#6	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200

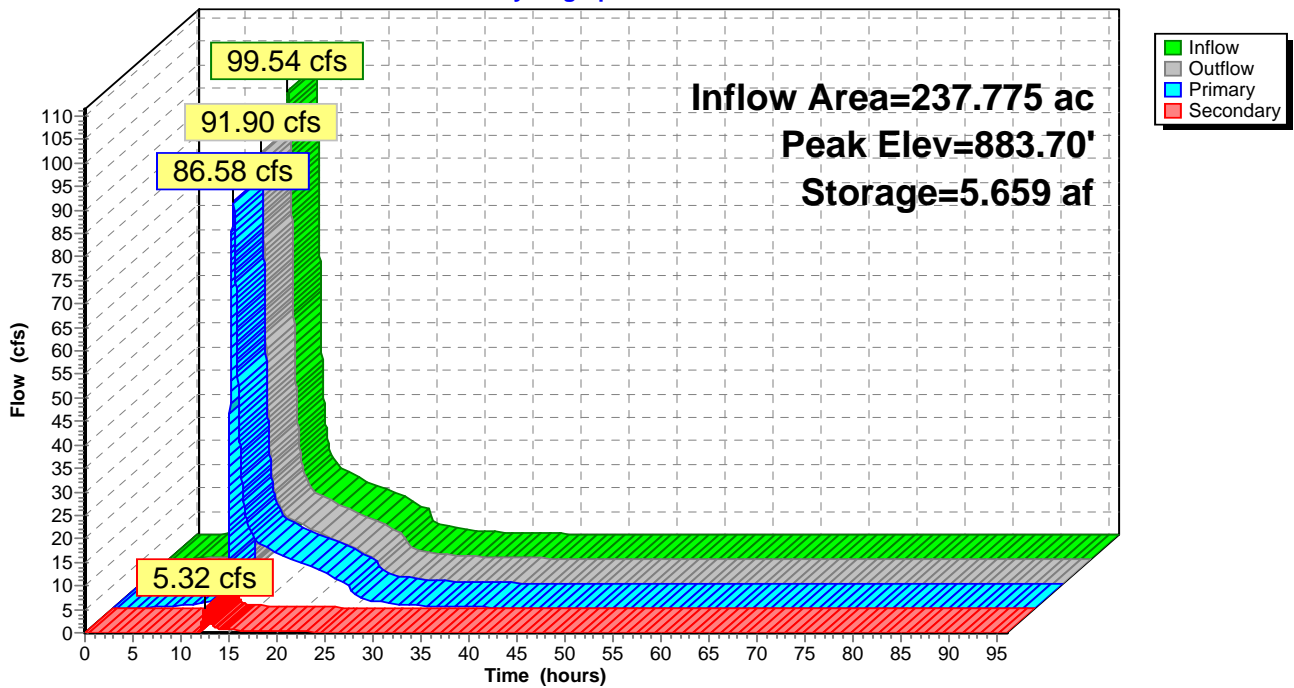
Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 1' Cc= 0.900
 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=86.58 cfs @ 12.52 hrs HW=883.70' TW=0.00' (Dynamic Tailwater)
 ↳1=**Broad-Crested Rectangular Weir** (Weir Controls 86.58 cfs @ 2.25 fps)

Secondary OutFlow Max=5.32 cfs @ 12.52 hrs HW=883.70' TW=882.90' (Dynamic Tailwater)
 ↳2=**RCP_Round 12"** (Barrel Controls 1.06 cfs @ 2.55 fps)
 ↳3=**RCP_Round 12"** (Barrel Controls 1.06 cfs @ 2.55 fps)
 ↳4=**RCP_Round 12"** (Barrel Controls 1.06 cfs @ 2.55 fps)
 ↳5=**RCP_Round 12"** (Barrel Controls 1.06 cfs @ 2.55 fps)
 ↳6=**RCP_Round 12"** (Barrel Controls 1.06 cfs @ 2.55 fps)

Pond 13P: P-13

Hydrograph



Summary for Pond 17P: W-2

[80] Warning: Exceeded Pond P-5/P-6 by 0.15' @ 40.43 hrs (0.12 cfs 0.597 af)

Inflow = 1.51 cfs @ 12.76 hrs, Volume= 0.492 af
 Outflow = 0.32 cfs @ 17.83 hrs, Volume= 0.351 af, Atten= 79%, Lag= 304.6 min
 Primary = 0.32 cfs @ 17.83 hrs, Volume= 0.351 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 929.35' @ 17.83 hrs Surf.Area= 1.149 ac Storage= 0.390 af

Plug-Flow detention time= 904.5 min calculated for 0.351 af (71% of inflow)
 Center-of-Mass det. time= 847.7 min (1,717.9 - 870.2)

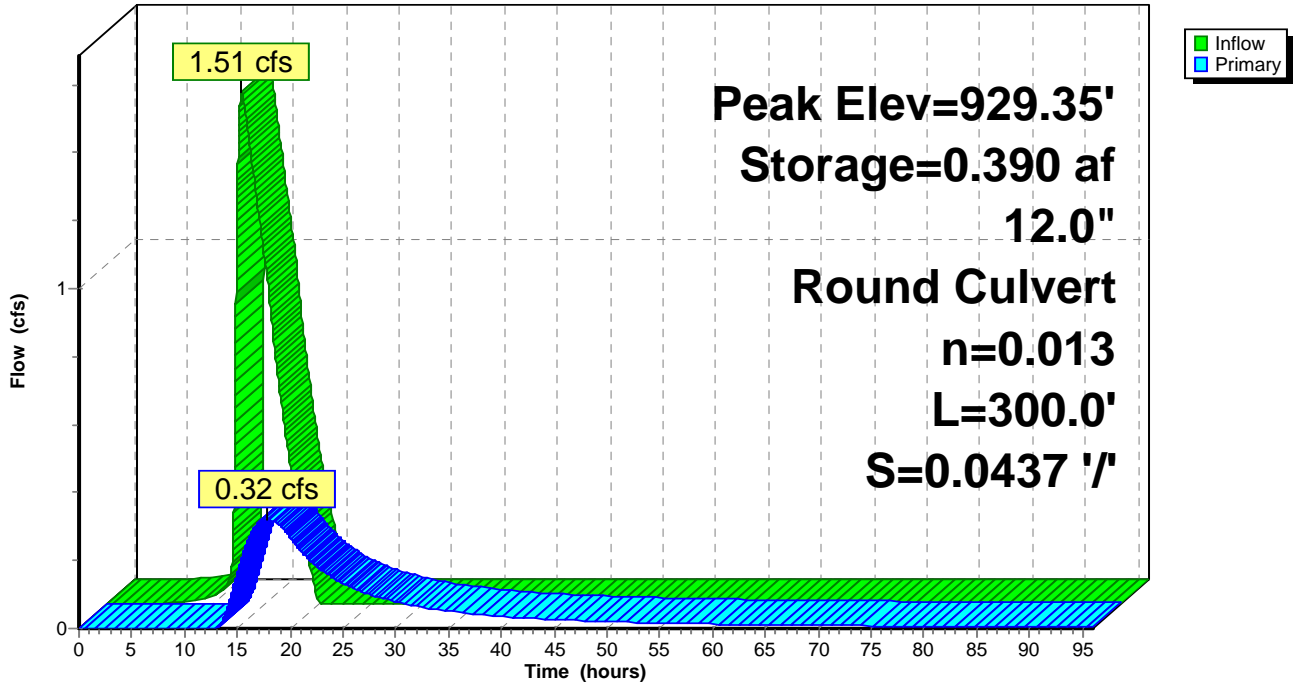
Volume	Invert	Avail.Storage	Storage Description
#1	929.00'	1.175 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
929.00	1.090	0.000	0.000
930.00	1.260	1.175	1.175

Device	Routing	Invert	Outlet Devices
#1	Primary	929.10'	12.0" Round RCP_Round 12" L= 300.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 929.10' / 916.00' S= 0.0437 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.32 cfs @ 17.83 hrs HW=929.35' TW=914.79' (Dynamic Tailwater)
 ↑1=RCP_Round 12" (Inlet Controls 0.32 cfs @ 2.12 fps)

Pond 17P: W-2

Hydrograph



Summary for Pond 36P: Culverts passing flow beneath Spine Road

Inflow Area = 52.790 ac, 0.00% Impervious, Inflow Depth = 0.80" for 2-Year event
 Inflow = 26.87 cfs @ 12.48 hrs, Volume= 3.502 af
 Outflow = 26.87 cfs @ 12.48 hrs, Volume= 3.502 af, Atten= 0%, Lag= 0.0 min
 Primary = 26.87 cfs @ 12.48 hrs, Volume= 3.502 af
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 887.11' @ 12.48 hrs Surf.Area= 0.000 ac Storage= 0.000 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 0.0 min (892.6 - 892.6)

Volume	Invert	Avail.Storage	Storage Description
#1	887.00'	0.026 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
887.00	0.000	0.000	0.000
887.50	0.002	0.001	0.001
890.50	0.007	0.014	0.014
892.00	0.009	0.012	0.026

Device	Routing	Invert	Outlet Devices
#1	Primary	887.00'	Special & User-Defined Head (feet) 0.00 0.10 0.20 0.30 0.40 0.50 5.00 Disch. (cfs) 0.000 25.000 50.000 75.000 100.000 127.000 127.000
#2	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#3	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#4	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#5	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#6	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#7	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900

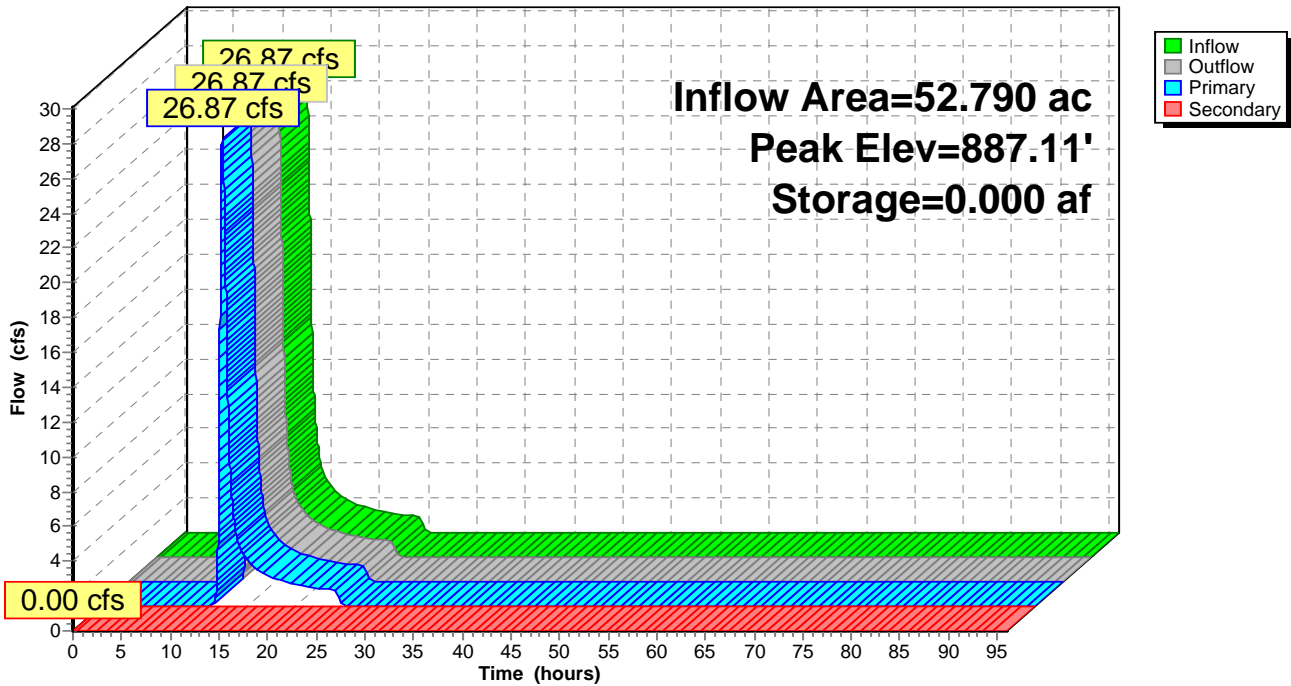
- #8 Secondary 887.50' n= 0.013, Flow Area= 1.77 sf **18.0" Round RCP_Round 18"**
 L= 100.0' RCP, groove end w/headwall, Ke= 0.200
 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/' Cc= 0.900
- #9 Secondary 887.50' n= 0.013, Flow Area= 1.77 sf **18.0" Round RCP_Round 18"**
 L= 100.0' RCP, groove end w/headwall, Ke= 0.200
 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/' Cc= 0.900
 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=26.87 cfs @ 12.48 hrs HW=887.11' TW=883.70' (Dynamic Tailwater)
 ↳1=Special & User-Defined (Custom Controls 26.87 cfs)

- Secondary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=887.00' TW=883.00' (Dynamic Tailwater)
- ↳2=RCP_Round 18" (Controls 0.00 cfs)
 - ↳3=RCP_Round 18" (Controls 0.00 cfs)
 - ↳4=RCP_Round 18" (Controls 0.00 cfs)
 - ↳5=RCP_Round 18" (Controls 0.00 cfs)
 - ↳6=RCP_Round 18" (Controls 0.00 cfs)
 - ↳7=RCP_Round 18" (Controls 0.00 cfs)
 - ↳8=RCP_Round 18" (Controls 0.00 cfs)
 - ↳9=RCP_Round 18" (Controls 0.00 cfs)

Pond 36P: Culverts passing flow beneath Spine Road

Hydrograph



Summary for Pond CRH-1: CRH-1

Inflow Area = 6.955 ac, 46.76% Impervious, Inflow Depth = 1.63" for 2-Year event
 Inflow = 10.87 cfs @ 12.15 hrs, Volume= 0.947 af
 Outflow = 4.80 cfs @ 12.48 hrs, Volume= 0.947 af, Atten= 56%, Lag= 19.6 min
 Discarded = 0.22 cfs @ 12.48 hrs, Volume= 0.471 af
 Primary = 4.58 cfs @ 12.48 hrs, Volume= 0.476 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 877.67' @ 12.48 hrs Surf.Area= 0.275 ac Storage= 0.354 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 270.2 min (1,064.9 - 794.7)

Volume	Invert	Avail.Storage	Storage Description
#1	876.00'	0.850 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
876.00	0.150	0.000	0.000
878.00	0.300	0.450	0.450
879.00	0.500	0.400	0.850

Device	Routing	Invert	Outlet Devices
#1	Discarded	876.00'	0.800 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 0.00'
#2	Primary	877.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 877.00' / 876.00' S= 0.0065 1/1' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Primary	877.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 877.00' / 876.00' S= 0.0065 1/1' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Discarded OutFlow Max=0.22 cfs @ 12.48 hrs HW=877.67' (Free Discharge)

└─1=Exfiltration (Controls 0.22 cfs)

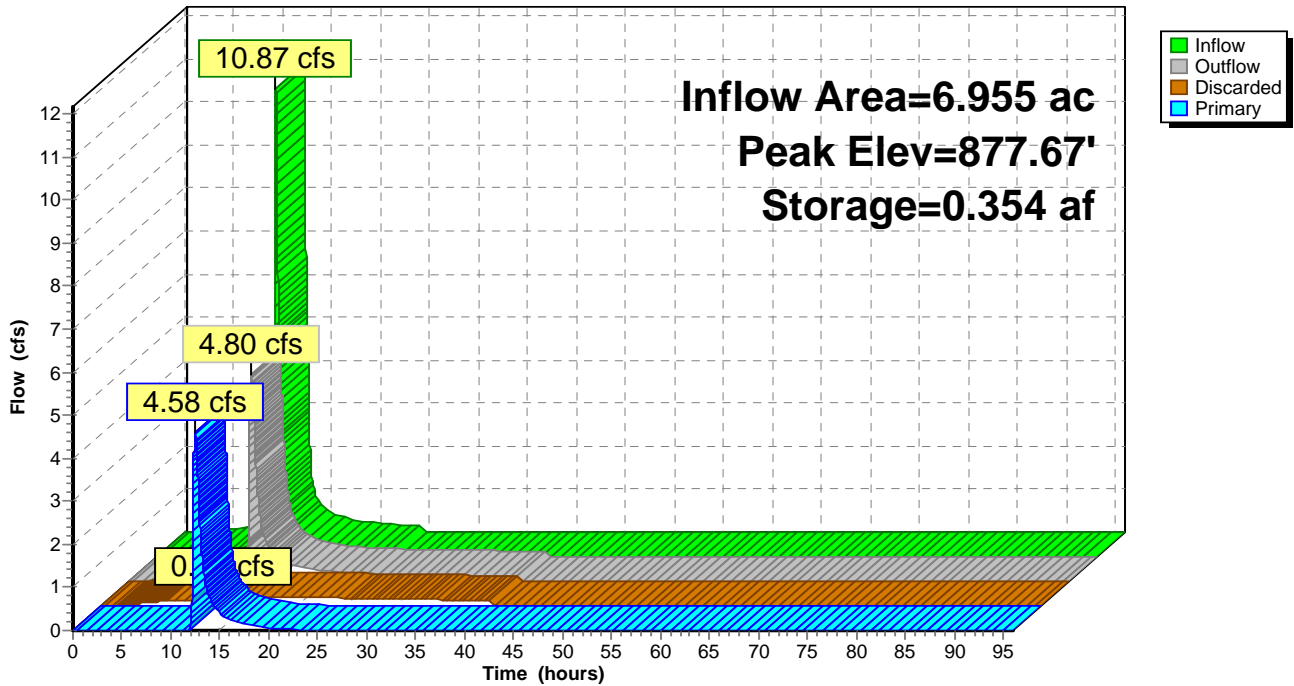
Primary OutFlow Max=4.58 cfs @ 12.48 hrs HW=877.67' (Free Discharge)

└─2=Culvert (Barrel Controls 2.29 cfs @ 3.72 fps)

└─3=Culvert (Barrel Controls 2.29 cfs @ 3.72 fps)

Pond CRH-1: CRH-1

Hydrograph



Summary for Pond CRH-2: CRH-2

Inflow Area = 10.214 ac, 37.73% Impervious, Inflow Depth = 1.47" for 2-Year event
 Inflow = 12.67 cfs @ 12.22 hrs, Volume= 1.253 af
 Outflow = 2.62 cfs @ 12.88 hrs, Volume= 1.254 af, Atten= 79%, Lag= 39.4 min
 Discarded = 0.33 cfs @ 12.88 hrs, Volume= 0.833 af
 Primary = 2.29 cfs @ 12.88 hrs, Volume= 0.420 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 882.04' @ 12.88 hrs Surf.Area= 0.404 ac Storage= 0.617 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 500.6 min (1,307.9 - 807.2)

Volume	Invert	Avail.Storage	Storage Description
#1	880.00'	1.600 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
880.00	0.200	0.000	0.000
882.00	0.400	0.600	0.600
884.00	0.600	1.000	1.600

Device	Routing	Invert	Outlet Devices
#1	Primary	881.50'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 881.50' / 881.00' S= 0.0032 ' /' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Primary	881.50'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 881.50' / 881.00' S= 0.0032 ' /' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Discarded	880.00'	0.800 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 0.00'

Discarded OutFlow Max=0.33 cfs @ 12.88 hrs HW=882.04' (Free Discharge)

↑ **3=Exfiltration** (Controls 0.33 cfs)

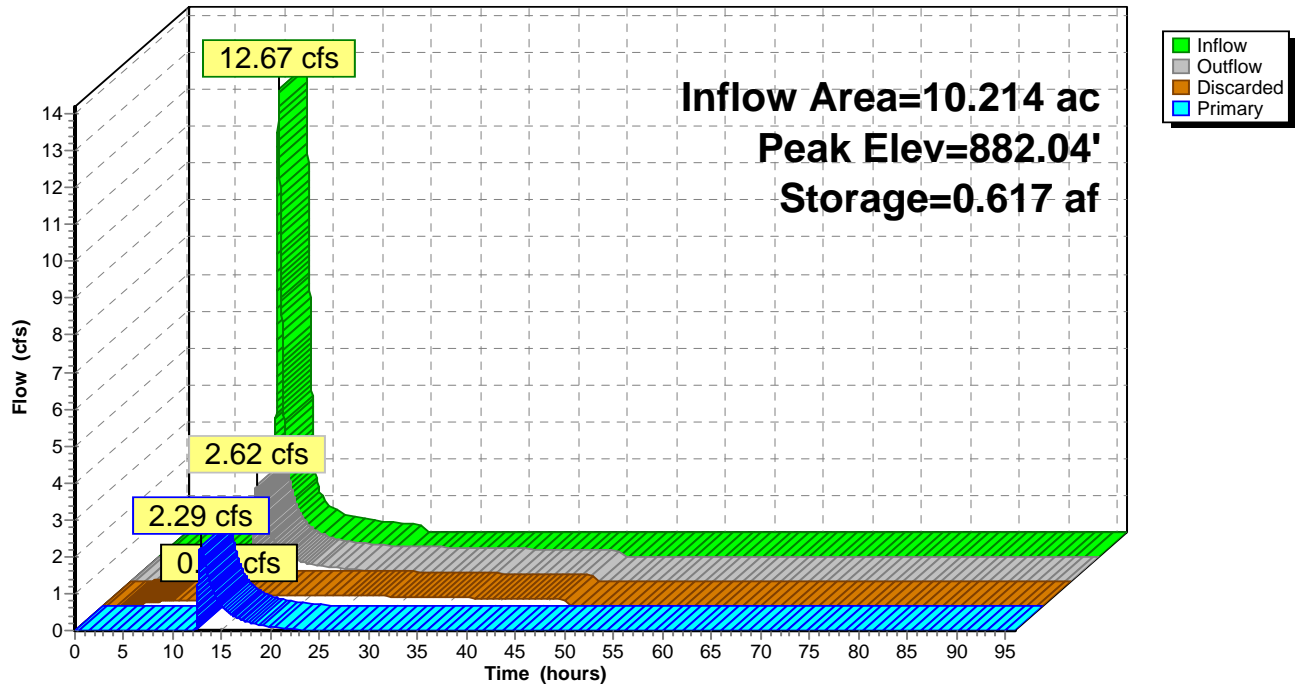
Primary OutFlow Max=2.29 cfs @ 12.88 hrs HW=882.04' TW=877.79' (Dynamic Tailwater)

↑ **1=Culvert** (Barrel Controls 1.15 cfs @ 2.51 fps)

↑ **2=Culvert** (Barrel Controls 1.15 cfs @ 2.51 fps)

Pond CRH-2: CRH-2

Hydrograph



Summary for Pond CRH-3: CRH-3

Inflow Area = 11.815 ac, 36.95% Impervious, Inflow Depth = 0.61" for 2-Year event
 Inflow = 2.97 cfs @ 12.04 hrs, Volume= 0.603 af
 Outflow = 1.17 cfs @ 14.13 hrs, Volume= 0.603 af, Atten= 61%, Lag= 125.4 min
 Discarded = 0.20 cfs @ 14.13 hrs, Volume= 0.381 af
 Primary = 0.97 cfs @ 14.13 hrs, Volume= 0.222 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 878.30' @ 14.13 hrs Surf.Area= 0.248 ac Storage= 0.259 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 351.8 min (1,209.1 - 857.3)

Volume	Invert	Avail.Storage	Storage Description
#1	877.00'	0.850 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
877.00	0.150	0.000	0.000
879.00	0.300	0.450	0.450
880.00	0.500	0.400	0.850

Device	Routing	Invert	Outlet Devices
#1	Discarded	877.00'	0.800 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 0.00'
#2	Primary	878.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 878.00' / 877.00' S= 0.0065 1/1' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Primary	878.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 878.00' / 877.00' S= 0.0065 1/1' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Discarded OutFlow Max=0.20 cfs @ 14.13 hrs HW=878.30' (Free Discharge)

└─1=Exfiltration (Controls 0.20 cfs)

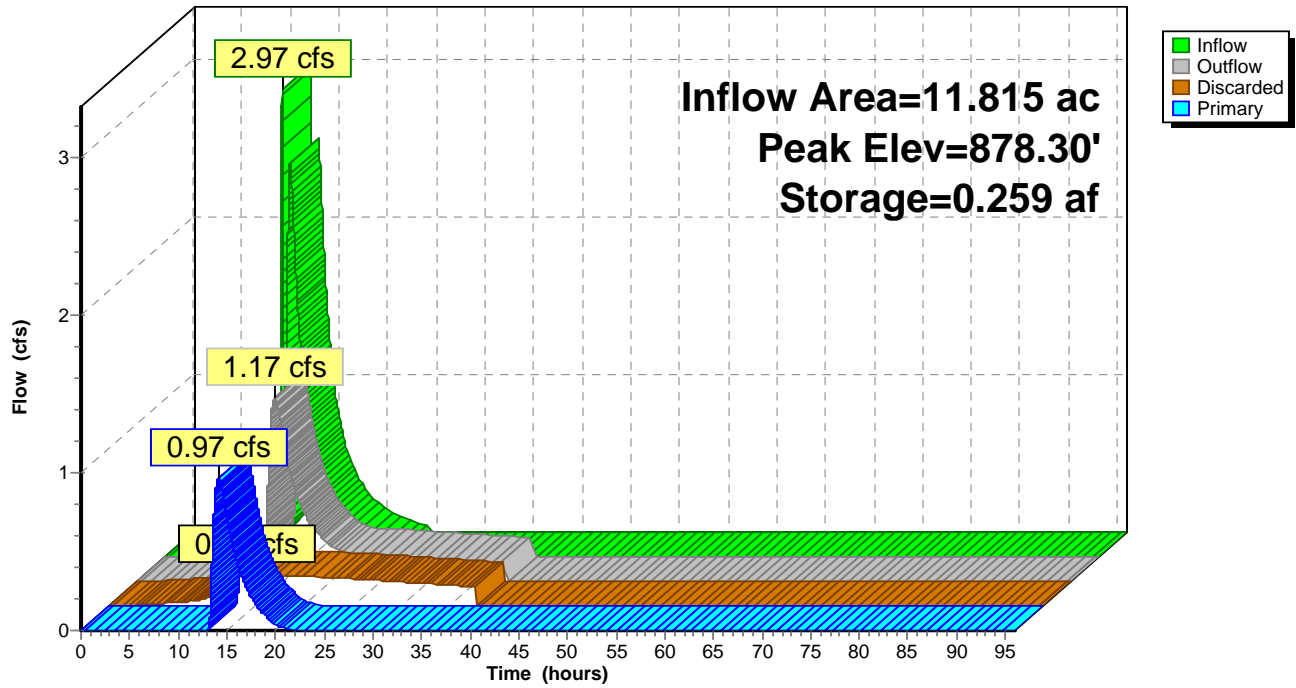
Primary OutFlow Max=0.97 cfs @ 14.13 hrs HW=878.30' (Free Discharge)

└─2=Culvert (Barrel Controls 0.48 cfs @ 2.44 fps)

└─3=Culvert (Barrel Controls 0.48 cfs @ 2.44 fps)

Pond CRH-3: CRH-3

Hydrograph



Summary for Pond P-5/P-6: P-5/P-6

Inflow Area = 43.346 ac, 18.61% Impervious, Inflow Depth = 1.13" for 2-Year event
 Inflow = 44.72 cfs @ 12.15 hrs, Volume= 4.086 af
 Outflow = 10.29 cfs @ 12.76 hrs, Volume= 4.079 af, Atten= 77%, Lag= 36.3 min
 Primary = 8.78 cfs @ 12.76 hrs, Volume= 3.587 af
 Secondary = 1.51 cfs @ 12.76 hrs, Volume= 0.492 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 929.00' Surf.Area= 1.975 ac Storage= 5.062 af
 Peak Elev= 929.88' @ 12.76 hrs Surf.Area= 2.207 ac Storage= 6.897 af (1.834 af above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= 231.0 min (1,058.6 - 827.5)

Volume	Invert	Avail.Storage	Storage Description
#1	926.00'	14.650 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
926.00	1.510	0.000	0.000
928.00	1.710	3.220	3.220
930.00	2.240	3.950	7.170
931.00	2.400	2.320	9.490
933.00	2.760	5.160	14.650

Device	Routing	Invert	Outlet Devices
#1	Primary	929.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	929.50'	7.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#3	Primary	930.50'	14.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#4	Secondary	929.00'	9.0" Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=8.78 cfs @ 12.76 hrs HW=929.88' TW=0.00' (Dynamic Tailwater)

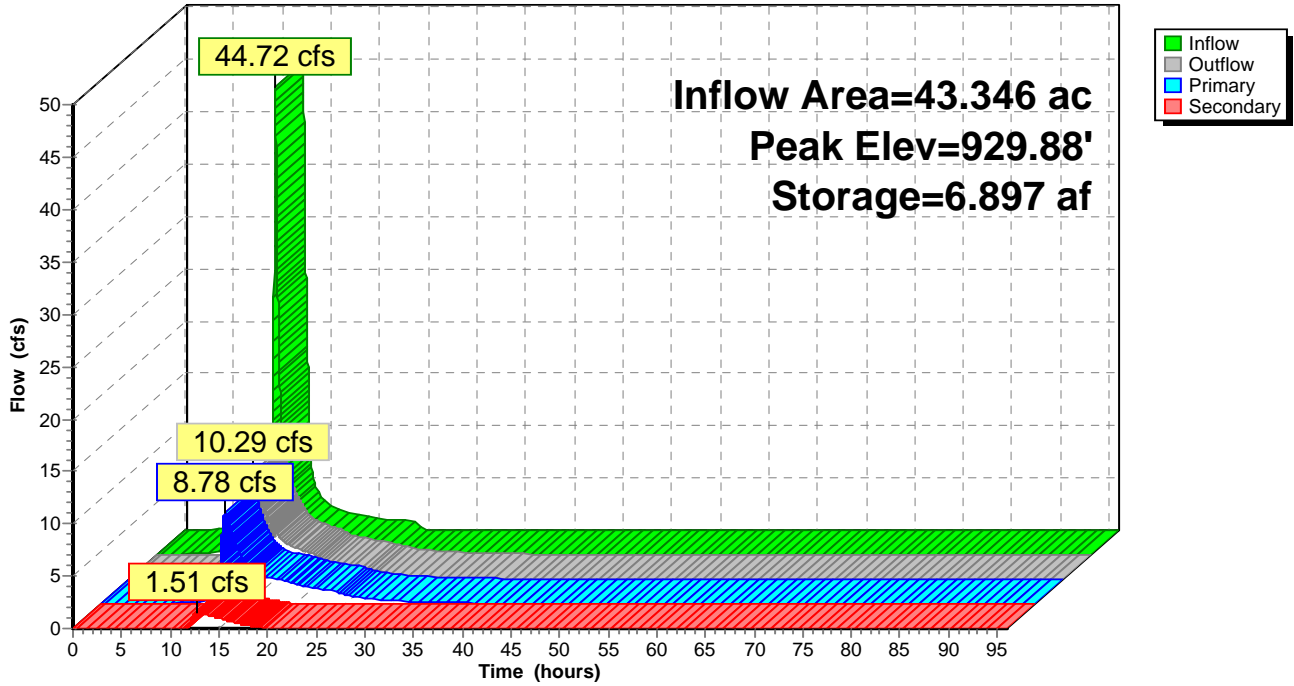
- ↑1=Orifice/Grate (Orifice Controls 3.54 cfs @ 4.51 fps)
- └2=Sharp-Crested Rectangular Weir (Weir Controls 5.24 cfs @ 2.01 fps)
- └3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

Secondary OutFlow Max=1.51 cfs @ 12.76 hrs HW=929.88' TW=929.08' (Dynamic Tailwater)

- ↑4=Orifice/Grate (Orifice Controls 1.51 cfs @ 3.41 fps)

Pond P-5/P-6: P-5/P-6

Hydrograph



Summary for Pond TI P: Thumb Infiltration (Thumb TP load only)

Inflow Area = 48.539 ac, 11.38% Impervious, Inflow Depth = 0.36" for 2-Year event
 Inflow = 10.90 cfs @ 12.32 hrs, Volume= 1.436 af
 Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 901.44' @ 26.34 hrs Surf.Area= 1.000 ac Storage= 1.436 af

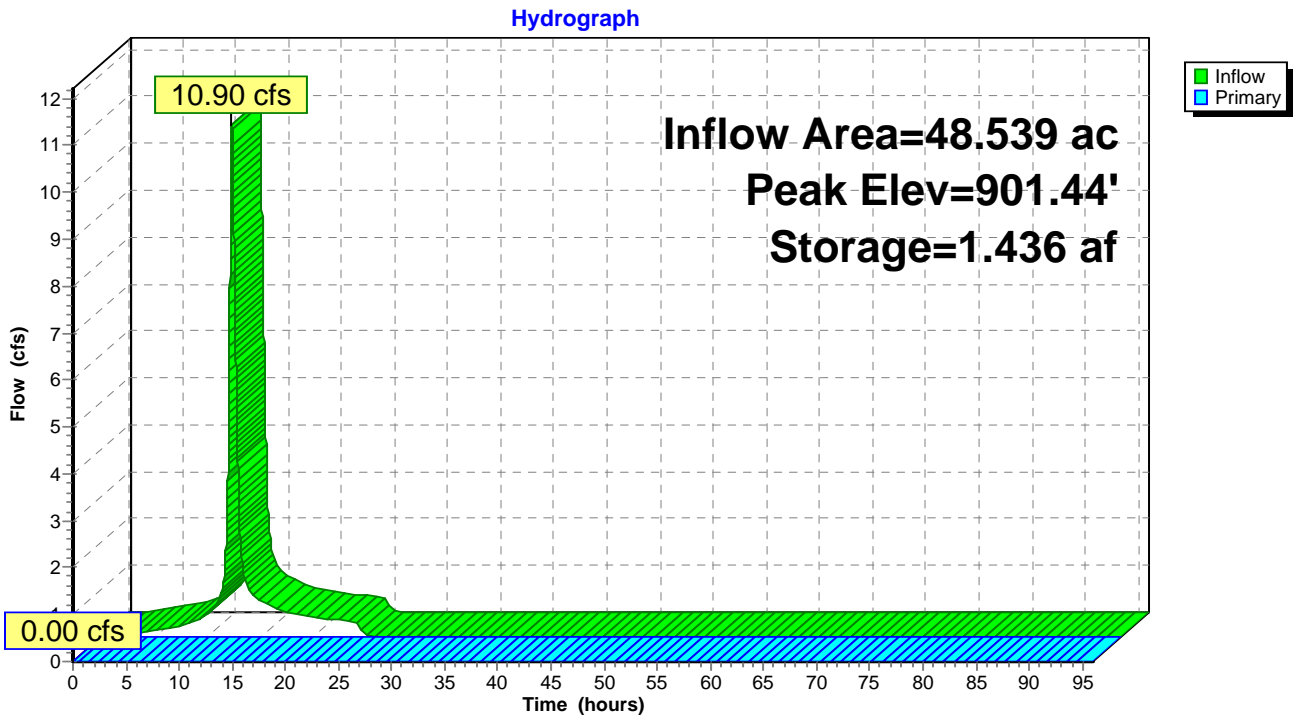
Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description
#1	900.00'	5.000 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
900.00	1.000	0.000	0.000
901.00	1.000	1.000	1.000
902.00	1.000	1.000	2.000
903.00	1.000	1.000	3.000
904.00	1.000	1.000	4.000
905.00	1.000	1.000	5.000

Device	Routing	Invert	Outlet Devices
#1	Primary	903.74'	1,000.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) 5.0' Crest Height

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=900.00' (Free Discharge)
 ↑1=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

Pond TI P: Thumb Infiltration (Thumb TP load only)



Summary for Pond W-1: W-1

Inflow Area = 1.000 ac, 10.00% Impervious, Inflow Depth = 5.57" for 2-Year event
 Inflow = 1.52 cfs @ 13.30 hrs, Volume= 0.464 af
 Outflow = 1.05 cfs @ 14.39 hrs, Volume= 0.464 af, Atten= 31%, Lag= 65.6 min
 Primary = 1.05 cfs @ 14.39 hrs, Volume= 0.464 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 914.97' @ 14.39 hrs Surf.Area= 0.695 ac Storage= 0.148 af

Plug-Flow detention time= 154.8 min calculated for 0.464 af (100% of inflow)
 Center-of-Mass det. time= 153.7 min (1,074.9 - 921.2)

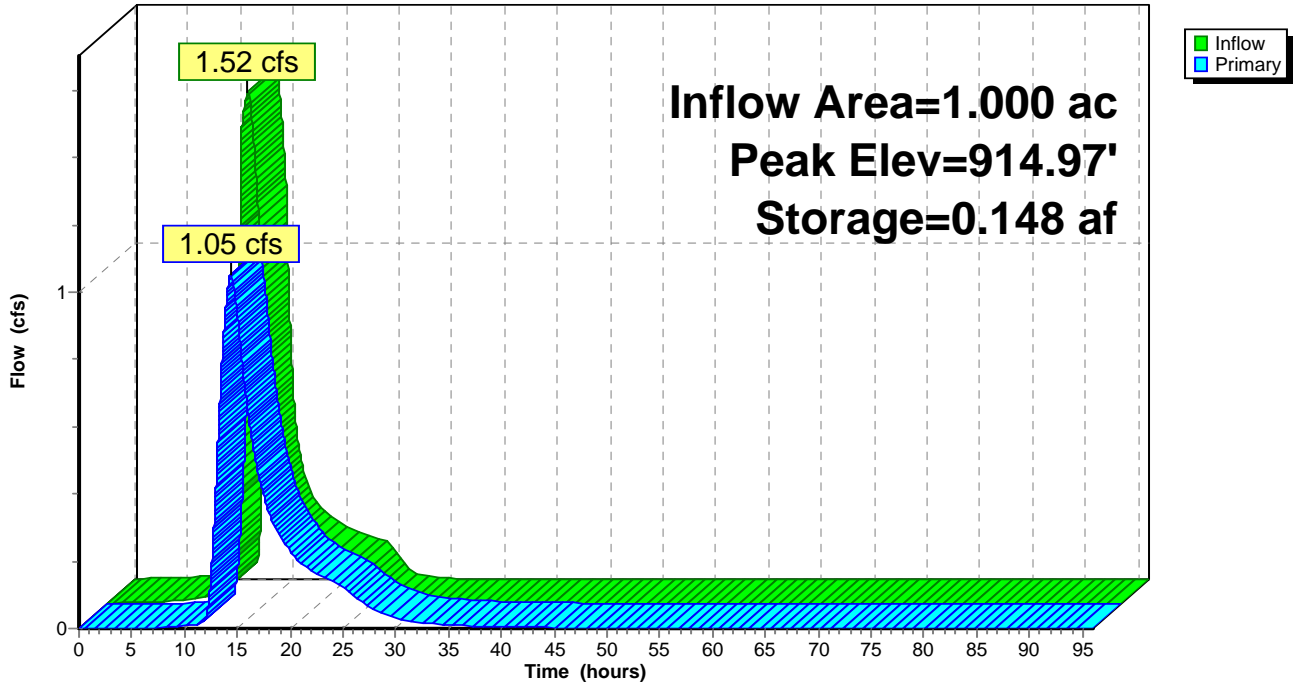
Volume	Invert	Avail.Storage	Storage Description
#1	914.75'	0.950 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
914.75	0.660	0.000	0.000
916.00	0.860	0.950	0.950

Device	Routing	Invert	Outlet Devices
#1	Primary	914.75'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=1.05 cfs @ 14.39 hrs HW=914.97' TW=0.00' (Dynamic Tailwater)
 ↑**1=Orifice/Grate** (Weir Controls 1.05 cfs @ 1.53 fps)

Pond W-1: W-1

Hydrograph



Summary for Pond W-3: W-3

Inflow = 0.32 cfs @ 17.83 hrs, Volume= 0.351 af
 Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

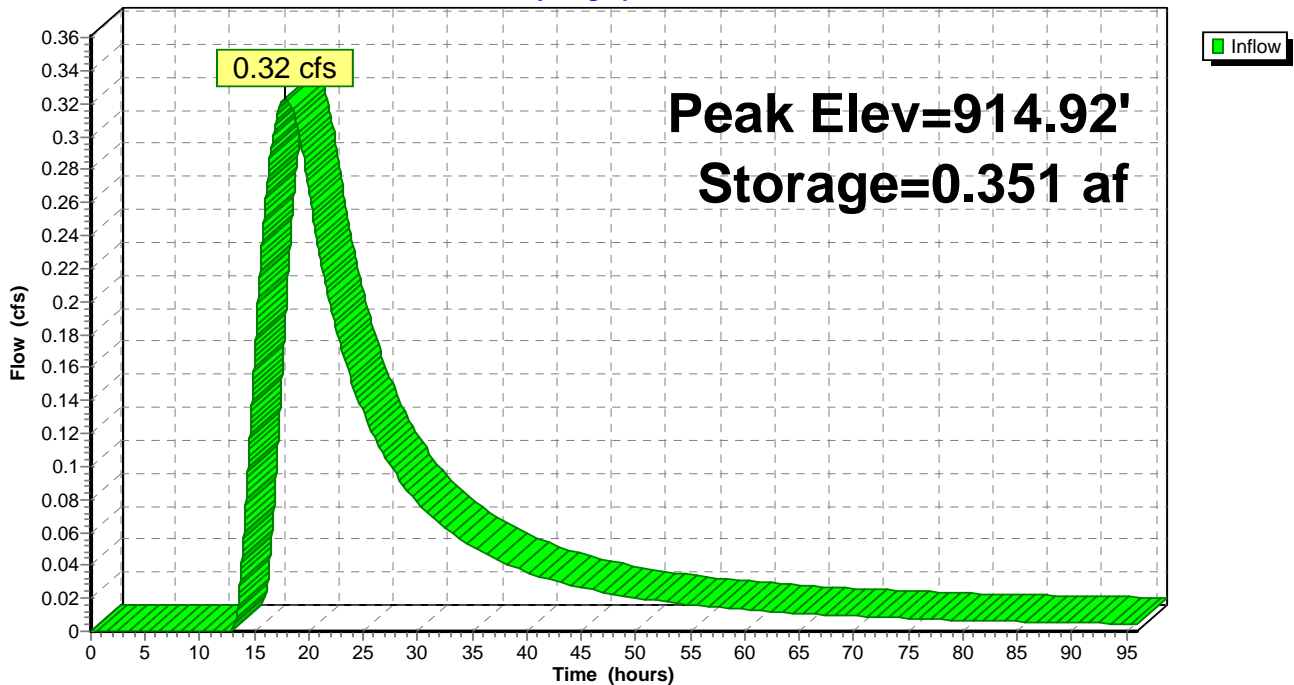
Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 914.92' @ 96.00 hrs Surf.Area= 2.067 ac Storage= 0.351 af

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description
#1	914.75'	2.680 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
914.75	2.040	0.000	0.000
915.00	2.080	0.515	0.515
916.00	2.250	2.165	2.680

Pond W-3: W-3

Hydrograph



Summary for Pond W-4: W-4

Inflow Area = 2.980 ac, 26.17% Impervious, Inflow Depth = 6.40" for 2-Year event
 Inflow = 4.39 cfs @ 12.08 hrs, Volume= 1.588 af
 Outflow = 2.16 cfs @ 16.02 hrs, Volume= 1.570 af, Atten= 51%, Lag= 236.1 min
 Primary = 2.16 cfs @ 16.02 hrs, Volume= 1.570 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 908.71' @ 16.02 hrs Surf.Area= 1.037 ac Storage= 0.649 af

Plug-Flow detention time= 324.3 min calculated for 1.570 af (99% of inflow)
 Center-of-Mass det. time= 315.3 min (1,234.3 - 919.0)

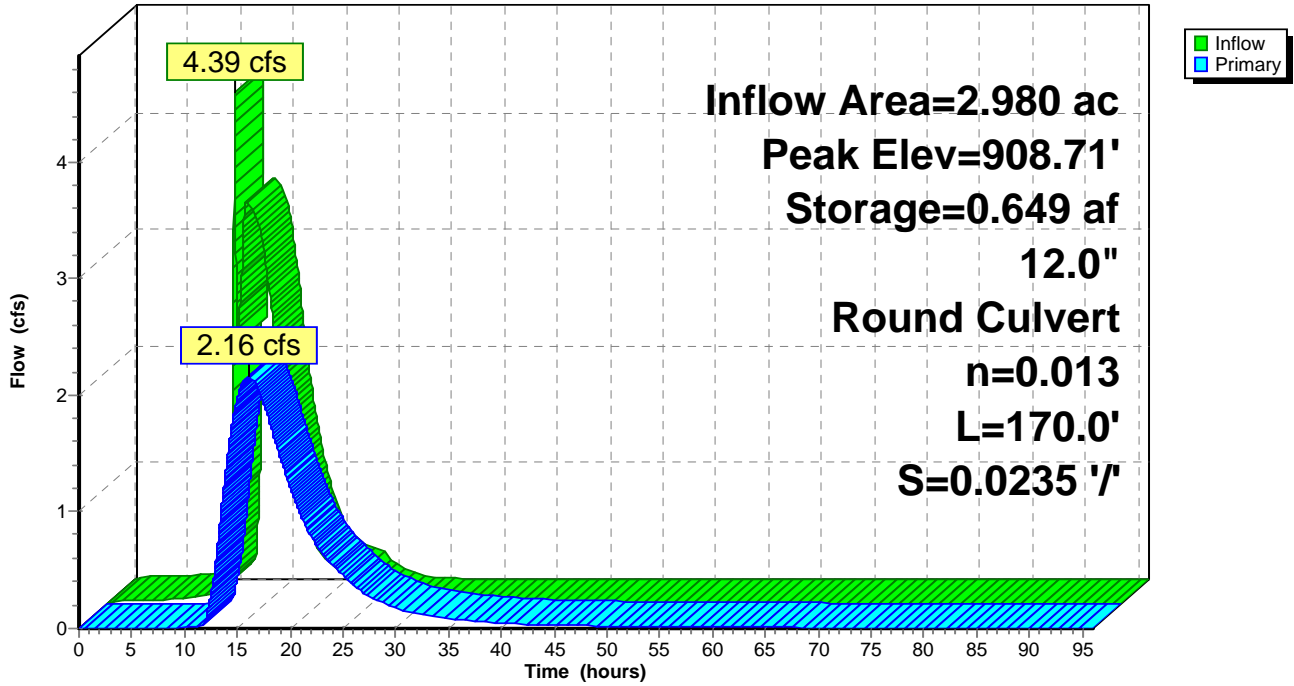
Volume	Invert	Avail.Storage	Storage Description
#1	908.00'	2.280 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
908.00	0.780	0.000	0.000
910.00	1.500	2.280	2.280

Device	Routing	Invert	Outlet Devices
#1	Primary	908.00'	12.0" Round RCP_Round 12" L= 170.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 908.00' / 904.00' S= 0.0235 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=2.16 cfs @ 16.02 hrs HW=908.71' TW=893.61' (Dynamic Tailwater)
 ↑1=RCP_Round 12" (Inlet Controls 2.16 cfs @ 3.60 fps)

Pond W-4: W-4

Hydrograph



Summary for Pond W-5: W-5

Inflow Area = 7.608 ac, 48.41% Impervious, Inflow Depth = 3.02" for 2-Year event
 Inflow = 19.24 cfs @ 12.02 hrs, Volume= 1.913 af
 Outflow = 2.91 cfs @ 13.19 hrs, Volume= 1.906 af, Atten= 85%, Lag= 70.2 min
 Primary = 2.91 cfs @ 13.19 hrs, Volume= 1.906 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 882.75' Surf.Area= 4.910 ac Storage= 3.412 af
 Peak Elev= 882.93' @ 13.19 hrs Surf.Area= 5.080 ac Storage= 4.298 af (0.886 af above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= 325.7 min (1,149.4 - 823.6)

Volume	Invert	Avail.Storage	Storage Description
#1	882.00'	7.390 af	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
882.00	4.190	0.000	0.000
883.00	5.150	4.670	4.670
883.49	5.950	2.720	7.390

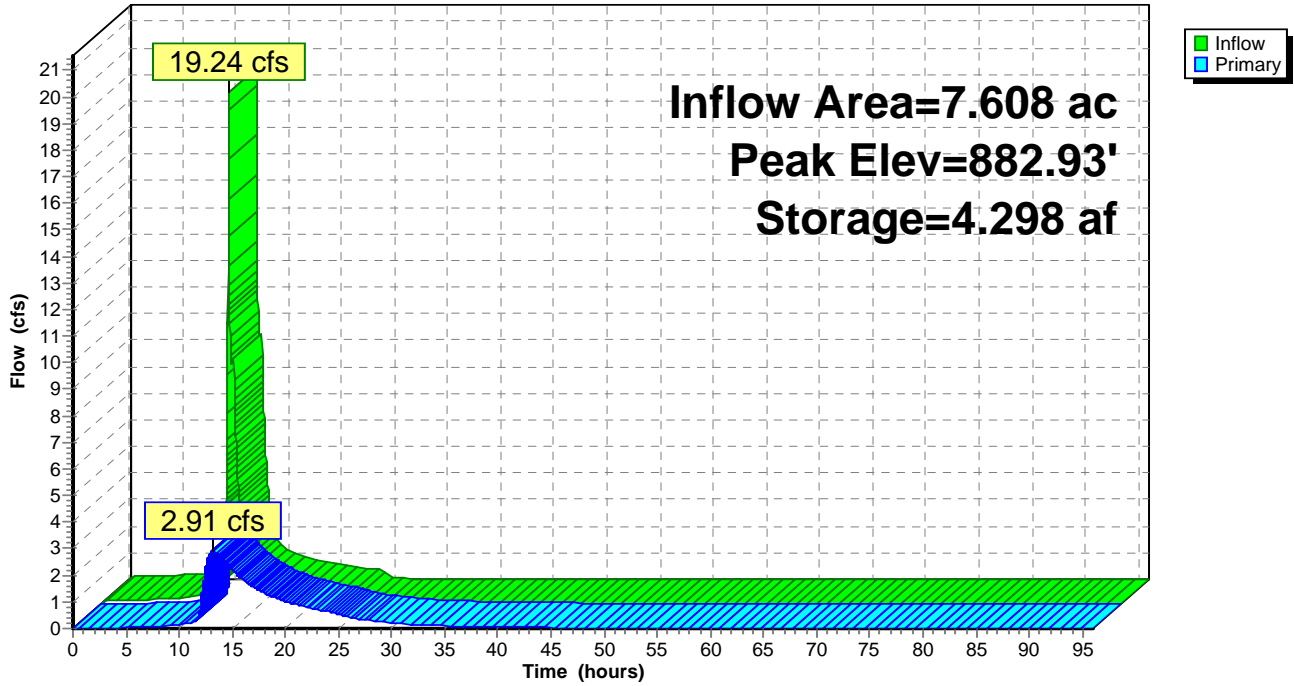
Device	Routing	Invert	Outlet Devices
#1	Primary	882.75'	6.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Primary	882.75'	6.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)

Primary OutFlow Max=2.91 cfs @ 13.19 hrs HW=882.93' TW=0.00' (Dynamic Tailwater)

- 1=Sharp-Crested Rectangular Weir (Weir Controls 1.46 cfs @ 1.38 fps)
- 2=Sharp-Crested Rectangular Weir (Weir Controls 1.46 cfs @ 1.38 fps)

Pond W-5: W-5

Hydrograph



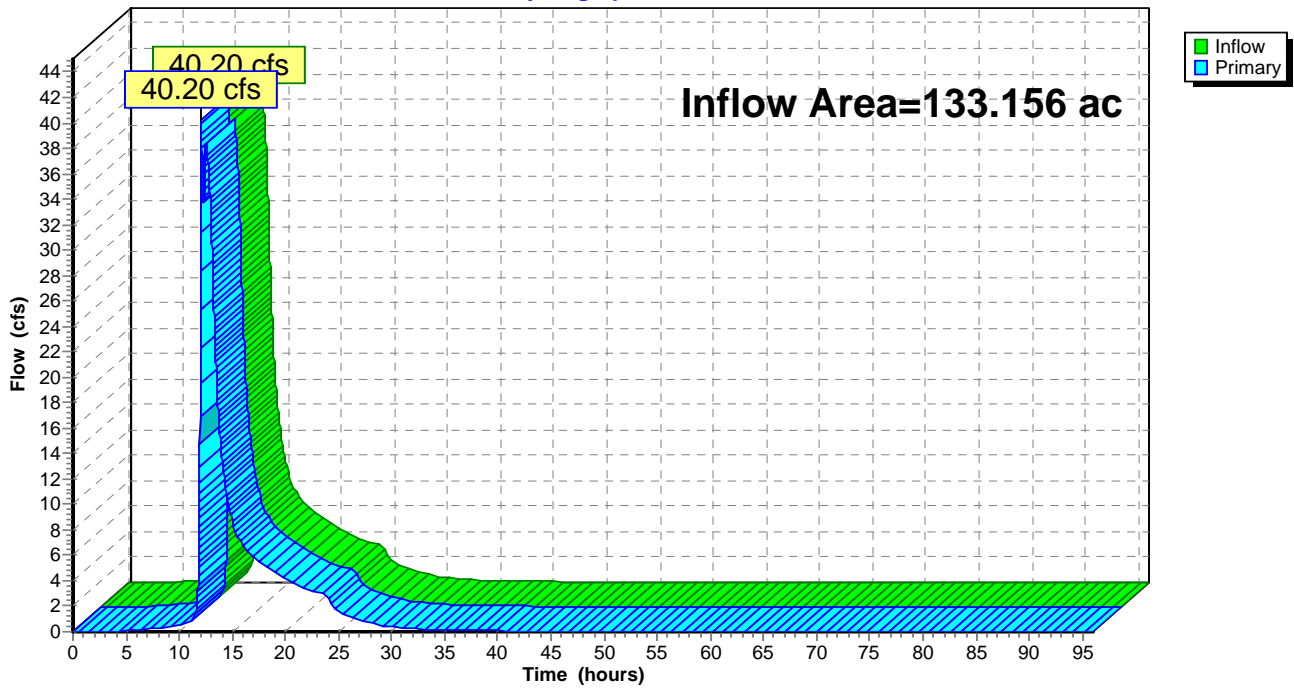
Summary for Link 53L: Sum of Outlet #2 Discharges to Round Lake

Inflow Area = 133.156 ac, 9.78% Impervious, Inflow Depth = 0.93" for 2-Year event
Inflow = 40.20 cfs @ 12.07 hrs, Volume= 10.286 af
Primary = 40.20 cfs @ 12.07 hrs, Volume= 10.286 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 53L: Sum of Outlet #2 Discharges to Round Lake

Hydrograph



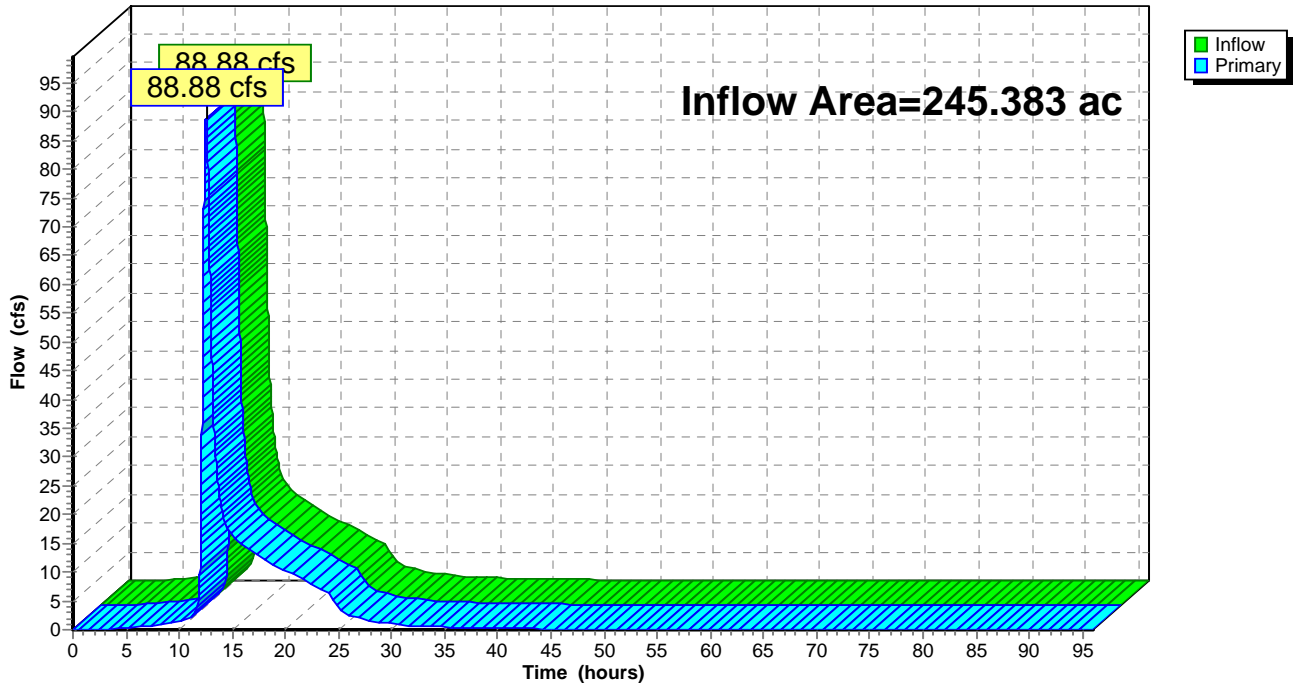
Summary for Link 54L: Sum of Discharges from P-13 and W-5

Inflow Area = 245.383 ac, 10.42% Impervious, Inflow Depth = 0.99" for 2-Year event
Inflow = 88.88 cfs @ 12.53 hrs, Volume= 20.165 af
Primary = 88.88 cfs @ 12.53 hrs, Volume= 20.165 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 54L: Sum of Discharges from P-13 and W-5

Hydrograph

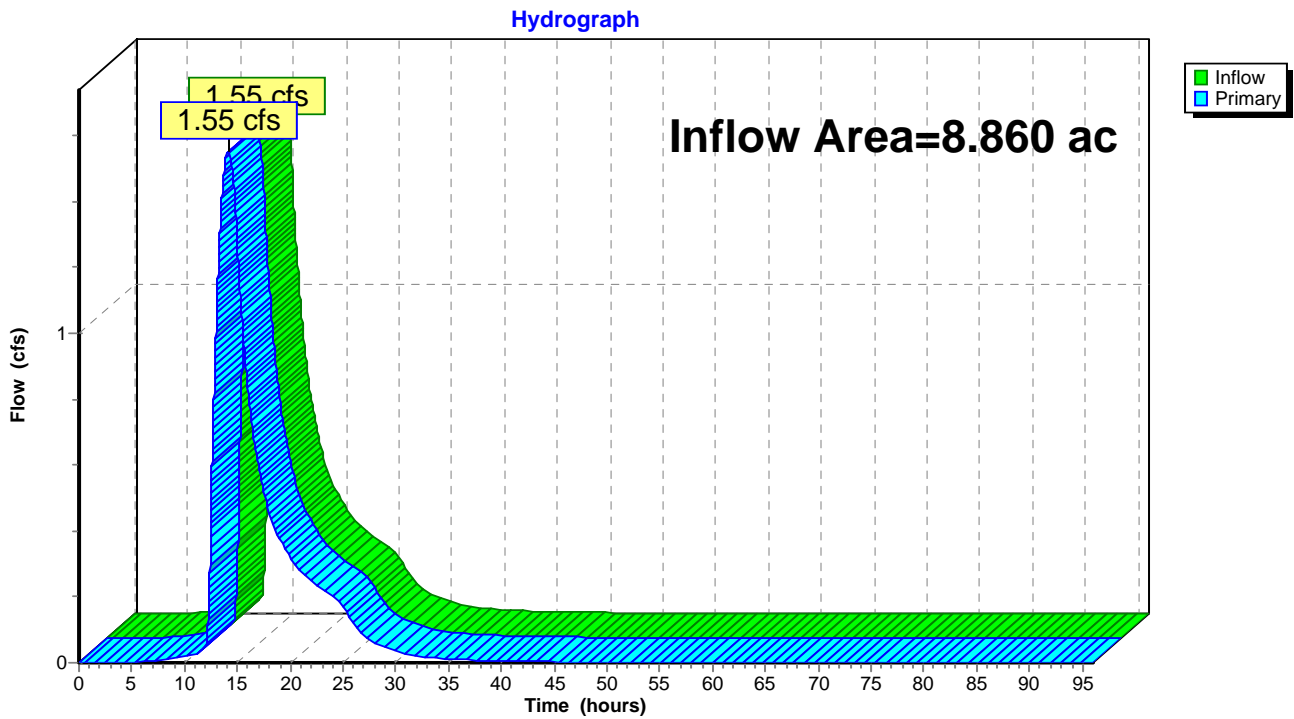


Summary for Link 55L: Sum of Outlet #1 Discharges to Round Lake

Inflow Area = 8.860 ac, 6.43% Impervious, Inflow Depth = 0.91" for 2-Year event
Inflow = 1.55 cfs @ 14.14 hrs, Volume= 0.675 af
Primary = 1.55 cfs @ 14.14 hrs, Volume= 0.675 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 55L: Sum of Outlet #1 Discharges to Round Lake



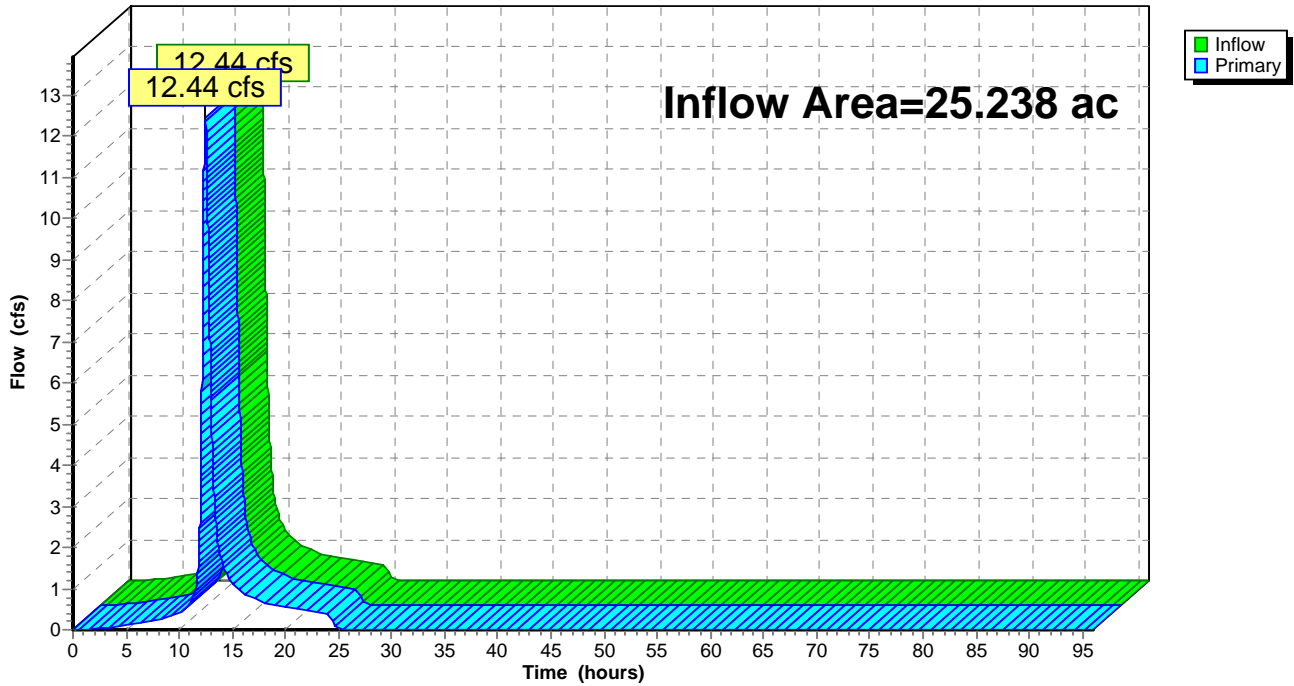
Summary for Link 57L: Outlet #3 Discharge to Round Lake

Inflow Area = 25.238 ac, 19.96% Impervious, Inflow Depth = 0.86" for 2-Year event
Inflow = 12.44 cfs @ 12.43 hrs, Volume= 1.805 af
Primary = 12.44 cfs @ 12.43 hrs, Volume= 1.805 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 57L: Outlet #3 Discharge to Round Lake

Hydrograph



Time span=0.00-96.00 hrs, dt=0.01 hrs, 9601 points
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: To Rice Creek	Runoff Area=1.601 ac 31.98% Impervious Runoff Depth=2.47" Tc=5.7 min CN=74/98 Runoff=5.55 cfs 0.330 af
Subcatchment 47S: Offsite Subbasin 51	Runoff Area=25.238 ac 19.96% Impervious Runoff Depth=1.72" Tc=32.8 min CN=65/98 Runoff=27.74 cfs 3.622 af
Subcatchment SB 1: SB 1	Runoff Area=52.150 ac 0.00% Impervious Runoff Depth=1.76" Tc=53.1 min CN=74/0 Runoff=49.76 cfs 7.644 af
Subcatchment SB 11: SB 11	Runoff Area=3.290 ac 36.78% Impervious Runoff Depth=2.66" Tc=11.7 min CN=74/100 Runoff=9.15 cfs 0.730 af
Subcatchment SB 12: SB 12	Runoff Area=1.390 ac 20.86% Impervious Runoff Depth=2.22" Tc=9.5 min CN=74/98 Runoff=3.67 cfs 0.258 af
Subcatchment SB 13: SB 13	Runoff Area=2.980 ac 26.17% Impervious Runoff Depth=2.40" Tc=9.4 min CN=74/100 Runoff=8.31 cfs 0.597 af
Subcatchment SB 14: SB 14	Runoff Area=10.230 ac 16.03% Impervious Runoff Depth=2.12" Tc=4.3 min CN=74/98 Runoff=33.20 cfs 1.804 af
Subcatchment SB 15: SB 15	Runoff Area=58.570 ac 0.05% Impervious Runoff Depth=1.76" Tc=31.3 min CN=74/98 Runoff=73.90 cfs 8.591 af
Subcatchment SB 16: SB 16	Runoff Area=32.440 ac 5.76% Impervious Runoff Depth=1.89" Tc=12.1 min CN=74/98 Runoff=66.93 cfs 5.102 af
Subcatchment SB 17: SB 17	Runoff Area=7.608 ac 48.41% Impervious Runoff Depth=2.95" Tc=4.3 min CN=74/100 Runoff=32.13 cfs 1.870 af
Subcatchment SB 18: SB 18	Runoff Area=52.790 ac 0.00% Impervious Runoff Depth=1.76" Tc=33.5 min CN=74/0 Runoff=64.04 cfs 7.738 af
Subcatchment SB 19: SB 19	Runoff Area=21.190 ac 0.00% Impervious Runoff Depth=1.76" Tc=24.7 min CN=74/0 Runoff=29.93 cfs 3.106 af
Subcatchment SB 2: SB 2	Runoff Area=11.067 ac 0.33% Impervious Runoff Depth=1.77" Tc=16.6 min CN=74/98 Runoff=18.86 cfs 1.629 af
Subcatchment SB 22: SB 22	Runoff Area=41.910 ac 0.00% Impervious Runoff Depth=0.36" Tc=41.0 min CN=49/0 Runoff=5.43 cfs 1.272 af
Subcatchment SB 24: SB 24	Runoff Area=5.043 ac 97.56% Impervious Runoff Depth=3.93" Tc=7.5 min CN=74/98 Runoff=24.79 cfs 1.651 af
Subcatchment SB 25: SB 25	Runoff Area=5.136 ac 95.72% Impervious Runoff Depth=3.89" Tc=10.7 min CN=74/98 Runoff=21.69 cfs 1.664 af

Subcatchment SB 26: SB 26	Runoff Area=14.335 ac 98.27% Impervious Runoff Depth=3.95" Tc=25.4 min CN=74/98 Runoff=41.75 cfs 4.713 af
Subcatchment SB 27: SB 27 (Thumb Road)	Runoff Area=6.629 ac 83.33% Impervious Runoff Depth=3.61" Tc=27.6 min CN=74/98 Runoff=17.05 cfs 1.996 af
Subcatchment SB 28: SB 28	Runoff Area=6.955 ac 46.76% Impervious Runoff Depth=2.80" Tc=14.6 min CN=74/98 Runoff=18.97 cfs 1.622 af
Subcatchment SB 29: SB 29	Runoff Area=10.214 ac 37.73% Impervious Runoff Depth=2.60" Tc=19.1 min CN=74/98 Runoff=23.01 cfs 2.212 af
Subcatchment SB 3: SB 3	Runoff Area=37.610 ac 7.68% Impervious Runoff Depth=1.93" Tc=15.3 min CN=74/98 Runoff=71.94 cfs 6.049 af
Subcatchment SB 4: SB 4	Runoff Area=0.600 ac 43.33% Impervious Runoff Depth=2.83" Tc=5.9 min CN=74/100 Runoff=2.26 cfs 0.141 af
Subcatchment SB 5: SB 5	Runoff Area=7.860 ac 5.98% Impervious Runoff Depth=1.89" Tc=59.3 min CN=74/98 Runoff=7.43 cfs 1.239 af
Subcatchment SB 6: SB 6	Runoff Area=1.000 ac 10.00% Impervious Runoff Depth=2.01" Tc=20.3 min CN=74/100 Runoff=1.72 cfs 0.167 af
Subcatchment SB 7: SB 7	Runoff Area=21.550 ac 0.00% Impervious Runoff Depth=1.76" Tc=5.7 min CN=74/0 Runoff=55.54 cfs 3.159 af
Subcatchment SB 8: SB 8	Runoff Area=29.580 ac 5.51% Impervious Runoff Depth=1.88" Tc=47.1 min CN=74/98 Runoff=31.84 cfs 4.638 af
Subcatchment SB 9: SB 9	Runoff Area=25.780 ac 0.12% Impervious Runoff Depth=1.76" Tc=30.0 min CN=74/98 Runoff=33.02 cfs 3.784 af
Subcatchment SB10: SB 10	Runoff Area=6.390 ac 4.85% Impervious Runoff Depth=1.87" Tc=7.3 min CN=74/98 Runoff=15.99 cfs 0.994 af
Pond 2 P: P-2	Peak Elev=924.98' Storage=1.125 af Inflow=59.33 cfs 10.925 af Outflow=59.01 cfs 10.925 af
Pond 4P: P-4	Peak Elev=916.17' Storage=1.005 af Inflow=7.43 cfs 1.239 af Primary=1.35 cfs 0.436 af Secondary=2.26 cfs 0.803 af Outflow=3.61 cfs 1.239 af
Pond 7P: P-7	Peak Elev=915.49' Storage=1.291 af Inflow=31.84 cfs 4.638 af Outflow=32.31 cfs 4.638 af
Pond 8P: P-8	Peak Elev=898.05' Storage=0.895 af Inflow=15.99 cfs 0.994 af 24.0" Round Culvert n=0.013 L=380.0' S=0.0028 '/' Outflow=3.70 cfs 0.989 af
Pond 9P: P-9	Peak Elev=915.44' Storage=0.464 af Inflow=58.97 cfs 8.423 af Outflow=58.93 cfs 8.423 af
Pond 10P: P-10 Lowered 1 ft	Peak Elev=897.64' Storage=1.230 af Inflow=25.80 cfs 8.191 af Primary=11.94 cfs 6.845 af Secondary=13.80 cfs 1.346 af Outflow=25.74 cfs 8.190 af

Pond 11P: P-11 Peak Elev=911.47' Storage=6.979 af Inflow=61.92 cfs 9.153 af
Primary=23.80 cfs 6.945 af Secondary=4.08 cfs 2.206 af Outflow=27.87 cfs 9.151 af

Pond 12P: P-12 Peak Elev=894.12' Storage=6.997 af Inflow=34.82 cfs 12.778 af
Outflow=24.47 cfs 12.770 af

Pond 13P: P-13 Peak Elev=884.19' Storage=6.735 af Inflow=213.60 cfs 38.914 af
Primary=189.63 cfs 37.025 af Secondary=11.45 cfs 1.886 af Outflow=201.08 cfs 38.911 af

Pond 17P: W-2 Peak Elev=929.45' Storage=0.512 af Inflow=2.25 cfs 0.809 af
12.0" Round Culvert n=0.013 L=300.0' S=0.0437 '/' Outflow=0.63 cfs 0.666 af

Pond 36P: Culverts passing flow beneath Peak Elev=887.26' Storage=0.000 af Inflow=64.04 cfs 7.738 af
Primary=64.04 cfs 7.738 af Secondary=0.00 cfs 0.000 af Outflow=64.04 cfs 7.738 af

Pond CRH-1: CRH-1 Peak Elev=878.12' Storage=0.489 af Inflow=18.97 cfs 1.622 af
Discarded=0.26 cfs 0.512 af Primary=11.80 cfs 1.110 af Outflow=12.06 cfs 1.622 af

Pond CRH-2: CRH-2 Peak Elev=882.67' Storage=0.888 af Inflow=23.01 cfs 2.212 af
Discarded=0.38 cfs 0.904 af Primary=9.82 cfs 1.308 af Outflow=10.19 cfs 2.212 af

Pond CRH-3: CRH-3 Peak Elev=878.90' Storage=0.421 af Inflow=10.84 cfs 1.637 af
Discarded=0.24 cfs 0.449 af Primary=7.98 cfs 1.188 af Outflow=8.22 cfs 1.637 af

Pond P-5/P-6: P-5/P-6 Peak Elev=930.49' Storage=8.298 af Inflow=91.40 cfs 7.854 af
Primary=26.70 cfs 7.038 af Secondary=2.25 cfs 0.809 af Outflow=28.95 cfs 7.848 af

Pond TI P: Thumb Infiltration (Thumb TP Peak Elev=903.27' Storage=3.268 af Inflow=18.36 cfs 3.268 af
Outflow=0.00 cfs 0.000 af

Pond W-1: W-1 Peak Elev=915.09' Storage=0.231 af Inflow=2.45 cfs 0.971 af
Outflow=2.00 cfs 0.970 af

Pond W-3: W-3 Peak Elev=915.07' Storage=0.666 af Inflow=0.63 cfs 0.666 af
Outflow=0.00 cfs 0.000 af

Pond W-4: W-4 Peak Elev=908.93' Storage=0.878 af Inflow=8.82 cfs 2.803 af
12.0" Round Culvert n=0.013 L=170.0' S=0.0235 '/' Outflow=3.11 cfs 2.784 af

Pond W-5: W-5 Peak Elev=883.06' Storage=4.998 af Inflow=34.78 cfs 3.757 af
Outflow=6.80 cfs 3.750 af

Link 53L: Sum of Outlet #2 Discharges to Round Lake Inflow=96.75 cfs 21.122 af
Primary=96.75 cfs 21.122 af

Link 54L: Sum of Discharges from P-13 and W-5 Inflow=194.85 cfs 40.774 af
Primary=194.85 cfs 40.774 af

Link 55L: Sum of Outlet #1 Discharges to Round Lake Inflow=3.08 cfs 1.406 af
Primary=3.08 cfs 1.406 af

Link 57L: Outlet #3 Discharge to Round Lake

Inflow=27.74 cfs 3.622 af

Primary=27.74 cfs 3.622 af

Total Runoff Area = 501.136 ac Runoff Volume = 78.323 af Average Runoff Depth = 1.88"
88.56% Pervious = 443.803 ac 11.44% Impervious = 57.333 ac

Summary for Subcatchment 1S: To Rice Creek

Runoff = 5.55 cfs @ 12.04 hrs, Volume= 0.330 af, Depth= 2.47"

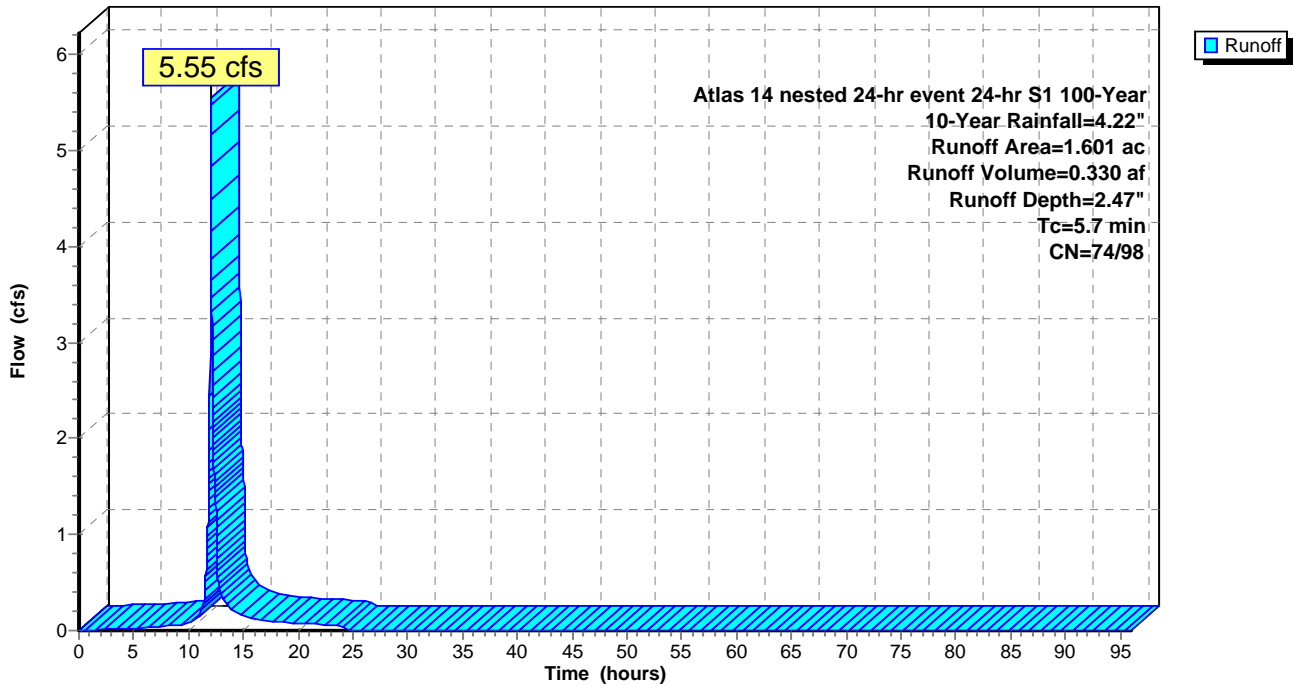
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 0.512	98	impervious
* 1.089	74	pervious
1.601	82	Weighted Average
1.089		68.02% Pervious Area
0.512		31.98% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.7					Direct Entry,

Subcatchment 1S: To Rice Creek

Hydrograph



Summary for Subcatchment 47S: Offsite Subbasin 51

Runoff = 27.74 cfs @ 12.43 hrs, Volume= 3.622 af, Depth= 1.72"

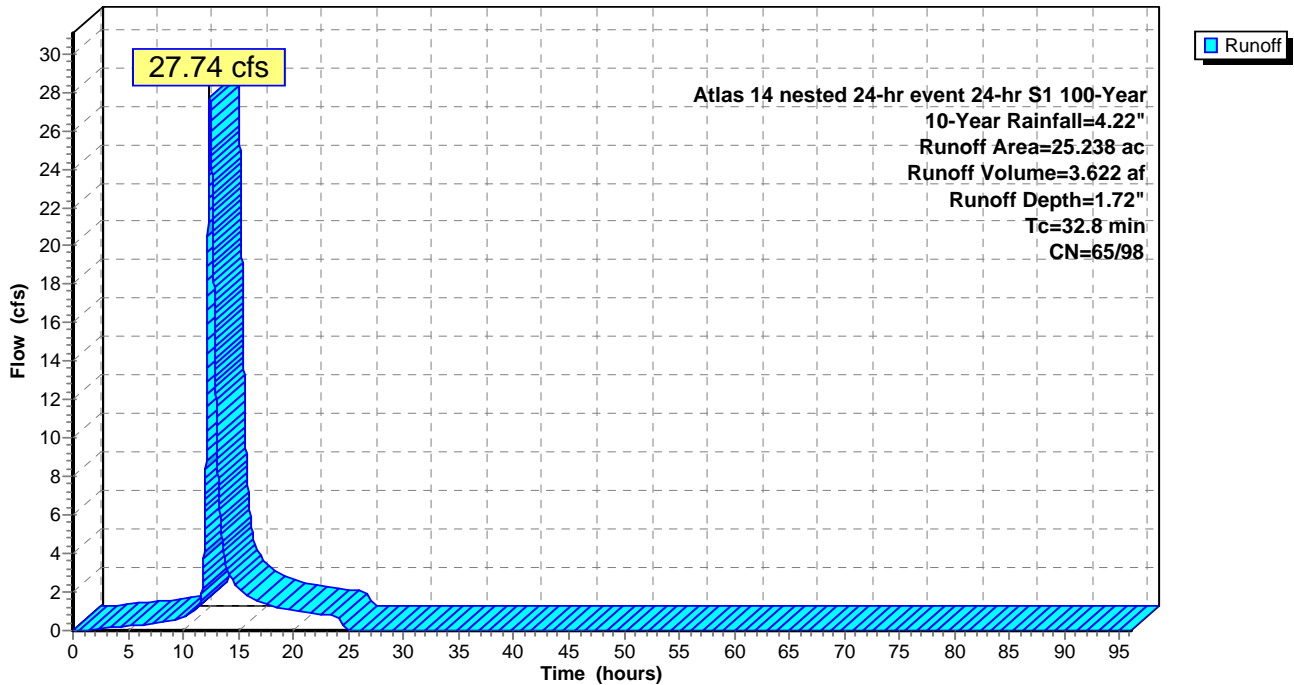
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 20.200	65	Offsite subbasin 51
* 5.038	98	
25.238	72	Weighted Average
20.200		80.04% Pervious Area
5.038		19.96% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.8					Direct Entry,

Subcatchment 47S: Offsite Subbasin 51

Hydrograph



Summary for Subcatchment SB 1: SB 1

Runoff = 49.76 cfs @ 12.74 hrs, Volume= 7.644 af, Depth= 1.76"

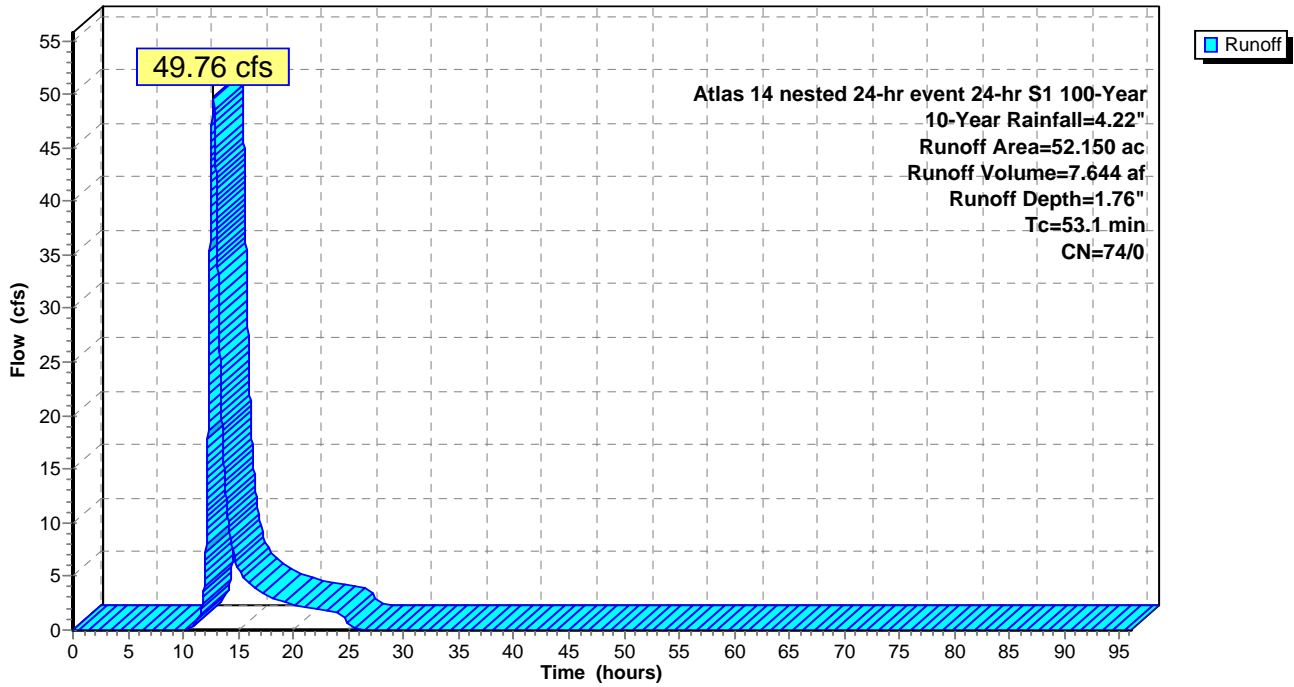
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 52.150	74	pervious
* 0.000	98	impervious
52.150	74	Weighted Average
52.150		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
53.1					Direct Entry,

Subcatchment SB 1: SB 1

Hydrograph



Summary for Subcatchment SB 11: SB 11

Runoff = 9.15 cfs @ 12.11 hrs, Volume= 0.730 af, Depth= 2.66"

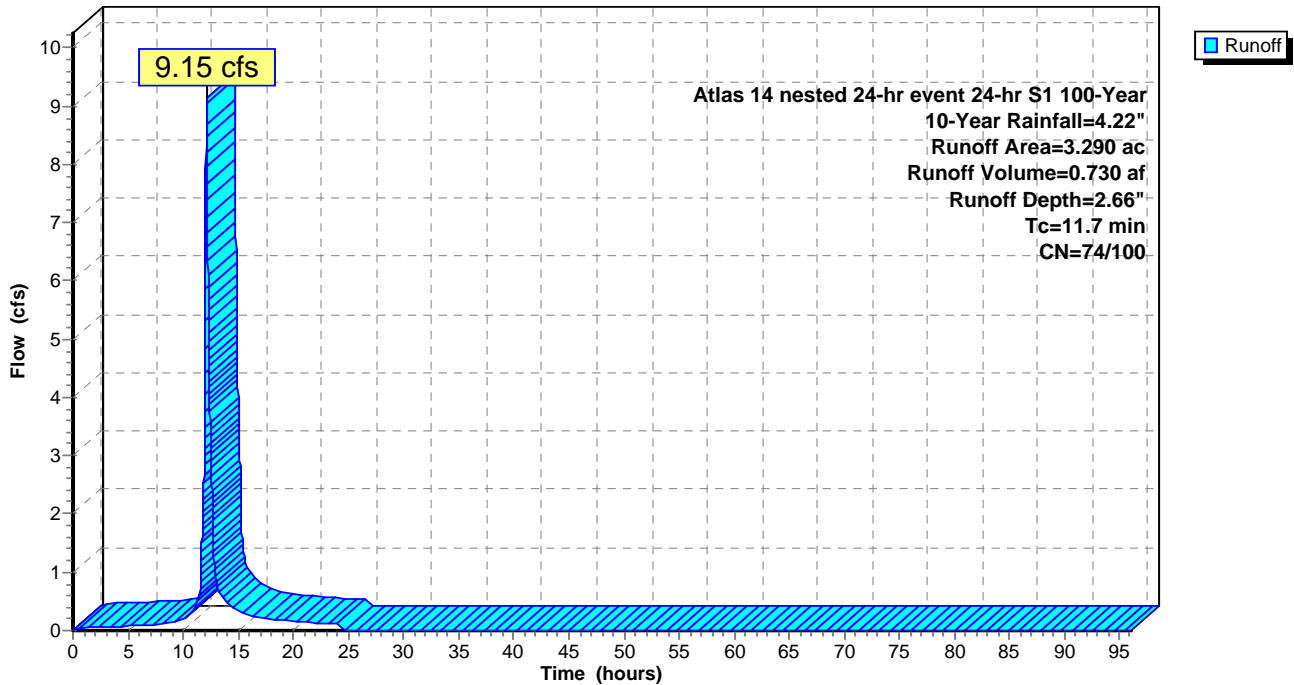
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 2.080	74	pervious
* 1.210	100	impervious
3.290	84	Weighted Average
2.080		63.22% Pervious Area
1.210		36.78% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.7					Direct Entry,

Subcatchment SB 11: SB 11

Hydrograph



Summary for Subcatchment SB 12: SB 12

Runoff = 3.67 cfs @ 12.08 hrs, Volume= 0.258 af, Depth= 2.22"

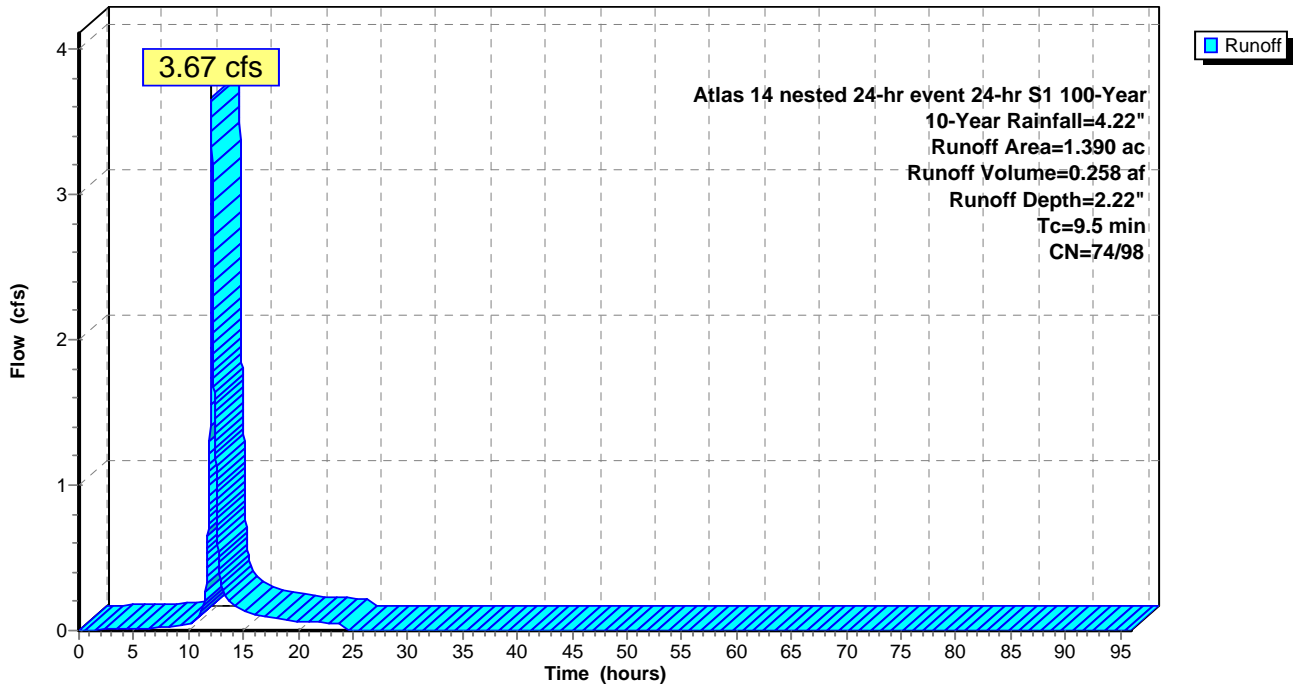
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 1.100	74	pervious
* 0.290	98	impervious
1.390	79	Weighted Average
1.100		79.14% Pervious Area
0.290		20.86% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.5					Direct Entry,

Subcatchment SB 12: SB 12

Hydrograph



Summary for Subcatchment SB 13: SB 13

Runoff = 8.31 cfs @ 12.08 hrs, Volume= 0.597 af, Depth= 2.40"

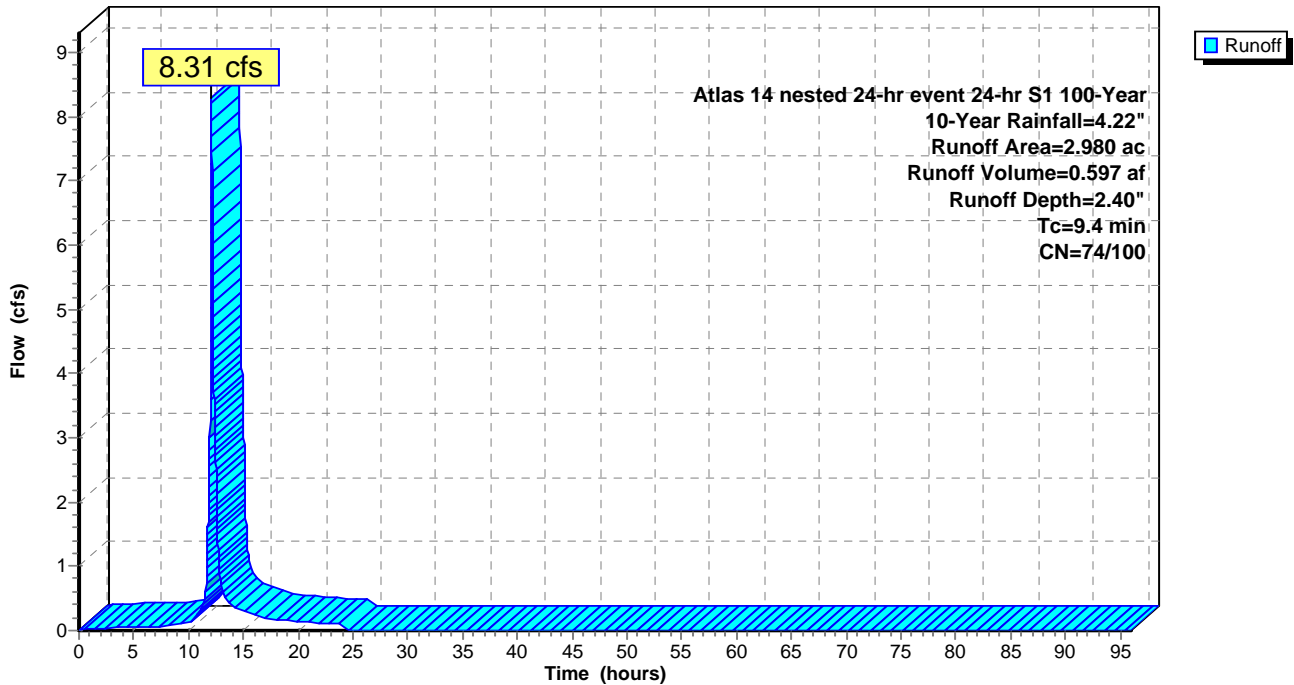
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 2.200	74	pervious
* 0.780	100	impervious
2.980	81	Weighted Average
2.200		73.83% Pervious Area
0.780		26.17% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.4					Direct Entry,

Subcatchment SB 13: SB 13

Hydrograph



Summary for Subcatchment SB 14: SB 14

Runoff = 33.20 cfs @ 12.02 hrs, Volume= 1.804 af, Depth= 2.12"

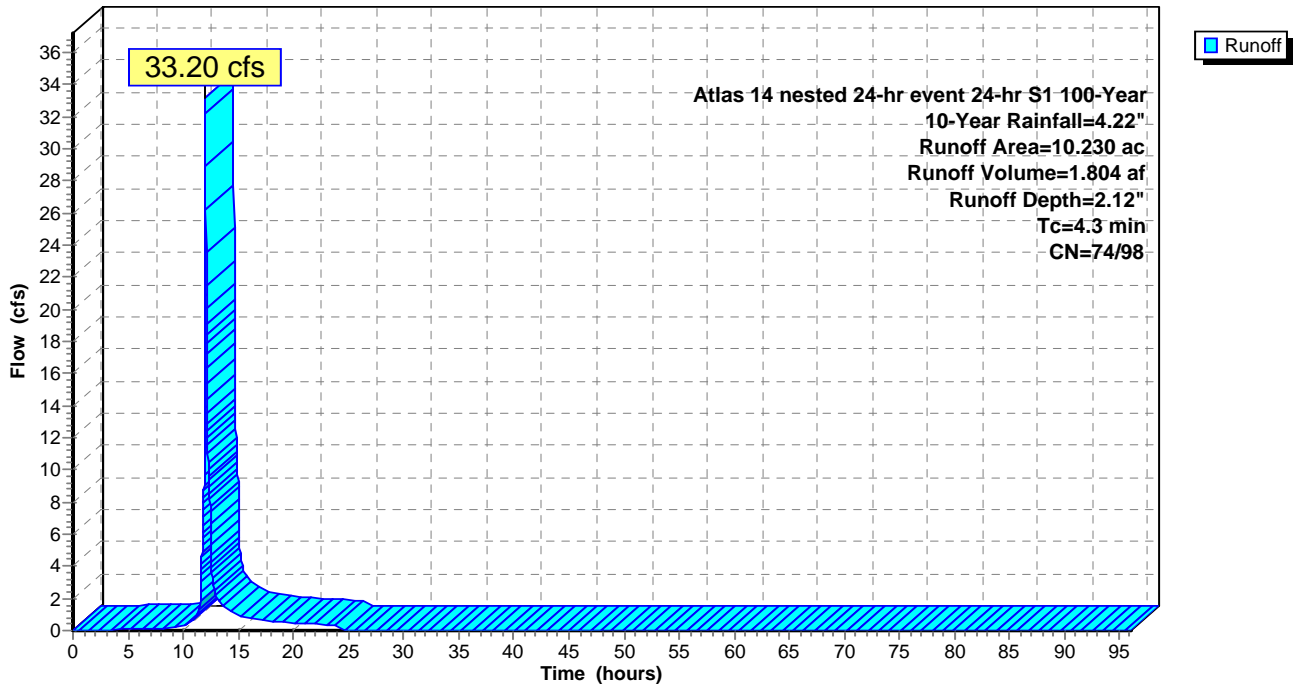
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 8.590	74	pervious
* 1.640	98	impervious
10.230	78	Weighted Average
8.590		83.97% Pervious Area
1.640		16.03% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.3					Direct Entry,

Subcatchment SB 14: SB 14

Hydrograph



Summary for Subcatchment SB 15: SB 15

Runoff = 73.90 cfs @ 12.42 hrs, Volume= 8.591 af, Depth= 1.76"

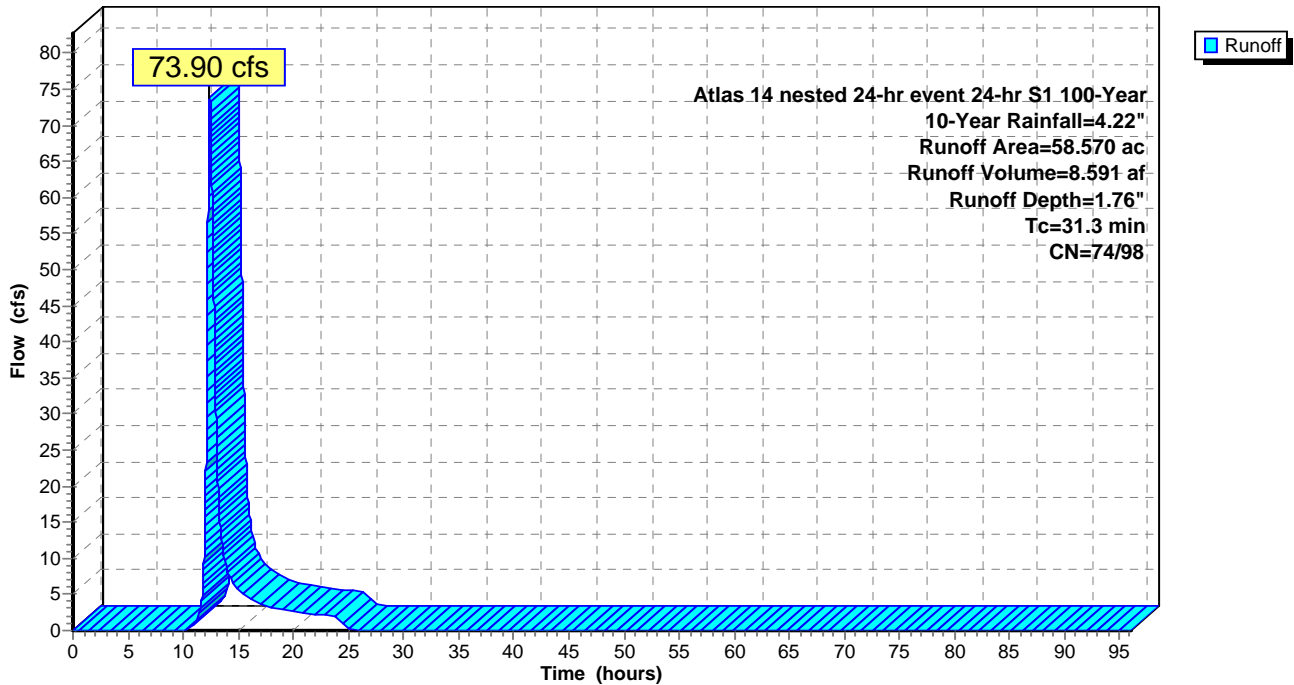
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 58.540	74	pervious
* 0.030	98	impervious
58.570	74	Weighted Average
58.540		99.95% Pervious Area
0.030		0.05% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
31.3					Direct Entry,

Subcatchment SB 15: SB 15

Hydrograph



Summary for Subcatchment SB 16: SB 16

Runoff = 66.93 cfs @ 12.13 hrs, Volume= 5.102 af, Depth= 1.89"

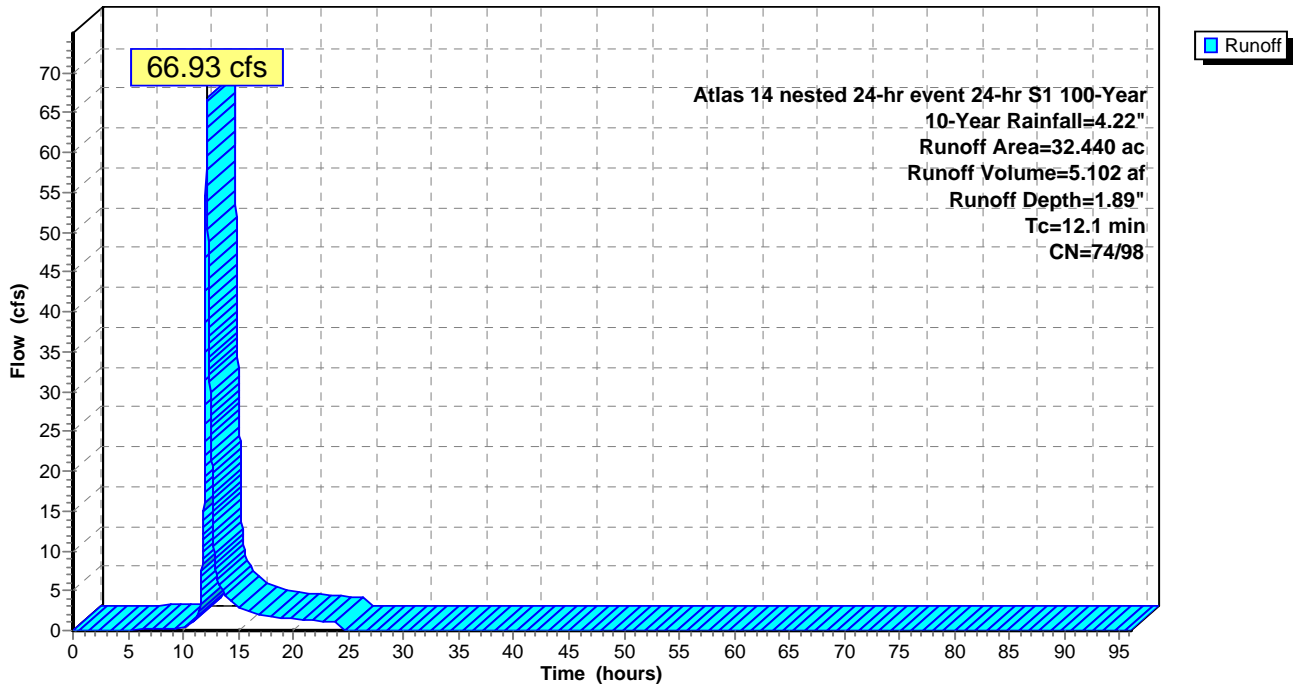
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 30.570	74	pervious
* 1.870	98	impervious
32.440	75	Weighted Average
30.570		94.24% Pervious Area
1.870		5.76% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.1					Direct Entry,

Subcatchment SB 16: SB 16

Hydrograph



Summary for Subcatchment SB 17: SB 17

Runoff = 32.13 cfs @ 12.02 hrs, Volume= 1.870 af, Depth= 2.95"

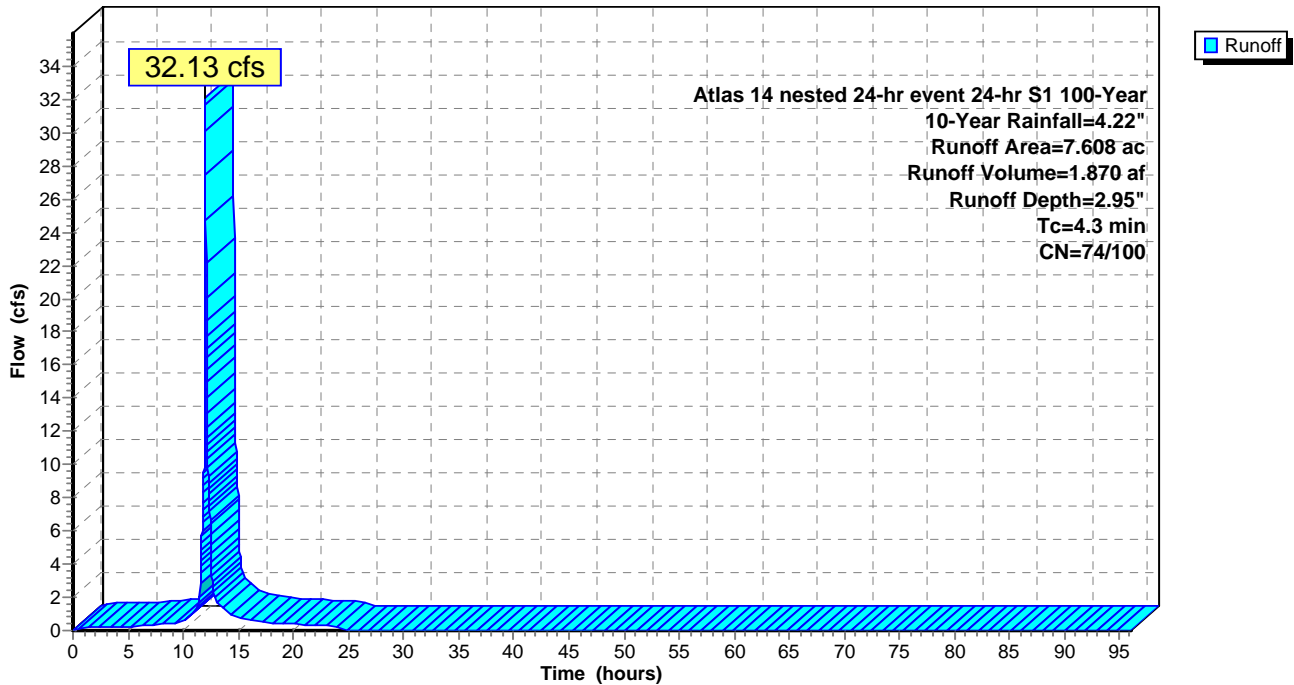
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 3.925	74	pervious
* 3.683	100	impervious
7.608	87	Weighted Average
3.925		51.59% Pervious Area
3.683		48.41% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.3					Direct Entry,

Subcatchment SB 17: SB 17

Hydrograph



Summary for Subcatchment SB 18: SB 18

Runoff = 64.04 cfs @ 12.46 hrs, Volume= 7.738 af, Depth= 1.76"

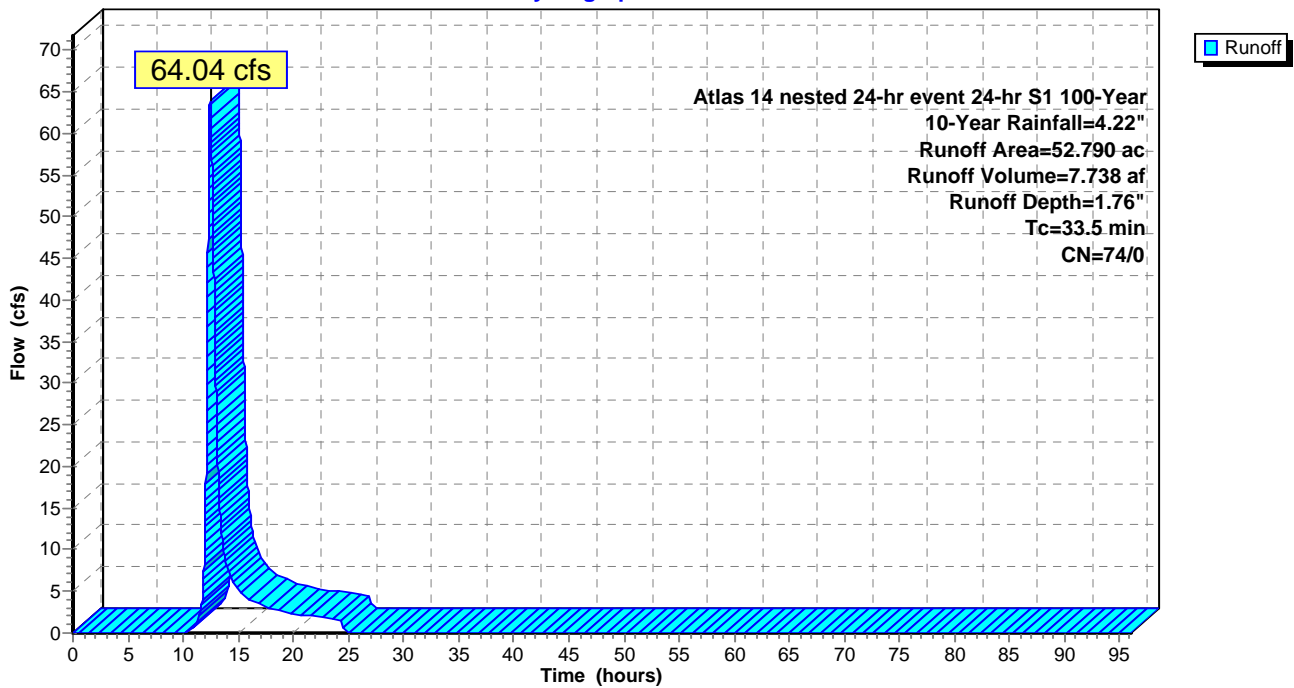
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 52.790	74	pervious
* 0.000	98	impervious
52.790	74	Weighted Average
52.790		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
33.5					Direct Entry,

Subcatchment SB 18: SB 18

Hydrograph



Summary for Subcatchment SB 19: SB 19

Runoff = 29.93 cfs @ 12.32 hrs, Volume= 3.106 af, Depth= 1.76"

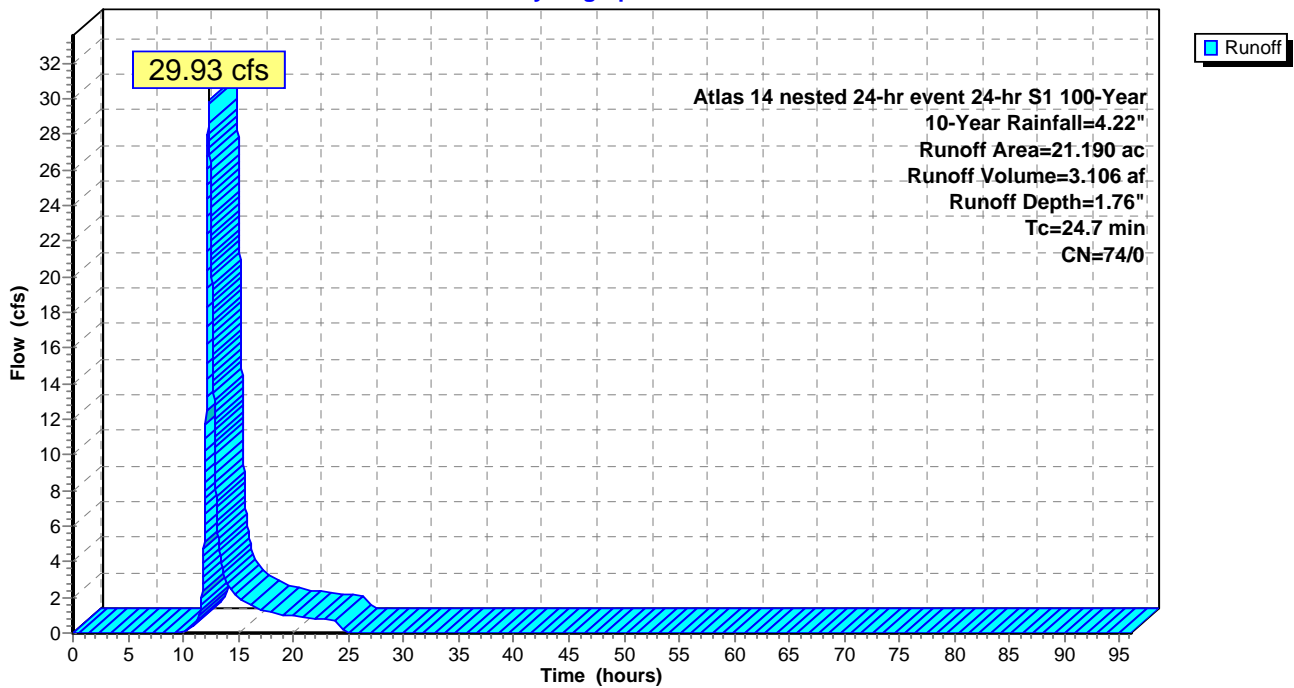
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 21.190	74	pervious
* 0.000	98	impervious
21.190	74	Weighted Average
21.190		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.7					Direct Entry,

Subcatchment SB 19: SB 19

Hydrograph



Summary for Subcatchment SB 2: SB 2

Runoff = 18.86 cfs @ 12.19 hrs, Volume= 1.629 af, Depth= 1.77"

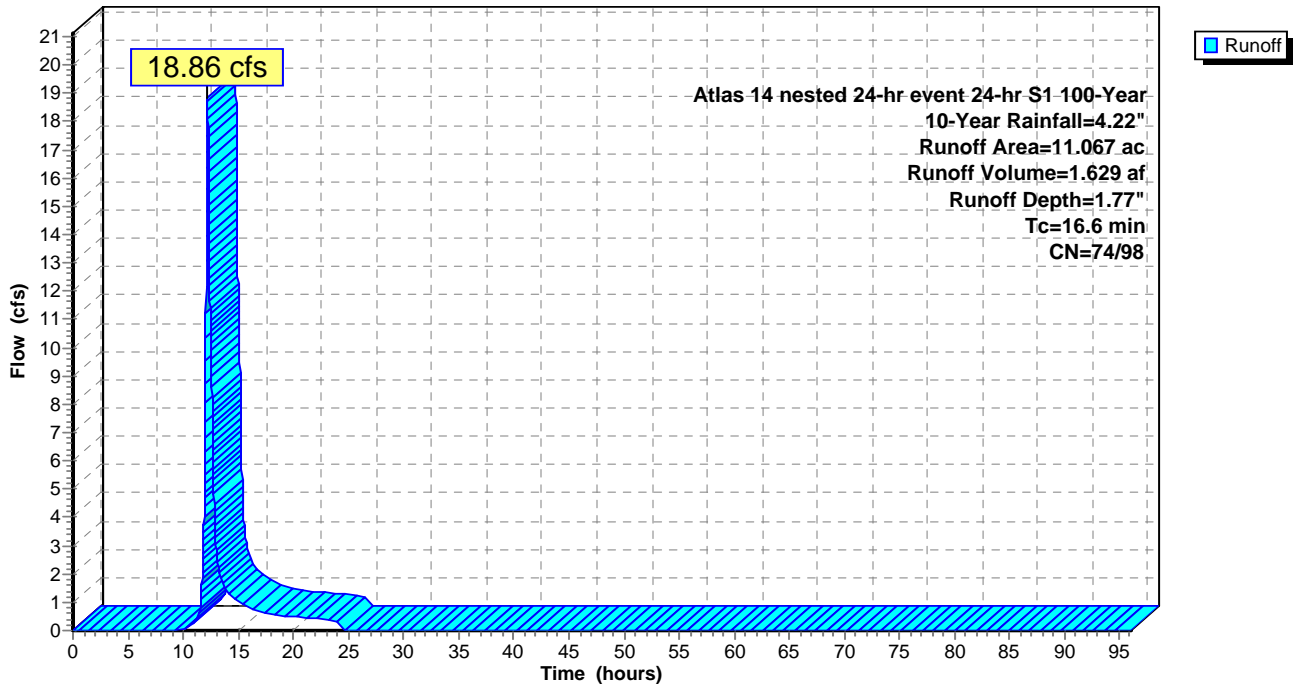
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 11.030	74	pervious
* 0.037	98	impervious
11.067	74	Weighted Average
11.030		99.67% Pervious Area
0.037		0.33% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.6					Direct Entry,

Subcatchment SB 2: SB 2

Hydrograph



Summary for Subcatchment SB 22: SB 22

Runoff = 5.43 cfs @ 12.81 hrs, Volume= 1.272 af, Depth= 0.36"

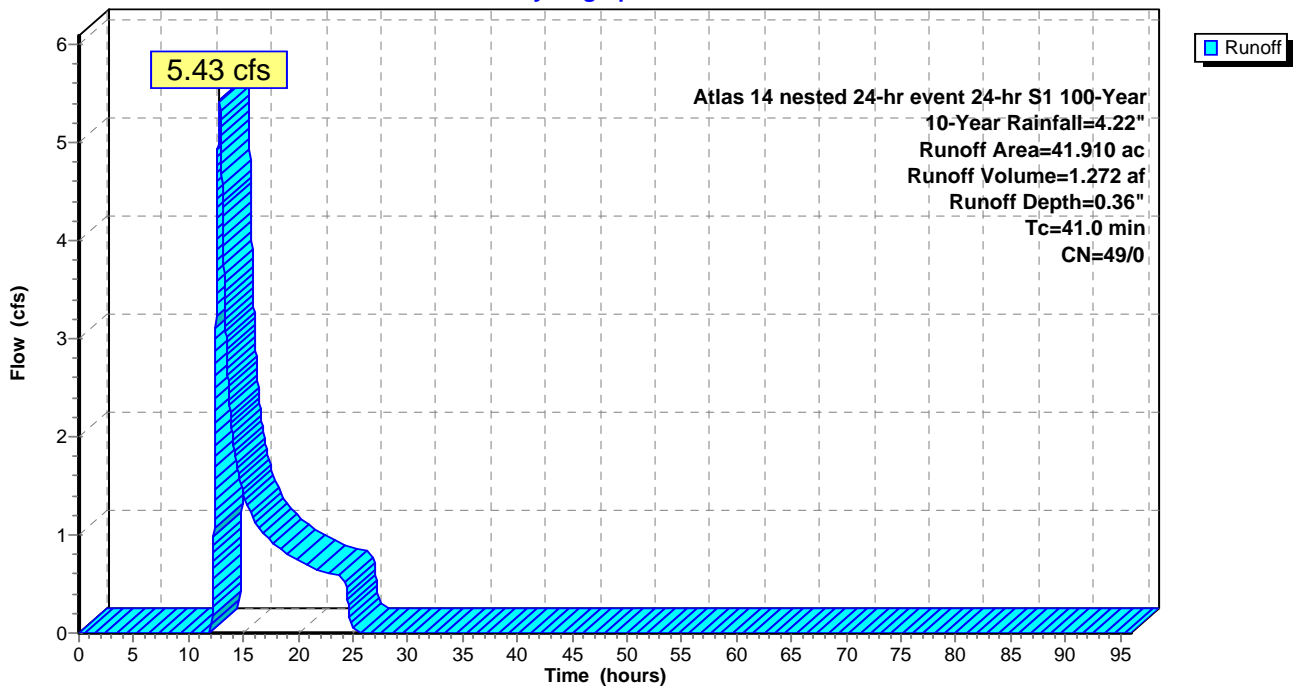
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 41.910	49	Pervious
* 0.000	98	Impervious
41.910	49	Weighted Average
41.910		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
41.0					Direct Entry,

Subcatchment SB 22: SB 22

Hydrograph



Summary for Subcatchment SB 24: SB 24

Runoff = 24.79 cfs @ 12.05 hrs, Volume= 1.651 af, Depth= 3.93"

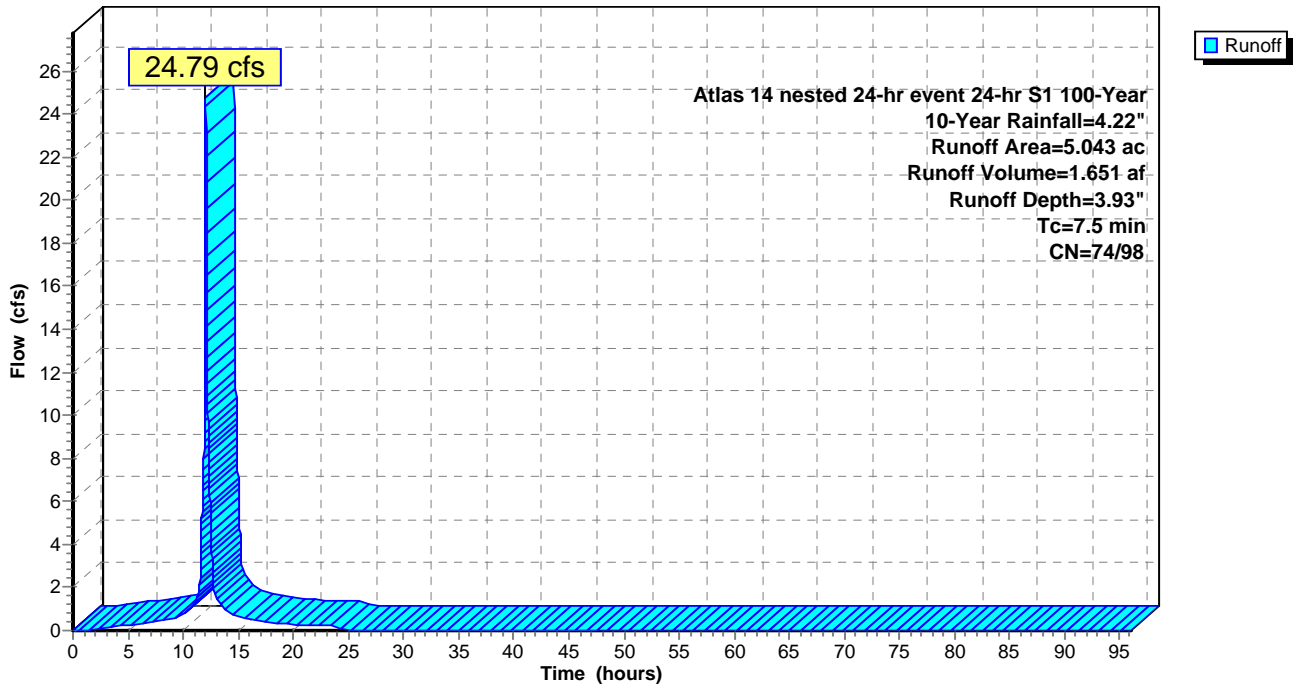
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 0.123	74	permiabile
* 4.920	98	impermiabile
5.043	97	Weighted Average
0.123		2.44% Pervious Area
4.920		97.56% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.5					Direct Entry,

Subcatchment SB 24: SB 24

Hydrograph



Summary for Subcatchment SB 25: SB 25

Runoff = 21.69 cfs @ 12.09 hrs, Volume= 1.664 af, Depth= 3.89"

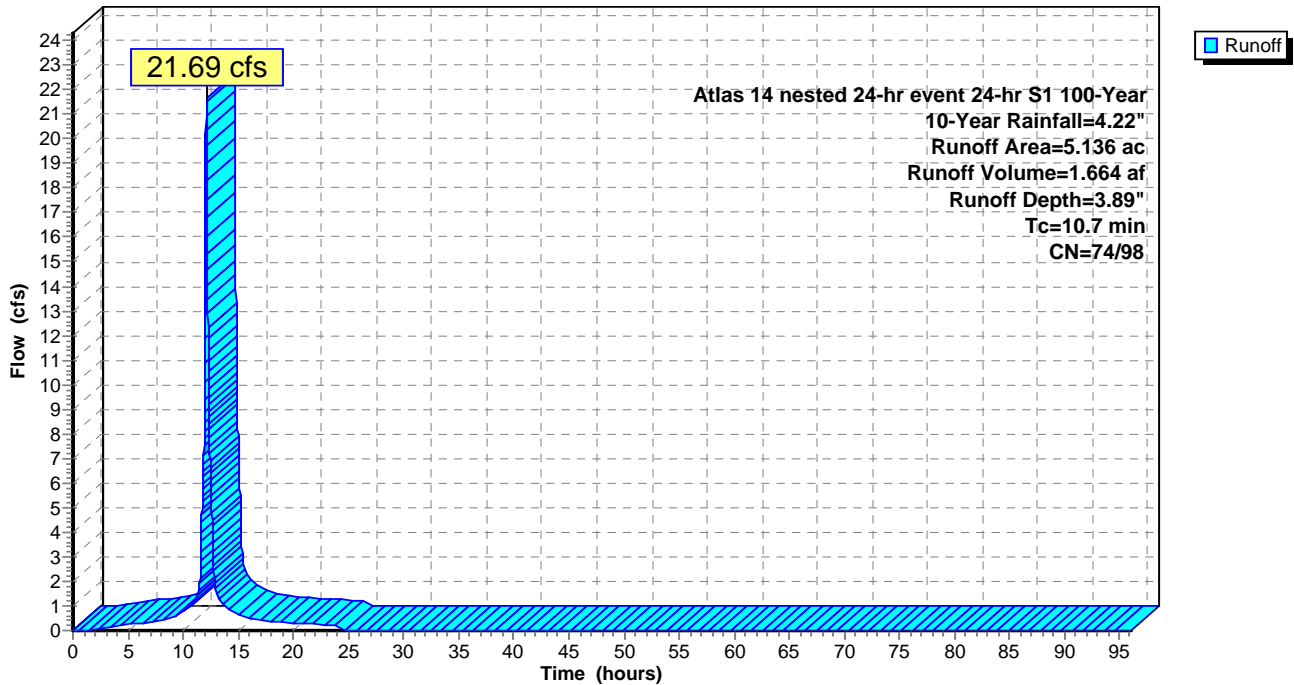
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 0.220	74	pervious
* 4.916	98	impervious
5.136	97	Weighted Average
0.220		4.28% Pervious Area
4.916		95.72% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7					Direct Entry,

Subcatchment SB 25: SB 25

Hydrograph



Summary for Subcatchment SB 26: SB 26

Runoff = 41.75 cfs @ 12.28 hrs, Volume= 4.713 af, Depth= 3.95"

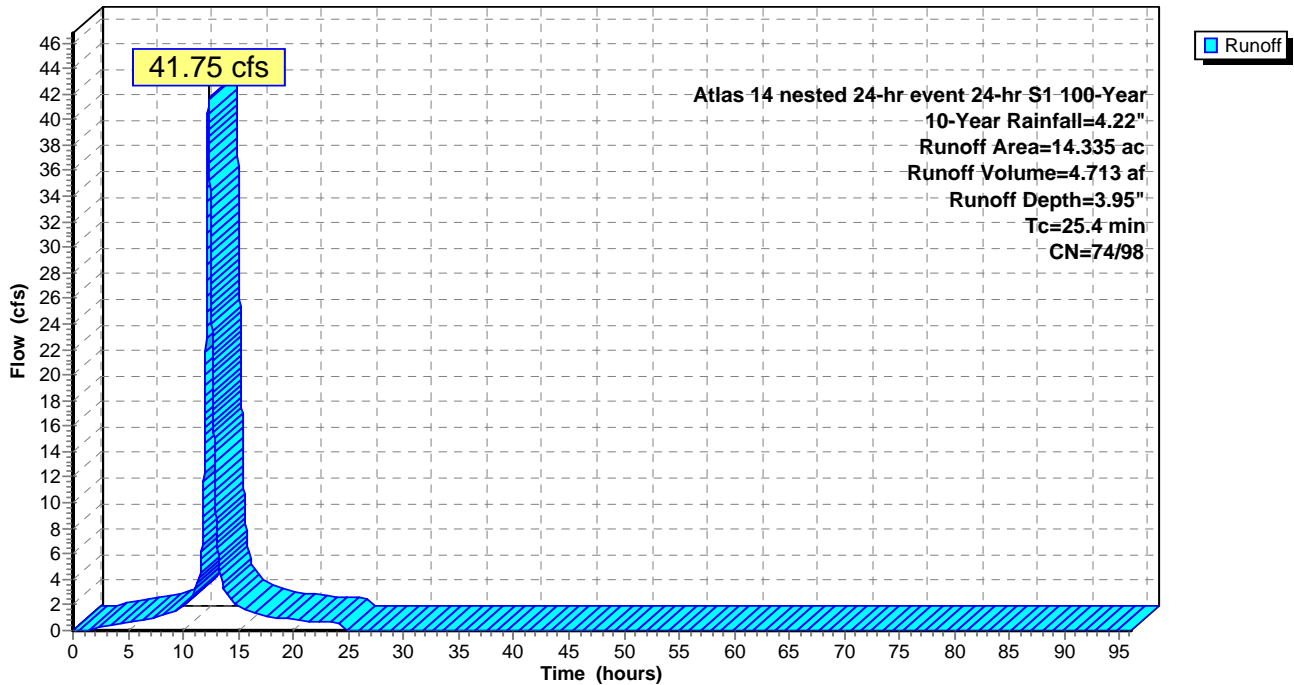
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 0.248	74	pervious
* 14.087	98	impervious
14.335	98	Weighted Average
0.248		1.73% Pervious Area
14.087		98.27% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
25.4					Direct Entry,

Subcatchment SB 26: SB 26

Hydrograph



Summary for Subcatchment SB 27: SB 27 (Thumb Road)

Runoff = 17.05 cfs @ 12.32 hrs, Volume= 1.996 af, Depth= 3.61"

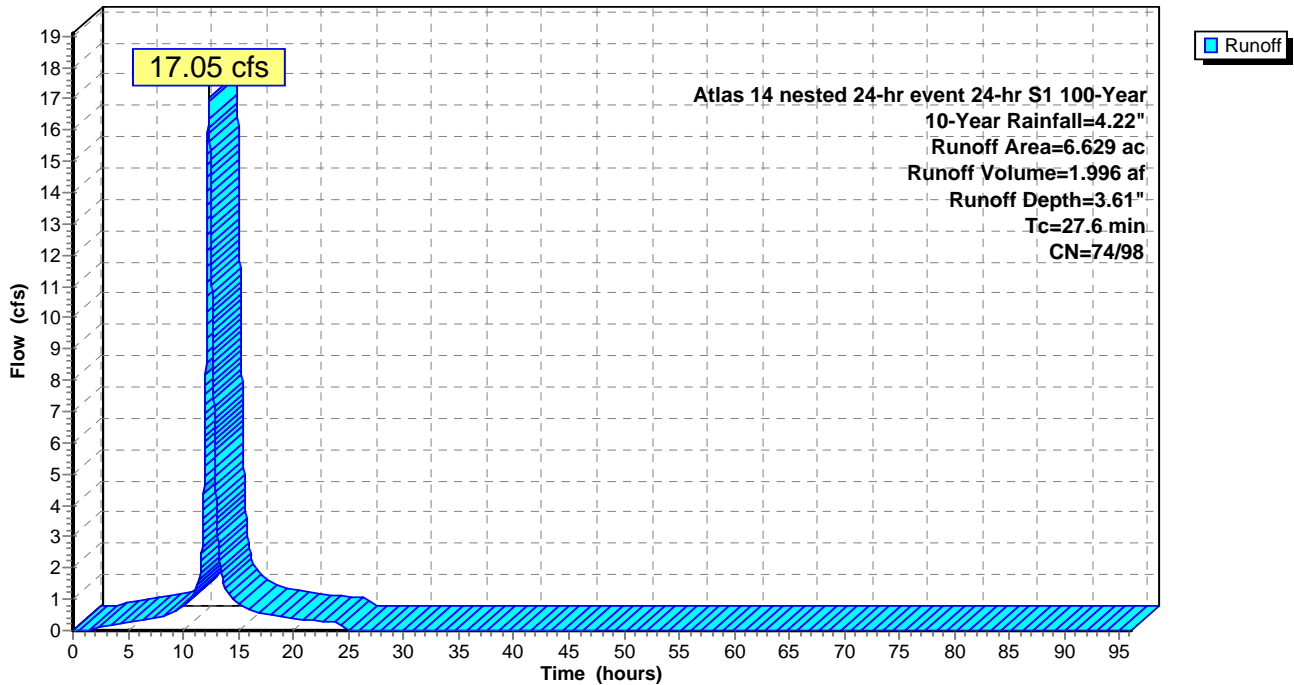
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 1.105	74	Pervious
* 5.524	98	Impervious
6.629	94	Weighted Average
1.105		16.67% Pervious Area
5.524		83.33% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.6					Direct Entry,

Subcatchment SB 27: SB 27 (Thumb Road)

Hydrograph



Summary for Subcatchment SB 28: SB 28

Runoff = 18.97 cfs @ 12.15 hrs, Volume= 1.622 af, Depth= 2.80"

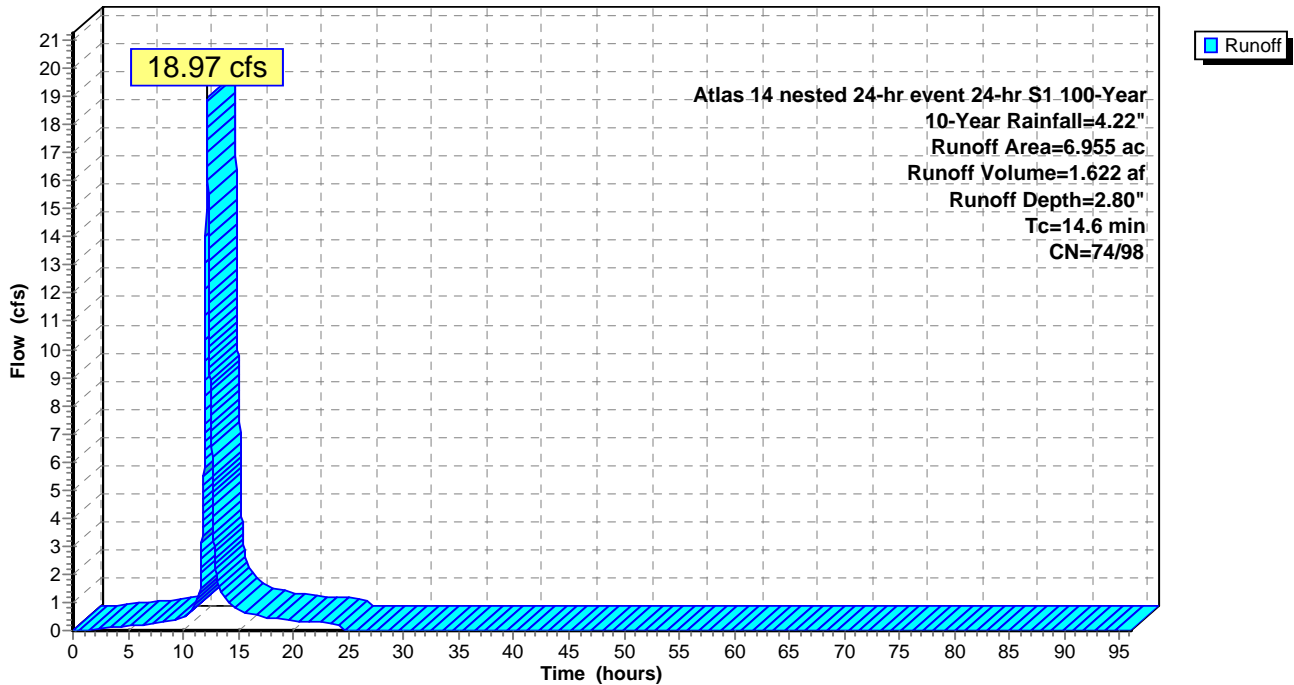
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 3.703	74	pervious
* 3.252	98	impervious
6.955	85	Weighted Average
3.703		53.24% Pervious Area
3.252		46.76% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.6					Direct Entry,

Subcatchment SB 28: SB 28

Hydrograph



Summary for Subcatchment SB 29: SB 29

Runoff = 23.01 cfs @ 12.22 hrs, Volume= 2.212 af, Depth= 2.60"

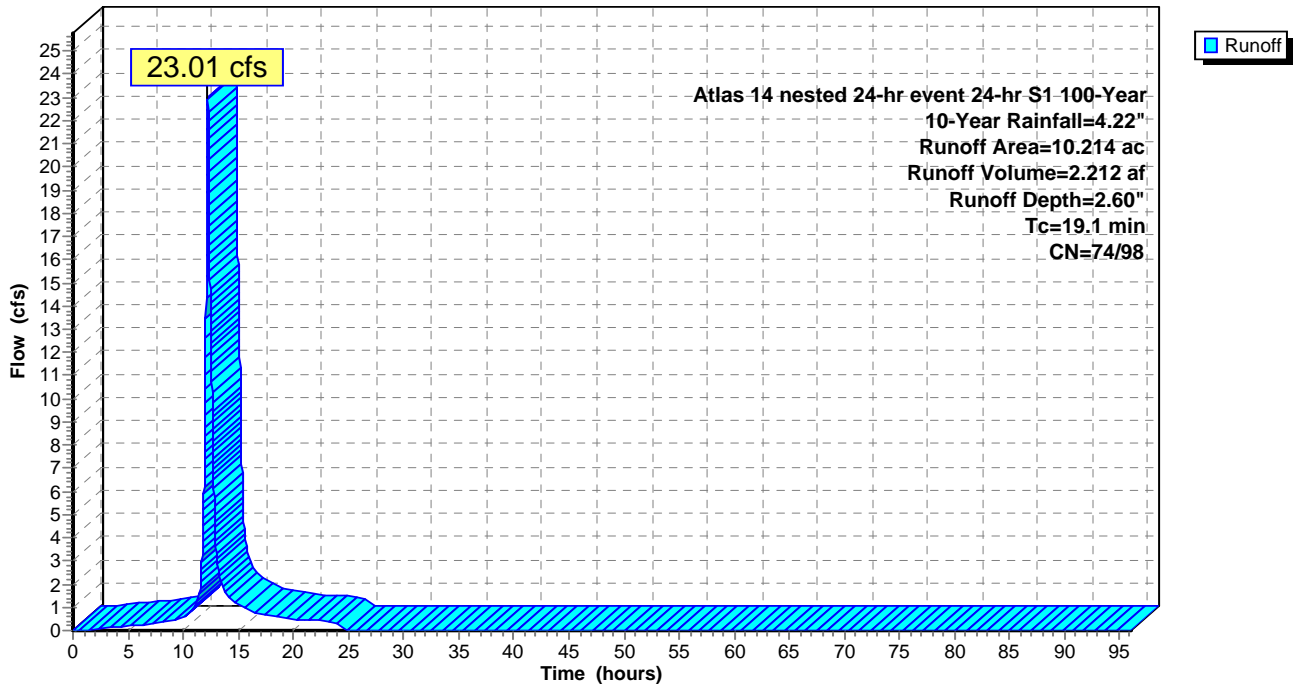
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 6.360	74	pervious
* 3.854	98	impervious
10.214	83	Weighted Average
6.360		62.27% Pervious Area
3.854		37.73% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.1					Direct Entry,

Subcatchment SB 29: SB 29

Hydrograph



Summary for Subcatchment SB 3: SB 3

Runoff = 71.94 cfs @ 12.17 hrs, Volume= 6.049 af, Depth= 1.93"

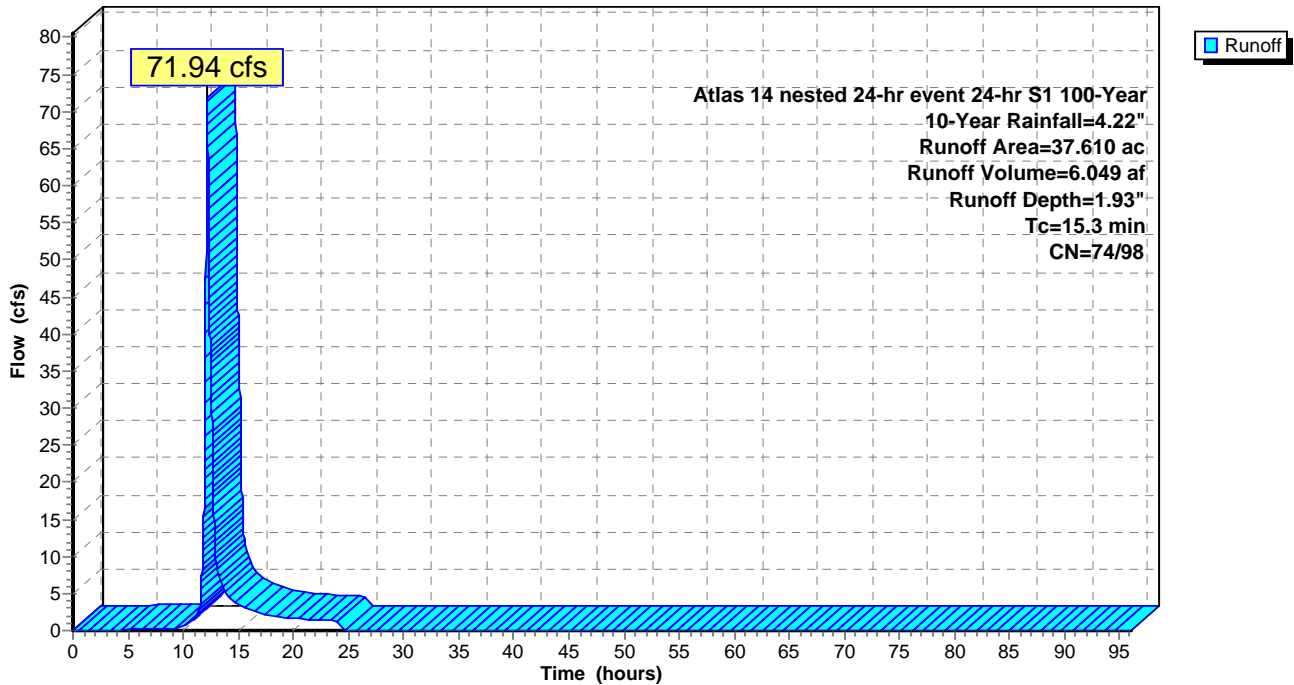
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 34.720	74	Pervious
* 2.890	98	Impervious
37.610	76	Weighted Average
34.720		92.32% Pervious Area
2.890		7.68% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.3					Direct Entry,

Subcatchment SB 3: SB 3

Hydrograph



Summary for Subcatchment SB 4: SB 4

Runoff = 2.26 cfs @ 12.04 hrs, Volume= 0.141 af, Depth= 2.83"

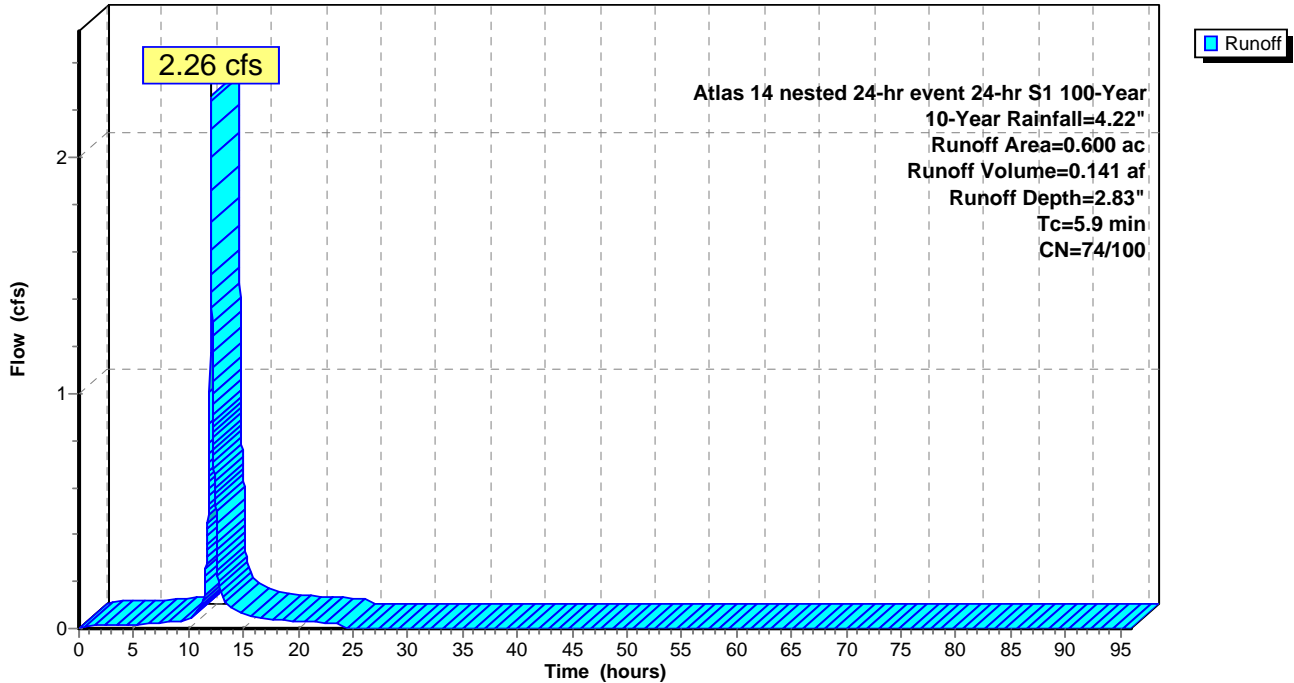
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 0.340	74	pervious
* 0.260	100	impervious
0.600	85	Weighted Average
0.340		56.67% Pervious Area
0.260		43.33% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.9					Direct Entry,

Subcatchment SB 4: SB 4

Hydrograph



Summary for Subcatchment SB 5: SB 5

Runoff = 7.43 cfs @ 12.84 hrs, Volume= 1.239 af, Depth= 1.89"

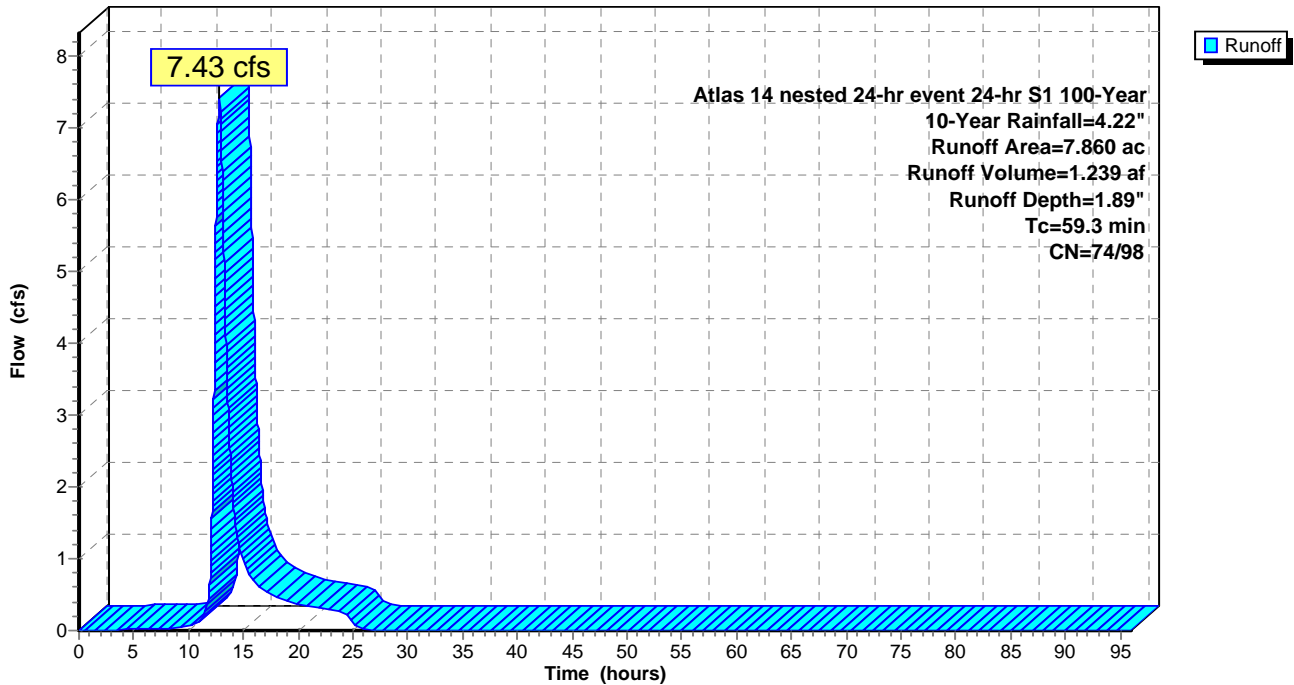
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 7.390	74	pervious
* 0.470	98	impervious
7.860	75	Weighted Average
7.390		94.02% Pervious Area
0.470		5.98% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
59.3					Direct Entry,

Subcatchment SB 5: SB 5

Hydrograph



Summary for Subcatchment SB 6: SB 6

Runoff = 1.72 cfs @ 12.25 hrs, Volume= 0.167 af, Depth= 2.01"

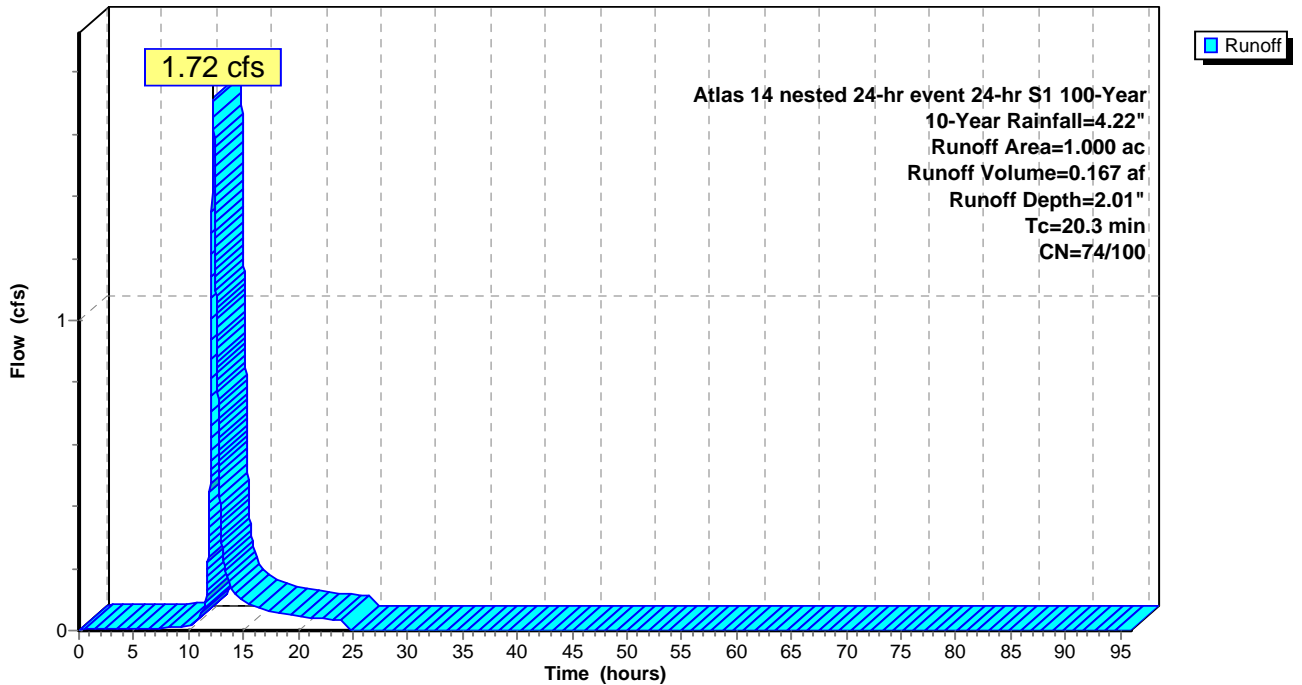
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 0.900	74	pervious
* 0.100	100	impervious
1.000	77	Weighted Average
0.900		90.00% Pervious Area
0.100		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.3					Direct Entry,

Subcatchment SB 6: SB 6

Hydrograph



Summary for Subcatchment SB 7: SB 7

Runoff = 55.54 cfs @ 12.04 hrs, Volume= 3.159 af, Depth= 1.76"

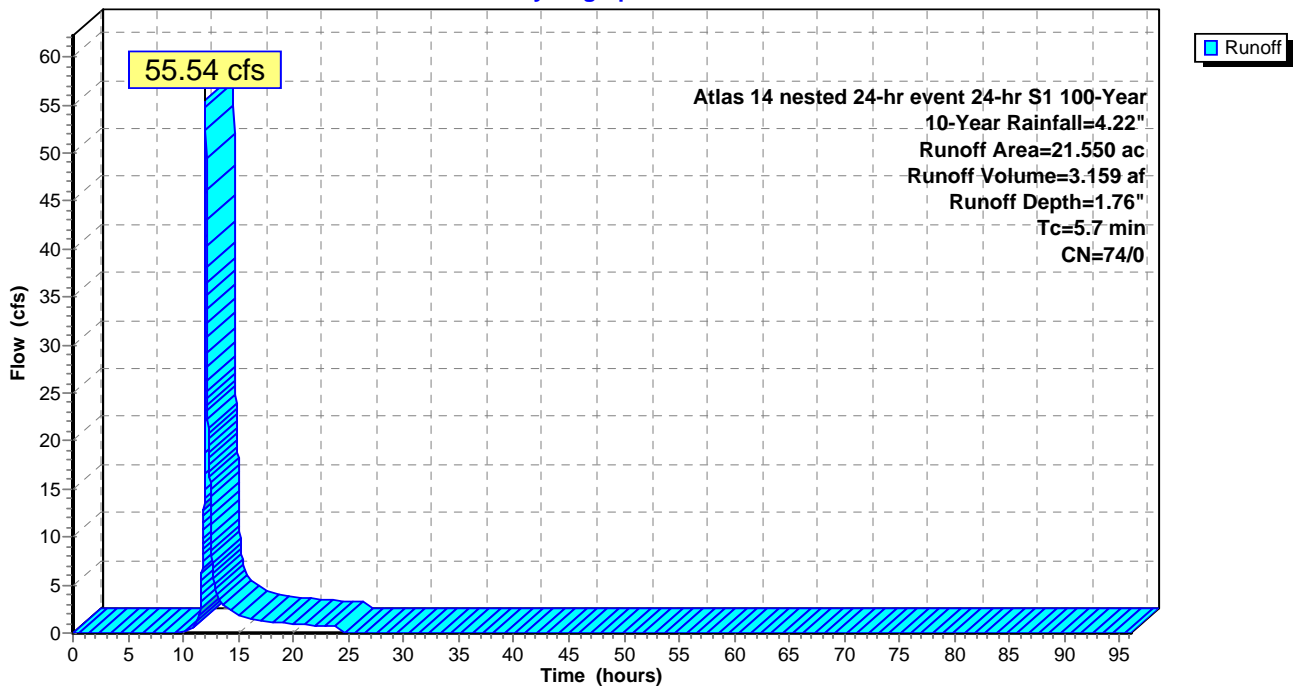
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 21.550	74	pervious
* 0.000	98	impervious
21.550	74	Weighted Average
21.550		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.7					Direct Entry,

Subcatchment SB 7: SB 7

Hydrograph



Summary for Subcatchment SB 8: SB 8

Runoff = 31.84 cfs @ 12.62 hrs, Volume= 4.638 af, Depth= 1.88"

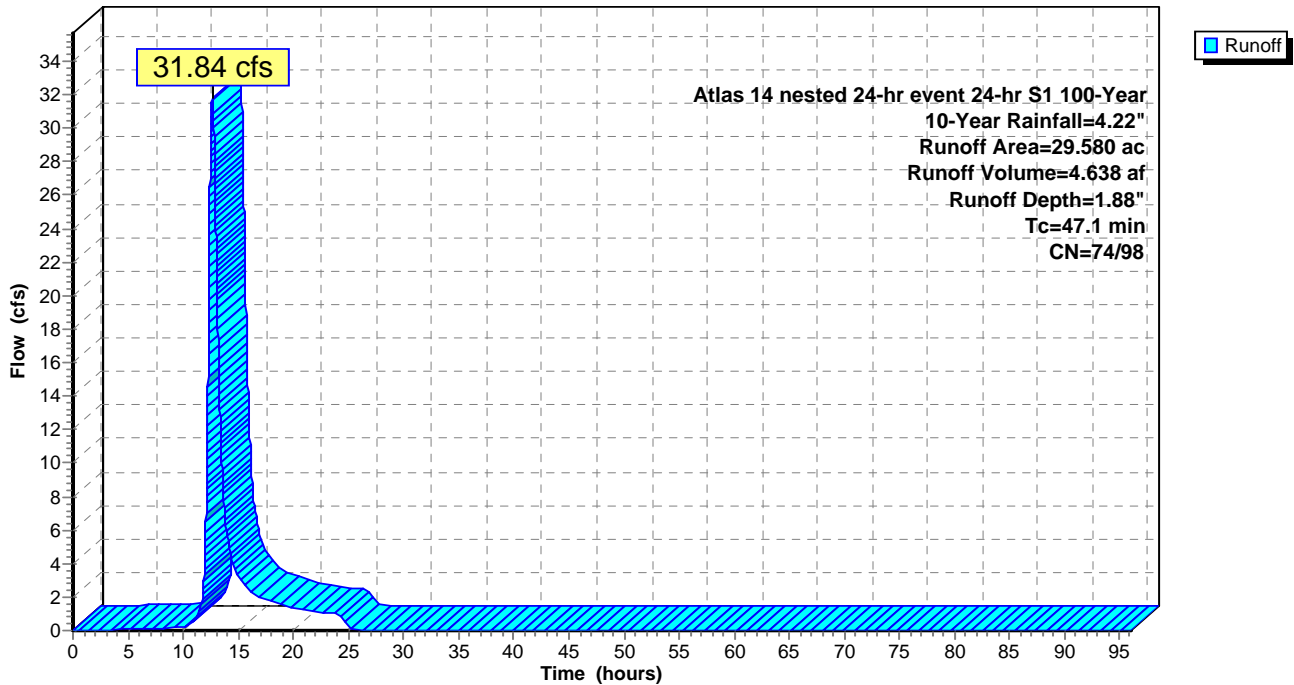
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 27.950	74	pervious
* 1.630	98	impervious
29.580	75	Weighted Average
27.950		94.49% Pervious Area
1.630		5.51% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
47.1					Direct Entry,

Subcatchment SB 8: SB 8

Hydrograph



Summary for Subcatchment SB 9: SB 9

Runoff = 33.02 cfs @ 12.40 hrs, Volume= 3.784 af, Depth= 1.76"

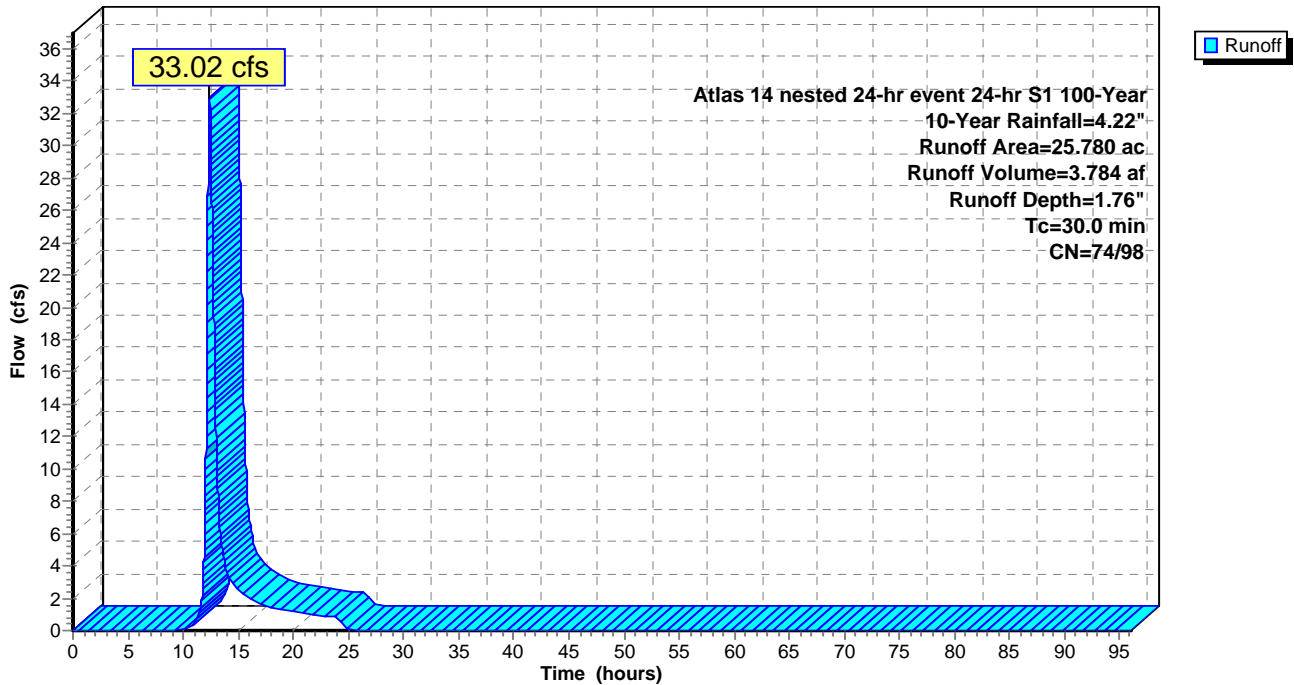
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 25.750	74	permiabile
* 0.030	98	impermiabile
25.780	74	Weighted Average
25.750		99.88% Pervious Area
0.030		0.12% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.0					Direct Entry,

Subcatchment SB 9: SB 9

Hydrograph



Summary for Subcatchment SB10: SB 10

Runoff = 15.99 cfs @ 12.06 hrs, Volume= 0.994 af, Depth= 1.87"

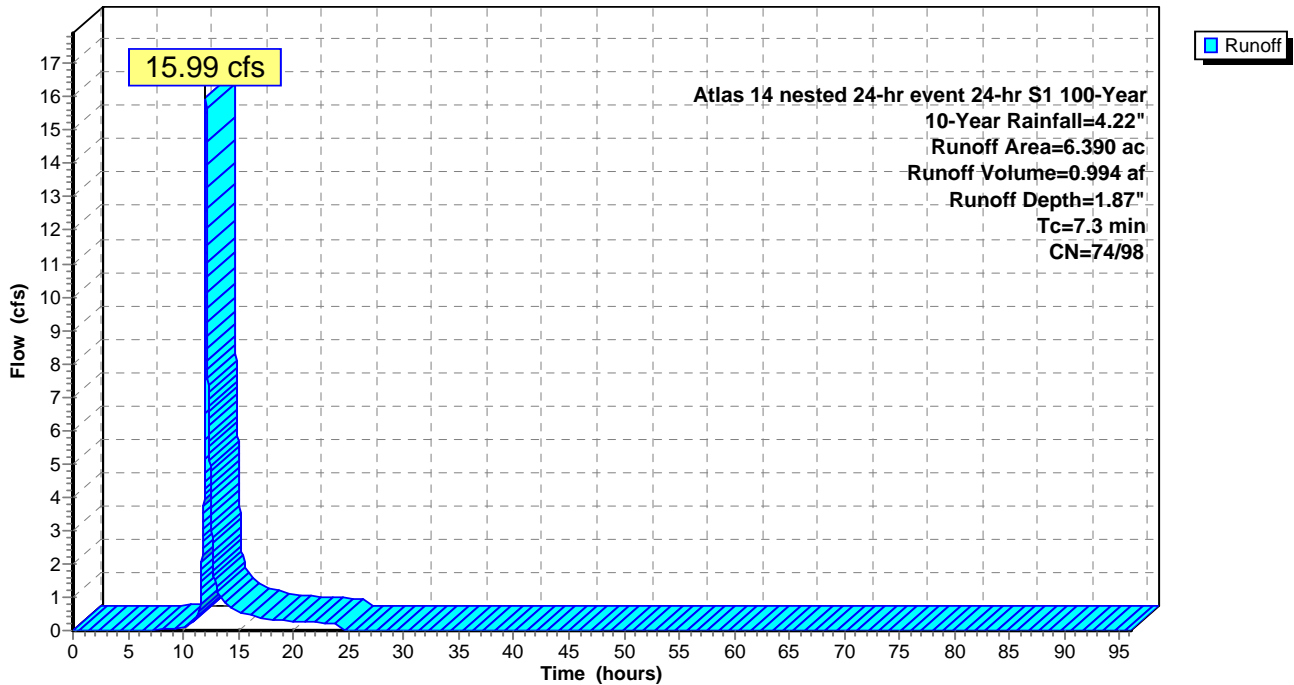
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 6.080	74	pervious
* 0.310	98	impervious
6.390	75	Weighted Average
6.080		95.15% Pervious Area
0.310		4.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.3					Direct Entry,

Subcatchment SB10: SB 10

Hydrograph



Summary for Pond 2 P: P-2

Inflow Area = 68.260 ac, 7.26% Impervious, Inflow Depth = 1.92" for 10-Year event
 Inflow = 59.33 cfs @ 12.57 hrs, Volume= 10.925 af
 Outflow = 59.01 cfs @ 12.64 hrs, Volume= 10.925 af, Atten= 1%, Lag= 3.9 min
 Primary = 59.01 cfs @ 12.64 hrs, Volume= 10.925 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 924.00' Surf.Area= 0.370 ac Storage= 0.730 af
 Peak Elev= 924.98' @ 12.64 hrs Surf.Area= 0.434 ac Storage= 1.125 af (0.395 af above start)

Plug-Flow detention time= 73.9 min calculated for 10.195 af (93% of inflow)
 Center-of-Mass det. time= 21.2 min (883.0 - 861.8)

Volume	Invert	Avail.Storage	Storage Description
#1	920.00'	1.600 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
920.00	0.100	0.000	0.000
922.00	0.130	0.230	0.230
924.00	0.370	0.500	0.730
926.00	0.500	0.870	1.600

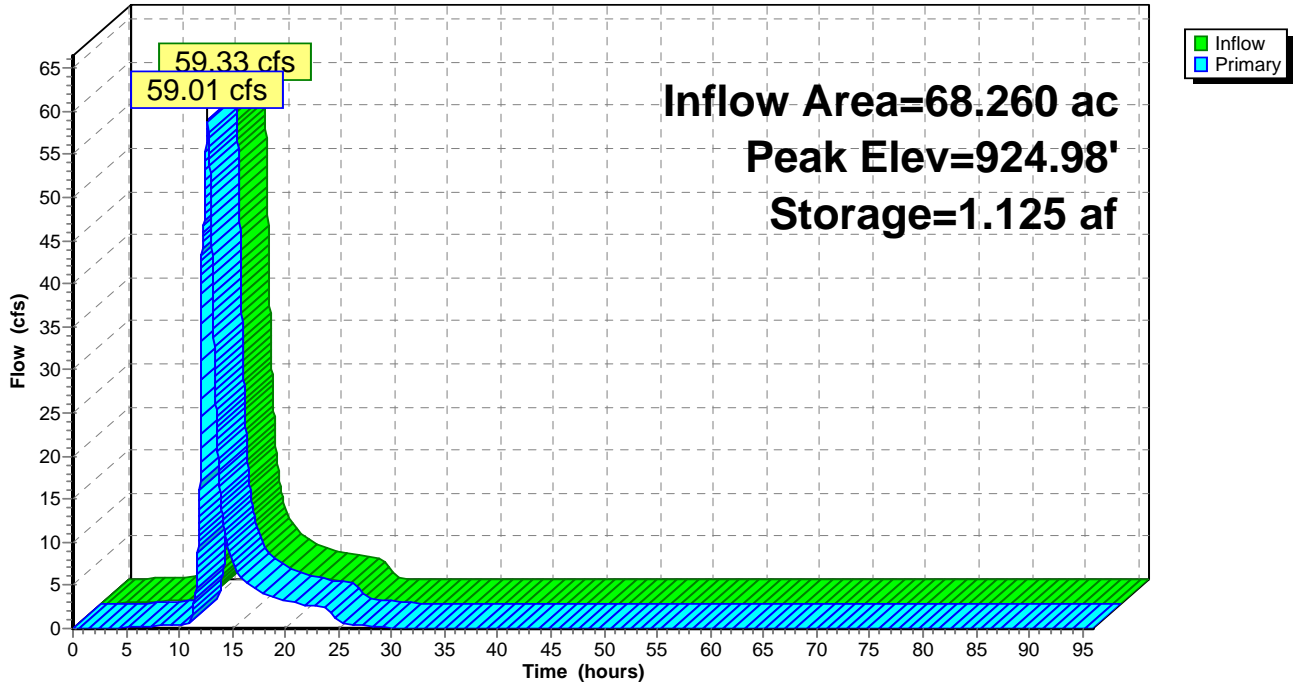
Device	Routing	Invert	Outlet Devices
#1	Primary	924.40'	40.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Primary	924.00'	6.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=59.01 cfs @ 12.64 hrs HW=924.98' TW=0.00' (Dynamic Tailwater)

- 1=Sharp-Crested Rectangular Weir (Weir Controls 58.07 cfs @ 2.50 fps)
- 2=Orifice/Grate (Orifice Controls 0.94 cfs @ 4.77 fps)

Pond 2 P: P-2

Hydrograph



Summary for Pond 4P: P-4

Inflow Area = 7.860 ac, 5.98% Impervious, Inflow Depth = 1.89" for 10-Year event
 Inflow = 7.43 cfs @ 12.84 hrs, Volume= 1.239 af
 Outflow = 3.61 cfs @ 13.47 hrs, Volume= 1.239 af, Atten= 51%, Lag= 38.2 min
 Primary = 1.35 cfs @ 13.50 hrs, Volume= 0.436 af
 Secondary = 2.26 cfs @ 13.42 hrs, Volume= 0.803 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 915.00' Surf.Area= 0.275 ac Storage= 0.646 af
 Peak Elev= 916.17' @ 13.50 hrs Surf.Area= 0.340 ac Storage= 1.005 af (0.359 af above start)

Plug-Flow detention time= 335.5 min calculated for 0.594 af (48% of inflow)
 Center-of-Mass det. time= 60.0 min (941.2 - 881.1)

Volume	Invert	Avail.Storage	Storage Description
#1	910.90'	1.728 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
910.90	0.070	0.000	0.000
912.00	0.090	0.088	0.088
914.00	0.220	0.310	0.398
916.00	0.330	0.550	0.948
918.00	0.450	0.780	1.728

Device	Routing	Invert	Outlet Devices
#1	Primary	915.00'	6.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Secondary	915.00'	9.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	915.95'	24.0" Round RCP_Round 24" L= 50.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 915.80' / 915.95' S= -0.0030 ' /' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Primary OutFlow Max=1.35 cfs @ 13.50 hrs HW=916.17' TW=0.00' (Dynamic Tailwater)

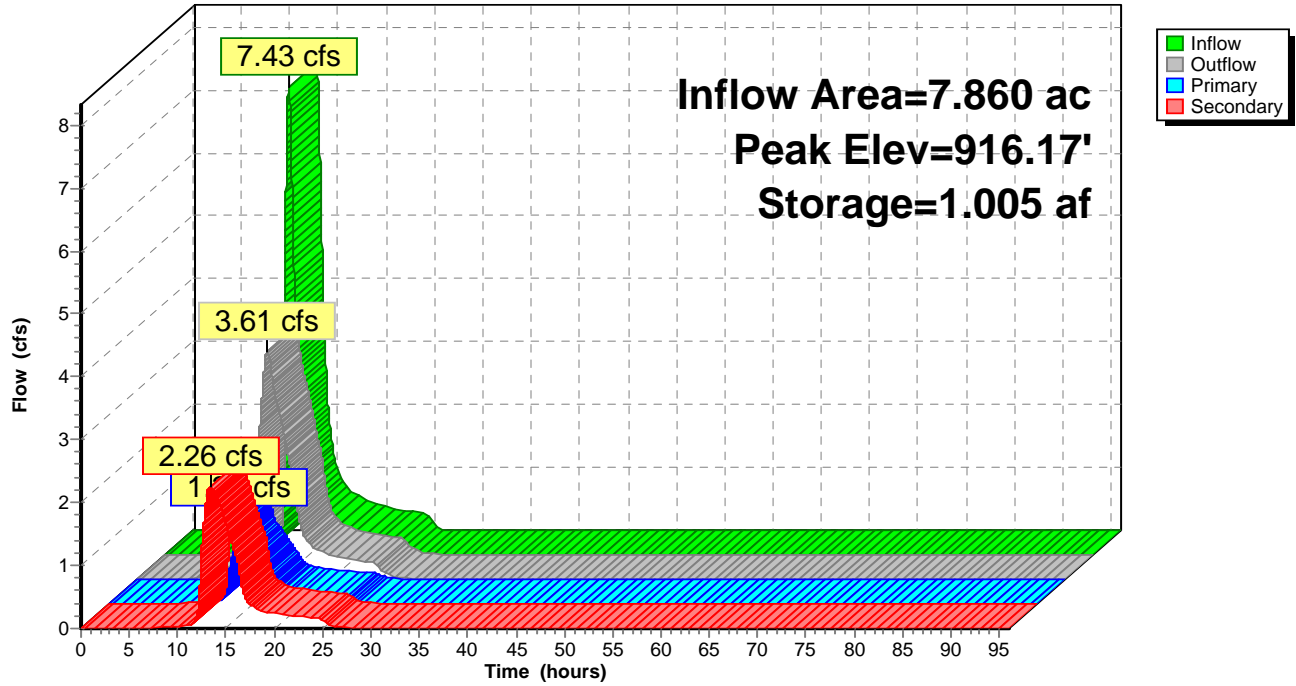
- ↑1=Orifice/Grate (Orifice Controls 1.02 cfs @ 5.21 fps)
- ↑3=RCP_Round 24" (Barrel Controls 0.33 cfs @ 1.25 fps)

Secondary OutFlow Max=2.26 cfs @ 13.42 hrs HW=916.17' TW=915.04' (Dynamic Tailwater)

- ↑2=Orifice/Grate (Orifice Controls 2.26 cfs @ 5.12 fps)

Pond 4P: P-4

Hydrograph



Summary for Pond 7P: P-7

[90] Warning: Qout>Qin may require smaller dt or Finer Routing

Inflow Area = 29.580 ac, 5.51% Impervious, Inflow Depth = 1.88" for 10-Year event
 Inflow = 31.84 cfs @ 12.62 hrs, Volume= 4.638 af
 Outflow = 32.31 cfs @ 12.68 hrs, Volume= 4.638 af, Atten= 0%, Lag= 3.8 min
 Primary = 32.31 cfs @ 12.68 hrs, Volume= 4.638 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 915.00' Surf.Area= 0.440 ac Storage= 1.062 af
 Peak Elev= 915.49' @ 12.62 hrs Surf.Area= 0.498 ac Storage= 1.291 af (0.228 af above start)

Plug-Flow detention time= 146.3 min calculated for 3.576 af (77% of inflow)
 Center-of-Mass det. time= 9.4 min (880.2 - 870.8)

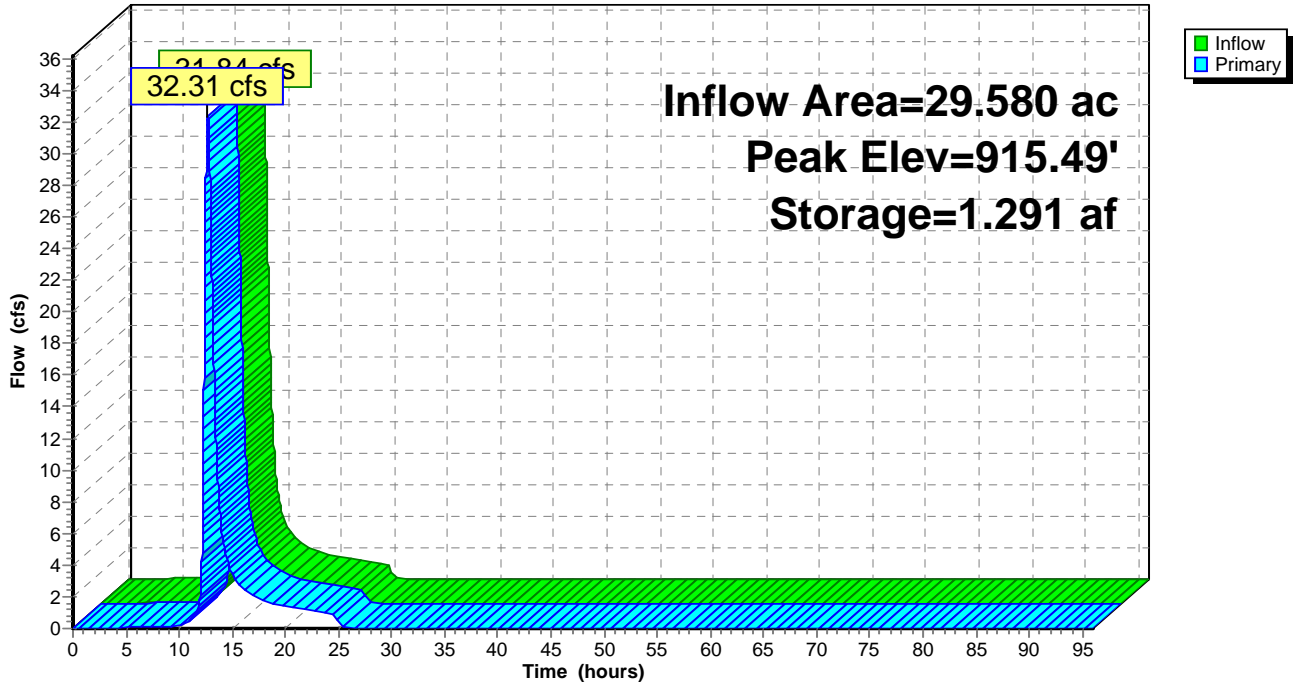
Volume	Invert	Avail.Storage	Storage Description
#1	910.95'	1.562 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
910.95	0.110	0.000	0.000
912.00	0.180	0.152	0.152
914.00	0.340	0.520	0.672
915.00	0.440	0.390	1.062
916.00	0.560	0.500	1.562

Device	Routing	Invert	Outlet Devices
#1	Primary	915.00'	75.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=32.64 cfs @ 12.68 hrs HW=915.48' TW=915.43' (Dynamic Tailwater)
 ↑**1=Broad-Crested Rectangular Weir** (Weir Controls 32.64 cfs @ 0.90 fps)

Pond 7P: P-7

Hydrograph



Summary for Pond 8P: P-8

Inflow Area = 6.390 ac, 4.85% Impervious, Inflow Depth = 1.87" for 10-Year event
 Inflow = 15.99 cfs @ 12.06 hrs, Volume= 0.994 af
 Outflow = 3.70 cfs @ 12.47 hrs, Volume= 0.989 af, Atten= 77%, Lag= 24.8 min
 Primary = 3.70 cfs @ 12.47 hrs, Volume= 0.989 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 897.00' Surf.Area= 0.300 ac Storage= 0.495 af
 Peak Elev= 898.05' @ 12.58 hrs Surf.Area= 0.452 ac Storage= 0.895 af (0.400 af above start)

Plug-Flow detention time= 544.3 min calculated for 0.494 af (50% of inflow)
 Center-of-Mass det. time= 189.5 min (1,024.6 - 835.1)

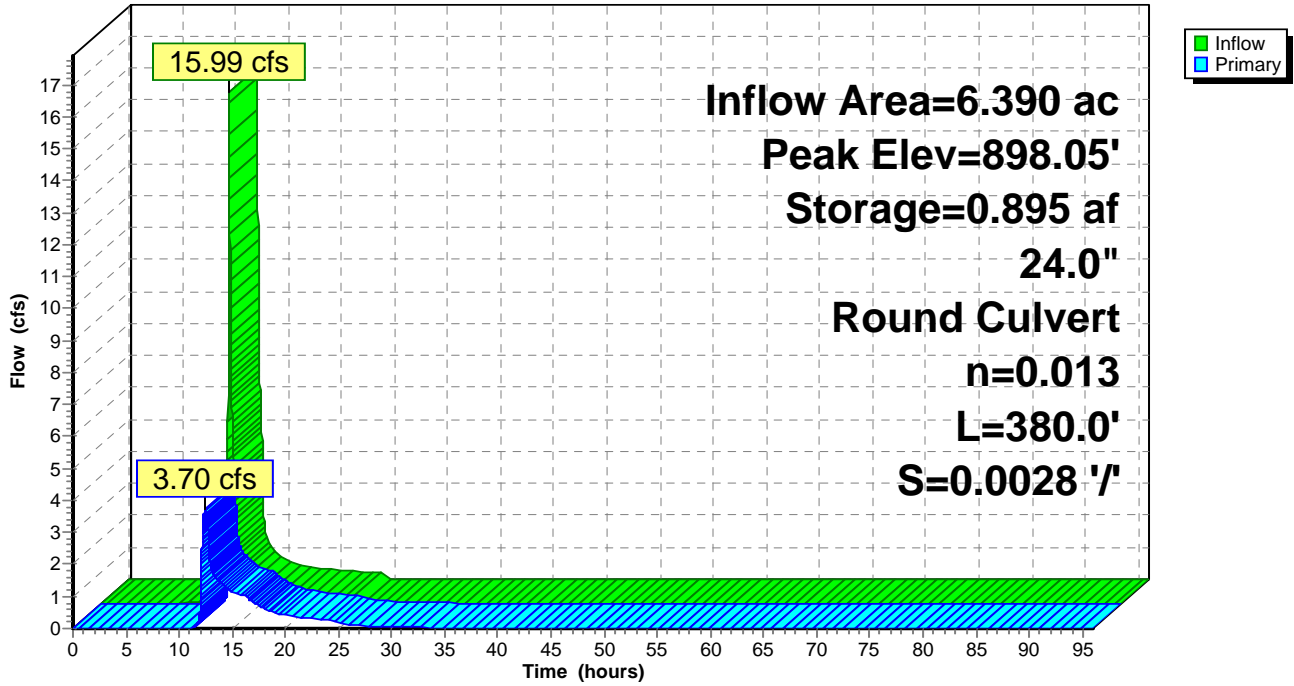
Volume	Invert	Avail.Storage	Storage Description
#1	893.00'	1.850 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
893.00	0.030	0.000	0.000
894.00	0.070	0.050	0.050
896.00	0.150	0.220	0.270
897.00	0.300	0.225	0.495
898.00	0.450	0.375	0.870
900.00	0.530	0.980	1.850

Device	Routing	Invert	Outlet Devices
#1	Primary	897.00'	24.0" Round RCP_Round 24" L= 380.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 897.00' / 895.94' S= 0.0028 1/'' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Primary OutFlow Max=3.68 cfs @ 12.47 hrs HW=898.04' TW=896.86' (Dynamic Tailwater)
 ↑1=RCP_Round 24" (Outlet Controls 3.68 cfs @ 3.25 fps)

Pond 8P: P-8

Hydrograph



Summary for Pond 9P: P-9

Inflow Area = 55.360 ac, 3.00% Impervious, Inflow Depth = 1.83" for 10-Year event
 Inflow = 58.97 cfs @ 12.53 hrs, Volume= 8.423 af
 Outflow = 58.93 cfs @ 12.55 hrs, Volume= 8.423 af, Atten= 0%, Lag= 1.2 min
 Primary = 58.93 cfs @ 12.55 hrs, Volume= 8.423 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 915.00' Surf.Area= 0.210 ac Storage= 0.353 af
 Peak Elev= 915.44' @ 12.55 hrs Surf.Area= 0.298 ac Storage= 0.464 af (0.111 af above start)

Plug-Flow detention time= 35.5 min calculated for 8.069 af (96% of inflow)
 Center-of-Mass det. time= 2.3 min (875.9 - 873.6)

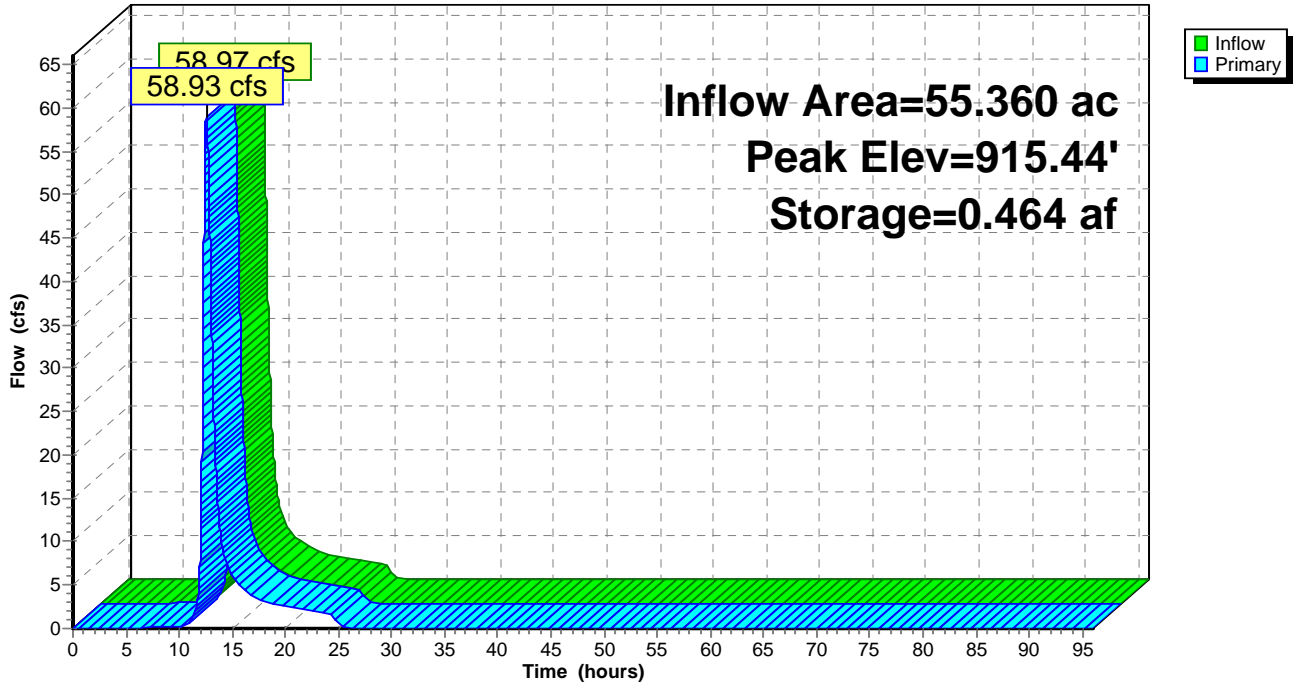
Volume	Invert	Avail.Storage	Storage Description
#1	910.50'	1.673 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
910.50	0.020	0.000	0.000
912.00	0.050	0.052	0.052
913.00	0.070	0.060	0.112
914.00	0.100	0.085	0.198
915.00	0.210	0.155	0.353
916.00	0.410	0.310	0.662
918.00	0.600	1.010	1.673

Device	Routing	Invert	Outlet Devices
#1	Primary	915.00'	80.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=58.93 cfs @ 12.55 hrs HW=915.44' TW=910.57' (Dynamic Tailwater)
 ↑**1=Broad-Crested Rectangular Weir** (Weir Controls 58.93 cfs @ 1.68 fps)

Pond 9P: P-9

Hydrograph



Summary for Pond 10P: P-10 Lowered 1 ft

[95] Warning: Outlet Device #1 rise exceeded

Inflow Area = 66.430 ac, 5.22% Impervious, Inflow Depth = 1.48" for 10-Year event
 Inflow = 25.80 cfs @ 13.18 hrs, Volume= 8.191 af
 Outflow = 25.74 cfs @ 13.23 hrs, Volume= 8.190 af, Atten= 0%, Lag= 3.0 min
 Primary = 11.94 cfs @ 13.23 hrs, Volume= 6.845 af
 Secondary = 13.80 cfs @ 13.23 hrs, Volume= 1.346 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 896.00' Surf.Area= 0.290 ac Storage= 0.700 af
 Peak Elev= 897.64' @ 13.23 hrs Surf.Area= 0.359 ac Storage= 1.230 af (0.530 af above start)

Plug-Flow detention time= 120.2 min calculated for 7.490 af (91% of inflow)
 Center-of-Mass det. time= 30.6 min (1,024.6 - 994.0)

Volume	Invert	Avail.Storage	Storage Description
#1	892.00'	1.760 af	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
892.00	0.120	0.000	0.000
893.00	0.140	0.130	0.130
895.00	0.190	0.330	0.460
896.00	0.290	0.240	0.700
897.00	0.330	0.310	1.010
899.00	0.420	0.750	1.760

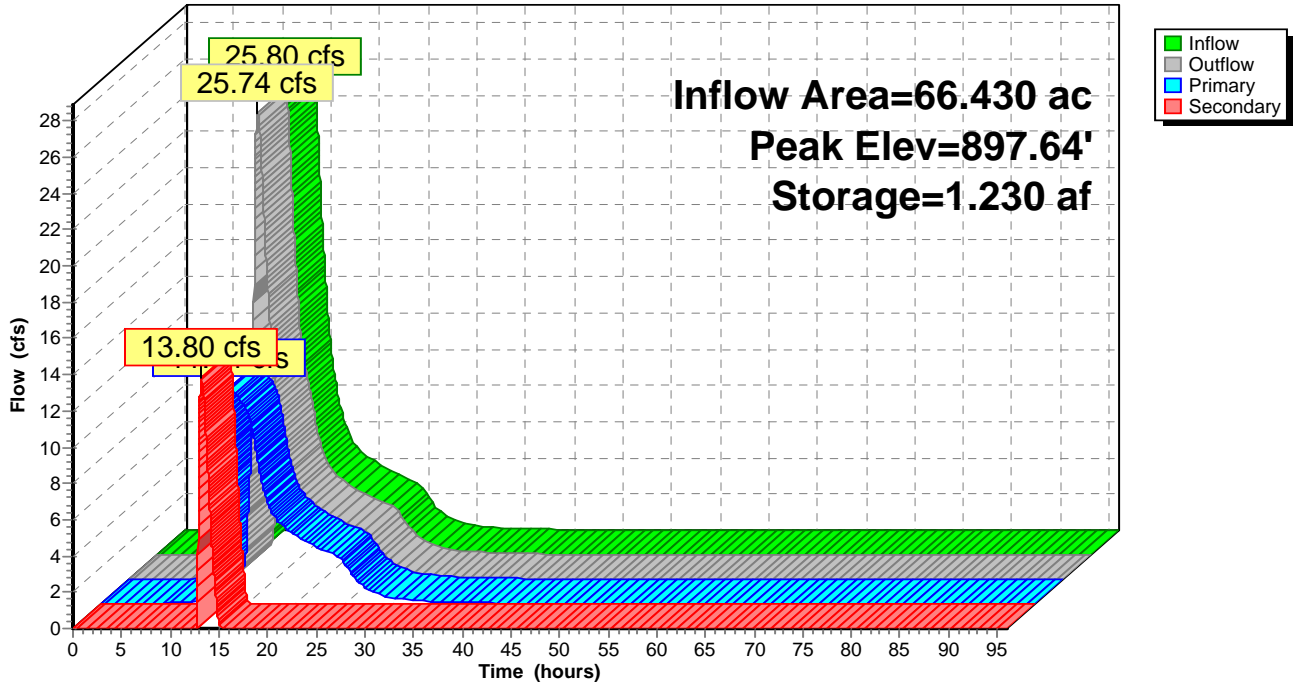
Device	Routing	Invert	Outlet Devices
#1	Primary	896.00'	2.5' long x 1.00' rise Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Secondary	897.40'	50.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=11.94 cfs @ 13.23 hrs HW=897.64' TW=893.98' (Dynamic Tailwater)
 ↖1=Sharp-Crested Rectangular Weir (Orifice Controls 11.94 cfs @ 5.19 fps)

Secondary OutFlow Max=13.80 cfs @ 13.23 hrs HW=897.64' TW=893.98' (Dynamic Tailwater)
 ↖2=Broad-Crested Rectangular Weir (Weir Controls 13.80 cfs @ 1.16 fps)

Pond 10P: P-10 Lowered 1 ft

Hydrograph



Summary for Pond 11P: P-11

Inflow Area = 58.650 ac, 4.89% Impervious, Inflow Depth = 1.87" for 10-Year event
 Inflow = 61.92 cfs @ 12.53 hrs, Volume= 9.153 af
 Outflow = 27.87 cfs @ 13.20 hrs, Volume= 9.151 af, Atten= 55%, Lag= 39.7 min
 Primary = 23.80 cfs @ 13.20 hrs, Volume= 6.945 af
 Secondary = 4.08 cfs @ 13.20 hrs, Volume= 2.206 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 909.00' Surf.Area= 1.210 ac Storage= 3.640 af
 Peak Elev= 911.47' @ 13.20 hrs Surf.Area= 1.497 ac Storage= 6.979 af (3.339 af above start)

Plug-Flow detention time= 380.0 min calculated for 5.511 af (60% of inflow)
 Center-of-Mass det. time= 127.9 min (996.0 - 868.2)

Volume	Invert	Avail.Storage	Storage Description
#1	905.00'	9.405 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
905.00	0.760	0.000	0.000
906.00	0.820	0.790	0.790
908.00	0.950	1.770	2.560
909.00	1.210	1.080	3.640
910.00	1.320	1.265	4.905
912.00	1.560	2.880	7.785
913.00	1.680	1.620	9.405

Device	Routing	Invert	Outlet Devices
#1	Primary	909.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	910.00'	24.0" Round RCP_Round 24" L= 200.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 910.00' / 909.00' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Primary	910.00'	24.0" Round RCP_Round 24" L= 200.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 910.00' / 909.00' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#4	Primary	912.00'	60.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88
#5	Secondary	909.00'	12.0" Round RCP_Round 12" L= 150.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 909.00' / 908.00' S= 0.0067 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=23.79 cfs @ 13.20 hrs HW=911.47' TW=897.64' (Dynamic Tailwater)

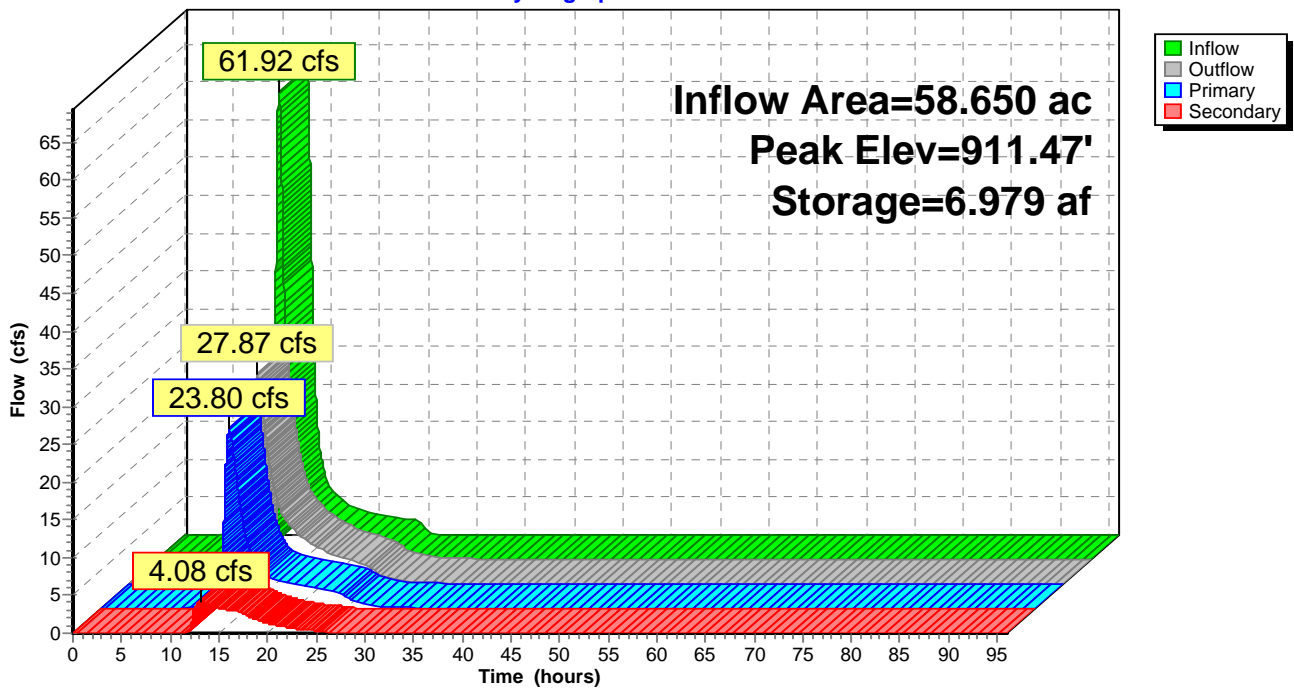
- 1=Orifice/Grate (Orifice Controls 5.95 cfs @ 7.57 fps)
- 2=RCP_Round 24" (Barrel Controls 8.92 cfs @ 5.02 fps)
- 3=RCP_Round 24" (Barrel Controls 8.92 cfs @ 5.02 fps)
- 4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Secondary OutFlow Max=4.08 cfs @ 13.20 hrs HW=911.47' TW=908.66' (Dynamic Tailwater)

- 5=RCP_Round 12" (Barrel Controls 4.08 cfs @ 5.19 fps)

Pond 11P: P-11

Hydrograph



Summary for Pond 12P: P-12

Inflow Area = 79.640 ac, 7.40% Impervious, Inflow Depth > 1.93" for 10-Year event
 Inflow = 34.82 cfs @ 12.02 hrs, Volume= 12.778 af
 Outflow = 24.47 cfs @ 13.83 hrs, Volume= 12.770 af, Atten= 30%, Lag= 108.4 min
 Primary = 24.47 cfs @ 13.83 hrs, Volume= 12.770 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 893.00' Surf.Area= 1.640 ac Storage= 5.075 af
 Peak Elev= 894.12' @ 13.83 hrs Surf.Area= 1.787 ac Storage= 6.997 af (1.922 af above start)

Plug-Flow detention time= 491.3 min calculated for 7.695 af (60% of inflow)
 Center-of-Mass det. time= 96.5 min (1,132.1 - 1,035.6)

Volume	Invert	Avail.Storage	Storage Description
#1	889.00'	10.590 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
889.00	1.070	0.000	0.000
890.00	1.150	1.110	1.110
892.00	1.330	2.480	3.590
893.00	1.640	1.485	5.075
894.00	1.770	1.705	6.780
896.00	2.040	3.810	10.590

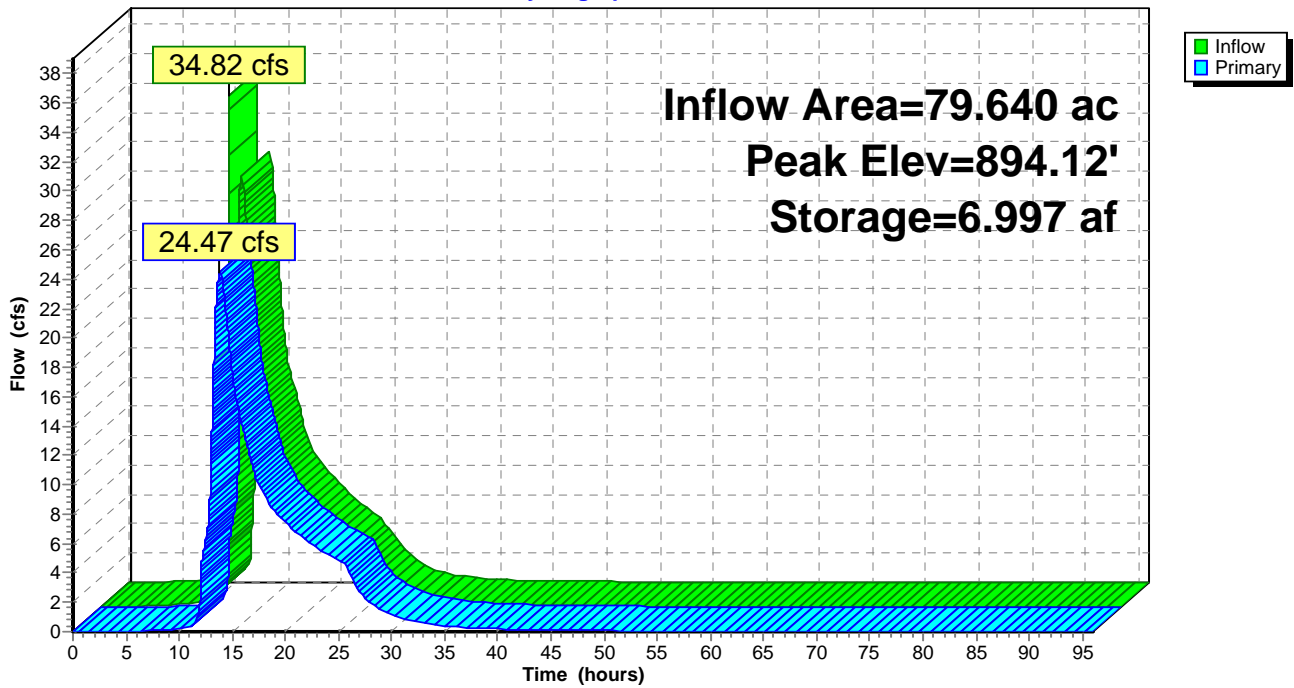
Device	Routing	Invert	Outlet Devices
#1	Primary	893.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	893.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf
#4	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf
#5	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf
#6	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf

Primary OutFlow Max=24.47 cfs @ 13.83 hrs HW=894.12' TW=883.49' (Dynamic Tailwater)

- 1=Orifice/Grate (Orifice Controls 4.01 cfs @ 5.10 fps)
- 2=Orifice/Grate (Orifice Controls 4.01 cfs @ 5.10 fps)
- 3=RCP_Arch 44x27 (Barrel Controls 4.12 cfs @ 3.27 fps)
- 4=RCP_Arch 44x27 (Barrel Controls 4.12 cfs @ 3.27 fps)
- 5=RCP_Arch 44x27 (Barrel Controls 4.12 cfs @ 3.27 fps)
- 6=RCP_Arch 44x27 (Barrel Controls 4.12 cfs @ 3.27 fps)

Pond 12P: P-12

Hydrograph



Summary for Pond 13P: P-13

Inflow Area = 237.775 ac, 9.20% Impervious, Inflow Depth = 1.96" for 10-Year event
 Inflow = 213.60 cfs @ 12.36 hrs, Volume= 38.914 af
 Outflow = 201.08 cfs @ 12.48 hrs, Volume= 38.911 af, Atten= 6%, Lag= 7.5 min
 Primary = 189.63 cfs @ 12.48 hrs, Volume= 37.025 af
 Secondary = 11.45 cfs @ 12.48 hrs, Volume= 1.886 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 883.00' Surf.Area= 1.870 ac Storage= 4.265 af
 Peak Elev= 884.19' @ 12.48 hrs Surf.Area= 2.290 ac Storage= 6.735 af (2.470 af above start)

Plug-Flow detention time= 130.0 min calculated for 34.646 af (89% of inflow)
 Center-of-Mass det. time= 17.0 min (955.7 - 938.7)

Volume	Invert	Avail.Storage	Storage Description
#1	878.00'	11.490 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
878.00	0.000	0.000	0.000
879.00	0.630	0.315	0.315
880.00	0.730	0.680	0.995
882.00	1.070	1.800	2.795
883.00	1.870	1.470	4.265
884.00	2.220	2.045	6.310
886.00	2.960	5.180	11.490

Device	Routing	Invert	Outlet Devices
#1	Primary	883.00'	55.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88
#2	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#3	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#4	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#5	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#6	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200

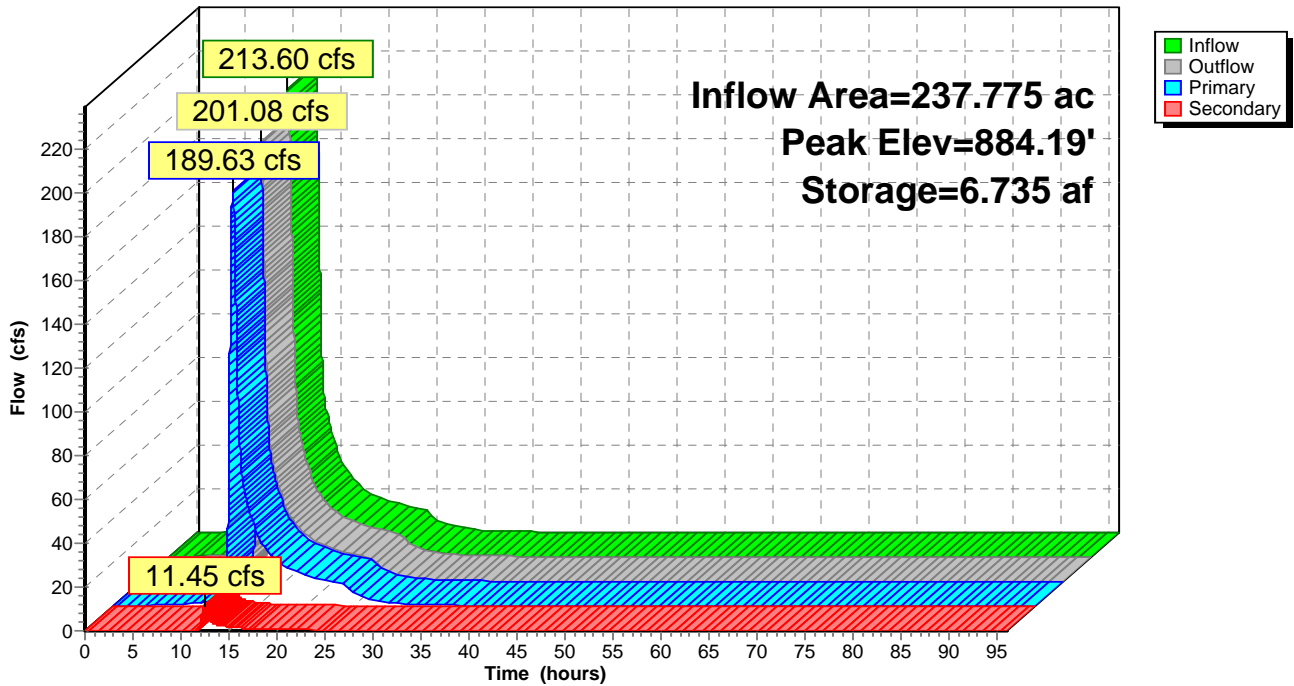
Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 1' Cc= 0.900
 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=189.63 cfs @ 12.48 hrs HW=884.19' TW=0.00' (Dynamic Tailwater)
 1=Broad-Crested Rectangular Weir (Weir Controls 189.63 cfs @ 2.90 fps)

Secondary OutFlow Max=11.45 cfs @ 12.48 hrs HW=884.19' TW=883.01' (Dynamic Tailwater)
 2=RCP_Round 12" (Barrel Controls 2.29 cfs @ 3.10 fps)
 3=RCP_Round 12" (Barrel Controls 2.29 cfs @ 3.10 fps)
 4=RCP_Round 12" (Barrel Controls 2.29 cfs @ 3.10 fps)
 5=RCP_Round 12" (Barrel Controls 2.29 cfs @ 3.10 fps)
 6=RCP_Round 12" (Barrel Controls 2.29 cfs @ 3.10 fps)

Pond 13P: P-13

Hydrograph



Summary for Pond 17P: W-2

[80] Warning: Exceeded Pond P-5/P-6 by 0.16' @ 37.70 hrs (0.15 cfs 0.642 af)

Inflow = 2.25 cfs @ 12.65 hrs, Volume= 0.809 af
 Outflow = 0.63 cfs @ 17.34 hrs, Volume= 0.666 af, Atten= 72%, Lag= 281.1 min
 Primary = 0.63 cfs @ 17.34 hrs, Volume= 0.666 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 929.45' @ 17.34 hrs Surf.Area= 1.167 ac Storage= 0.512 af

Plug-Flow detention time= 707.8 min calculated for 0.666 af (82% of inflow)
 Center-of-Mass det. time= 649.7 min (1,547.3 - 897.6)

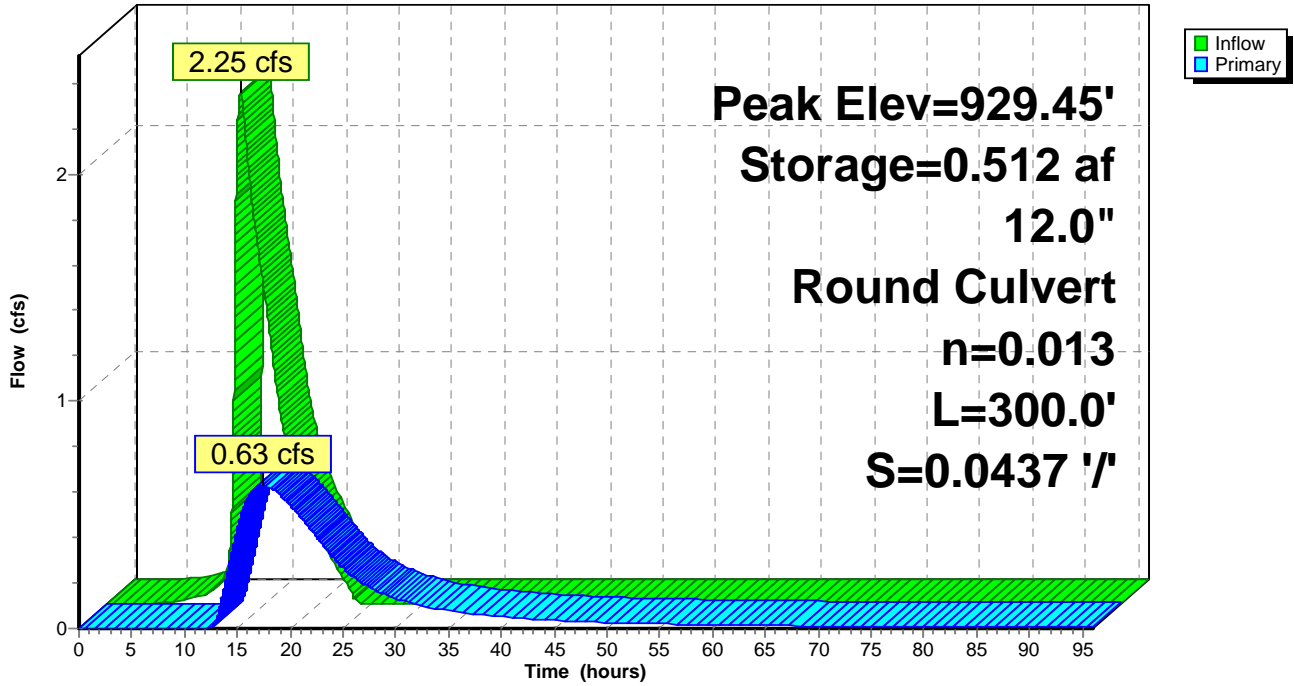
Volume	Invert	Avail.Storage	Storage Description
#1	929.00'	1.175 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
929.00	1.090	0.000	0.000
930.00	1.260	1.175	1.175

Device	Routing	Invert	Outlet Devices
#1	Primary	929.10'	12.0" Round RCP_Round 12" L= 300.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 929.10' / 916.00' S= 0.0437 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.63 cfs @ 17.34 hrs HW=929.45' TW=914.82' (Dynamic Tailwater)
 ↑1=RCP_Round 12" (Inlet Controls 0.63 cfs @ 2.53 fps)

Pond 17P: W-2

Hydrograph



Summary for Pond 36P: Culverts passing flow beneath Spine Road

Inflow Area = 52.790 ac, 0.00% Impervious, Inflow Depth = 1.76" for 10-Year event
 Inflow = 64.04 cfs @ 12.46 hrs, Volume= 7.738 af
 Outflow = 64.04 cfs @ 12.46 hrs, Volume= 7.738 af, Atten= 0%, Lag= 0.0 min
 Primary = 64.04 cfs @ 12.46 hrs, Volume= 7.738 af
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 887.26' @ 12.46 hrs Surf.Area= 0.001 ac Storage= 0.000 af

Plug-Flow detention time= 0.0 min calculated for 7.737 af (100% of inflow)
 Center-of-Mass det. time= 0.0 min (869.1 - 869.1)

Volume	Invert	Avail.Storage	Storage Description
#1	887.00'	0.026 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
887.00	0.000	0.000	0.000
887.50	0.002	0.001	0.001
890.50	0.007	0.014	0.014
892.00	0.009	0.012	0.026

Device	Routing	Invert	Outlet Devices
#1	Primary	887.00'	Special & User-Defined Head (feet) 0.00 0.10 0.20 0.30 0.40 0.50 5.00 Disch. (cfs) 0.000 25.000 50.000 75.000 100.000 127.000 127.000
#2	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#3	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#4	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#5	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#6	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#7	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900

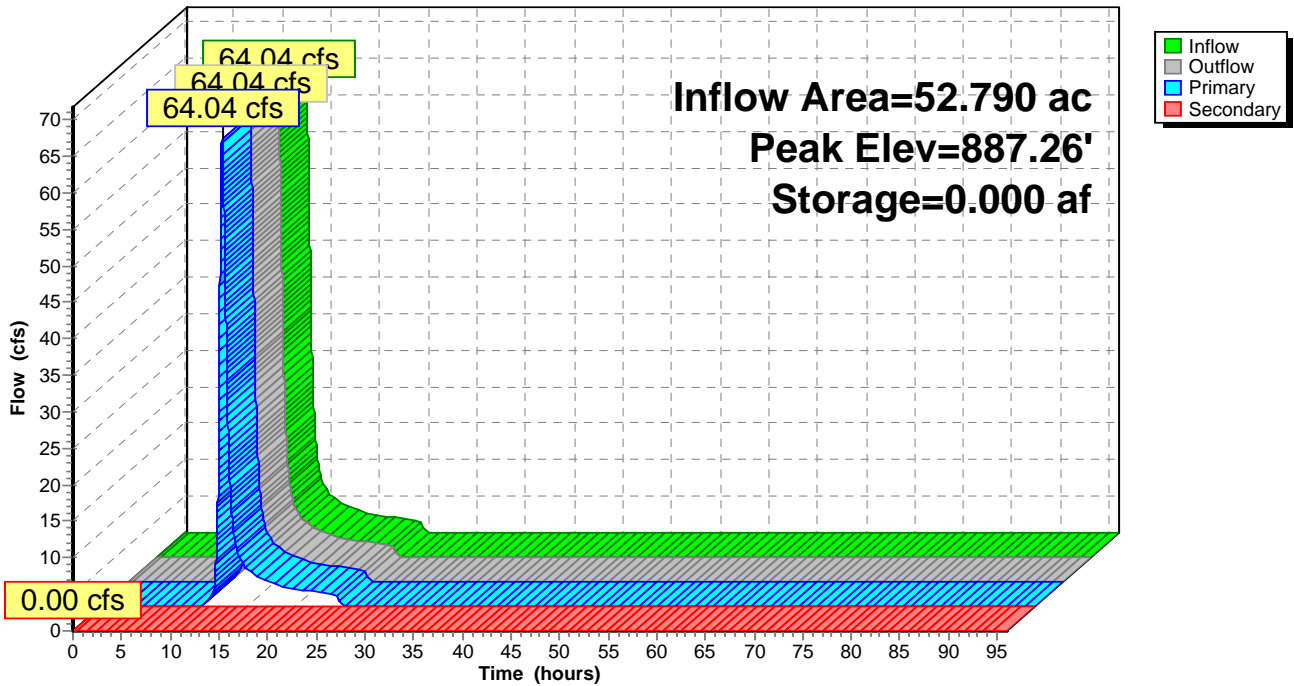
#8	Secondary	887.50'	n= 0.013, Flow Area= 1.77 sf 18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/' Cc= 0.900
#9	Secondary	887.50'	n= 0.013, Flow Area= 1.77 sf 18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=64.03 cfs @ 12.46 hrs HW=887.26' TW=884.19' (Dynamic Tailwater)
 ↳ **1=Special & User-Defined** (Custom Controls 64.03 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=887.00' TW=883.00' (Dynamic Tailwater)
 ↳ **2=RCP_Round 18"** (Controls 0.00 cfs)
 ↳ **3=RCP_Round 18"** (Controls 0.00 cfs)
 ↳ **4=RCP_Round 18"** (Controls 0.00 cfs)
 ↳ **5=RCP_Round 18"** (Controls 0.00 cfs)
 ↳ **6=RCP_Round 18"** (Controls 0.00 cfs)
 ↳ **7=RCP_Round 18"** (Controls 0.00 cfs)
 ↳ **8=RCP_Round 18"** (Controls 0.00 cfs)
 ↳ **9=RCP_Round 18"** (Controls 0.00 cfs)

Pond 36P: Culverts passing flow beneath Spine Road

Hydrograph



Summary for Pond CRH-1: CRH-1

Inflow Area = 6.955 ac, 46.76% Impervious, Inflow Depth = 2.80" for 10-Year event
 Inflow = 18.97 cfs @ 12.15 hrs, Volume= 1.622 af
 Outflow = 12.06 cfs @ 12.33 hrs, Volume= 1.622 af, Atten= 36%, Lag= 10.6 min
 Discarded = 0.26 cfs @ 12.33 hrs, Volume= 0.512 af
 Primary = 11.80 cfs @ 12.33 hrs, Volume= 1.110 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 878.12' @ 12.33 hrs Surf.Area= 0.325 ac Storage= 0.489 af

Plug-Flow detention time= 182.4 min calculated for 1.622 af (100% of inflow)
 Center-of-Mass det. time= 182.5 min (971.9 - 789.5)

Volume	Invert	Avail.Storage	Storage Description
#1	876.00'	0.850 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
876.00	0.150	0.000	0.000
878.00	0.300	0.450	0.450
879.00	0.500	0.400	0.850

Device	Routing	Invert	Outlet Devices
#1	Discarded	876.00'	0.800 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 0.00'
#2	Primary	877.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 877.00' / 876.00' S= 0.0065 1/1' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Primary	877.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 877.00' / 876.00' S= 0.0065 1/1' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Discarded OutFlow Max=0.26 cfs @ 12.33 hrs HW=878.12' (Free Discharge)

└─1=Exfiltration (Controls 0.26 cfs)

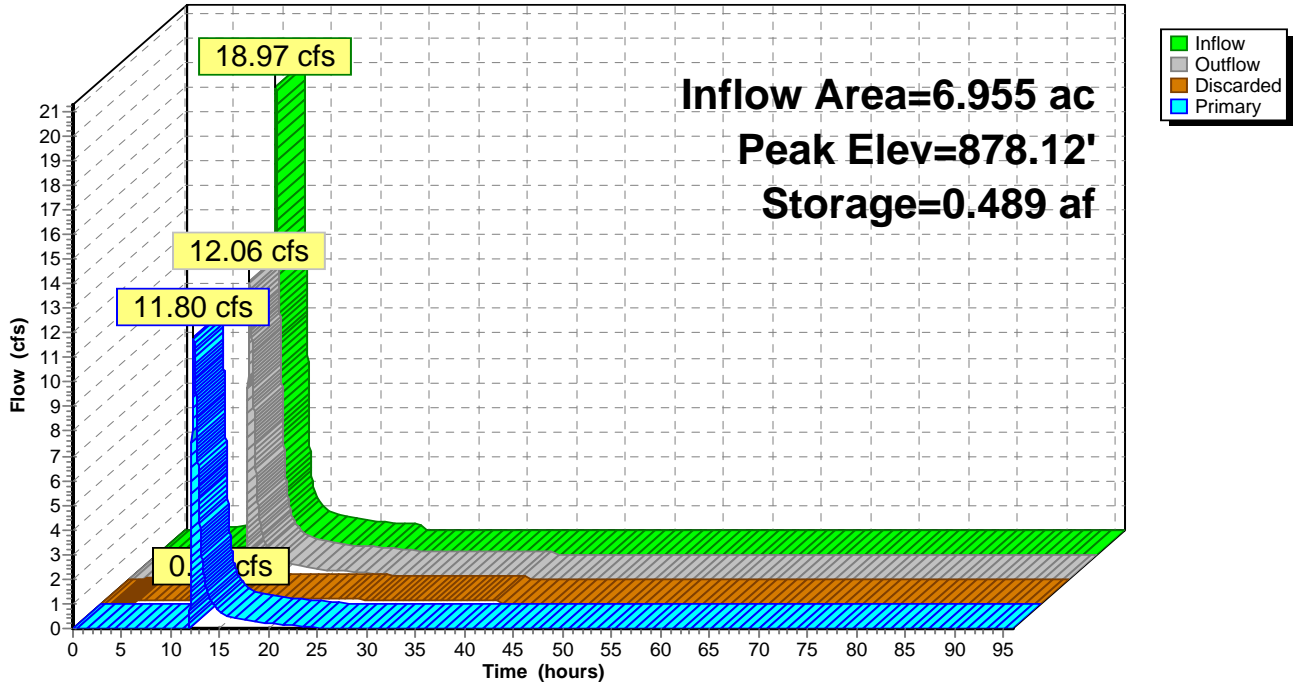
Primary OutFlow Max=11.80 cfs @ 12.33 hrs HW=878.12' (Free Discharge)

└─2=Culvert (Barrel Controls 5.90 cfs @ 4.69 fps)

└─3=Culvert (Barrel Controls 5.90 cfs @ 4.69 fps)

Pond CRH-1: CRH-1

Hydrograph



Summary for Pond CRH-2: CRH-2

Inflow Area = 10.214 ac, 37.73% Impervious, Inflow Depth = 2.60" for 10-Year event
 Inflow = 23.01 cfs @ 12.22 hrs, Volume= 2.212 af
 Outflow = 10.19 cfs @ 12.62 hrs, Volume= 2.212 af, Atten= 56%, Lag= 24.1 min
 Discarded = 0.38 cfs @ 12.62 hrs, Volume= 0.904 af
 Primary = 9.82 cfs @ 12.62 hrs, Volume= 1.308 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 882.67' @ 12.62 hrs Surf.Area= 0.467 ac Storage= 0.888 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 323.8 min (1,125.6 - 801.7)

Volume	Invert	Avail.Storage	Storage Description
#1	880.00'	1.600 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
880.00	0.200	0.000	0.000
882.00	0.400	0.600	0.600
884.00	0.600	1.000	1.600

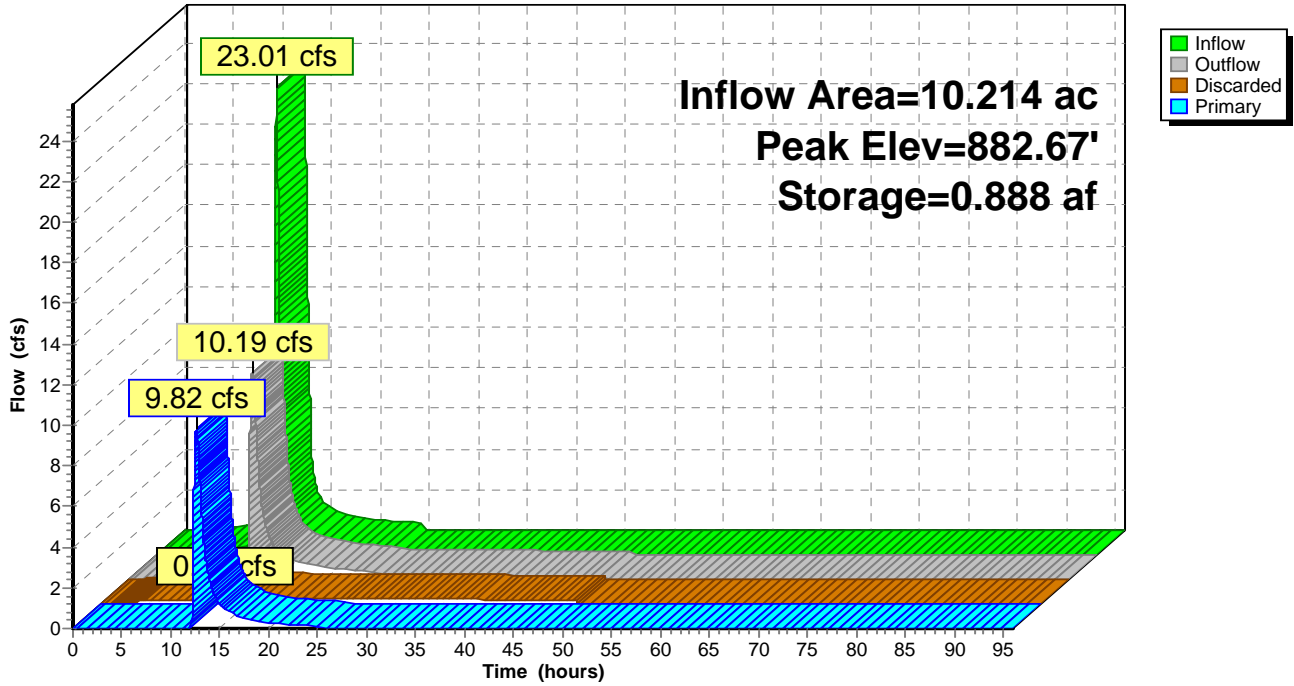
Device	Routing	Invert	Outlet Devices
#1	Primary	881.50'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 881.50' / 881.00' S= 0.0032 ' /' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Primary	881.50'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 881.50' / 881.00' S= 0.0032 ' /' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Discarded	880.00'	0.800 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 0.00'

Discarded OutFlow Max=0.38 cfs @ 12.62 hrs HW=882.67' (Free Discharge)
 ↳ **3=Exfiltration** (Controls 0.38 cfs)

Primary OutFlow Max=9.82 cfs @ 12.62 hrs HW=882.67' TW=878.71' (Dynamic Tailwater)
 ↳ **1=Culvert** (Barrel Controls 4.91 cfs @ 3.72 fps)
 ↳ **2=Culvert** (Barrel Controls 4.91 cfs @ 3.72 fps)

Pond CRH-2: CRH-2

Hydrograph



Summary for Pond CRH-3: CRH-3

Inflow Area = 11.815 ac, 36.95% Impervious, Inflow Depth = 1.66" for 10-Year event
 Inflow = 10.84 cfs @ 12.54 hrs, Volume= 1.637 af
 Outflow = 8.22 cfs @ 12.91 hrs, Volume= 1.637 af, Atten= 24%, Lag= 22.1 min
 Discarded = 0.24 cfs @ 12.91 hrs, Volume= 0.449 af
 Primary = 7.98 cfs @ 12.91 hrs, Volume= 1.188 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 878.90' @ 12.91 hrs Surf.Area= 0.293 ac Storage= 0.421 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 170.2 min (1,018.1 - 847.9)

Volume	Invert	Avail.Storage	Storage Description
#1	877.00'	0.850 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
877.00	0.150	0.000	0.000
879.00	0.300	0.450	0.450
880.00	0.500	0.400	0.850

Device	Routing	Invert	Outlet Devices
#1	Discarded	877.00'	0.800 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 0.00'
#2	Primary	878.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 878.00' / 877.00' S= 0.0065 1/1' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Primary	878.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 878.00' / 877.00' S= 0.0065 1/1' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Discarded OutFlow Max=0.24 cfs @ 12.91 hrs HW=878.90' (Free Discharge)

└─1=Exfiltration (Controls 0.24 cfs)

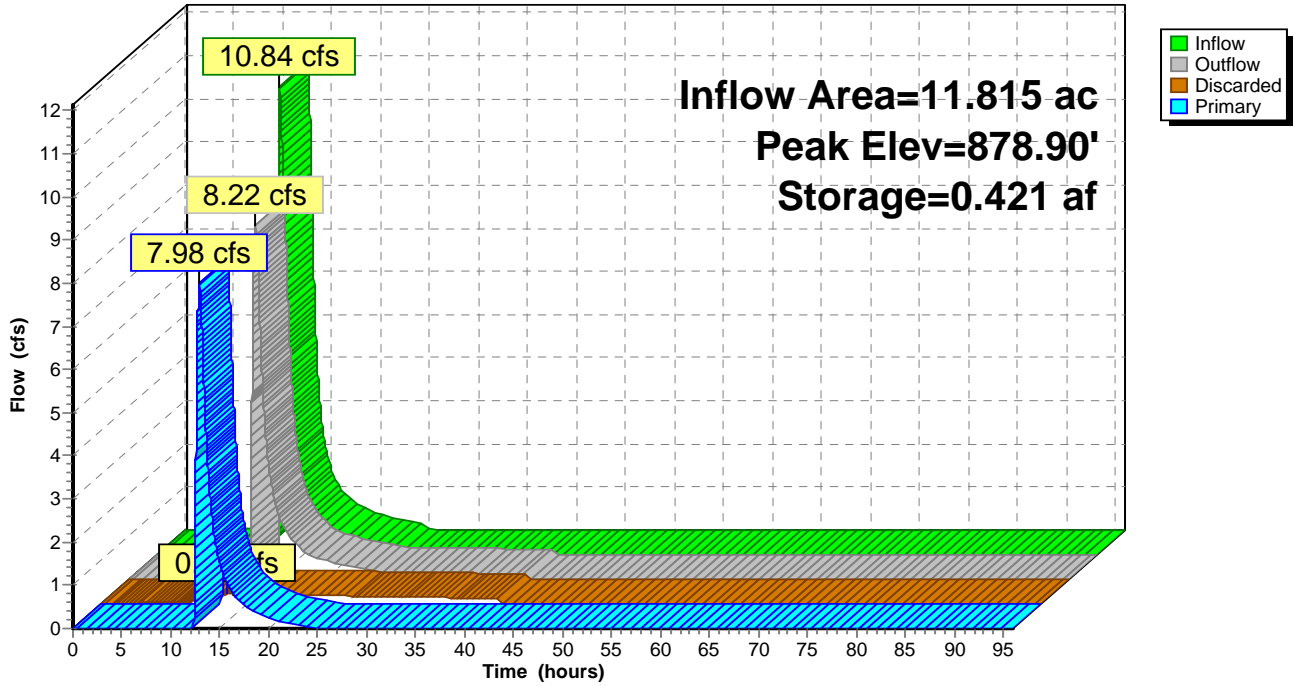
Primary OutFlow Max=7.98 cfs @ 12.91 hrs HW=878.90' (Free Discharge)

└─2=Culvert (Barrel Controls 3.99 cfs @ 4.27 fps)

└─3=Culvert (Barrel Controls 3.99 cfs @ 4.27 fps)

Pond CRH-3: CRH-3

Hydrograph



Summary for Pond P-5/P-6: P-5/P-6

Inflow Area = 43.346 ac, 18.61% Impervious, Inflow Depth = 2.17" for 10-Year event
 Inflow = 91.40 cfs @ 12.15 hrs, Volume= 7.854 af
 Outflow = 28.95 cfs @ 12.65 hrs, Volume= 7.848 af, Atten= 68%, Lag= 30.1 min
 Primary = 26.70 cfs @ 12.65 hrs, Volume= 7.038 af
 Secondary = 2.25 cfs @ 12.65 hrs, Volume= 0.809 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 929.00' Surf.Area= 1.975 ac Storage= 5.062 af
 Peak Elev= 930.49' @ 12.65 hrs Surf.Area= 2.319 ac Storage= 8.298 af (3.236 af above start)

Plug-Flow detention time= 660.8 min calculated for 2.785 af (35% of inflow)
 Center-of-Mass det. time= 172.6 min (991.5 - 819.0)

Volume	Invert	Avail.Storage	Storage Description
#1	926.00'	14.650 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
926.00	1.510	0.000	0.000
928.00	1.710	3.220	3.220
930.00	2.240	3.950	7.170
931.00	2.400	2.320	9.490
933.00	2.760	5.160	14.650

Device	Routing	Invert	Outlet Devices
#1	Primary	929.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	929.50'	7.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#3	Primary	930.50'	14.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#4	Secondary	929.00'	9.0" Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=26.69 cfs @ 12.65 hrs HW=930.49' TW=0.00' (Dynamic Tailwater)

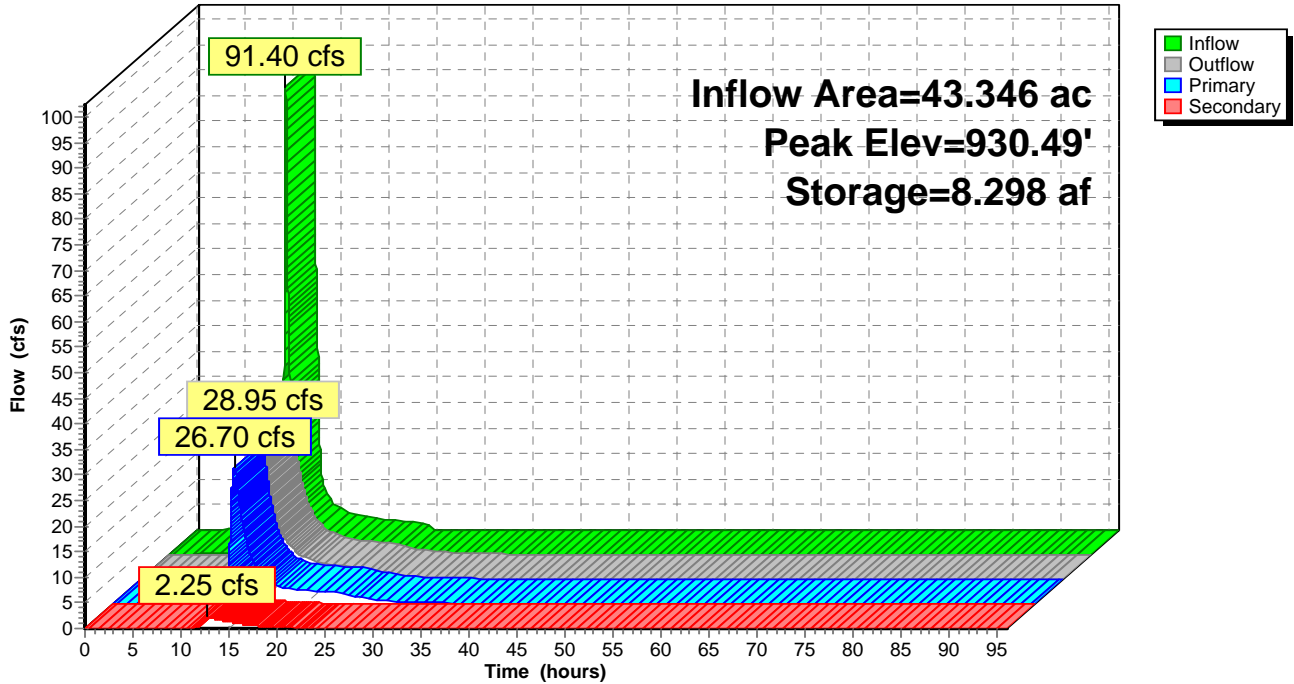
- ↑ 1=Orifice/Grate (Orifice Controls 4.62 cfs @ 5.89 fps)
- └ 2=Sharp-Crested Rectangular Weir (Weir Controls 22.07 cfs @ 3.26 fps)
- └ 3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

Secondary OutFlow Max=2.25 cfs @ 12.65 hrs HW=930.49' TW=929.13' (Dynamic Tailwater)

- ↑ 4=Orifice/Grate (Orifice Controls 2.25 cfs @ 5.10 fps)

Pond P-5/P-6: P-5/P-6

Hydrograph



Summary for Pond TI P: Thumb Infiltration (Thumb TP load only)

Inflow Area = 48.539 ac, 11.38% Impervious, Inflow Depth = 0.81" for 10-Year event
 Inflow = 18.36 cfs @ 12.36 hrs, Volume= 3.268 af
 Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 903.27' @ 26.34 hrs Surf.Area= 1.000 ac Storage= 3.268 af

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= (not calculated: no outflow)

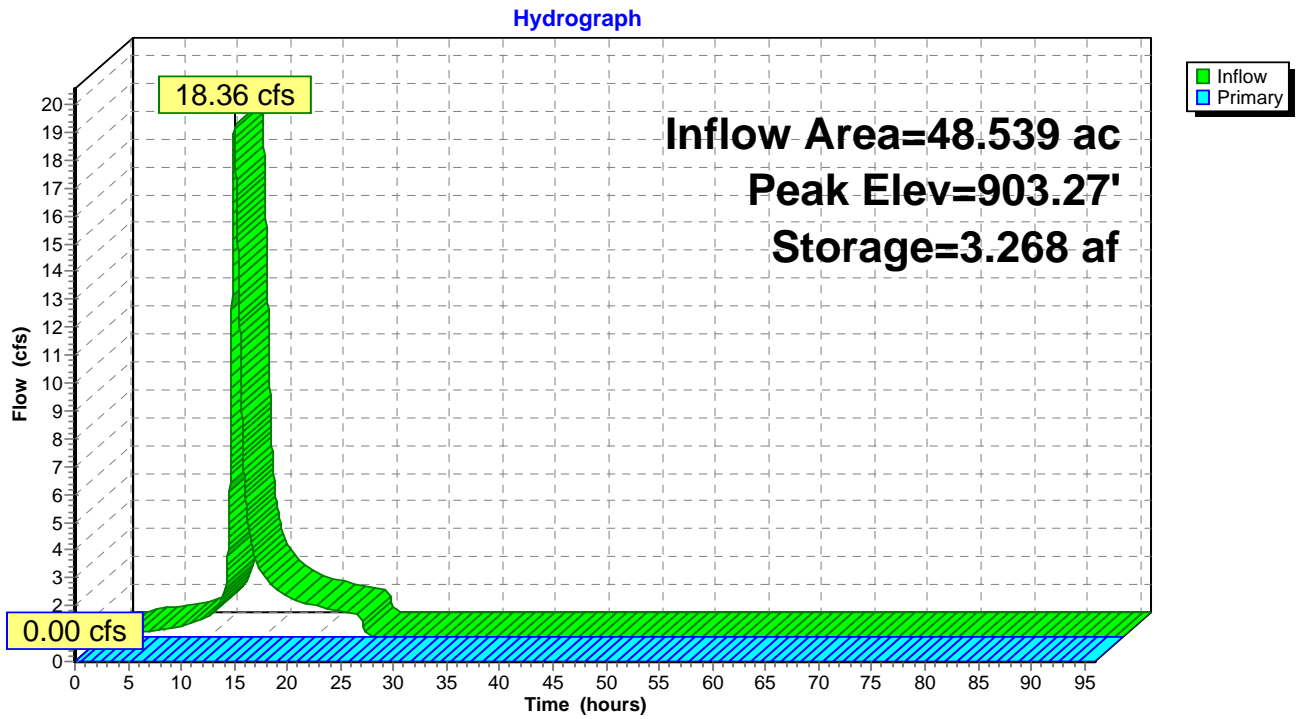
Volume	Invert	Avail.Storage	Storage Description
#1	900.00'	5.000 af	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
900.00	1.000	0.000	0.000
901.00	1.000	1.000	1.000
902.00	1.000	1.000	2.000
903.00	1.000	1.000	3.000
904.00	1.000	1.000	4.000
905.00	1.000	1.000	5.000

Device	Routing	Invert	Outlet Devices
#1	Primary	903.74'	1,000.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) 5.0' Crest Height

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=900.00' (Free Discharge)
 ↑1=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

Pond TI P: Thumb Infiltration (Thumb TP load only)



Summary for Pond W-1: W-1

Inflow Area = 1.000 ac, 10.00% Impervious, Inflow Depth = 11.65" for 10-Year event
 Inflow = 2.45 cfs @ 13.29 hrs, Volume= 0.971 af
 Outflow = 2.00 cfs @ 14.54 hrs, Volume= 0.970 af, Atten= 18%, Lag= 74.8 min
 Primary = 2.00 cfs @ 14.54 hrs, Volume= 0.970 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 915.09' @ 14.54 hrs Surf.Area= 0.714 ac Storage= 0.231 af

Plug-Flow detention time= 117.7 min calculated for 0.970 af (100% of inflow)
 Center-of-Mass det. time= 117.1 min (1,034.4 - 917.2)

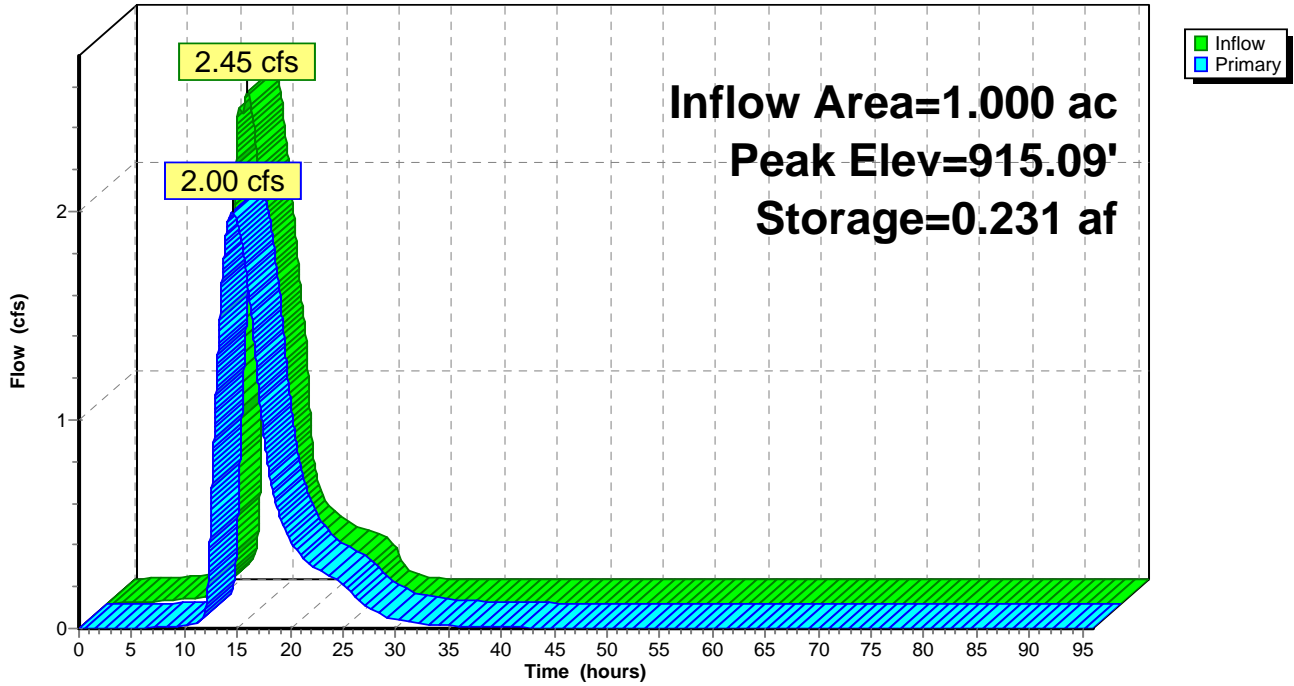
Volume	Invert	Avail.Storage	Storage Description
#1	914.75'	0.950 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
914.75	0.660	0.000	0.000
916.00	0.860	0.950	0.950

Device	Routing	Invert	Outlet Devices
#1	Primary	914.75'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=2.00 cfs @ 14.54 hrs HW=915.09' TW=0.00' (Dynamic Tailwater)
 ↑1=Orifice/Grate (Weir Controls 2.00 cfs @ 1.90 fps)

Pond W-1: W-1

Hydrograph



Summary for Pond W-3: W-3

Inflow = 0.63 cfs @ 17.34 hrs, Volume= 0.666 af
 Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

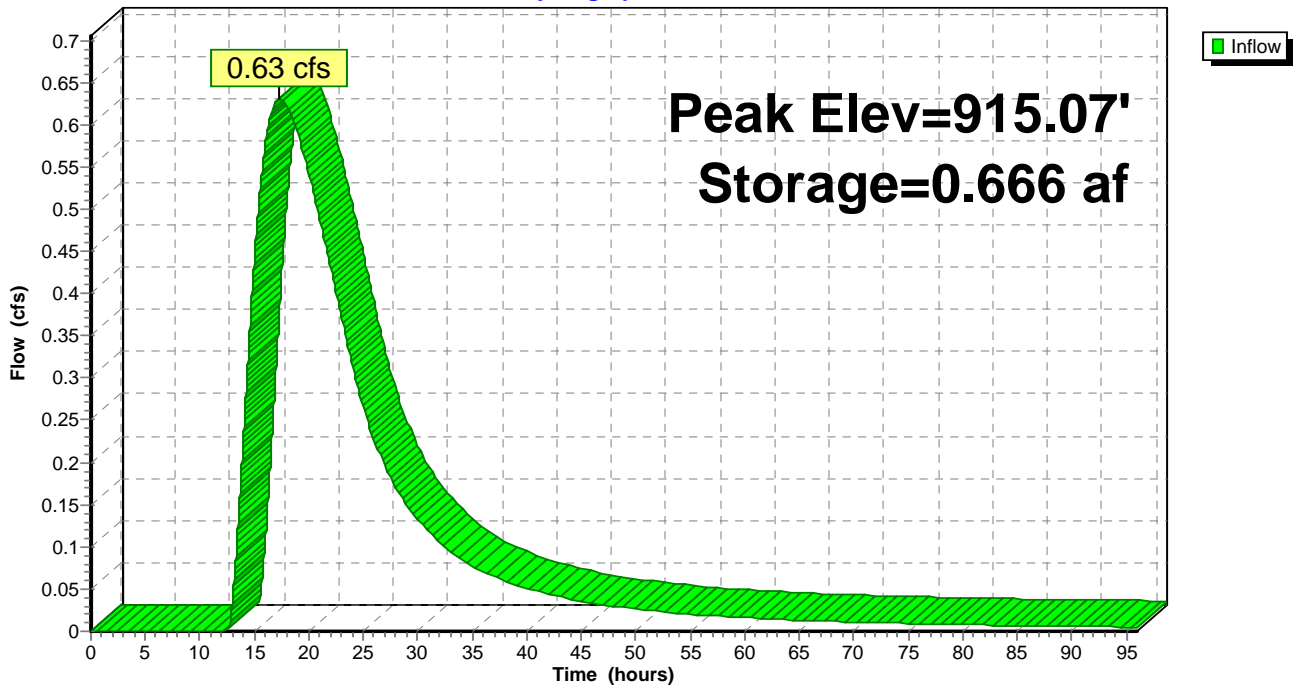
Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 915.07' @ 96.00 hrs Surf.Area= 2.092 ac Storage= 0.666 af

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description
#1	914.75'	2.680 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
914.75	2.040	0.000	0.000
915.00	2.080	0.515	0.515
916.00	2.250	2.165	2.680

Pond W-3: W-3

Hydrograph



Summary for Pond W-4: W-4

Inflow Area = 2.980 ac, 26.17% Impervious, Inflow Depth = 11.29" for 10-Year event
 Inflow = 8.82 cfs @ 12.08 hrs, Volume= 2.803 af
 Outflow = 3.11 cfs @ 16.96 hrs, Volume= 2.784 af, Atten= 65%, Lag= 292.4 min
 Primary = 3.11 cfs @ 16.96 hrs, Volume= 2.784 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 908.93' @ 16.96 hrs Surf.Area= 1.114 ac Storage= 0.878 af

Plug-Flow detention time= 264.8 min calculated for 2.783 af (99% of inflow)
 Center-of-Mass det. time= 259.8 min (1,211.6 - 951.8)

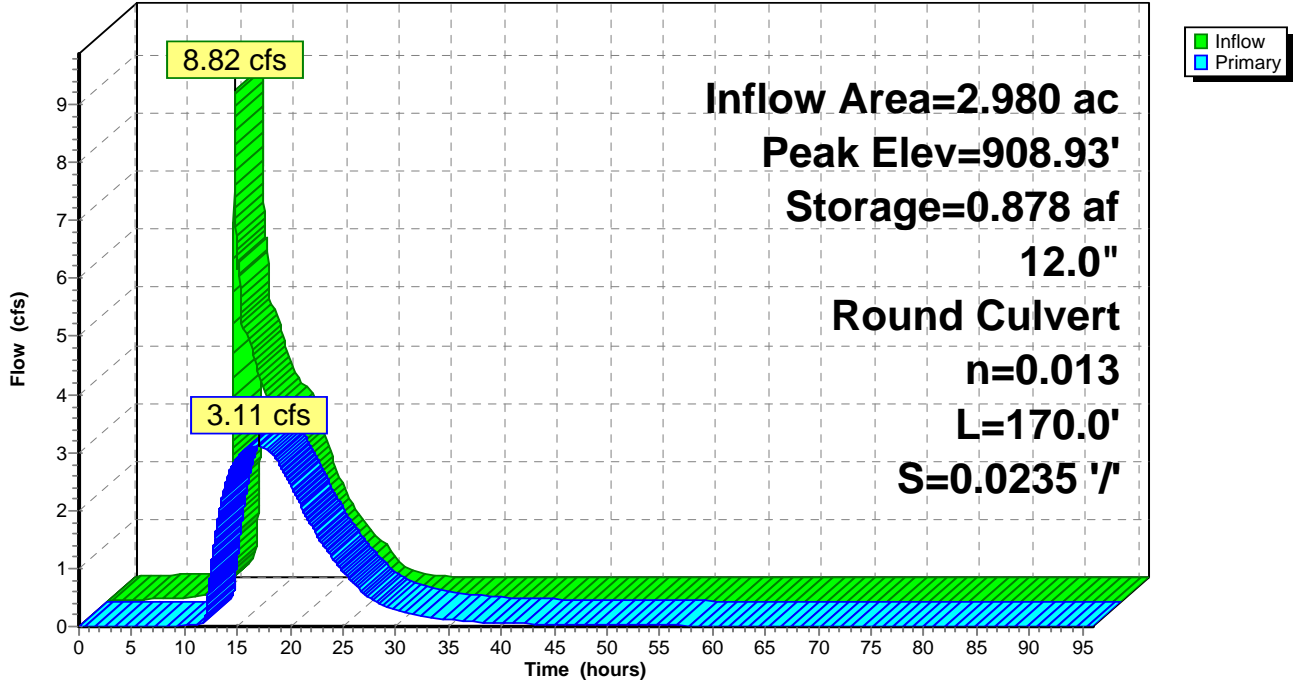
Volume	Invert	Avail.Storage	Storage Description
#1	908.00'	2.280 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
908.00	0.780	0.000	0.000
910.00	1.500	2.280	2.280

Device	Routing	Invert	Outlet Devices
#1	Primary	908.00'	12.0" Round RCP_Round 12" L= 170.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 908.00' / 904.00' S= 0.0235 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=3.11 cfs @ 16.96 hrs HW=908.93' TW=893.81' (Dynamic Tailwater)
 ↳1=RCP_Round 12" (Inlet Controls 3.11 cfs @ 4.10 fps)

Pond W-4: W-4

Hydrograph



Summary for Pond W-5: W-5

Inflow Area = 7.608 ac, 48.41% Impervious, Inflow Depth = 5.93" for 10-Year event
 Inflow = 34.78 cfs @ 12.02 hrs, Volume= 3.757 af
 Outflow = 6.80 cfs @ 13.11 hrs, Volume= 3.750 af, Atten= 80%, Lag= 65.1 min
 Primary = 6.80 cfs @ 13.11 hrs, Volume= 3.750 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 882.75' Surf.Area= 4.910 ac Storage= 3.412 af
 Peak Elev= 883.06' @ 13.11 hrs Surf.Area= 5.253 ac Storage= 4.998 af (1.586 af above start)

Plug-Flow detention time= 1,663.5 min calculated for 0.337 af (9% of inflow)
 Center-of-Mass det. time= 252.4 min (1,083.4 - 831.0)

Volume	Invert	Avail.Storage	Storage Description
#1	882.00'	7.390 af	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
882.00	4.190	0.000	0.000
883.00	5.150	4.670	4.670
883.49	5.950	2.720	7.390

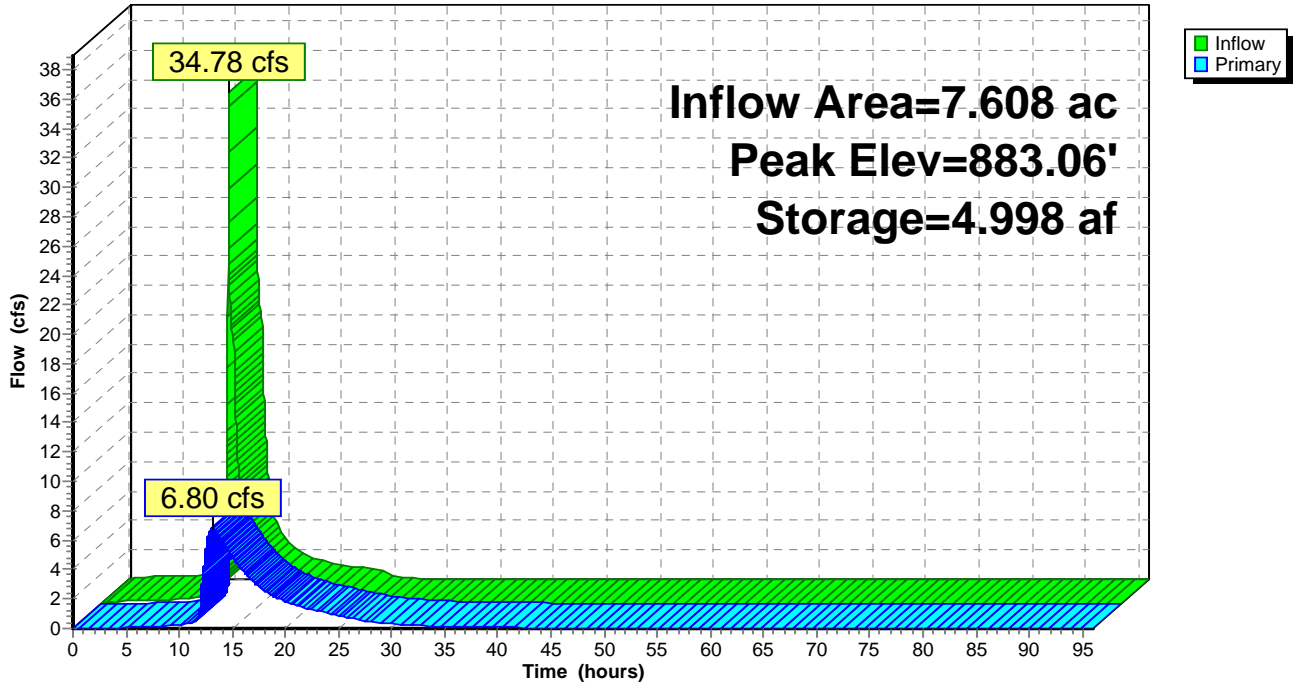
Device	Routing	Invert	Outlet Devices
#1	Primary	882.75'	6.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Primary	882.75'	6.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)

Primary OutFlow Max=6.80 cfs @ 13.11 hrs HW=883.06' TW=0.00' (Dynamic Tailwater)

- 1=Sharp-Crested Rectangular Weir (Weir Controls 3.40 cfs @ 1.83 fps)
- 2=Sharp-Crested Rectangular Weir (Weir Controls 3.40 cfs @ 1.83 fps)

Pond W-5: W-5

Hydrograph



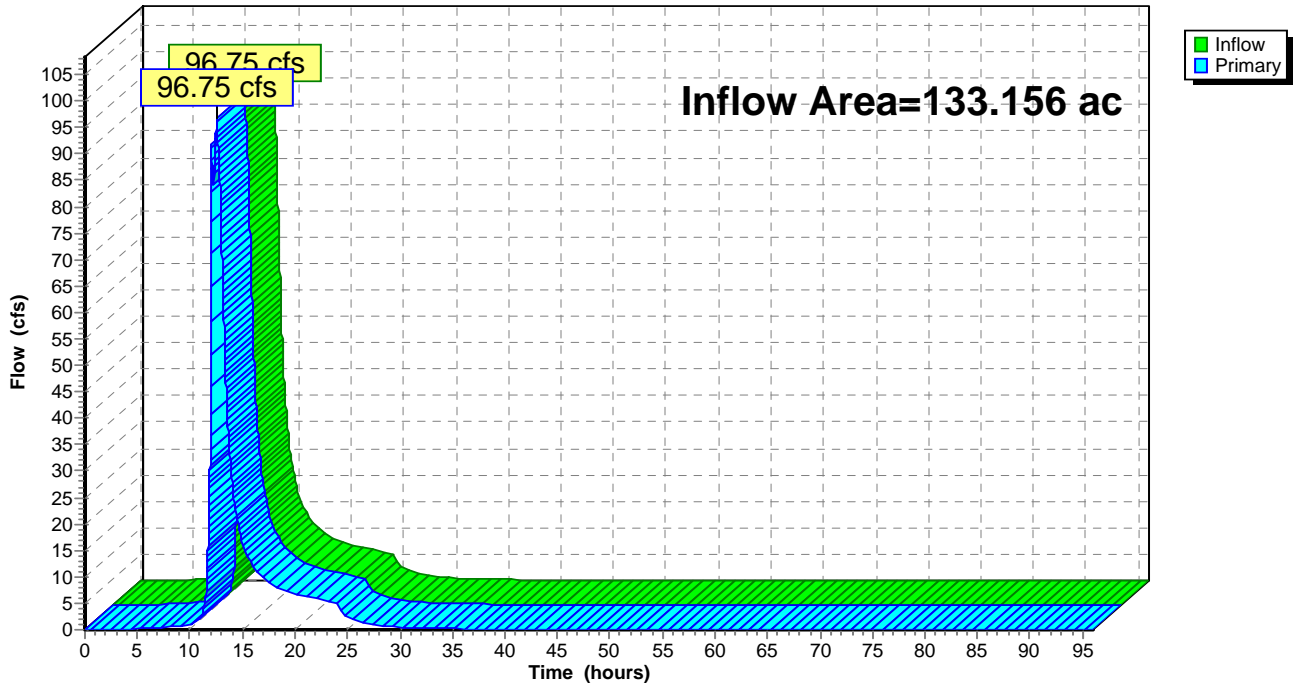
Summary for Link 53L: Sum of Outlet #2 Discharges to Round Lake

Inflow Area = 133.156 ac, 9.78% Impervious, Inflow Depth = 1.90" for 10-Year event
Inflow = 96.75 cfs @ 12.54 hrs, Volume= 21.122 af
Primary = 96.75 cfs @ 12.54 hrs, Volume= 21.122 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 53L: Sum of Outlet #2 Discharges to Round Lake

Hydrograph

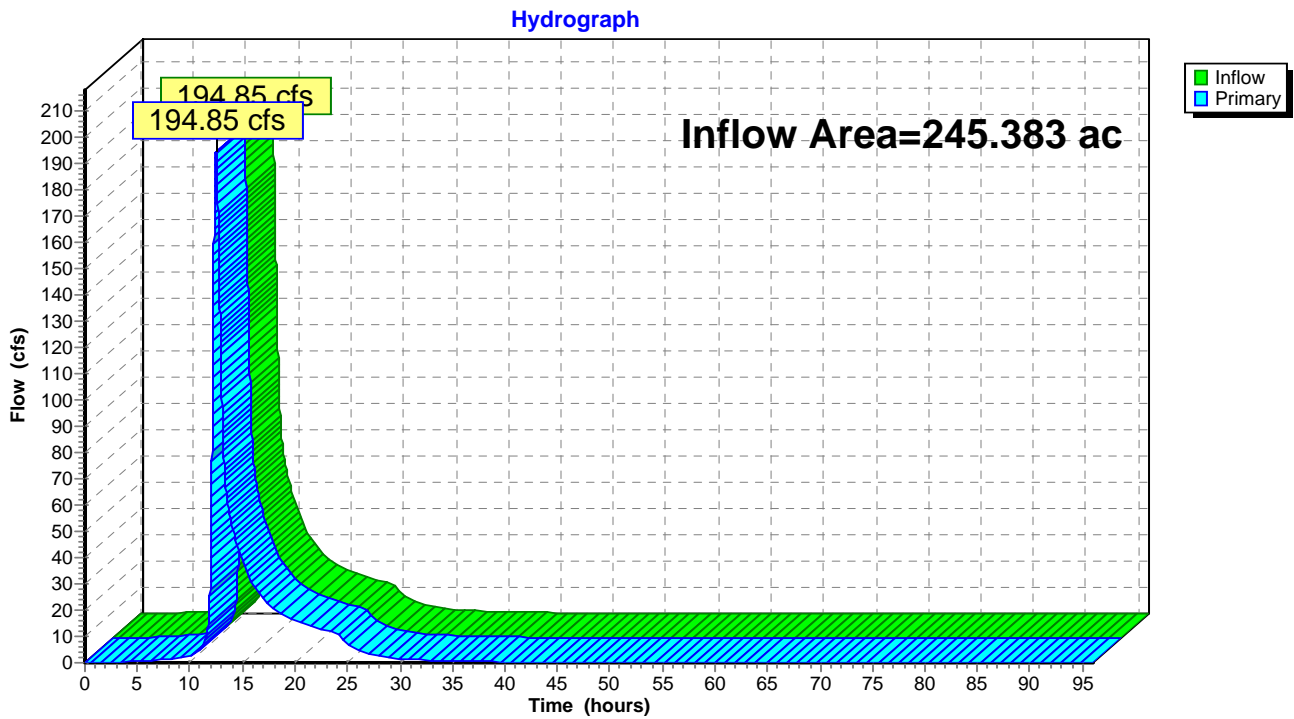


Summary for Link 54L: Sum of Discharges from P-13 and W-5

Inflow Area = 245.383 ac, 10.42% Impervious, Inflow Depth = 1.99" for 10-Year event
Inflow = 194.85 cfs @ 12.48 hrs, Volume= 40.774 af
Primary = 194.85 cfs @ 12.48 hrs, Volume= 40.774 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 54L: Sum of Discharges from P-13 and W-5

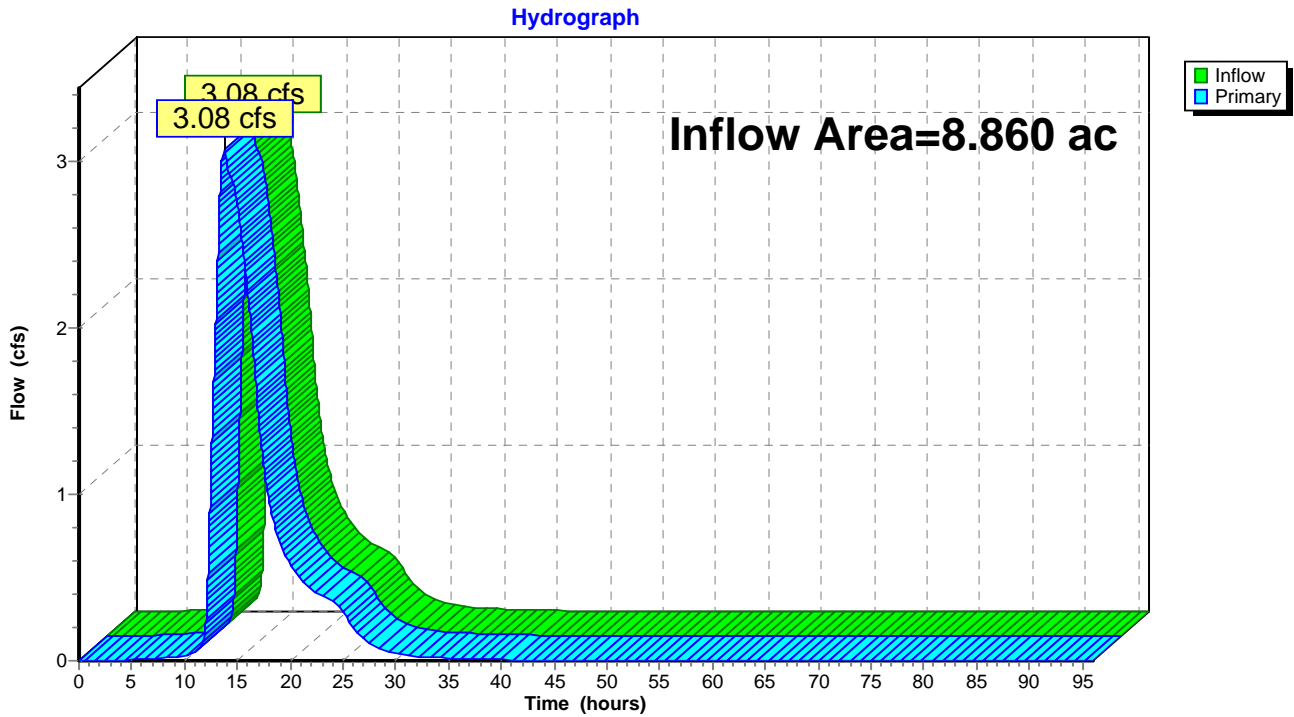


Summary for Link 55L: Sum of Outlet #1 Discharges to Round Lake

Inflow Area = 8.860 ac, 6.43% Impervious, Inflow Depth = 1.90" for 10-Year event
Inflow = 3.08 cfs @ 13.77 hrs, Volume= 1.406 af
Primary = 3.08 cfs @ 13.77 hrs, Volume= 1.406 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 55L: Sum of Outlet #1 Discharges to Round Lake



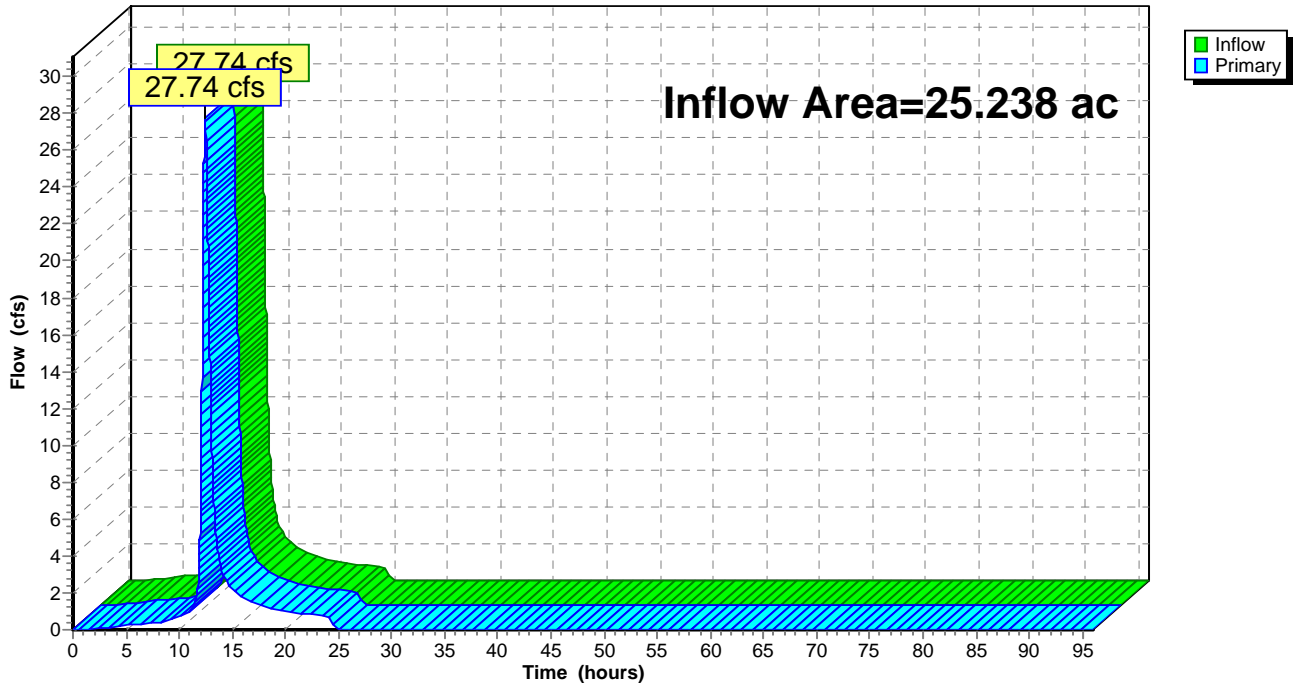
Summary for Link 57L: Outlet #3 Discharge to Round Lake

Inflow Area = 25.238 ac, 19.96% Impervious, Inflow Depth = 1.72" for 10-Year event
Inflow = 27.74 cfs @ 12.43 hrs, Volume= 3.622 af
Primary = 27.74 cfs @ 12.43 hrs, Volume= 3.622 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 57L: Outlet #3 Discharge to Round Lake

Hydrograph



Time span=0.00-96.00 hrs, dt=0.01 hrs, 9601 points
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: To Rice Creek	Runoff Area=1.601 ac 31.98% Impervious Runoff Depth=5.20" Tc=5.7 min CN=74/98 Runoff=11.87 cfs 0.693 af
Subcatchment 47S: Offsite Subbasin 51	Runoff Area=25.238 ac 19.96% Impervious Runoff Depth=4.09" Tc=32.8 min CN=65/98 Runoff=70.31 cfs 8.599 af
Subcatchment SB 1: SB 1	Runoff Area=52.150 ac 0.00% Impervious Runoff Depth=4.31" Tc=53.1 min CN=74/0 Runoff=125.09 cfs 18.747 af
Subcatchment SB 11: SB 11	Runoff Area=3.290 ac 36.78% Impervious Runoff Depth=5.42" Tc=11.7 min CN=74/100 Runoff=19.16 cfs 1.485 af
Subcatchment SB 12: SB 12	Runoff Area=1.390 ac 20.86% Impervious Runoff Depth=4.89" Tc=9.5 min CN=74/98 Runoff=8.27 cfs 0.566 af
Subcatchment SB 13: SB 13	Runoff Area=2.980 ac 26.17% Impervious Runoff Depth=5.10" Tc=9.4 min CN=74/100 Runoff=18.22 cfs 1.266 af
Subcatchment SB 14: SB 14	Runoff Area=10.230 ac 16.03% Impervious Runoff Depth=4.76" Tc=4.3 min CN=74/98 Runoff=76.19 cfs 4.054 af
Subcatchment SB 15: SB 15	Runoff Area=58.570 ac 0.05% Impervious Runoff Depth=4.32" Tc=31.3 min CN=74/98 Runoff=185.61 cfs 21.062 af
Subcatchment SB 16: SB 16	Runoff Area=32.440 ac 5.76% Impervious Runoff Depth=4.47" Tc=12.1 min CN=74/98 Runoff=163.65 cfs 12.092 af
Subcatchment SB 17: SB 17	Runoff Area=7.608 ac 48.41% Impervious Runoff Depth=5.76" Tc=4.3 min CN=74/100 Runoff=64.20 cfs 3.655 af
Subcatchment SB 18: SB 18	Runoff Area=52.790 ac 0.00% Impervious Runoff Depth=4.31" Tc=33.5 min CN=74/0 Runoff=161.15 cfs 18.977 af
Subcatchment SB 19: SB 19	Runoff Area=21.190 ac 0.00% Impervious Runoff Depth=4.31" Tc=24.7 min CN=74/0 Runoff=75.36 cfs 7.618 af
Subcatchment SB 2: SB 2	Runoff Area=11.067 ac 0.33% Impervious Runoff Depth=4.32" Tc=16.6 min CN=74/98 Runoff=47.56 cfs 3.987 af
Subcatchment SB 22: SB 22	Runoff Area=41.910 ac 0.00% Impervious Runoff Depth=1.75" Tc=41.0 min CN=49/0 Runoff=40.68 cfs 6.106 af
Subcatchment SB 24: SB 24	Runoff Area=5.043 ac 97.56% Impervious Runoff Depth=7.00" Tc=7.5 min CN=74/98 Runoff=43.43 cfs 2.943 af
Subcatchment SB 25: SB 25	Runoff Area=5.136 ac 95.72% Impervious Runoff Depth=6.95" Tc=10.7 min CN=74/98 Runoff=38.17 cfs 2.976 af

Interim Spine Road_HydroCADAtlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

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Subcatchment SB 26: SB 26	Runoff Area=14.335 ac 98.27% Impervious Runoff Depth=7.02" Tc=25.4 min CN=74/98 Runoff=73.07 cfs 8.390 af
Subcatchment SB 27: SB 27 (Thumb Road)	Runoff Area=6.629 ac 83.33% Impervious Runoff Depth=6.61" Tc=27.6 min CN=74/98 Runoff=30.89 cfs 3.652 af
Subcatchment SB 28: SB 28	Runoff Area=6.955 ac 46.76% Impervious Runoff Depth=5.60" Tc=14.6 min CN=74/98 Runoff=38.37 cfs 3.247 af
Subcatchment SB 29: SB 29	Runoff Area=10.214 ac 37.73% Impervious Runoff Depth=5.35" Tc=19.1 min CN=74/98 Runoff=48.13 cfs 4.557 af
Subcatchment SB 3: SB 3	Runoff Area=37.610 ac 7.68% Impervious Runoff Depth=4.53" Tc=15.3 min CN=74/98 Runoff=173.90 cfs 14.184 af
Subcatchment SB 4: SB 4	Runoff Area=0.600 ac 43.33% Impervious Runoff Depth=5.61" Tc=5.9 min CN=74/100 Runoff=4.61 cfs 0.281 af
Subcatchment SB 5: SB 5	Runoff Area=7.860 ac 5.98% Impervious Runoff Depth=4.48" Tc=59.3 min CN=74/98 Runoff=18.03 cfs 2.934 af
Subcatchment SB 6: SB 6	Runoff Area=1.000 ac 10.00% Impervious Runoff Depth=4.61" Tc=20.3 min CN=74/100 Runoff=4.09 cfs 0.384 af
Subcatchment SB 7: SB 7	Runoff Area=21.550 ac 0.00% Impervious Runoff Depth=4.31" Tc=5.7 min CN=74/0 Runoff=140.12 cfs 7.747 af
Subcatchment SB 8: SB 8	Runoff Area=29.580 ac 5.51% Impervious Runoff Depth=4.47" Tc=47.1 min CN=74/98 Runoff=77.55 cfs 11.008 af
Subcatchment SB 9: SB 9	Runoff Area=25.780 ac 0.12% Impervious Runoff Depth=4.32" Tc=30.0 min CN=74/98 Runoff=83.43 cfs 9.275 af
Subcatchment SB10: SB 10	Runoff Area=6.390 ac 4.85% Impervious Runoff Depth=4.45" Tc=7.3 min CN=74/98 Runoff=39.26 cfs 2.368 af
Pond 2 P: P-2	Peak Elev=925.48' Storage=1.347 af Inflow=146.98 cfs 25.678 af Outflow=146.30 cfs 25.678 af
Pond 4P: P-4	Peak Elev=917.34' Storage=1.444 af Inflow=18.03 cfs 2.934 af Primary=8.50 cfs 1.519 af Secondary=3.10 cfs 1.415 af Outflow=11.60 cfs 2.934 af
Pond 7P: P-7	Peak Elev=915.86' Storage=1.484 af Inflow=77.55 cfs 11.008 af Outflow=78.63 cfs 11.008 af
Pond 8P: P-8	Peak Elev=899.07' Storage=1.375 af Inflow=39.26 cfs 2.368 af 24.0" Round Culvert n=0.013 L=380.0' S=0.0028 '/' Outflow=10.88 cfs 2.363 af
Pond 9P: P-9	Peak Elev=915.78' Storage=0.576 af Inflow=146.92 cfs 20.283 af Outflow=146.74 cfs 20.283 af
Pond 10P: P-10 Lowered 1 ft	Peak Elev=898.35' Storage=1.496 af Inflow=139.63 cfs 21.209 af Primary=15.29 cfs 10.343 af Secondary=123.93 cfs 10.865 af Outflow=139.22 cfs 21.208 af

Pond 11P: P-11 Peak Elev=912.68' Storage=8.867 af Inflow=153.04 cfs 21.768 af
Primary=130.96 cfs 18.280 af Secondary=4.94 cfs 3.486 af Outflow=135.90 cfs 21.766 af

Pond 12P: P-12 Peak Elev=895.49' Storage=9.562 af Inflow=148.48 cfs 29.994 af
Outflow=116.29 cfs 29.986 af

Pond 13P: P-13 Peak Elev=885.22' Storage=9.306 af Inflow=524.41 cfs 90.508 af
Primary=485.21 cfs 86.324 af Secondary=18.37 cfs 4.181 af Outflow=503.58 cfs 90.505 af

Pond 17P: W-2 Peak Elev=929.56' Storage=0.638 af Inflow=3.09 cfs 1.318 af
12.0" Round Culvert n=0.013 L=300.0' S=0.0437 '/' Outflow=1.02 cfs 1.173 af

Pond 36P: Culverts passing flow Peak Elev=888.46' Storage=0.003 af Inflow=161.15 cfs 18.977 af
Primary=127.00 cfs 18.235 af Secondary=34.15 cfs 0.742 af Outflow=161.15 cfs 18.977 af

Pond CRH-1: CRH-1 Peak Elev=878.81' Storage=0.760 af Inflow=38.37 cfs 3.247 af
Discarded=0.37 cfs 0.563 af Primary=25.16 cfs 2.685 af Outflow=25.53 cfs 3.247 af

Pond CRH-2: CRH-2 Peak Elev=883.78' Storage=1.468 af Inflow=48.13 cfs 4.557 af
Discarded=0.47 cfs 0.986 af Primary=27.38 cfs 3.571 af Outflow=27.85 cfs 4.557 af

Pond CRH-3: CRH-3 Peak Elev=879.83' Storage=0.769 af Inflow=30.15 cfs 4.265 af
Discarded=0.38 cfs 0.519 af Primary=25.52 cfs 3.745 af Outflow=25.90 cfs 4.265 af

Pond P-5/P-6: P-5/P-6 Peak Elev=931.48' Storage=10.663 af Inflow=208.93 cfs 17.441 af
Primary=109.91 cfs 16.116 af Secondary=3.09 cfs 1.318 af Outflow=112.99 cfs 17.434 af

Pond TI P: Thumb Infiltration (Thumb TP Peak Elev=903.80' Storage=3.804 af Inflow=62.42 cfs 9.759 af
Outflow=53.02 cfs 6.019 af

Pond W-1: W-1 Peak Elev=915.28' Storage=0.371 af Inflow=5.53 cfs 1.799 af
Outflow=2.75 cfs 1.799 af

Pond W-3: W-3 Peak Elev=915.31' Storage=1.173 af Inflow=1.02 cfs 1.173 af
Outflow=0.00 cfs 0.000 af

Pond W-4: W-4 Peak Elev=909.25' Storage=1.261 af Inflow=21.33 cfs 4.752 af
12.0" Round Culvert n=0.013 L=170.0' S=0.0235 '/' Outflow=4.11 cfs 4.732 af

Pond W-5: W-5 Peak Elev=883.30' Storage=6.307 af Inflow=73.62 cfs 7.835 af
Outflow=15.85 cfs 7.828 af

Link 53L: Sum of Outlet #2 Discharges to Round Lake Inflow=276.90 cfs 49.540 af
Primary=276.90 cfs 49.540 af

Link 54L: Sum of Discharges from P-13 and W-5 Inflow=498.50 cfs 94.152 af
Primary=498.50 cfs 94.152 af

Link 55L: Sum of Outlet #1 Discharges to Round Lake Inflow=11.11 cfs 3.318 af
Primary=11.11 cfs 3.318 af

Link 57L: Outlet #3 Discharge to Round Lake

Inflow=70.31 cfs 8.599 af

Primary=70.31 cfs 8.599 af

Total Runoff Area = 501.136 ac Runoff Volume = 182.855 af Average Runoff Depth = 4.38"
88.56% Pervious = 443.803 ac 11.44% Impervious = 57.333 ac

Summary for Subcatchment 1S: To Rice Creek

Runoff = 11.87 cfs @ 12.03 hrs, Volume= 0.693 af, Depth= 5.20"

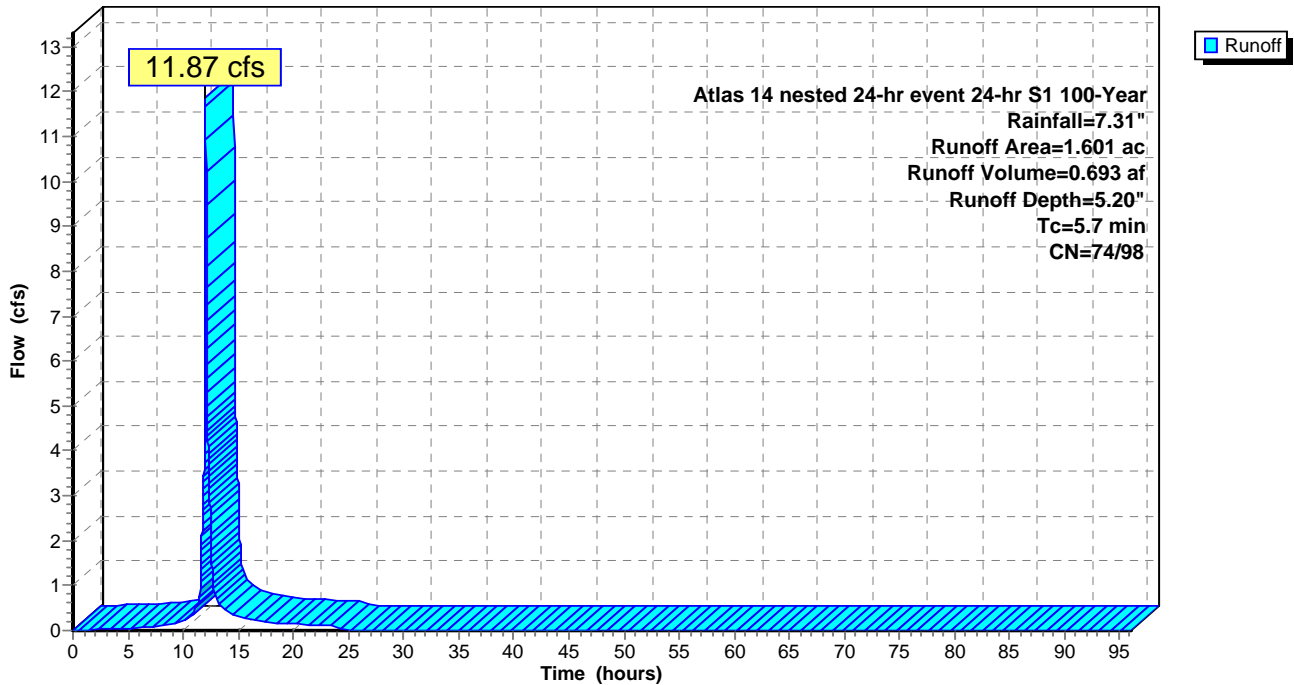
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 0.512	98	impervious
* 1.089	74	pervious
1.601	82	Weighted Average
1.089		68.02% Pervious Area
0.512		31.98% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.7					Direct Entry,

Subcatchment 1S: To Rice Creek

Hydrograph



Summary for Subcatchment 47S: Offsite Subbasin 51

Runoff = 70.31 cfs @ 12.43 hrs, Volume= 8.599 af, Depth= 4.09"

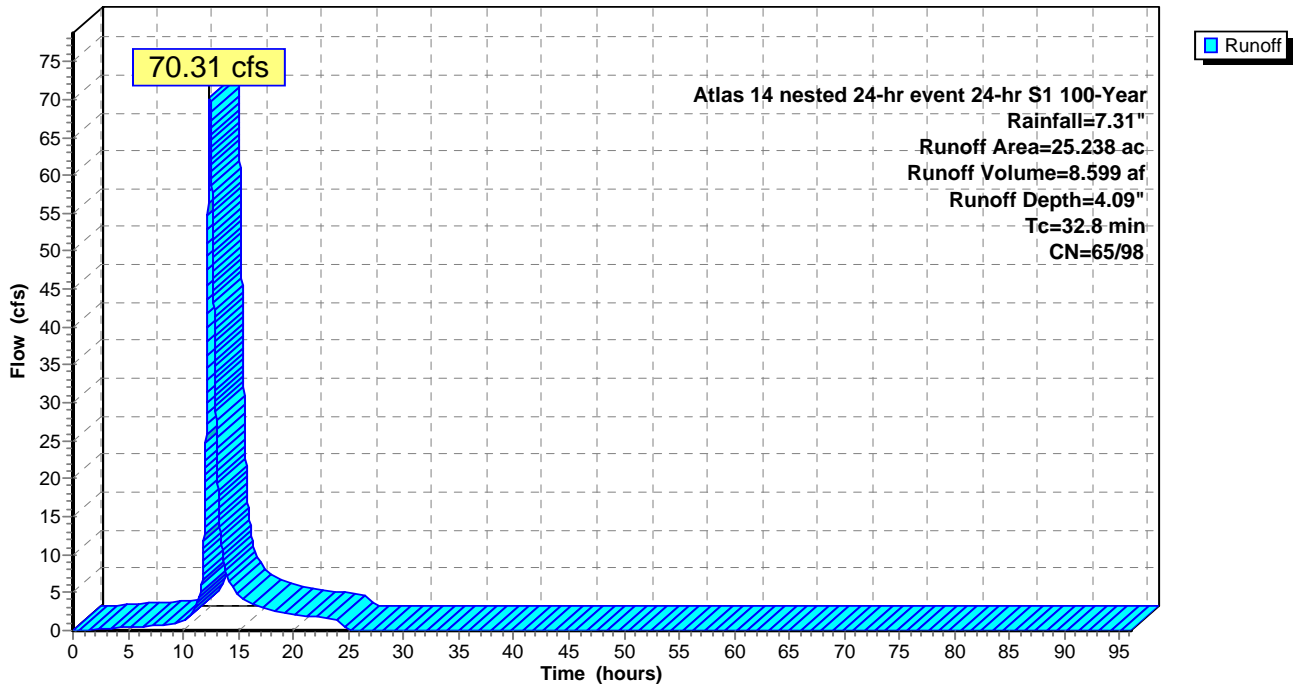
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 20.200	65	Offsite subbasin 51
* 5.038	98	
25.238	72	Weighted Average
20.200		80.04% Pervious Area
5.038		19.96% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.8					Direct Entry,

Subcatchment 47S: Offsite Subbasin 51

Hydrograph



Summary for Subcatchment SB 1: SB 1

Runoff = 125.09 cfs @ 12.69 hrs, Volume= 18.747 af, Depth= 4.31"

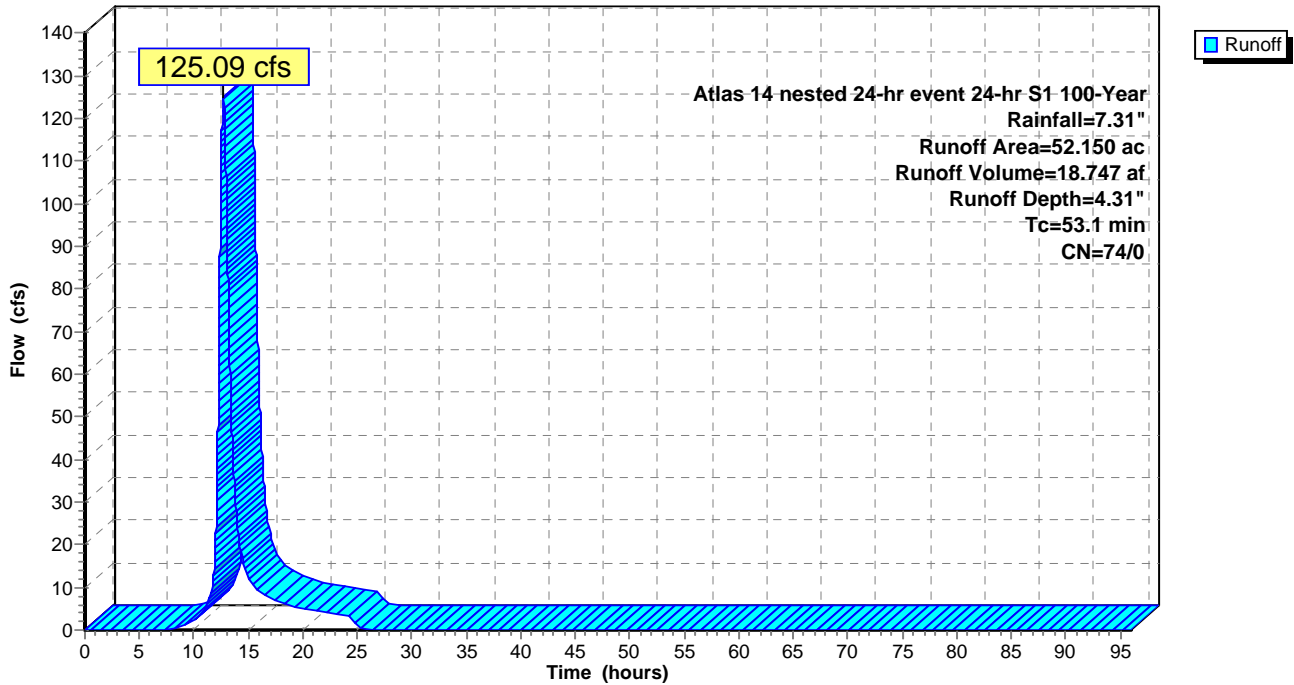
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 52.150	74	pervious
* 0.000	98	impervious
52.150	74	Weighted Average
52.150		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
53.1					Direct Entry,

Subcatchment SB 1: SB 1

Hydrograph



Summary for Subcatchment SB 11: SB 11

Runoff = 19.16 cfs @ 12.11 hrs, Volume= 1.485 af, Depth= 5.42"

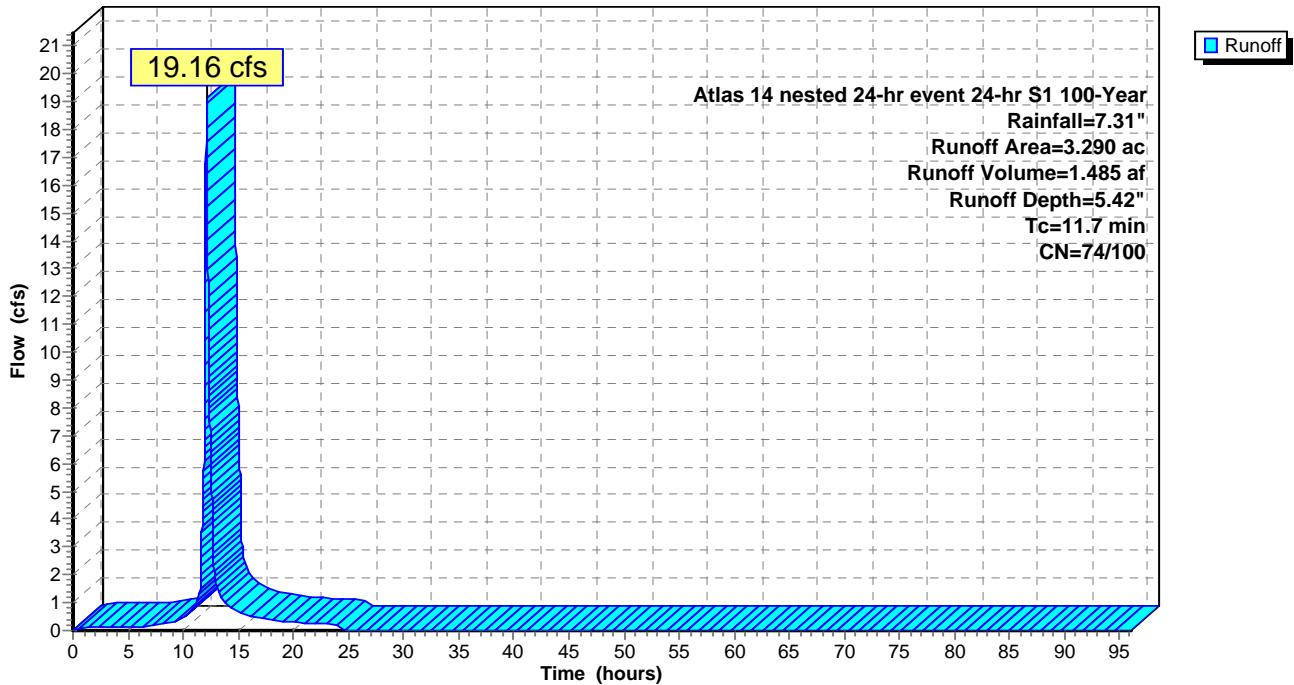
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 2.080	74	pervious
* 1.210	100	impervious
3.290	84	Weighted Average
2.080		63.22% Pervious Area
1.210		36.78% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.7					Direct Entry,

Subcatchment SB 11: SB 11

Hydrograph



Summary for Subcatchment SB 12: SB 12

Runoff = 8.27 cfs @ 12.08 hrs, Volume= 0.566 af, Depth= 4.89"

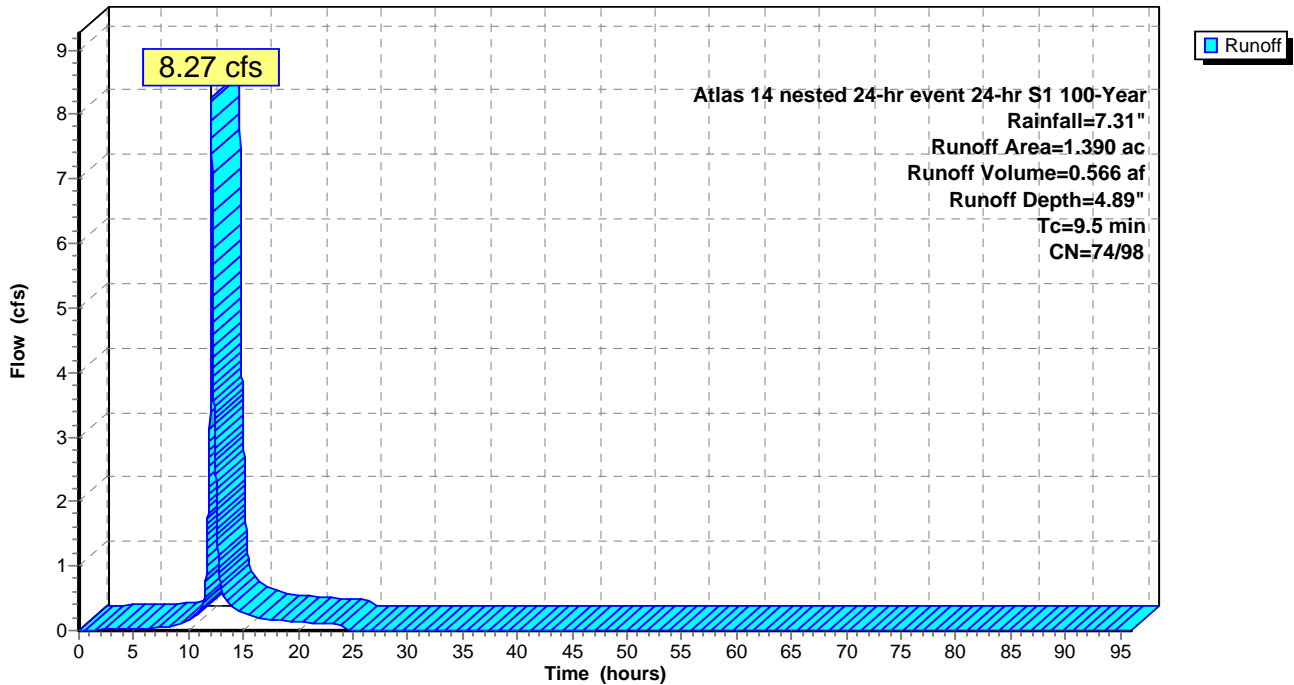
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 1.100	74	pervious
* 0.290	98	impervious
1.390	79	Weighted Average
1.100		79.14% Pervious Area
0.290		20.86% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.5					Direct Entry,

Subcatchment SB 12: SB 12

Hydrograph



Summary for Subcatchment SB 13: SB 13

Runoff = 18.22 cfs @ 12.08 hrs, Volume= 1.266 af, Depth= 5.10"

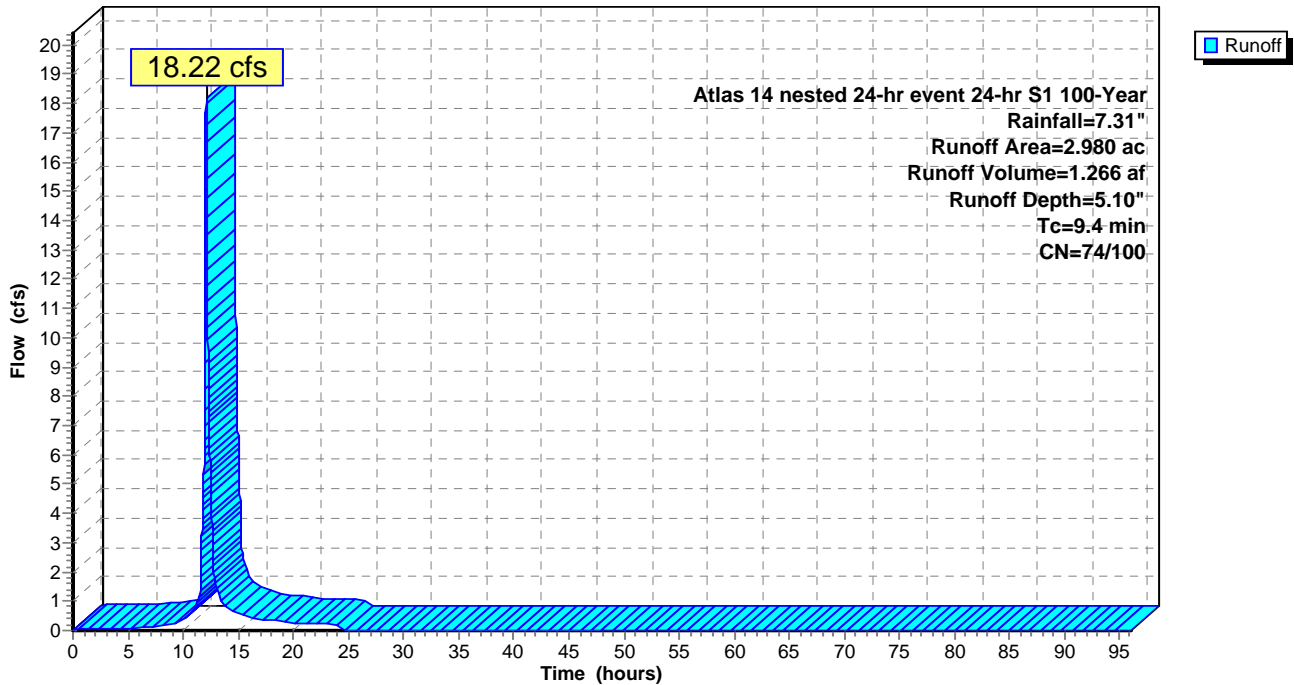
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 2.200	74	pervious
* 0.780	100	impervious
2.980	81	Weighted Average
2.200		73.83% Pervious Area
0.780		26.17% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.4					Direct Entry,

Subcatchment SB 13: SB 13

Hydrograph



Summary for Subcatchment SB 14: SB 14

Runoff = 76.19 cfs @ 12.02 hrs, Volume= 4.054 af, Depth= 4.76"

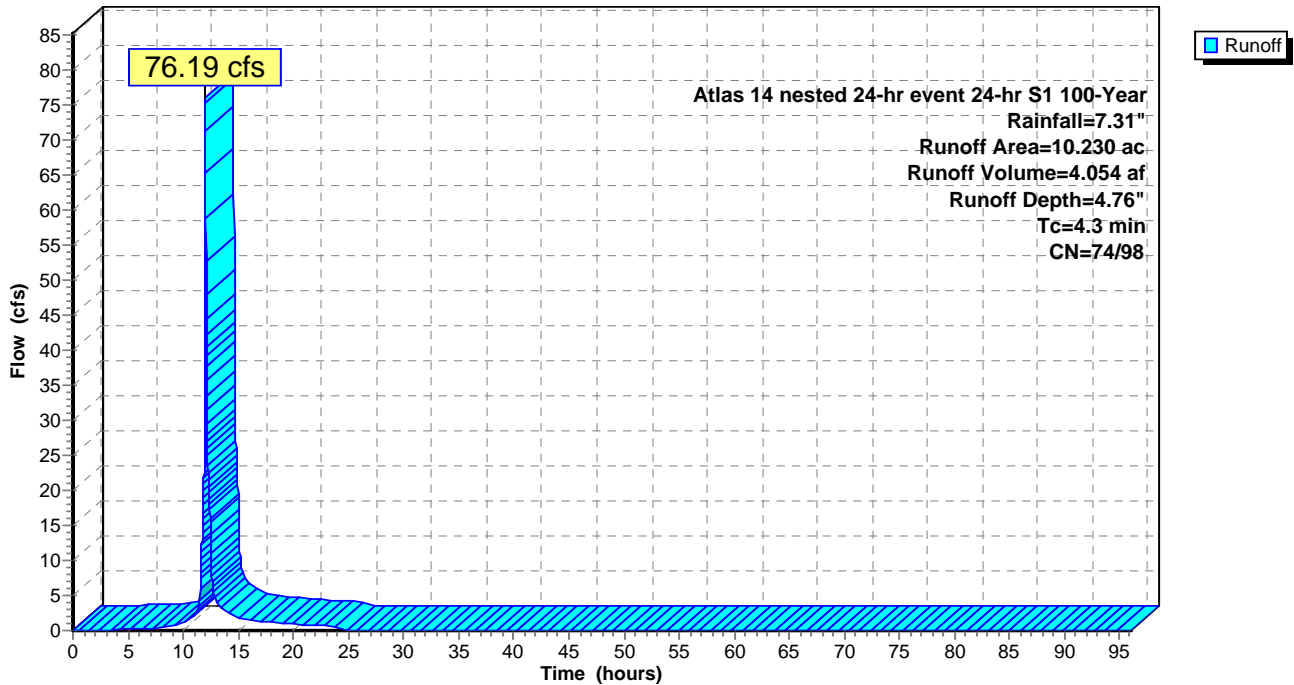
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 8.590	74	pervious
* 1.640	98	impervious
10.230	78	Weighted Average
8.590		83.97% Pervious Area
1.640		16.03% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.3					Direct Entry,

Subcatchment SB 14: SB 14

Hydrograph



Summary for Subcatchment SB 15: SB 15

Runoff = 185.61 cfs @ 12.41 hrs, Volume= 21.062 af, Depth= 4.32"

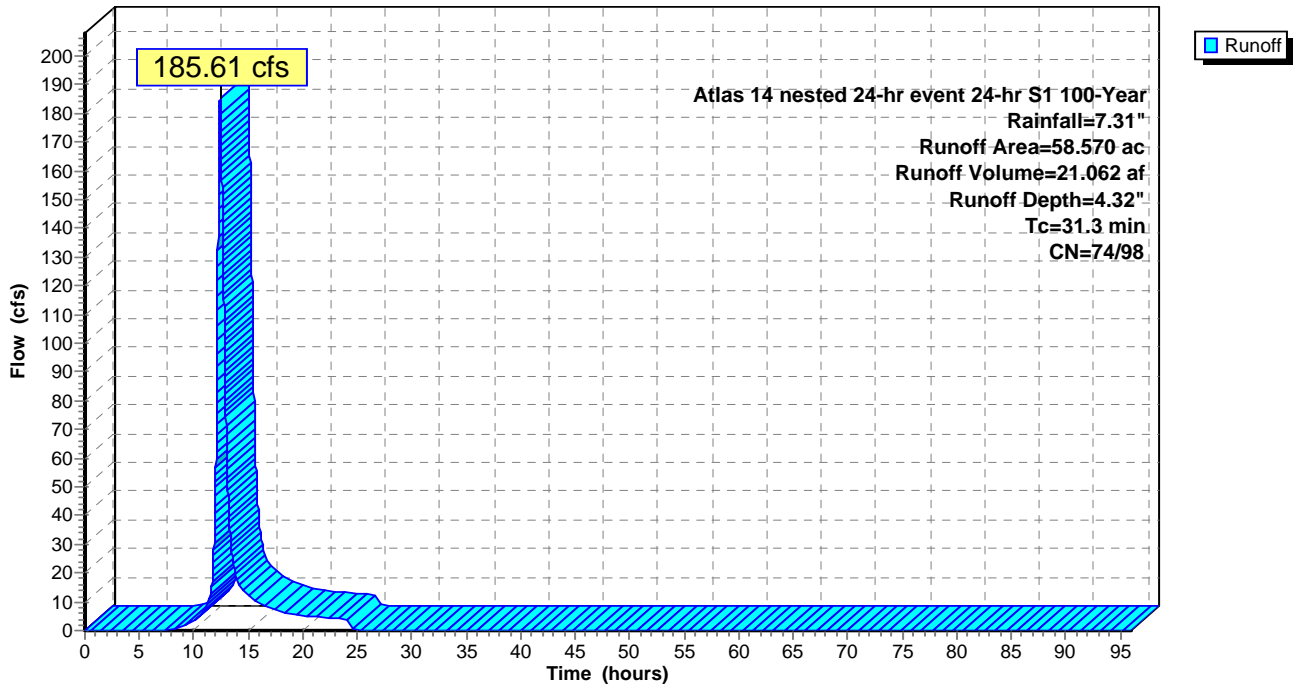
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 58.540	74	pervious
* 0.030	98	impervious
58.570	74	Weighted Average
58.540		99.95% Pervious Area
0.030		0.05% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
31.3					Direct Entry,

Subcatchment SB 15: SB 15

Hydrograph



Summary for Subcatchment SB 16: SB 16

Runoff = 163.65 cfs @ 12.12 hrs, Volume= 12.092 af, Depth= 4.47"

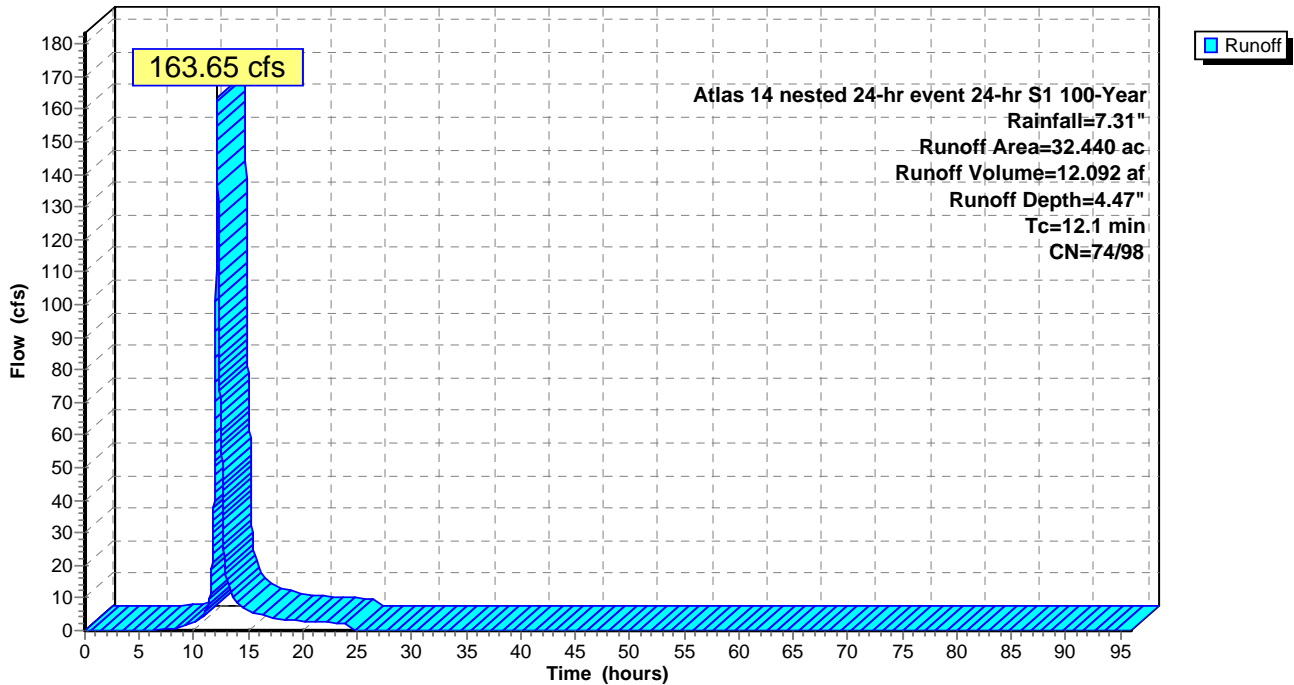
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 30.570	74	pervious
* 1.870	98	impervious
32.440	75	Weighted Average
30.570		94.24% Pervious Area
1.870		5.76% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.1					Direct Entry,

Subcatchment SB 16: SB 16

Hydrograph



Summary for Subcatchment SB 17: SB 17

Runoff = 64.20 cfs @ 12.02 hrs, Volume= 3.655 af, Depth= 5.76"

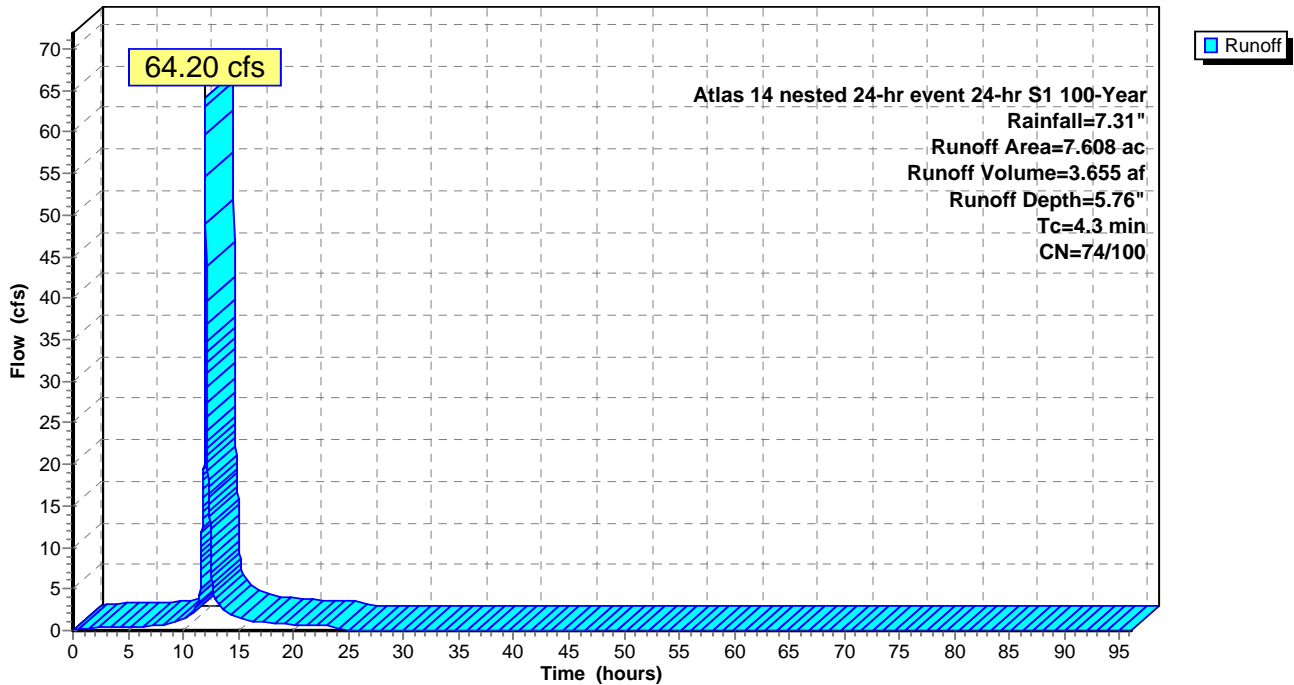
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 3.925	74	pervious
* 3.683	100	impervious
7.608	87	Weighted Average
3.925		51.59% Pervious Area
3.683		48.41% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.3					Direct Entry,

Subcatchment SB 17: SB 17

Hydrograph



Summary for Subcatchment SB 18: SB 18

Runoff = 161.15 cfs @ 12.43 hrs, Volume= 18.977 af, Depth= 4.31"

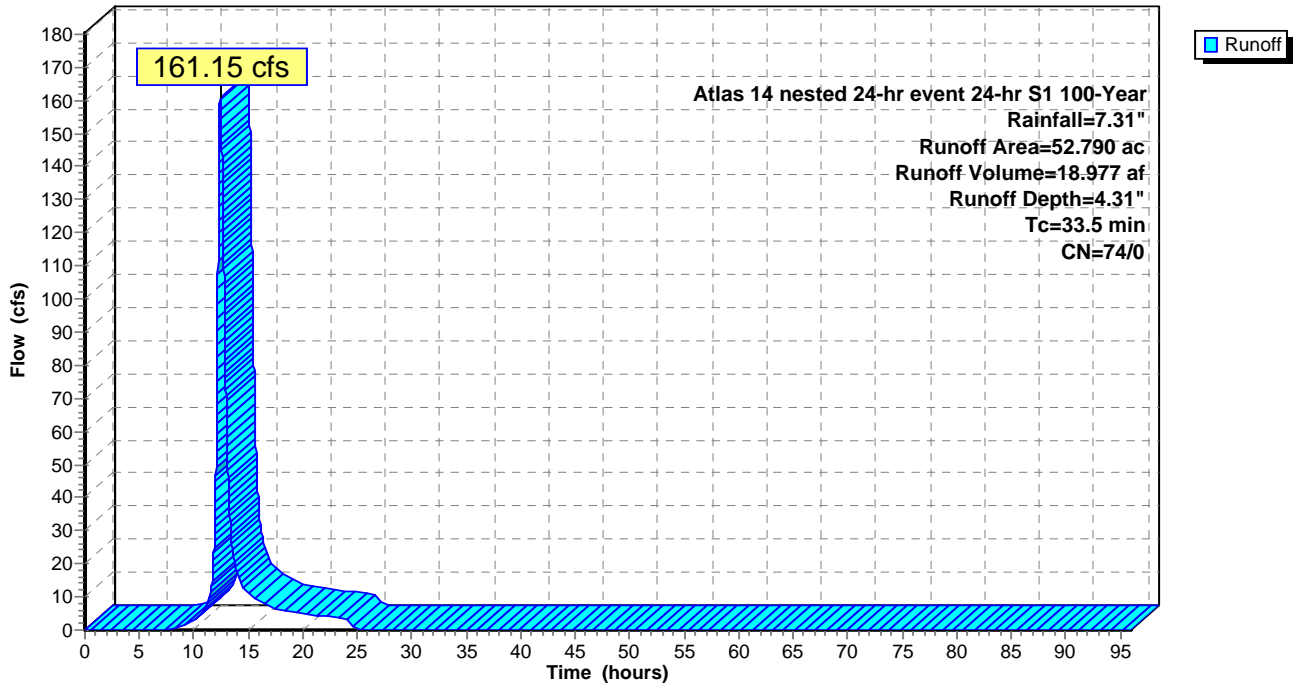
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 52.790	74	pervious
* 0.000	98	impervious
52.790	74	Weighted Average
52.790		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
33.5					Direct Entry,

Subcatchment SB 18: SB 18

Hydrograph



Summary for Subcatchment SB 19: SB 19

Runoff = 75.36 cfs @ 12.30 hrs, Volume= 7.618 af, Depth= 4.31"

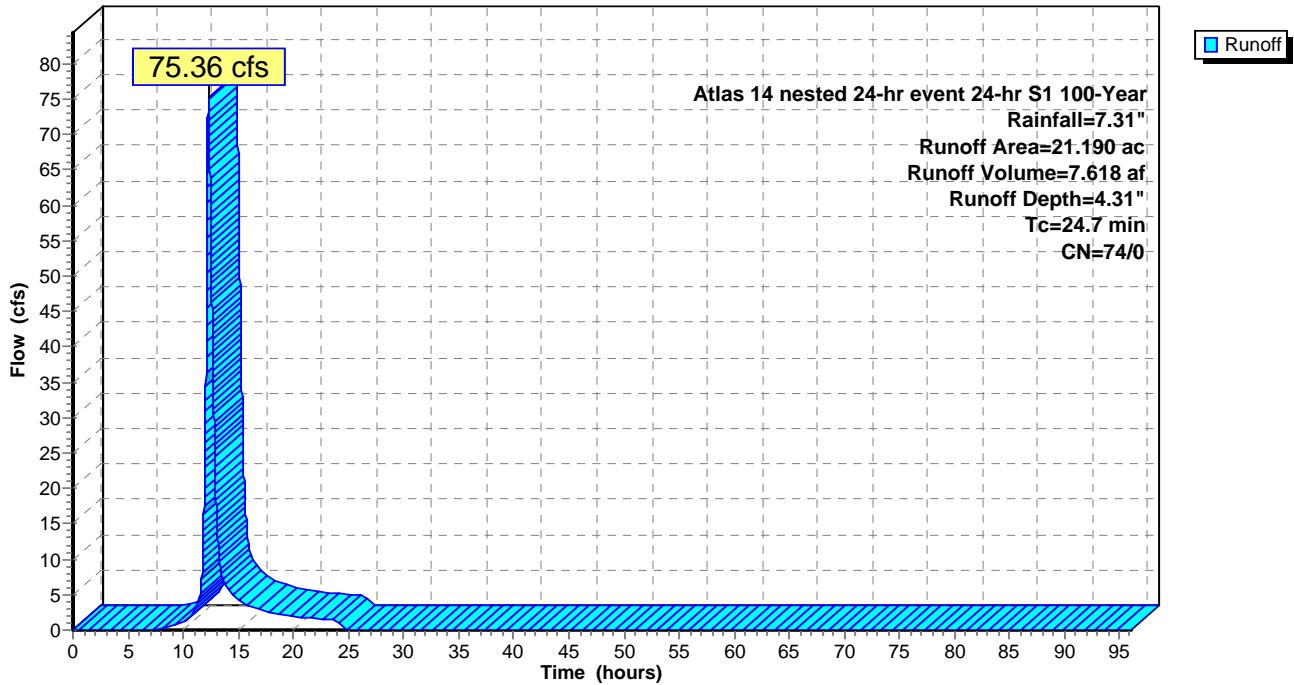
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 21.190	74	pervious
* 0.000	98	impervious
21.190	74	Weighted Average
21.190		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.7					Direct Entry,

Subcatchment SB 19: SB 19

Hydrograph



Summary for Subcatchment SB 2: SB 2

Runoff = 47.56 cfs @ 12.19 hrs, Volume= 3.987 af, Depth= 4.32"

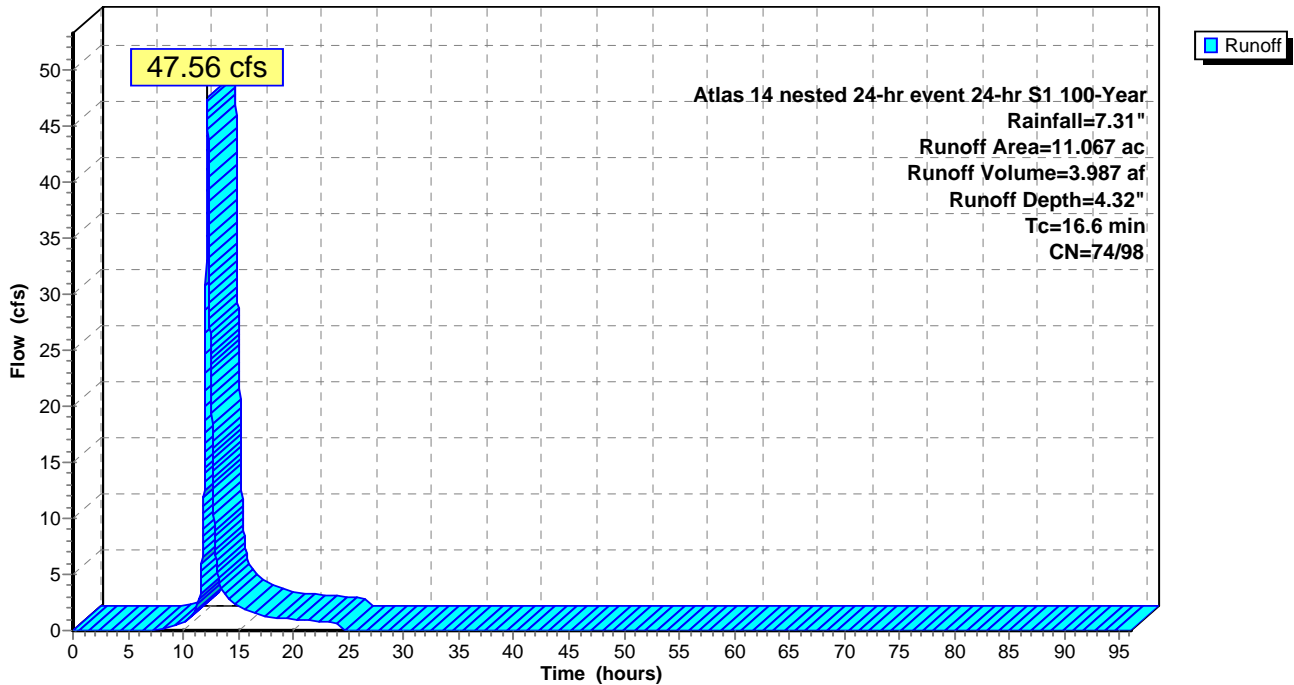
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 11.030	74	pervious
* 0.037	98	impervious
11.067	74	Weighted Average
11.030		99.67% Pervious Area
0.037		0.33% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.6					Direct Entry,

Subcatchment SB 2: SB 2

Hydrograph



Summary for Subcatchment SB 22: SB 22

Runoff = 40.68 cfs @ 12.62 hrs, Volume= 6.106 af, Depth= 1.75"

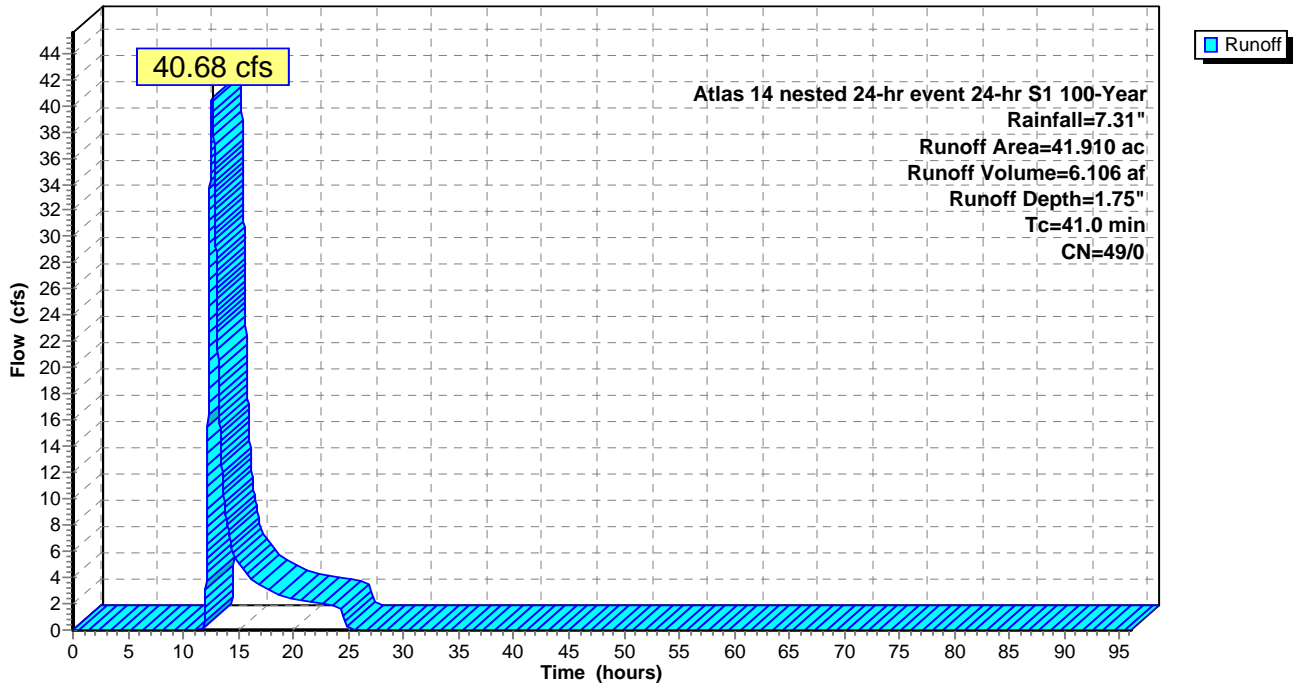
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 41.910	49	Pervious
* 0.000	98	Impervious
41.910	49	Weighted Average
41.910		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
41.0					Direct Entry,

Subcatchment SB 22: SB 22

Hydrograph



Summary for Subcatchment SB 24: SB 24

Runoff = 43.43 cfs @ 12.05 hrs, Volume= 2.943 af, Depth= 7.00"

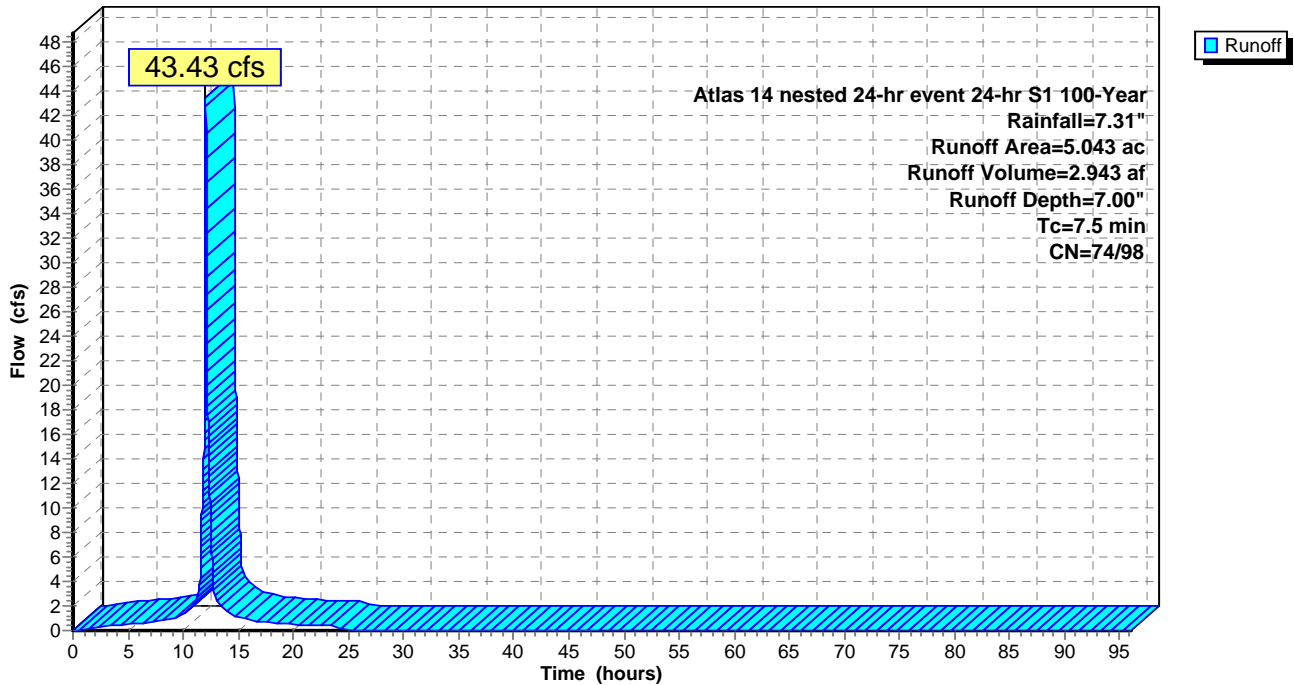
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 0.123	74	permiabile
* 4.920	98	impermiabile
5.043	97	Weighted Average
0.123		2.44% Pervious Area
4.920		97.56% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.5					Direct Entry,

Subcatchment SB 24: SB 24

Hydrograph



Summary for Subcatchment SB 25: SB 25

Runoff = 38.17 cfs @ 12.09 hrs, Volume= 2.976 af, Depth= 6.95"

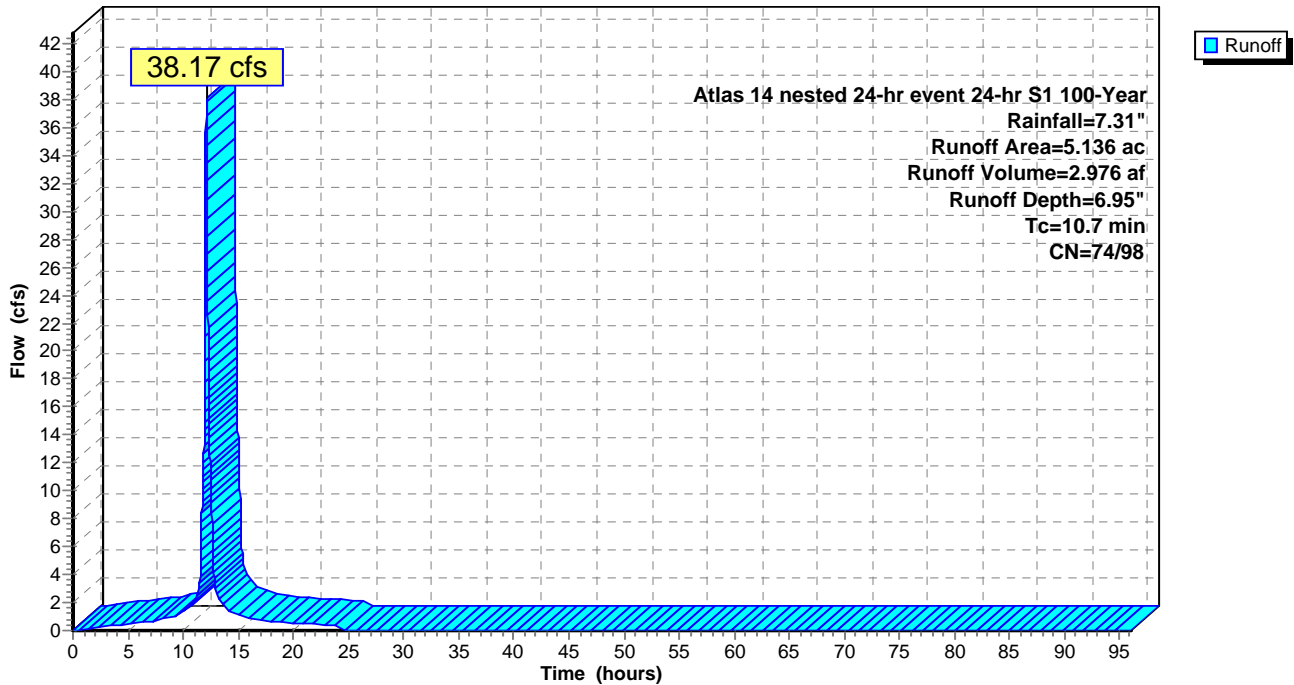
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 0.220	74	pervious
* 4.916	98	impervious
5.136	97	Weighted Average
0.220		4.28% Pervious Area
4.916		95.72% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7					Direct Entry,

Subcatchment SB 25: SB 25

Hydrograph



Summary for Subcatchment SB 26: SB 26

Runoff = 73.07 cfs @ 12.28 hrs, Volume= 8.390 af, Depth= 7.02"

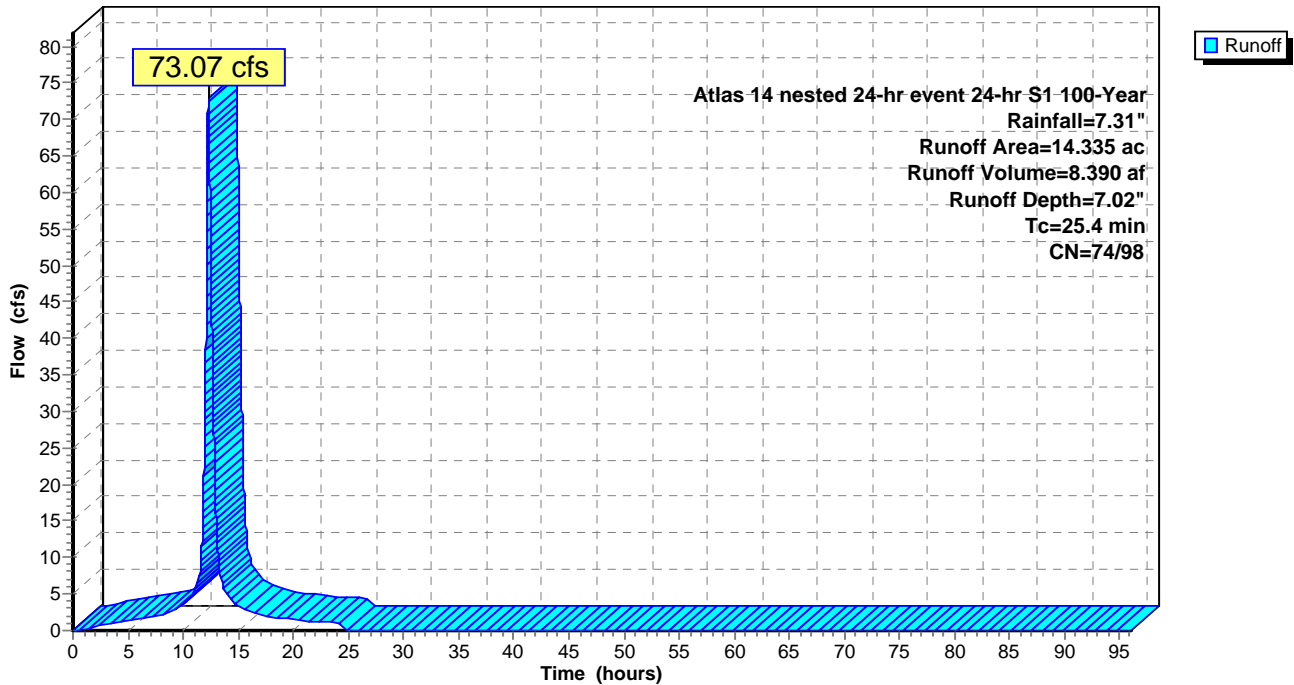
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 0.248	74	pervious
* 14.087	98	impervious
14.335	98	Weighted Average
0.248		1.73% Pervious Area
14.087		98.27% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
25.4					Direct Entry,

Subcatchment SB 26: SB 26

Hydrograph



Summary for Subcatchment SB 27: SB 27 (Thumb Road)

Runoff = 30.89 cfs @ 12.32 hrs, Volume= 3.652 af, Depth= 6.61"

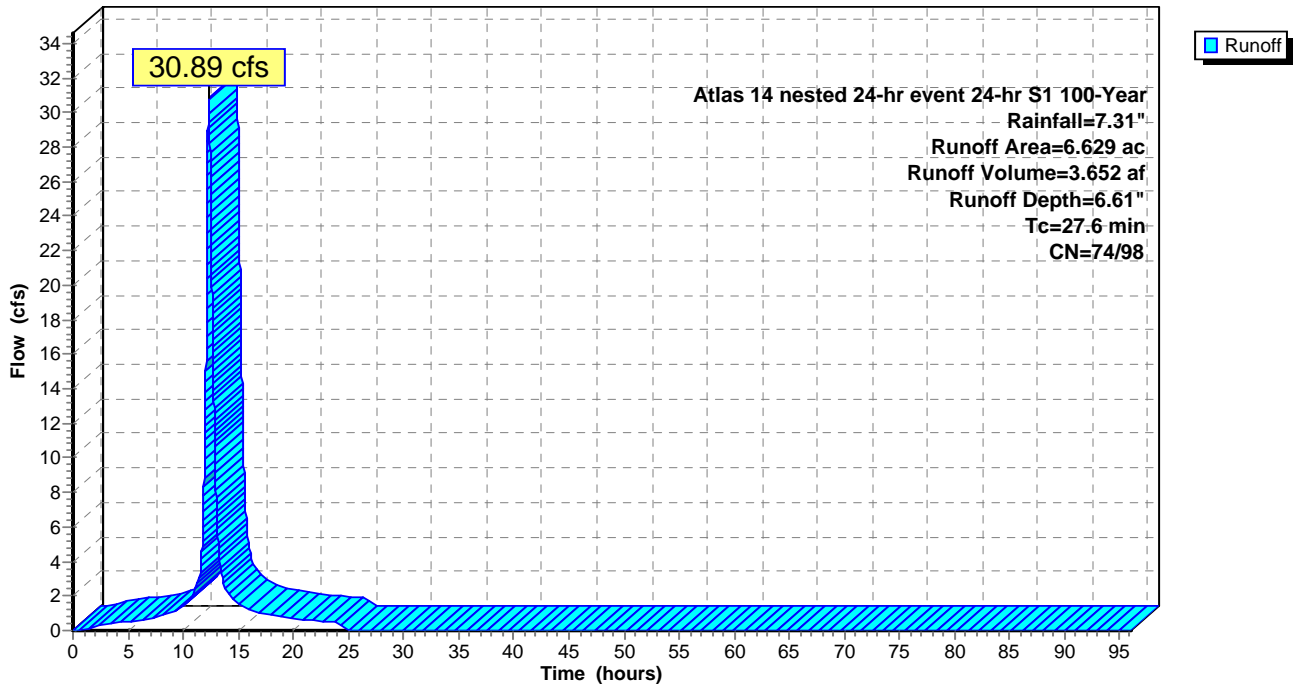
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 1.105	74	Pervious
* 5.524	98	Impervious
6.629	94	Weighted Average
1.105		16.67% Pervious Area
5.524		83.33% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.6					Direct Entry,

Subcatchment SB 27: SB 27 (Thumb Road)

Hydrograph



Summary for Subcatchment SB 28: SB 28

Runoff = 38.37 cfs @ 12.15 hrs, Volume= 3.247 af, Depth= 5.60"

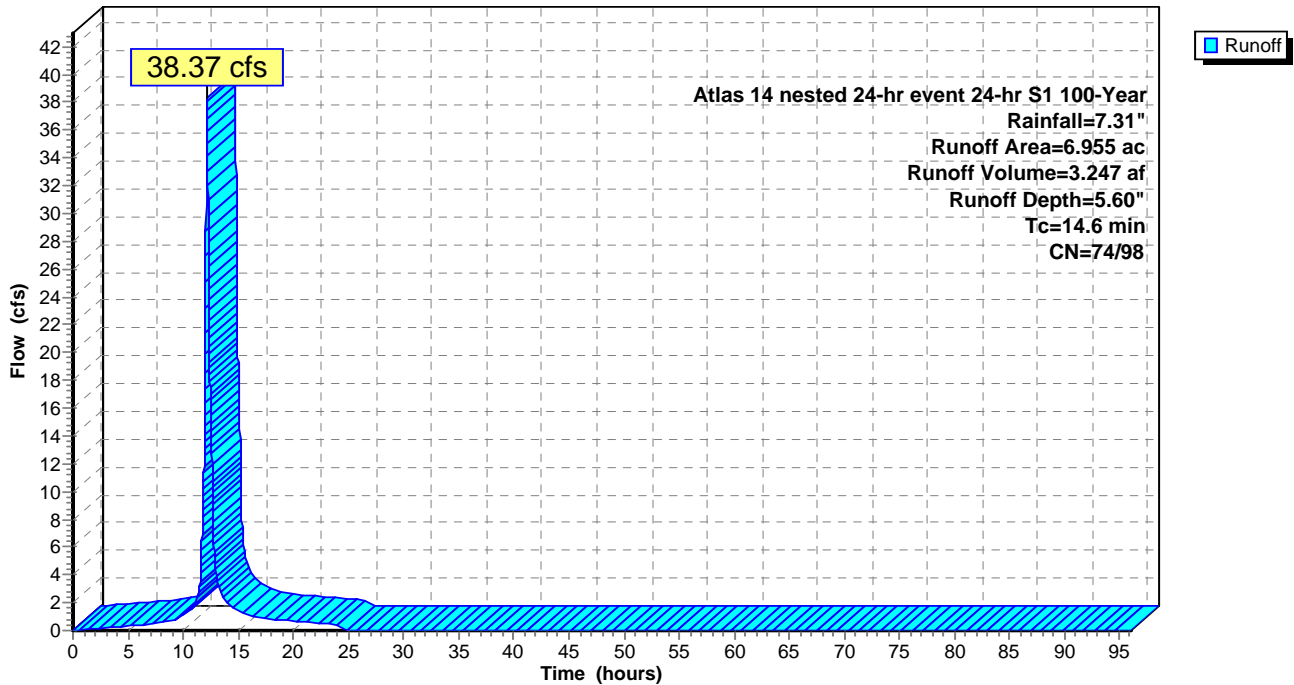
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 3.703	74	pervious
* 3.252	98	impervious
6.955	85	Weighted Average
3.703		53.24% Pervious Area
3.252		46.76% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.6					Direct Entry,

Subcatchment SB 28: SB 28

Hydrograph



Summary for Subcatchment SB 29: SB 29

Runoff = 48.13 cfs @ 12.21 hrs, Volume= 4.557 af, Depth= 5.35"

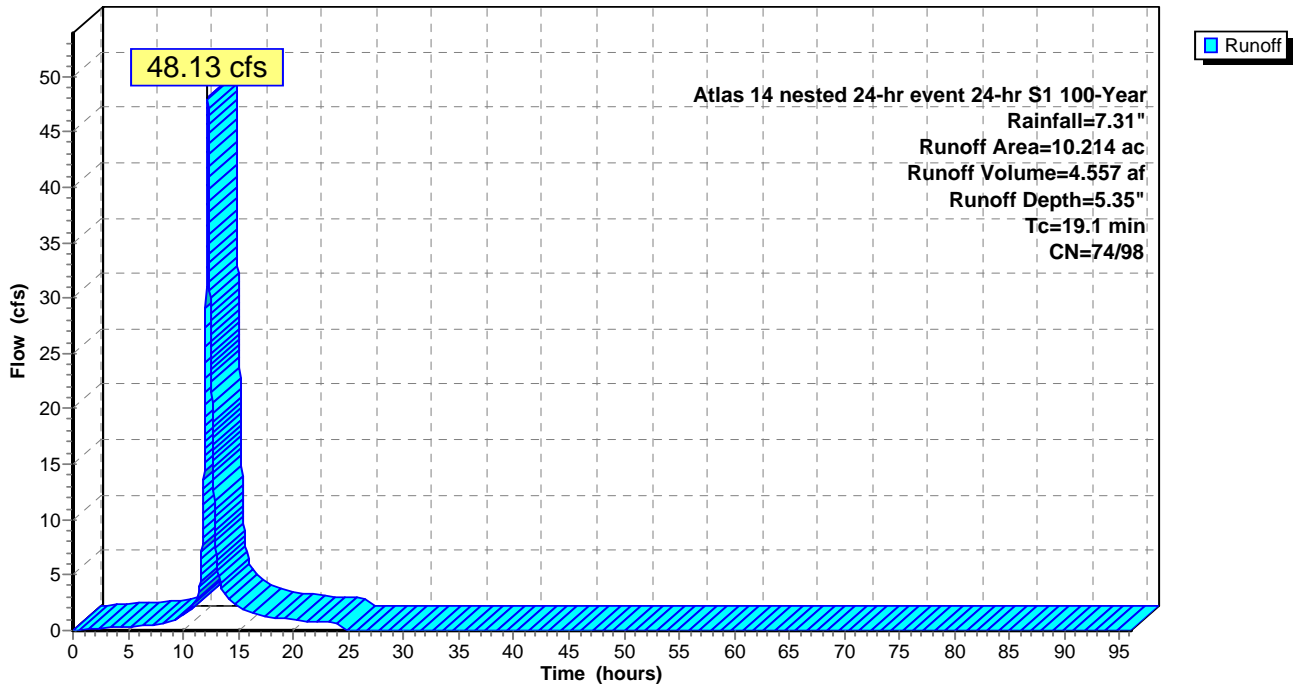
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 6.360	74	pervious
* 3.854	98	impervious
10.214	83	Weighted Average
6.360		62.27% Pervious Area
3.854		37.73% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.1					Direct Entry,

Subcatchment SB 29: SB 29

Hydrograph



Summary for Subcatchment SB 3: SB 3

Runoff = 173.90 cfs @ 12.16 hrs, Volume= 14.184 af, Depth= 4.53"

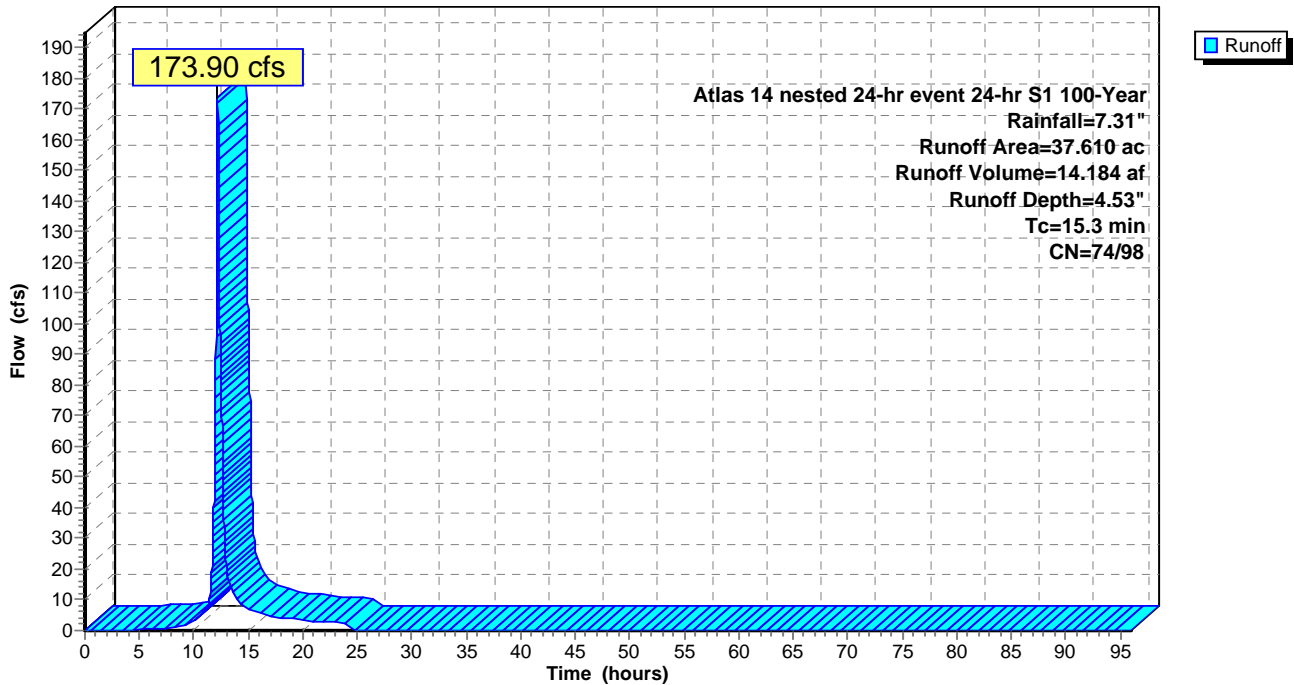
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 34.720	74	Pervious
* 2.890	98	Impervious
37.610	76	Weighted Average
34.720		92.32% Pervious Area
2.890		7.68% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.3					Direct Entry,

Subcatchment SB 3: SB 3

Hydrograph



Summary for Subcatchment SB 4: SB 4

Runoff = 4.61 cfs @ 12.04 hrs, Volume= 0.281 af, Depth= 5.61"

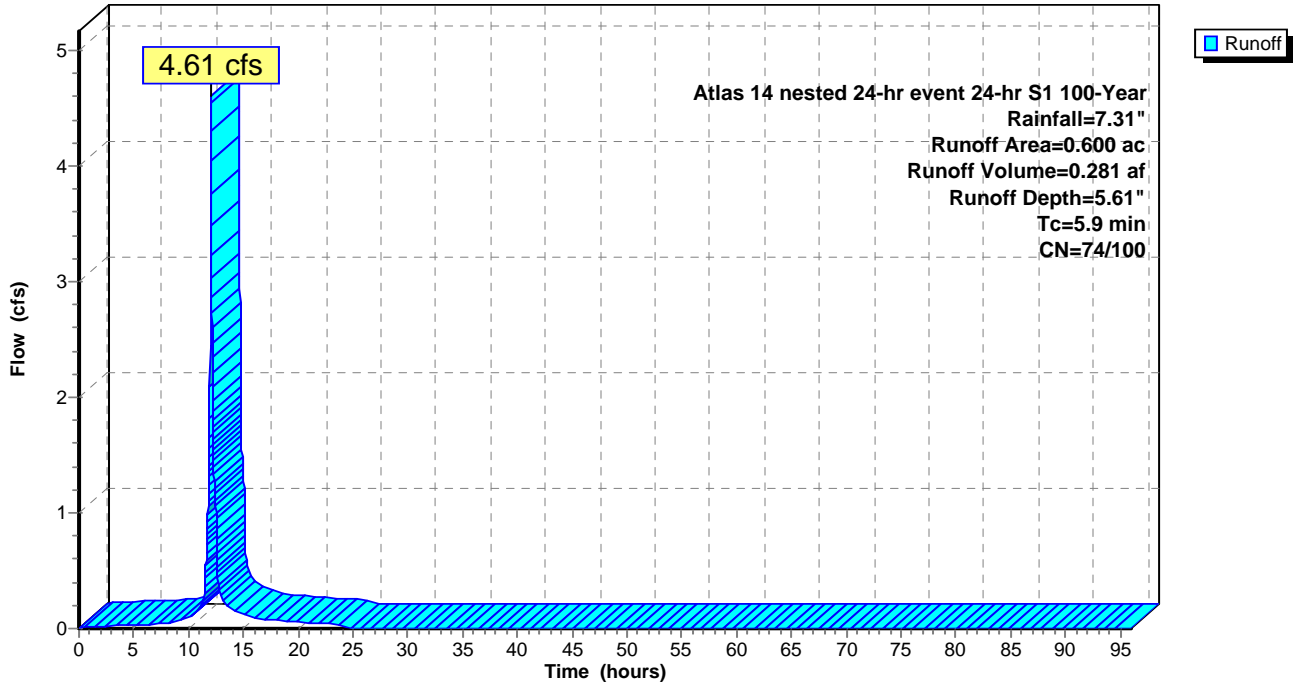
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 0.340	74	pervious
* 0.260	100	impervious
0.600	85	Weighted Average
0.340		56.67% Pervious Area
0.260		43.33% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.9					Direct Entry,

Subcatchment SB 4: SB 4

Hydrograph



Summary for Subcatchment SB 5: SB 5

Runoff = 18.03 cfs @ 12.78 hrs, Volume= 2.934 af, Depth= 4.48"

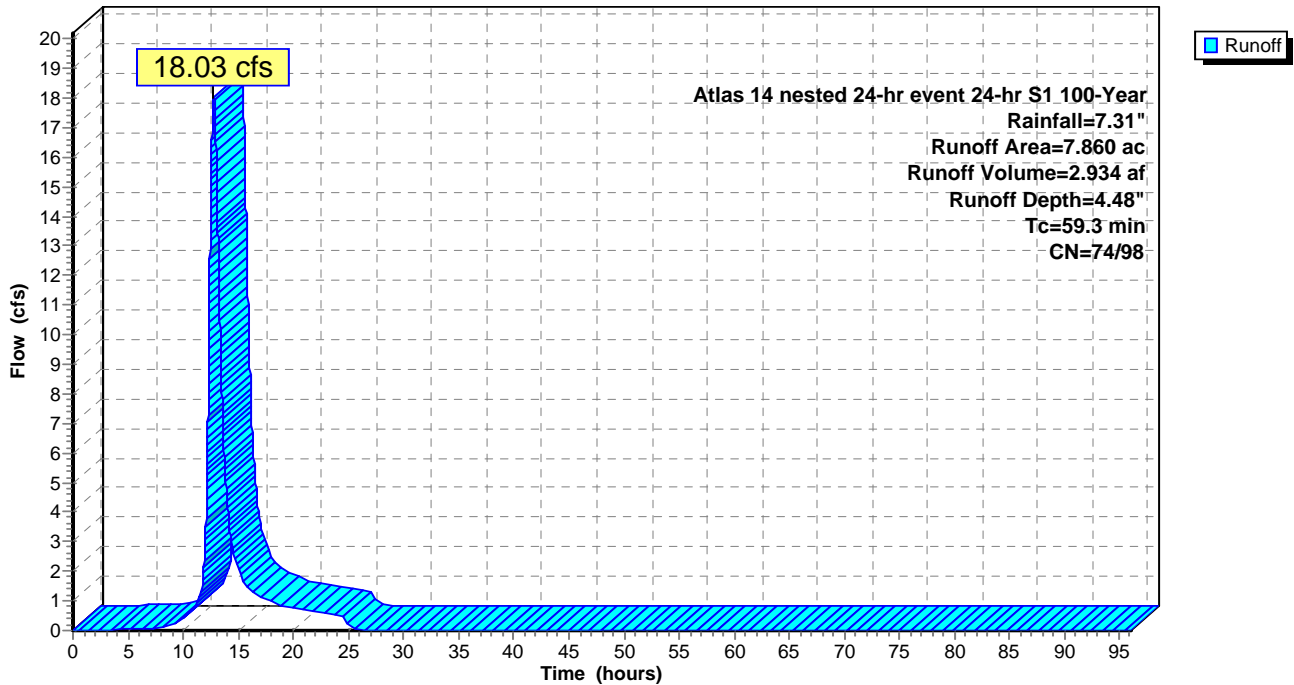
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 7.390	74	pervious
* 0.470	98	impervious
7.860	75	Weighted Average
7.390		94.02% Pervious Area
0.470		5.98% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
59.3					Direct Entry,

Subcatchment SB 5: SB 5

Hydrograph



Summary for Subcatchment SB 6: SB 6

Runoff = 4.09 cfs @ 12.24 hrs, Volume= 0.384 af, Depth= 4.61"

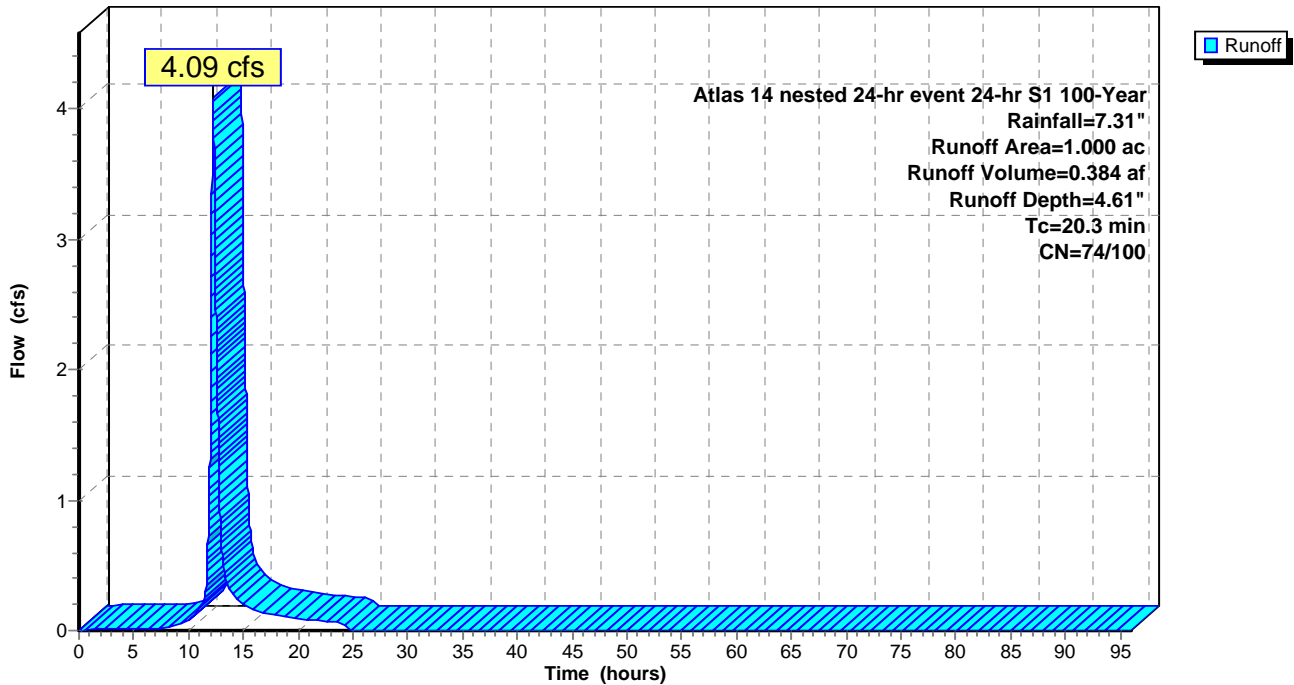
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 0.900	74	pervious
* 0.100	100	impervious
1.000	77	Weighted Average
0.900		90.00% Pervious Area
0.100		10.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.3					Direct Entry,

Subcatchment SB 6: SB 6

Hydrograph



Summary for Subcatchment SB 7: SB 7

Runoff = 140.12 cfs @ 12.04 hrs, Volume= 7.747 af, Depth= 4.31"

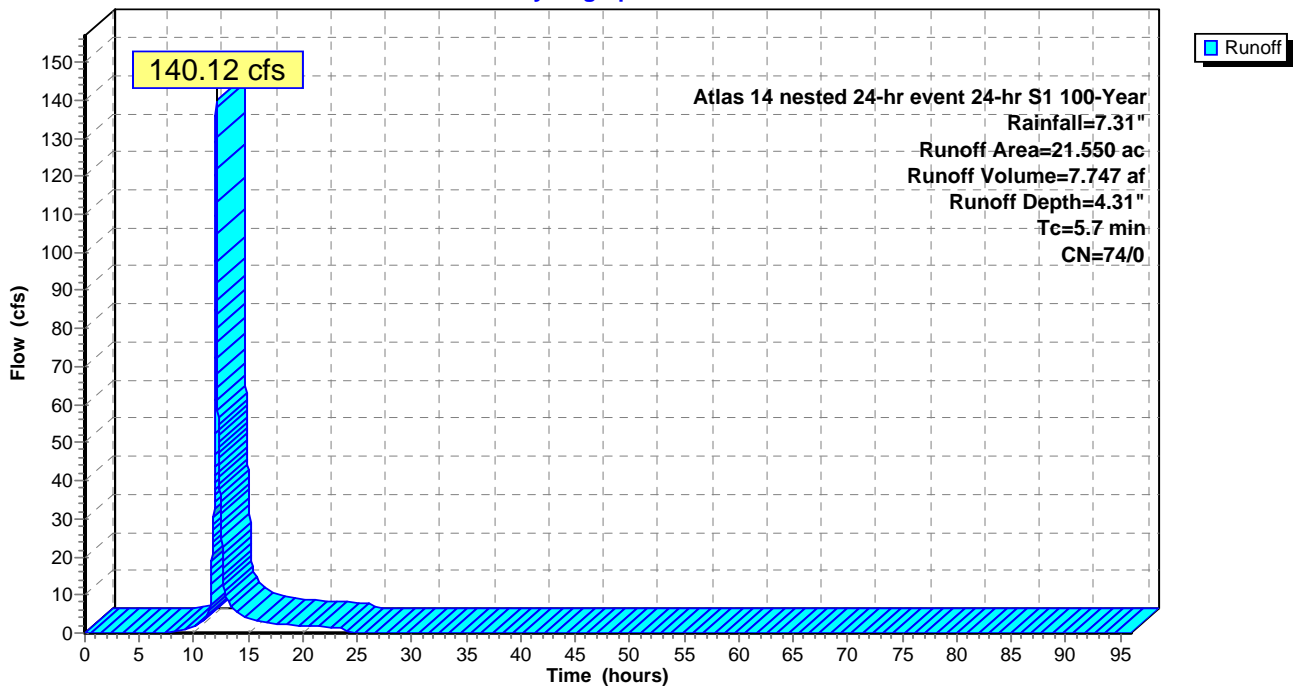
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 21.550	74	pervious
* 0.000	98	impervious
21.550	74	Weighted Average
21.550		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.7					Direct Entry,

Subcatchment SB 7: SB 7

Hydrograph



Summary for Subcatchment SB 8: SB 8

Runoff = 77.55 cfs @ 12.61 hrs, Volume= 11.008 af, Depth= 4.47"

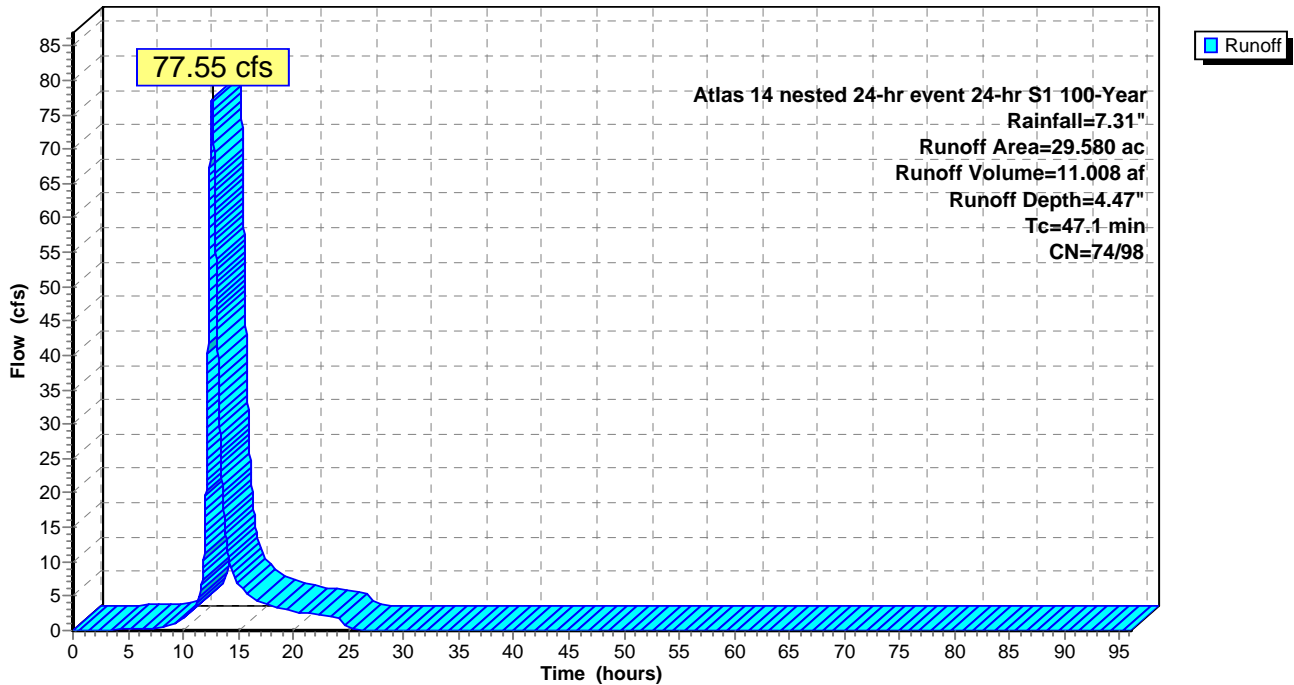
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 27.950	74	pervious
* 1.630	98	impervious
29.580	75	Weighted Average
27.950		94.49% Pervious Area
1.630		5.51% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
47.1					Direct Entry,

Subcatchment SB 8: SB 8

Hydrograph



Summary for Subcatchment SB 9: SB 9

Runoff = 83.43 cfs @ 12.37 hrs, Volume= 9.275 af, Depth= 4.32"

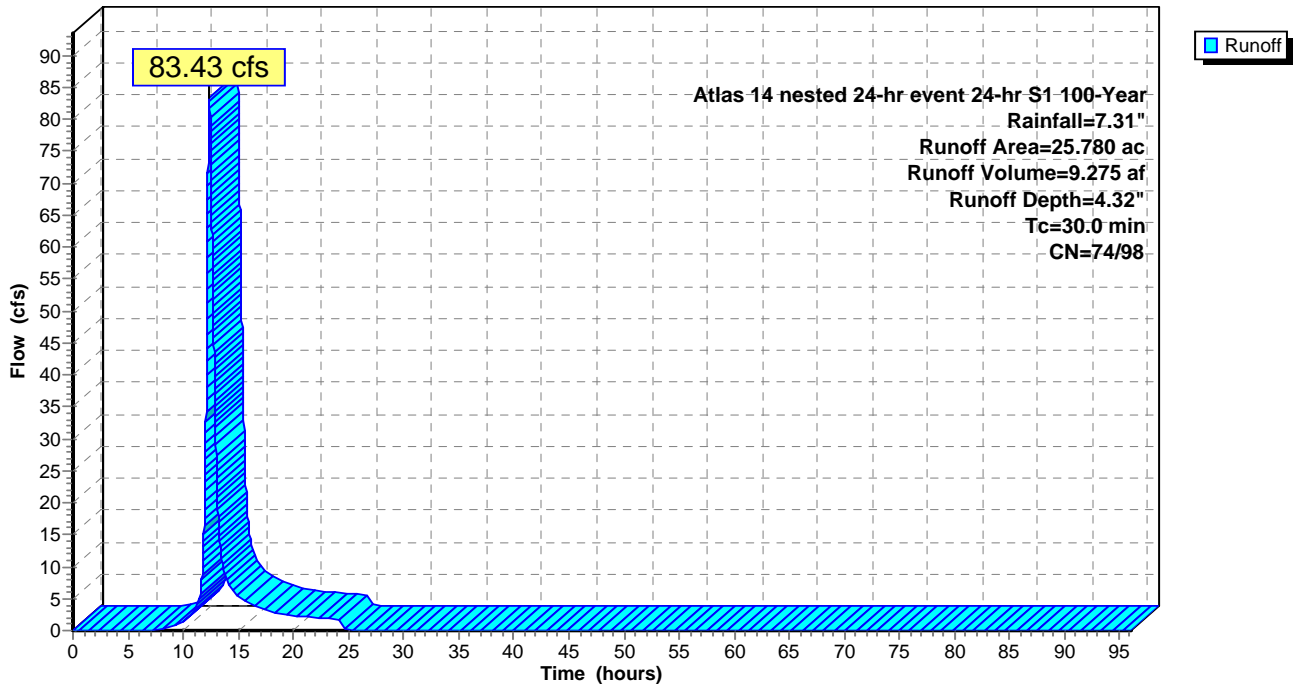
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 25.750	74	permiabile
* 0.030	98	impermiabile
25.780	74	Weighted Average
25.750		99.88% Pervious Area
0.030		0.12% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.0					Direct Entry,

Subcatchment SB 9: SB 9

Hydrograph



Summary for Subcatchment SB10: SB 10

Runoff = 39.26 cfs @ 12.05 hrs, Volume= 2.368 af, Depth= 4.45"

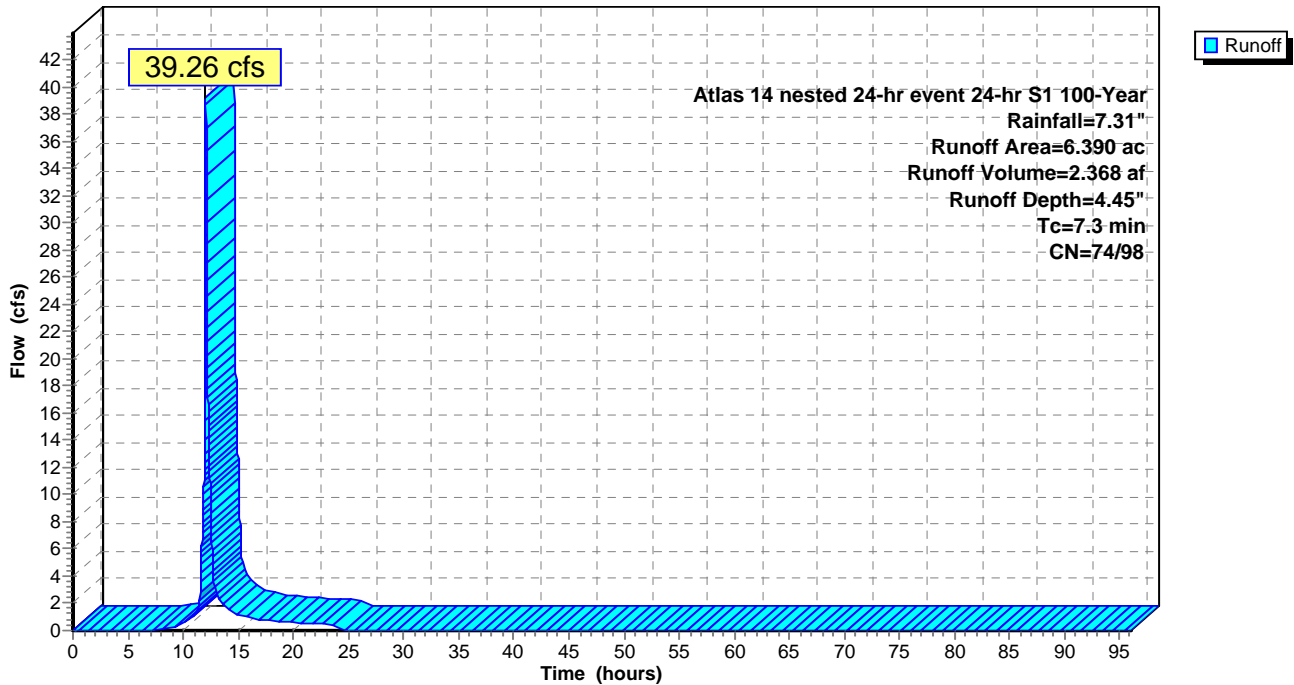
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 6.080	74	pervious
* 0.310	98	impervious
6.390	75	Weighted Average
6.080		95.15% Pervious Area
0.310		4.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.3					Direct Entry,

Subcatchment SB10: SB 10

Hydrograph



Summary for Pond 2 P: P-2

Inflow Area = 68.260 ac, 7.26% Impervious, Inflow Depth = 4.51" for 100-Year event
 Inflow = 146.98 cfs @ 12.57 hrs, Volume= 25.678 af
 Outflow = 146.30 cfs @ 12.62 hrs, Volume= 25.678 af, Atten= 0%, Lag= 2.6 min
 Primary = 146.30 cfs @ 12.62 hrs, Volume= 25.678 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 924.00' Surf.Area= 0.370 ac Storage= 0.730 af
 Peak Elev= 925.48' @ 12.62 hrs Surf.Area= 0.466 ac Storage= 1.347 af (0.617 af above start)

Plug-Flow detention time= 38.1 min calculated for 24.948 af (97% of inflow)
 Center-of-Mass det. time= 11.7 min (855.2 - 843.5)

Volume	Invert	Avail.Storage	Storage Description
#1	920.00'	1.600 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
920.00	0.100	0.000	0.000
922.00	0.130	0.230	0.230
924.00	0.370	0.500	0.730
926.00	0.500	0.870	1.600

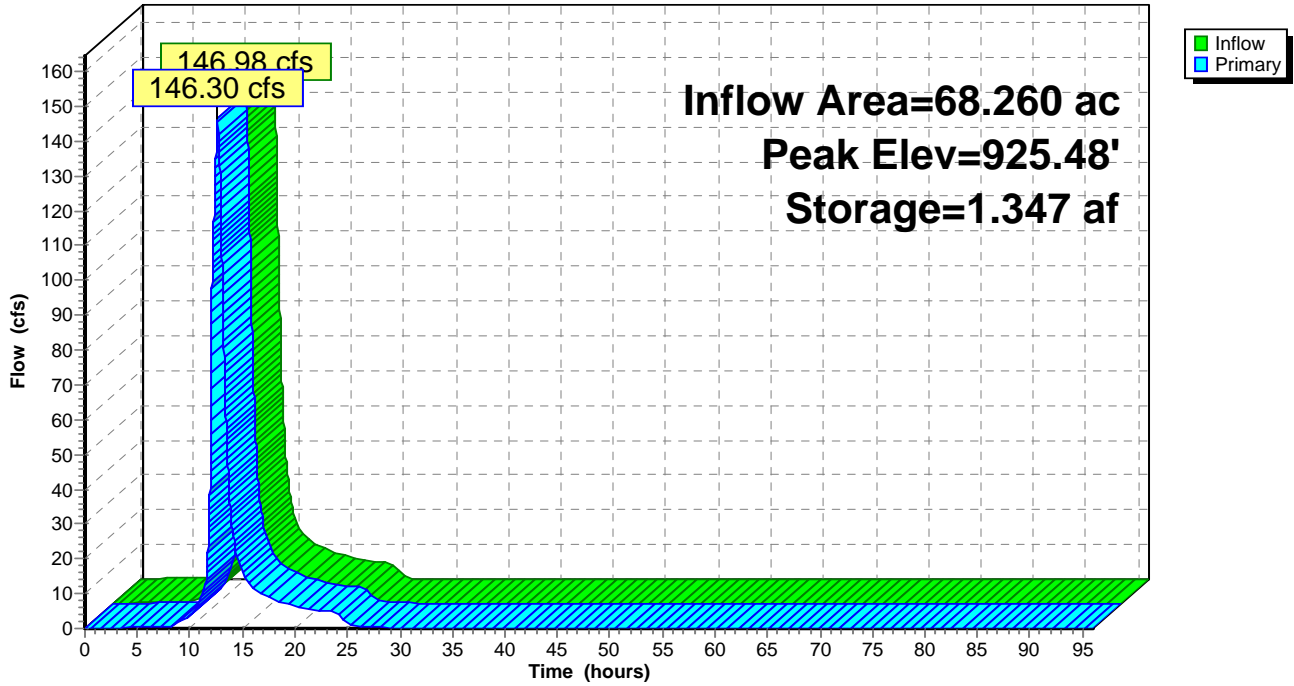
Device	Routing	Invert	Outlet Devices
#1	Primary	924.40'	40.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Primary	924.00'	6.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=146.30 cfs @ 12.62 hrs HW=925.48' TW=0.00' (Dynamic Tailwater)

- 1=Sharp-Crested Rectangular Weir (Weir Controls 145.15 cfs @ 3.39 fps)
- 2=Orifice/Grate (Orifice Controls 1.15 cfs @ 5.85 fps)

Pond 2 P: P-2

Hydrograph



Summary for Pond 4P: P-4

Inflow Area = 7.860 ac, 5.98% Impervious, Inflow Depth = 4.48" for 100-Year event
 Inflow = 18.03 cfs @ 12.78 hrs, Volume= 2.934 af
 Outflow = 11.60 cfs @ 13.25 hrs, Volume= 2.934 af, Atten= 36%, Lag= 28.5 min
 Primary = 8.50 cfs @ 13.26 hrs, Volume= 1.519 af
 Secondary = 3.10 cfs @ 13.22 hrs, Volume= 1.415 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 915.00' Surf.Area= 0.275 ac Storage= 0.646 af
 Peak Elev= 917.34' @ 13.26 hrs Surf.Area= 0.410 ac Storage= 1.444 af (0.798 af above start)

Plug-Flow detention time= 192.9 min calculated for 2.288 af (78% of inflow)
 Center-of-Mass det. time= 57.0 min (917.8 - 860.7)

Volume	Invert	Avail.Storage	Storage Description
#1	910.90'	1.728 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
910.90	0.070	0.000	0.000
912.00	0.090	0.088	0.088
914.00	0.220	0.310	0.398
916.00	0.330	0.550	0.948
918.00	0.450	0.780	1.728

Device	Routing	Invert	Outlet Devices
#1	Primary	915.00'	6.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Secondary	915.00'	9.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	915.95'	24.0" Round RCP_Round 24" L= 50.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 915.80' / 915.95' S= -0.0030 ' /' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Primary OutFlow Max=8.50 cfs @ 13.26 hrs HW=917.34' TW=0.00' (Dynamic Tailwater)

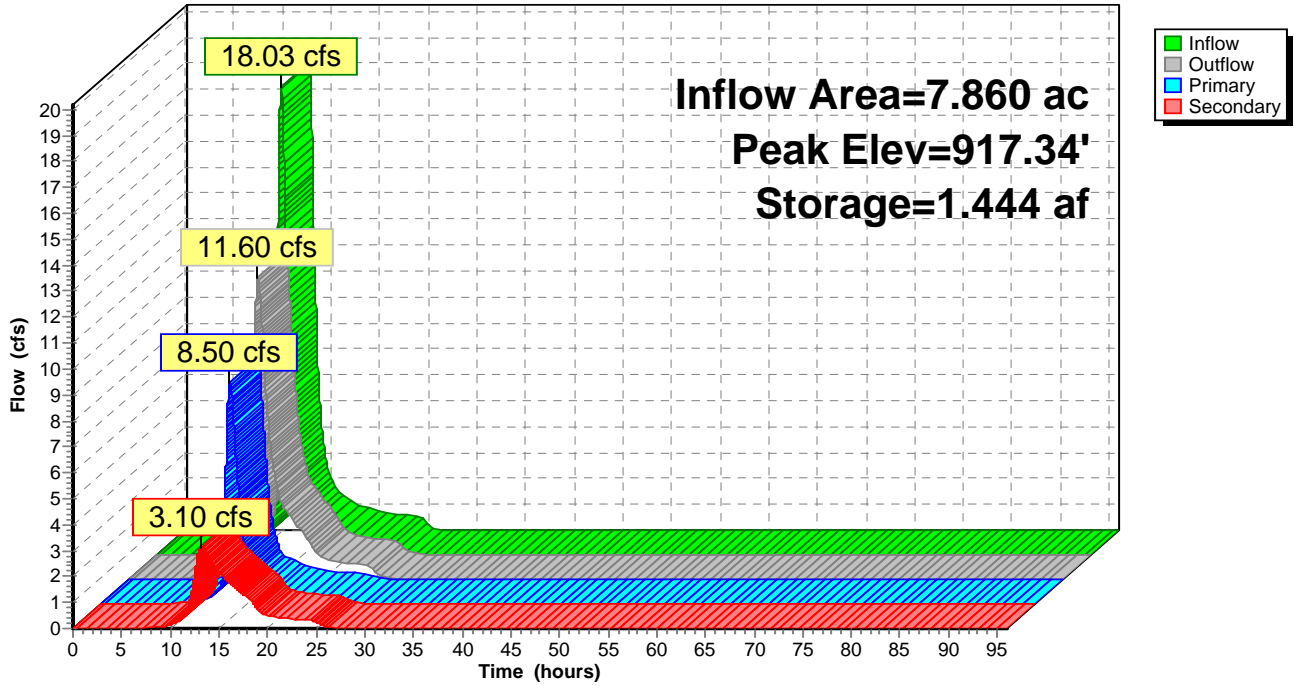
- ↑1=Orifice/Grate (Orifice Controls 1.45 cfs @ 7.36 fps)
- ↑3=RCP_Round 24" (Barrel Controls 7.06 cfs @ 3.76 fps)

Secondary OutFlow Max=3.10 cfs @ 13.22 hrs HW=917.34' TW=915.22' (Dynamic Tailwater)

- ↑2=Orifice/Grate (Orifice Controls 3.10 cfs @ 7.01 fps)

Pond 4P: P-4

Hydrograph



Summary for Pond 7P: P-7

[90] Warning: Qout>Qin may require smaller dt or Finer Routing

Inflow Area = 29.580 ac, 5.51% Impervious, Inflow Depth = 4.47" for 100-Year event
 Inflow = 77.55 cfs @ 12.61 hrs, Volume= 11.008 af
 Outflow = 78.63 cfs @ 12.65 hrs, Volume= 11.008 af, Atten= 0%, Lag= 2.1 min
 Primary = 78.63 cfs @ 12.65 hrs, Volume= 11.008 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 915.00' Surf.Area= 0.440 ac Storage= 1.062 af
 Peak Elev= 915.86' @ 12.56 hrs Surf.Area= 0.543 ac Storage= 1.484 af (0.422 af above start)

Plug-Flow detention time= 77.7 min calculated for 9.945 af (90% of inflow)
 Center-of-Mass det. time= 7.1 min (857.2 - 850.1)

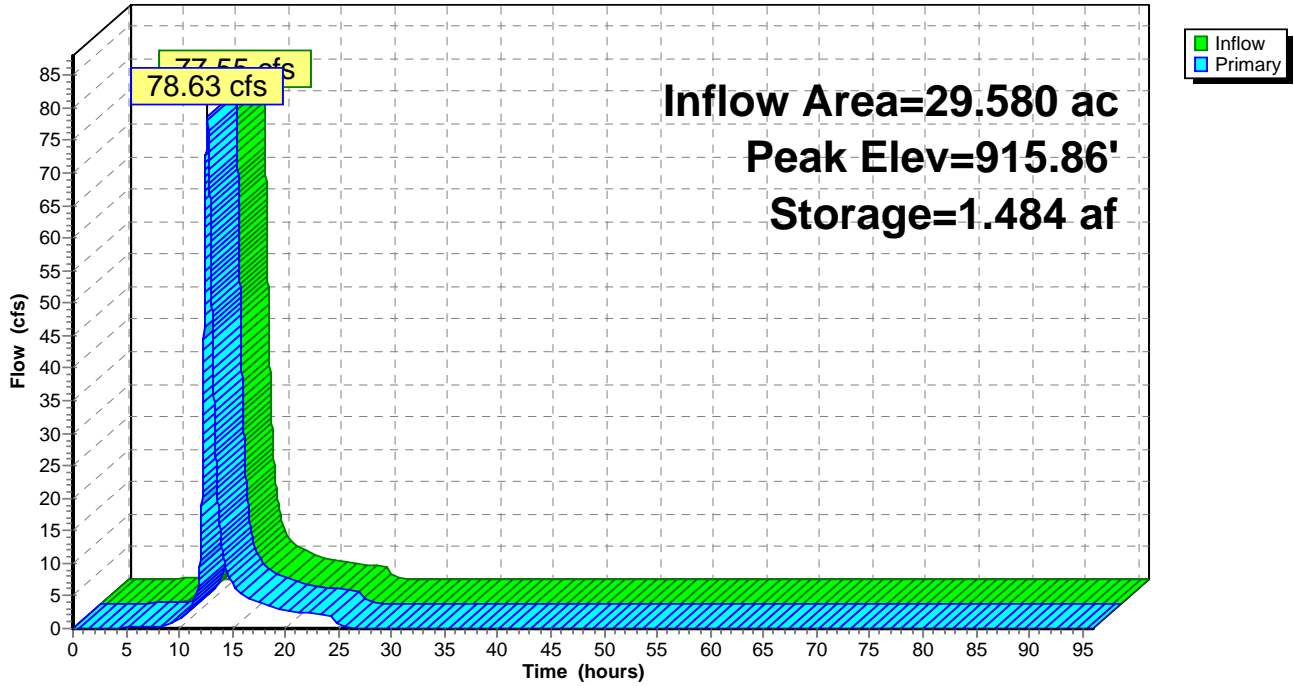
Volume	Invert	Avail.Storage	Storage Description
#1	910.95'	1.562 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
910.95	0.110	0.000	0.000
912.00	0.180	0.152	0.152
914.00	0.340	0.520	0.672
915.00	0.440	0.390	1.062
916.00	0.560	0.500	1.562

Device	Routing	Invert	Outlet Devices
#1	Primary	915.00'	75.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=79.64 cfs @ 12.65 hrs HW=915.85' TW=915.75' (Dynamic Tailwater)
 ↑1=**Broad-Crested Rectangular Weir** (Weir Controls 79.64 cfs @ 1.25 fps)

Pond 7P: P-7

Hydrograph



Summary for Pond 8P: P-8

Inflow Area = 6.390 ac, 4.85% Impervious, Inflow Depth = 4.45" for 100-Year event
 Inflow = 39.26 cfs @ 12.05 hrs, Volume= 2.368 af
 Outflow = 10.88 cfs @ 12.31 hrs, Volume= 2.363 af, Atten= 72%, Lag= 15.5 min
 Primary = 10.88 cfs @ 12.31 hrs, Volume= 2.363 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 897.00' Surf.Area= 0.300 ac Storage= 0.495 af
 Peak Elev= 899.07' @ 12.55 hrs Surf.Area= 0.493 ac Storage= 1.375 af (0.880 af above start)

Plug-Flow detention time= 268.6 min calculated for 1.868 af (79% of inflow)
 Center-of-Mass det. time= 127.9 min (941.9 - 813.9)

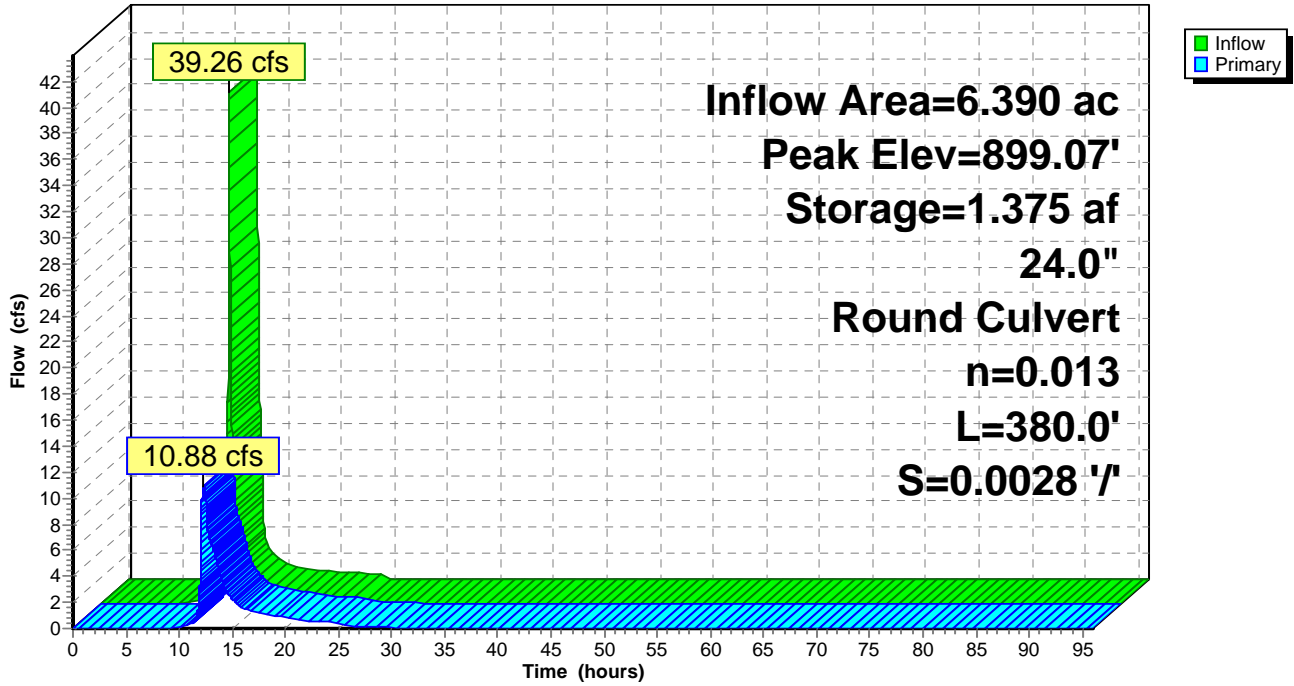
Volume	Invert	Avail.Storage	Storage Description
#1	893.00'	1.850 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
893.00	0.030	0.000	0.000
894.00	0.070	0.050	0.050
896.00	0.150	0.220	0.270
897.00	0.300	0.225	0.495
898.00	0.450	0.375	0.870
900.00	0.530	0.980	1.850

Device	Routing	Invert	Outlet Devices
#1	Primary	897.00'	24.0" Round RCP_Round 24" L= 380.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 897.00' / 895.94' S= 0.0028 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Primary OutFlow Max=10.83 cfs @ 12.31 hrs HW=899.04' TW=897.70' (Dynamic Tailwater)
 ↳1=RCP_Round 24" (Outlet Controls 10.83 cfs @ 4.20 fps)

Pond 8P: P-8

Hydrograph



Summary for Pond 9P: P-9

Inflow Area = 55.360 ac, 3.00% Impervious, Inflow Depth = 4.40" for 100-Year event
 Inflow = 146.92 cfs @ 12.50 hrs, Volume= 20.283 af
 Outflow = 146.74 cfs @ 12.51 hrs, Volume= 20.283 af, Atten= 0%, Lag= 0.7 min
 Primary = 146.74 cfs @ 12.51 hrs, Volume= 20.283 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 915.00' Surf.Area= 0.210 ac Storage= 0.353 af
 Peak Elev= 915.78' @ 12.51 hrs Surf.Area= 0.365 ac Storage= 0.576 af (0.223 af above start)

Plug-Flow detention time= 18.2 min calculated for 19.928 af (98% of inflow)
 Center-of-Mass det. time= 1.8 min (851.5 - 849.7)

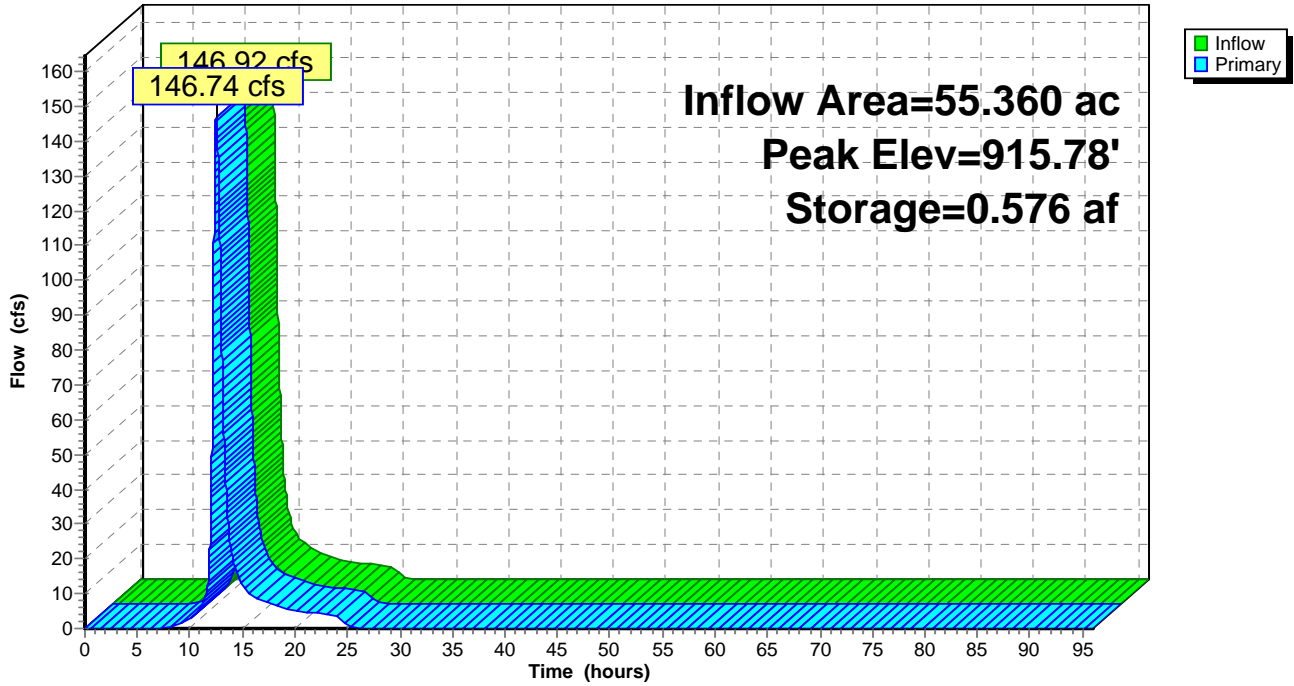
Volume	Invert	Avail.Storage	Storage Description
#1	910.50'	1.673 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
910.50	0.020	0.000	0.000
912.00	0.050	0.052	0.052
913.00	0.070	0.060	0.112
914.00	0.100	0.085	0.198
915.00	0.210	0.155	0.353
916.00	0.410	0.310	0.662
918.00	0.600	1.010	1.673

Device	Routing	Invert	Outlet Devices
#1	Primary	915.00'	80.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=146.74 cfs @ 12.51 hrs HW=915.78' TW=912.45' (Dynamic Tailwater)
 ↑**1=Broad-Crested Rectangular Weir** (Weir Controls 146.74 cfs @ 2.36 fps)

Pond 9P: P-9

Hydrograph



Summary for Pond 10P: P-10 Lowered 1 ft

[95] Warning: Outlet Device #1 rise exceeded

Inflow Area = 66.430 ac, 5.22% Impervious, Inflow Depth = 3.83" for 100-Year event
 Inflow = 139.63 cfs @ 12.70 hrs, Volume= 21.209 af
 Outflow = 139.22 cfs @ 12.72 hrs, Volume= 21.208 af, Atten= 0%, Lag= 1.5 min
 Primary = 15.29 cfs @ 12.72 hrs, Volume= 10.343 af
 Secondary = 123.93 cfs @ 12.72 hrs, Volume= 10.865 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 896.00' Surf.Area= 0.290 ac Storage= 0.700 af
 Peak Elev= 898.35' @ 12.72 hrs Surf.Area= 0.391 ac Storage= 1.496 af (0.796 af above start)

Plug-Flow detention time= 55.6 min calculated for 20.506 af (97% of inflow)
 Center-of-Mass det. time= 17.3 min (930.7 - 913.4)

Volume	Invert	Avail.Storage	Storage Description
#1	892.00'	1.760 af	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
892.00	0.120	0.000	0.000
893.00	0.140	0.130	0.130
895.00	0.190	0.330	0.460
896.00	0.290	0.240	0.700
897.00	0.330	0.310	1.010
899.00	0.420	0.750	1.760

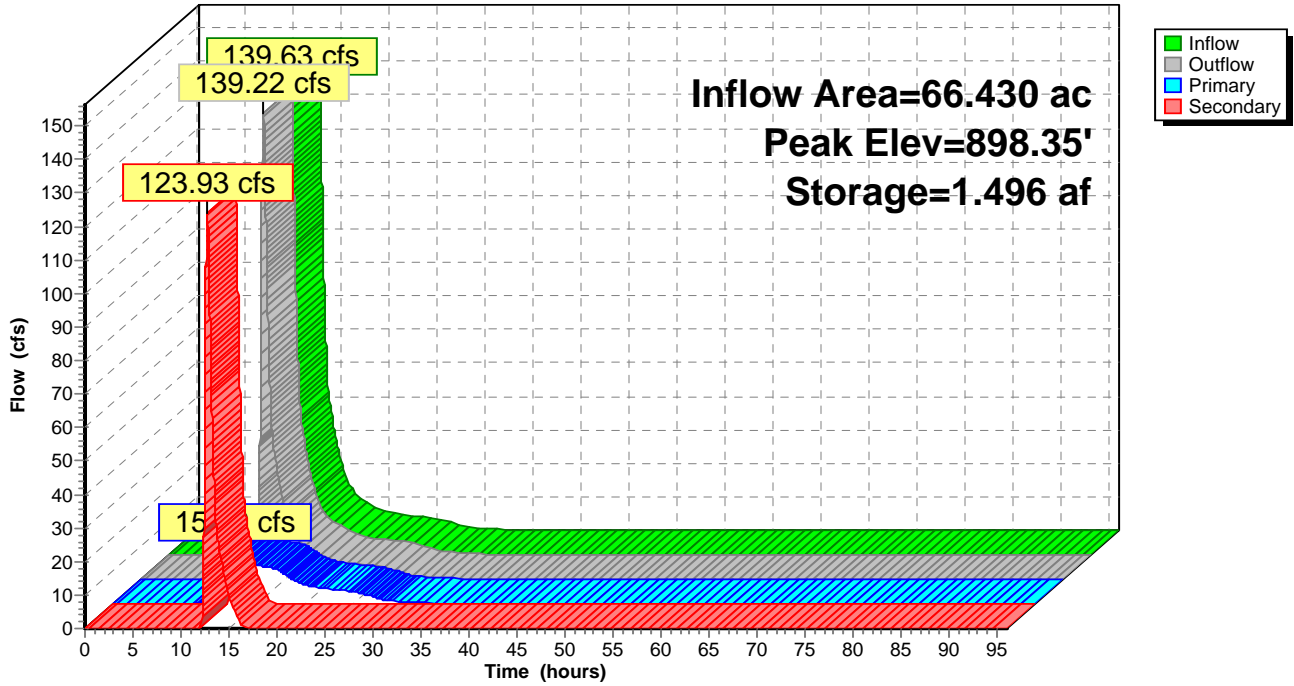
Device	Routing	Invert	Outlet Devices
#1	Primary	896.00'	2.5' long x 1.00' rise Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Secondary	897.40'	50.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=15.29 cfs @ 12.72 hrs HW=898.35' TW=895.15' (Dynamic Tailwater)
 ↑1=Sharp-Crested Rectangular Weir (Orifice Controls 15.29 cfs @ 6.65 fps)

Secondary OutFlow Max=123.91 cfs @ 12.72 hrs HW=898.35' TW=895.15' (Dynamic Tailwater)
 ↑2=Broad-Crested Rectangular Weir (Weir Controls 123.91 cfs @ 2.61 fps)

Pond 10P: P-10 Lowered 1 ft

Hydrograph



Summary for Pond 11P: P-11

Inflow Area = 58.650 ac, 4.89% Impervious, Inflow Depth = 4.45" for 100-Year event
 Inflow = 153.04 cfs @ 12.50 hrs, Volume= 21.768 af
 Outflow = 135.90 cfs @ 12.71 hrs, Volume= 21.766 af, Atten= 11%, Lag= 12.3 min
 Primary = 130.96 cfs @ 12.71 hrs, Volume= 18.280 af
 Secondary = 4.94 cfs @ 12.67 hrs, Volume= 3.486 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 909.00' Surf.Area= 1.210 ac Storage= 3.640 af
 Peak Elev= 912.68' @ 12.71 hrs Surf.Area= 1.641 ac Storage= 8.867 af (5.227 af above start)

Plug-Flow detention time= 203.7 min calculated for 18.124 af (83% of inflow)
 Center-of-Mass det. time= 89.2 min (935.6 - 846.4)

Volume	Invert	Avail.Storage	Storage Description
#1	905.00'	9.405 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
905.00	0.760	0.000	0.000
906.00	0.820	0.790	0.790
908.00	0.950	1.770	2.560
909.00	1.210	1.080	3.640
910.00	1.320	1.265	4.905
912.00	1.560	2.880	7.785
913.00	1.680	1.620	9.405

Device	Routing	Invert	Outlet Devices
#1	Primary	909.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	910.00'	24.0" Round RCP_Round 24" L= 200.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 910.00' / 909.00' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Primary	910.00'	24.0" Round RCP_Round 24" L= 200.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 910.00' / 909.00' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#4	Primary	912.00'	60.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88
#5	Secondary	909.00'	12.0" Round RCP_Round 12" L= 150.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 909.00' / 908.00' S= 0.0067 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=130.95 cfs @ 12.71 hrs HW=912.68' TW=898.35' (Dynamic Tailwater)

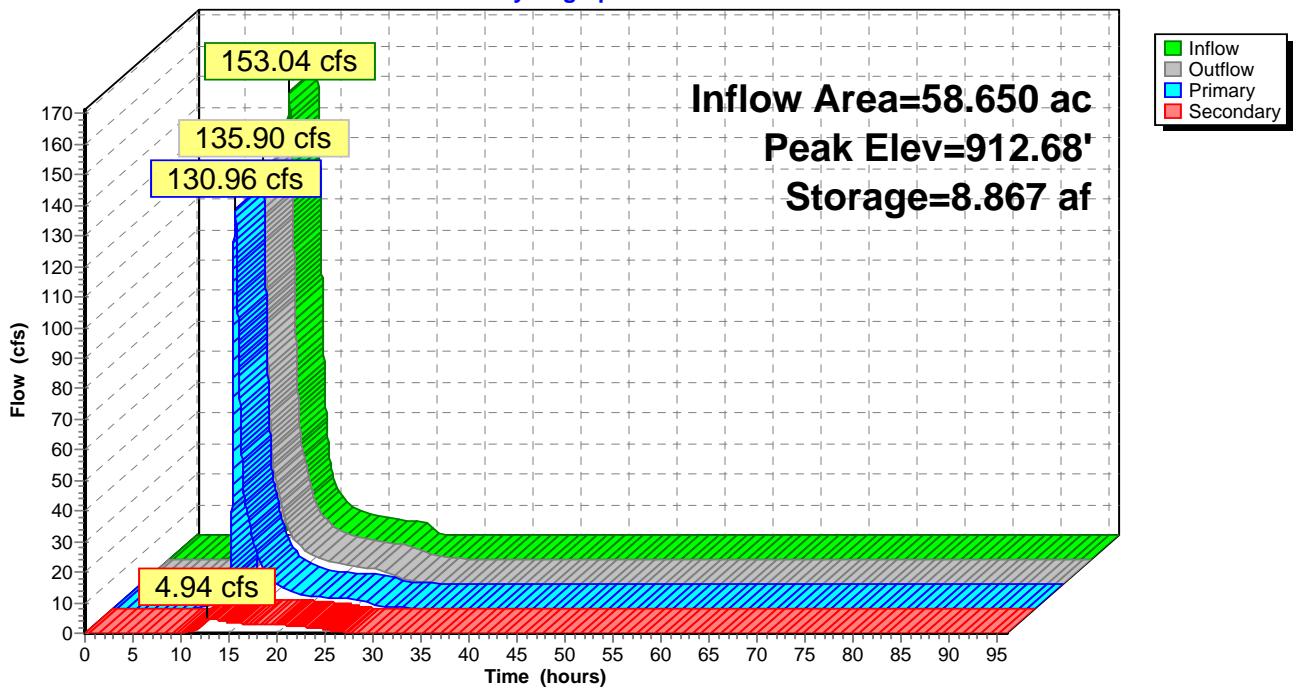
- 1=Orifice/Grate (Orifice Controls 7.25 cfs @ 9.23 fps)
- 2=RCP_Round 24" (Barrel Controls 16.98 cfs @ 5.41 fps)
- 3=RCP_Round 24" (Barrel Controls 16.98 cfs @ 5.41 fps)
- 4=Broad-Crested Rectangular Weir (Weir Controls 89.73 cfs @ 2.21 fps)

Secondary OutFlow Max=4.94 cfs @ 12.67 hrs HW=912.67' TW=909.04' (Dynamic Tailwater)

- 5=RCP_Round 12" (Outlet Controls 4.94 cfs @ 6.29 fps)

Pond 11P: P-11

Hydrograph



Summary for Pond 12P: P-12

Inflow Area = 79.640 ac, 7.40% Impervious, Inflow Depth = 4.52" for 100-Year event
 Inflow = 148.48 cfs @ 12.72 hrs, Volume= 29.994 af
 Outflow = 116.29 cfs @ 13.01 hrs, Volume= 29.986 af, Atten= 22%, Lag= 17.3 min
 Primary = 116.29 cfs @ 13.01 hrs, Volume= 29.986 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 893.00' Surf.Area= 1.640 ac Storage= 5.075 af
 Peak Elev= 895.49' @ 13.01 hrs Surf.Area= 1.971 ac Storage= 9.562 af (4.487 af above start)

Plug-Flow detention time= 231.9 min calculated for 24.911 af (83% of inflow)
 Center-of-Mass det. time= 62.7 min (1,021.9 - 959.1)

Volume	Invert	Avail.Storage	Storage Description
#1	889.00'	10.590 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
889.00	1.070	0.000	0.000
890.00	1.150	1.110	1.110
892.00	1.330	2.480	3.590
893.00	1.640	1.485	5.075
894.00	1.770	1.705	6.780
896.00	2.040	3.810	10.590

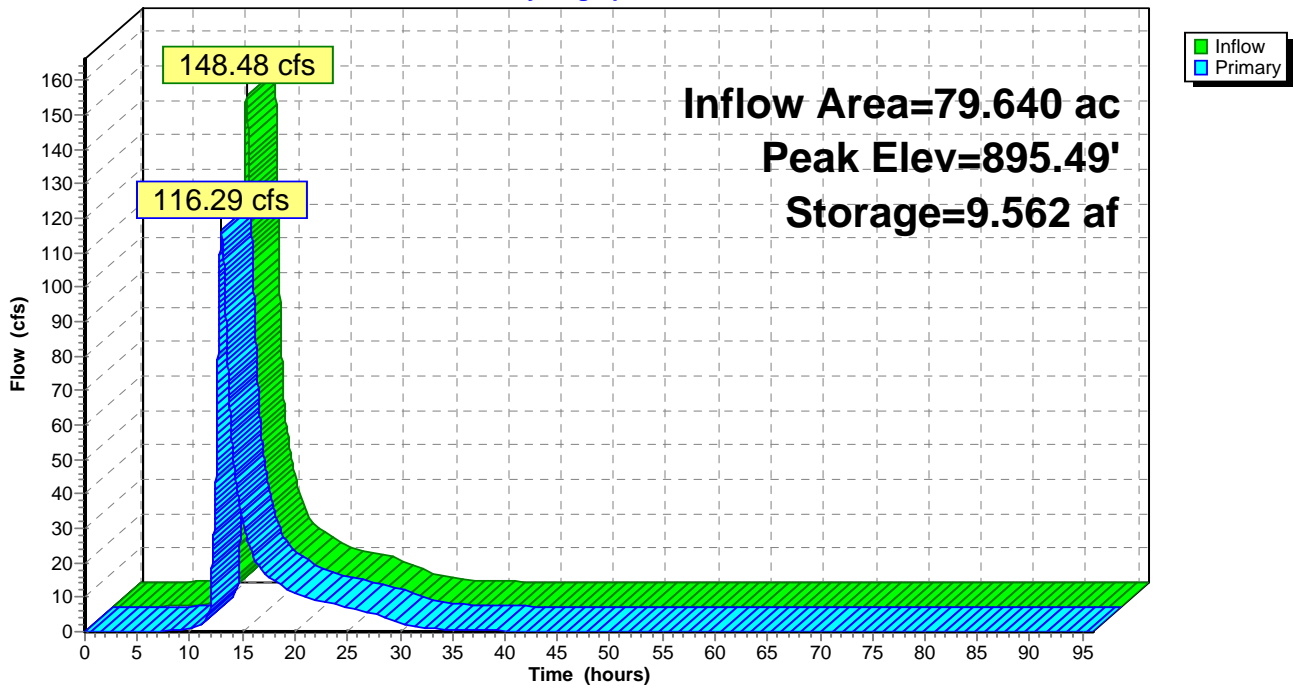
Device	Routing	Invert	Outlet Devices
#1	Primary	893.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	893.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf
#4	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf
#5	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf
#6	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf

Primary OutFlow Max=116.29 cfs @ 13.01 hrs HW=895.49' TW=884.60' (Dynamic Tailwater)

- 1=Orifice/Grate (Orifice Controls 5.96 cfs @ 7.59 fps)
- 2=Orifice/Grate (Orifice Controls 5.96 cfs @ 7.59 fps)
- 3=RCP_Arch 44x27 (Barrel Controls 26.09 cfs @ 5.45 fps)
- 4=RCP_Arch 44x27 (Barrel Controls 26.09 cfs @ 5.45 fps)
- 5=RCP_Arch 44x27 (Barrel Controls 26.09 cfs @ 5.45 fps)
- 6=RCP_Arch 44x27 (Barrel Controls 26.09 cfs @ 5.45 fps)

Pond 12P: P-12

Hydrograph



Summary for Pond 13P: P-13

Inflow Area = 237.775 ac, 9.20% Impervious, Inflow Depth = 4.57" for 100-Year event
 Inflow = 524.41 cfs @ 12.35 hrs, Volume= 90.508 af
 Outflow = 503.58 cfs @ 12.46 hrs, Volume= 90.505 af, Atten= 4%, Lag= 6.4 min
 Primary = 485.21 cfs @ 12.46 hrs, Volume= 86.324 af
 Secondary = 18.37 cfs @ 12.46 hrs, Volume= 4.181 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 883.00' Surf.Area= 1.870 ac Storage= 4.265 af
 Peak Elev= 885.22' @ 12.46 hrs Surf.Area= 2.673 ac Storage= 9.306 af (5.041 af above start)

Plug-Flow detention time= 68.7 min calculated for 86.231 af (95% of inflow)
 Center-of-Mass det. time= 12.6 min (903.8 - 891.2)

Volume	Invert	Avail.Storage	Storage Description
#1	878.00'	11.490 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
878.00	0.000	0.000	0.000
879.00	0.630	0.315	0.315
880.00	0.730	0.680	0.995
882.00	1.070	1.800	2.795
883.00	1.870	1.470	4.265
884.00	2.220	2.045	6.310
886.00	2.960	5.180	11.490

Device	Routing	Invert	Outlet Devices
#1	Primary	883.00'	55.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88
#2	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#3	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#4	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#5	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#6	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200

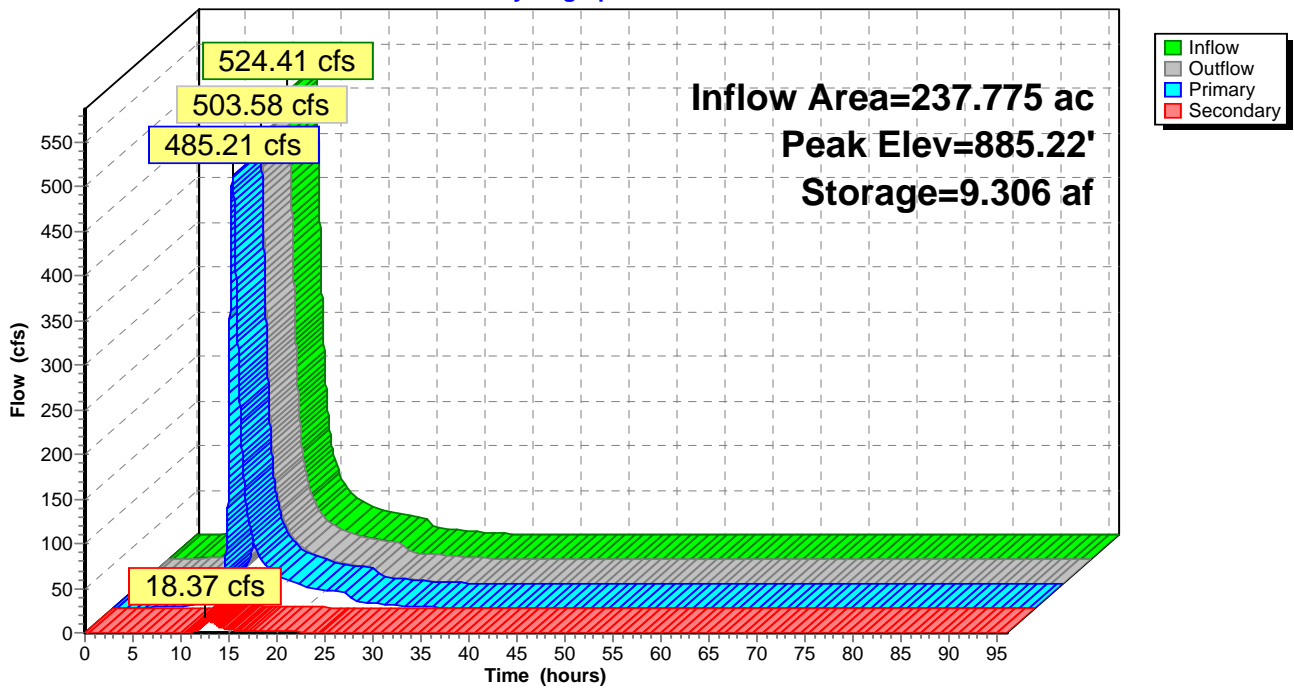
Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 1' Cc= 0.900
 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=485.21 cfs @ 12.46 hrs HW=885.22' TW=0.00' (Dynamic Tailwater)
 1=Broad-Crested Rectangular Weir (Weir Controls 485.21 cfs @ 3.97 fps)

Secondary OutFlow Max=18.37 cfs @ 12.46 hrs HW=885.22' TW=883.24' (Dynamic Tailwater)
 2=RCP_Round 12" (Barrel Controls 3.67 cfs @ 4.68 fps)
 3=RCP_Round 12" (Barrel Controls 3.67 cfs @ 4.68 fps)
 4=RCP_Round 12" (Barrel Controls 3.67 cfs @ 4.68 fps)
 5=RCP_Round 12" (Barrel Controls 3.67 cfs @ 4.68 fps)
 6=RCP_Round 12" (Barrel Controls 3.67 cfs @ 4.68 fps)

Pond 13P: P-13

Hydrograph



Summary for Pond 17P: W-2

[80] Warning: Exceeded Pond P-5/P-6 by 0.17' @ 36.12 hrs (0.20 cfs 0.727 af)

Inflow = 3.09 cfs @ 12.39 hrs, Volume= 1.318 af
 Outflow = 1.02 cfs @ 16.83 hrs, Volume= 1.173 af, Atten= 67%, Lag= 266.4 min
 Primary = 1.02 cfs @ 16.83 hrs, Volume= 1.173 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 929.56' @ 16.83 hrs Surf.Area= 1.185 ac Storage= 0.638 af

Plug-Flow detention time= 577.2 min calculated for 1.173 af (89% of inflow)
 Center-of-Mass det. time= 523.1 min (1,453.9 - 930.9)

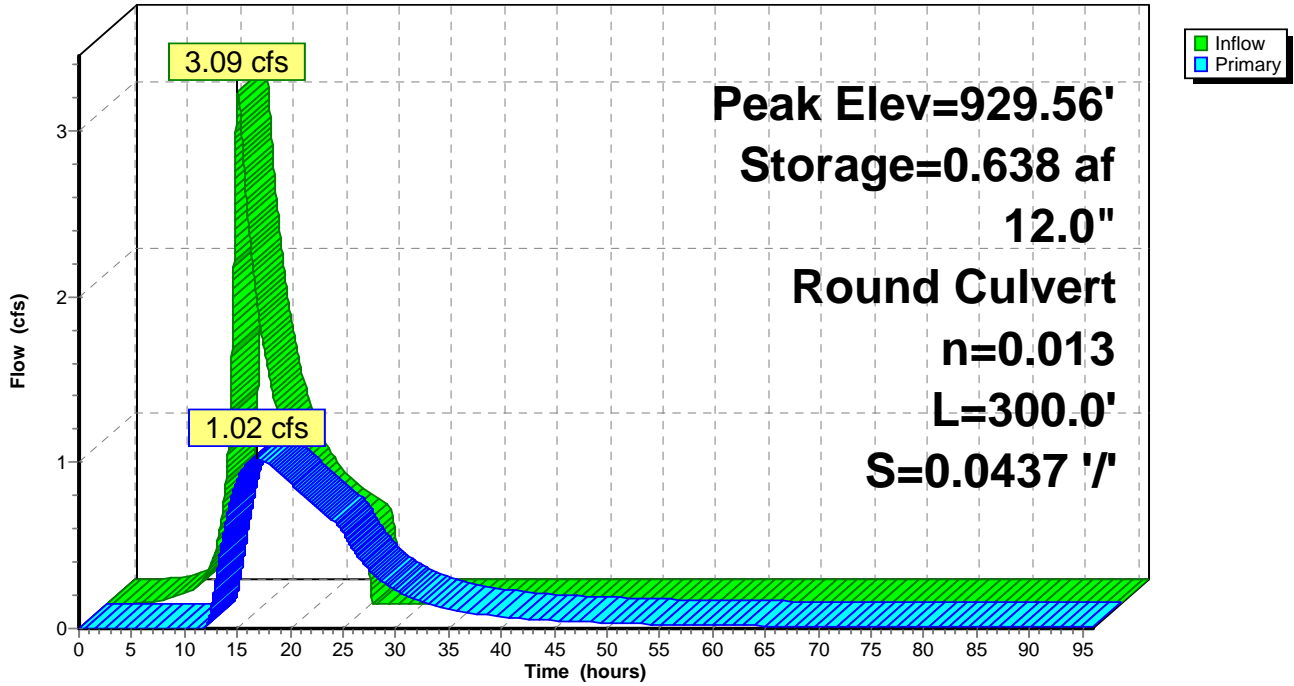
Volume	Invert	Avail.Storage	Storage Description
#1	929.00'	1.175 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
929.00	1.090	0.000	0.000
930.00	1.260	1.175	1.175

Device	Routing	Invert	Outlet Devices
#1	Primary	929.10'	12.0" Round RCP_Round 12" L= 300.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 929.10' / 916.00' S= 0.0437 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=1.02 cfs @ 16.83 hrs HW=929.56' TW=914.88' (Dynamic Tailwater)
 ↑1=RCP_Round 12" (Inlet Controls 1.02 cfs @ 2.89 fps)

Pond 17P: W-2

Hydrograph



Summary for Pond 36P: Culverts passing flow beneath Spine Road

Inflow Area = 52.790 ac, 0.00% Impervious, Inflow Depth = 4.31" for 100-Year event
 Inflow = 161.15 cfs @ 12.43 hrs, Volume= 18.977 af
 Outflow = 161.15 cfs @ 12.43 hrs, Volume= 18.977 af, Atten= 0%, Lag= 0.0 min
 Primary = 127.00 cfs @ 12.25 hrs, Volume= 18.235 af
 Secondary = 34.15 cfs @ 12.43 hrs, Volume= 0.742 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 888.46' @ 12.43 hrs Surf.Area= 0.004 ac Storage= 0.003 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 0.0 min (844.2 - 844.2)

Volume	Invert	Avail.Storage	Storage Description
#1	887.00'	0.026 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
887.00	0.000	0.000	0.000
887.50	0.002	0.001	0.001
890.50	0.007	0.014	0.014
892.00	0.009	0.012	0.026

Device	Routing	Invert	Outlet Devices
#1	Primary	887.00'	Special & User-Defined Head (feet) 0.00 0.10 0.20 0.30 0.40 0.50 5.00 Disch. (cfs) 0.000 25.000 50.000 75.000 100.000 127.000 127.000
#2	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#3	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#4	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#5	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#6	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#7	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 '/ Cc= 0.900

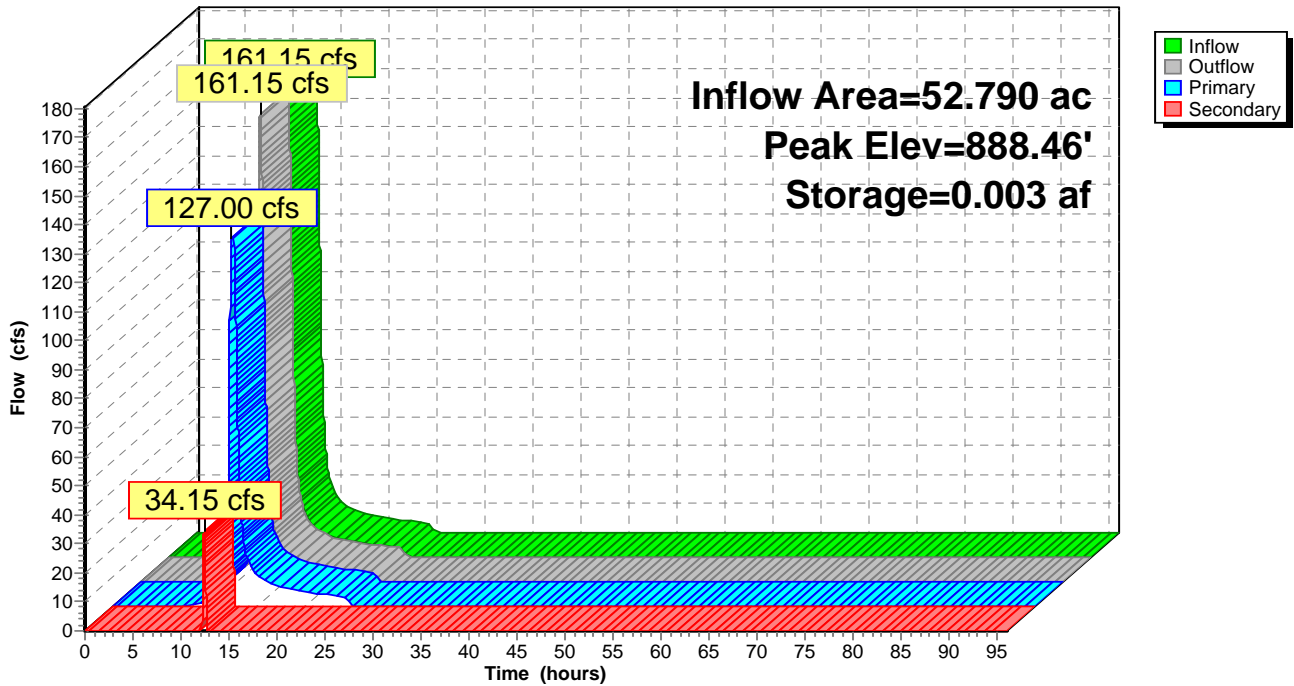
- #8 Secondary 887.50' n= 0.013, Flow Area= 1.77 sf **18.0" Round RCP_Round 18"**
 L= 100.0' RCP, groove end w/headwall, Ke= 0.200
 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/' Cc= 0.900
- #9 Secondary 887.50' n= 0.013, Flow Area= 1.77 sf **18.0" Round RCP_Round 18"**
 L= 100.0' RCP, groove end w/headwall, Ke= 0.200
 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/' Cc= 0.900
 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=127.00 cfs @ 12.25 hrs HW=887.60' TW=884.93' (Dynamic Tailwater)
 ↳1=Special & User-Defined (Custom Controls 127.00 cfs)

Secondary OutFlow Max=34.12 cfs @ 12.43 hrs HW=888.46' TW=885.22' (Dynamic Tailwater)
 ↳2=RCP_Round 18" (Barrel Controls 4.26 cfs @ 5.08 fps)
 ↳3=RCP_Round 18" (Barrel Controls 4.26 cfs @ 5.08 fps)
 ↳4=RCP_Round 18" (Barrel Controls 4.26 cfs @ 5.08 fps)
 ↳5=RCP_Round 18" (Barrel Controls 4.26 cfs @ 5.08 fps)
 ↳6=RCP_Round 18" (Barrel Controls 4.26 cfs @ 5.08 fps)
 ↳7=RCP_Round 18" (Barrel Controls 4.26 cfs @ 5.08 fps)
 ↳8=RCP_Round 18" (Barrel Controls 4.26 cfs @ 5.08 fps)
 ↳9=RCP_Round 18" (Barrel Controls 4.26 cfs @ 5.08 fps)

Pond 36P: Culverts passing flow beneath Spine Road

Hydrograph



Summary for Pond CRH-1: CRH-1

Inflow Area = 6.955 ac, 46.76% Impervious, Inflow Depth = 5.60" for 100-Year event
 Inflow = 38.37 cfs @ 12.15 hrs, Volume= 3.247 af
 Outflow = 25.53 cfs @ 12.31 hrs, Volume= 3.247 af, Atten= 33%, Lag= 9.5 min
 Discarded = 0.37 cfs @ 12.31 hrs, Volume= 0.563 af
 Primary = 25.16 cfs @ 12.31 hrs, Volume= 2.685 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 878.81' @ 12.31 hrs Surf.Area= 0.463 ac Storage= 0.760 af

Plug-Flow detention time= 114.2 min calculated for 3.247 af (100% of inflow)
 Center-of-Mass det. time= 114.3 min (895.5 - 781.2)

Volume	Invert	Avail.Storage	Storage Description
#1	876.00'	0.850 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
876.00	0.150	0.000	0.000
878.00	0.300	0.450	0.450
879.00	0.500	0.400	0.850

Device	Routing	Invert	Outlet Devices
#1	Discarded	876.00'	0.800 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 0.00'
#2	Primary	877.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 877.00' / 876.00' S= 0.0065 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Primary	877.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 877.00' / 876.00' S= 0.0065 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Discarded OutFlow Max=0.37 cfs @ 12.31 hrs HW=878.81' (Free Discharge)

↳ **1=Exfiltration** (Controls 0.37 cfs)

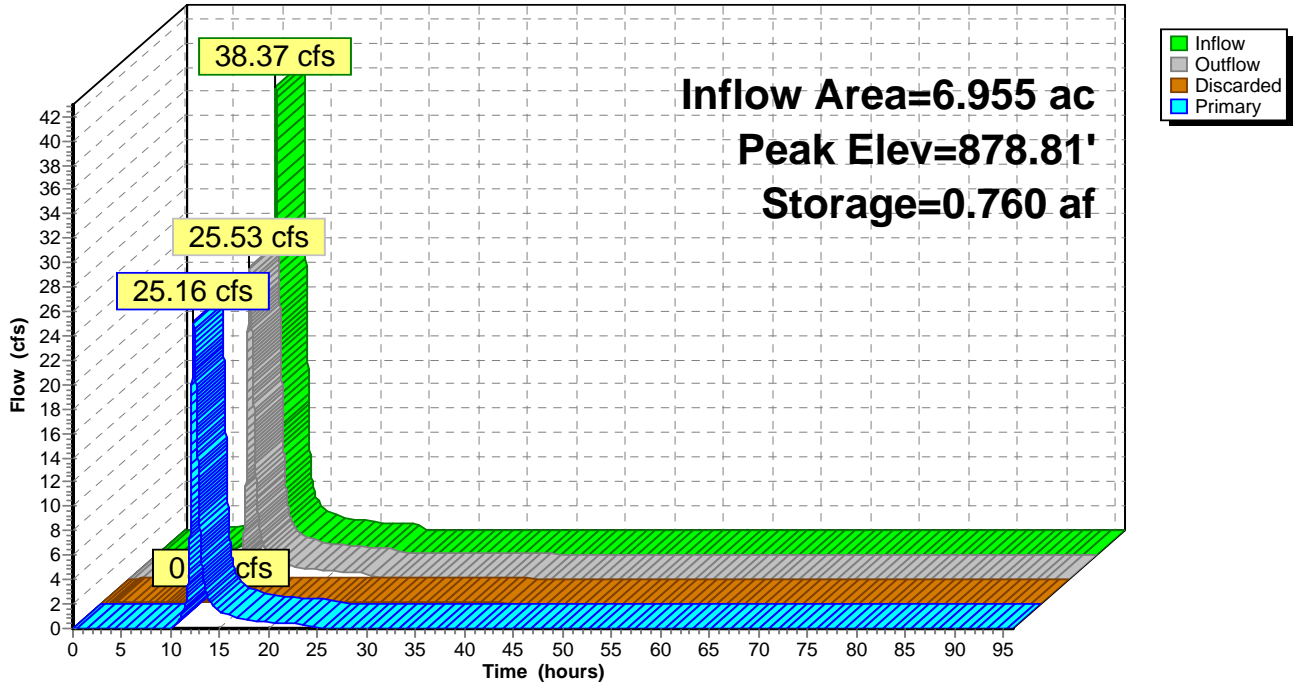
Primary OutFlow Max=25.15 cfs @ 12.31 hrs HW=878.81' (Free Discharge)

↳ **2=Culvert** (Barrel Controls 12.58 cfs @ 5.53 fps)

↳ **3=Culvert** (Barrel Controls 12.58 cfs @ 5.53 fps)

Pond CRH-1: CRH-1

Hydrograph



Summary for Pond CRH-2: CRH-2

Inflow Area = 10.214 ac, 37.73% Impervious, Inflow Depth = 5.35" for 100-Year event
 Inflow = 48.13 cfs @ 12.21 hrs, Volume= 4.557 af
 Outflow = 27.85 cfs @ 12.47 hrs, Volume= 4.557 af, Atten= 42%, Lag= 15.7 min
 Discarded = 0.47 cfs @ 12.47 hrs, Volume= 0.986 af
 Primary = 27.38 cfs @ 12.47 hrs, Volume= 3.571 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 883.78' @ 12.47 hrs Surf.Area= 0.578 ac Storage= 1.468 af

Plug-Flow detention time= 190.2 min calculated for 4.557 af (100% of inflow)
 Center-of-Mass det. time= 190.4 min (982.8 - 792.4)

Volume	Invert	Avail.Storage	Storage Description
#1	880.00'	1.600 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
880.00	0.200	0.000	0.000
882.00	0.400	0.600	0.600
884.00	0.600	1.000	1.600

Device	Routing	Invert	Outlet Devices
#1	Primary	881.50'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 881.50' / 881.00' S= 0.0032 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Primary	881.50'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 881.50' / 881.00' S= 0.0032 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Discarded	880.00'	0.800 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 0.00'

Discarded OutFlow Max=0.47 cfs @ 12.47 hrs HW=883.78' (Free Discharge)

↳ **3=Exfiltration** (Controls 0.47 cfs)

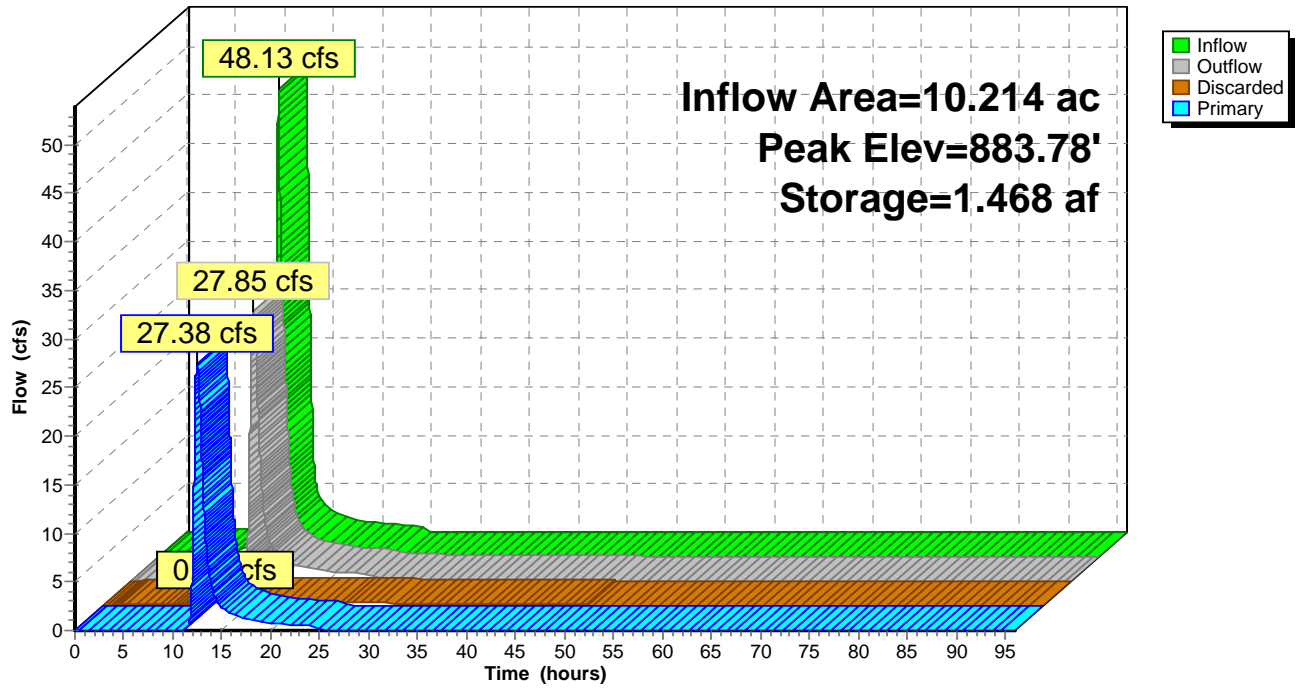
Primary OutFlow Max=27.38 cfs @ 12.47 hrs HW=883.78' TW=879.69' (Dynamic Tailwater)

↳ **1=Culvert** (Barrel Controls 13.69 cfs @ 4.79 fps)

↳ **2=Culvert** (Barrel Controls 13.69 cfs @ 4.79 fps)

Pond CRH-2: CRH-2

Hydrograph



Summary for Pond CRH-3: CRH-3

Inflow Area = 11.815 ac, 36.95% Impervious, Inflow Depth = 4.33" for 100-Year event
 Inflow = 30.15 cfs @ 12.44 hrs, Volume= 4.265 af
 Outflow = 25.90 cfs @ 12.71 hrs, Volume= 4.265 af, Atten= 14%, Lag= 15.7 min
 Discarded = 0.38 cfs @ 12.71 hrs, Volume= 0.519 af
 Primary = 25.52 cfs @ 12.71 hrs, Volume= 3.745 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 879.83' @ 12.71 hrs Surf.Area= 0.466 ac Storage= 0.769 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 82.2 min (919.3 - 837.1)

Volume	Invert	Avail.Storage	Storage Description
#1	877.00'	0.850 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
877.00	0.150	0.000	0.000
879.00	0.300	0.450	0.450
880.00	0.500	0.400	0.850

Device	Routing	Invert	Outlet Devices
#1	Discarded	877.00'	0.800 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 0.00'
#2	Primary	878.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 878.00' / 877.00' S= 0.0065 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Primary	878.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 878.00' / 877.00' S= 0.0065 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Discarded OutFlow Max=0.38 cfs @ 12.71 hrs HW=879.83' (Free Discharge)

↑**1=Exfiltration** (Controls 0.38 cfs)

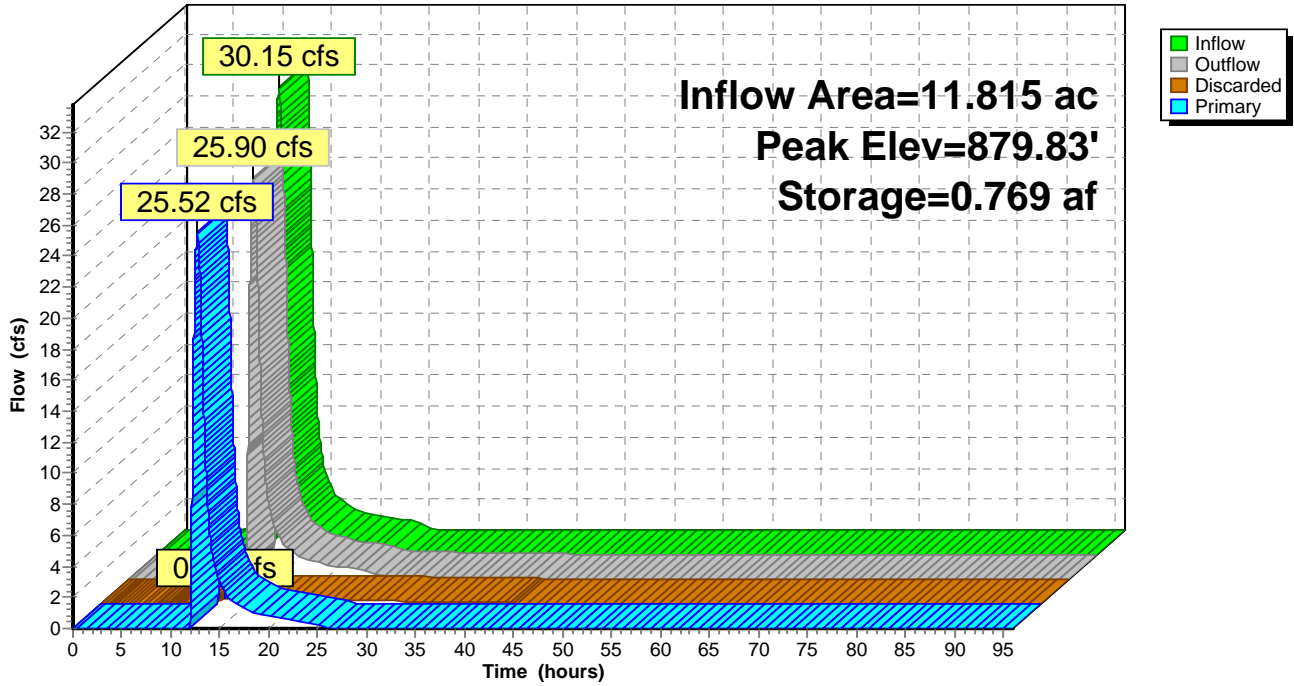
Primary OutFlow Max=25.52 cfs @ 12.71 hrs HW=879.83' (Free Discharge)

↑**2=Culvert** (Barrel Controls 12.76 cfs @ 5.55 fps)

↑**3=Culvert** (Barrel Controls 12.76 cfs @ 5.55 fps)

Pond CRH-3: CRH-3

Hydrograph



Summary for Pond P-5/P-6: P-5/P-6

Inflow Area = 43.346 ac, 18.61% Impervious, Inflow Depth = 4.83" for 100-Year event
 Inflow = 208.93 cfs @ 12.15 hrs, Volume= 17.441 af
 Outflow = 112.99 cfs @ 12.39 hrs, Volume= 17.434 af, Atten= 46%, Lag= 14.5 min
 Primary = 109.91 cfs @ 12.39 hrs, Volume= 16.116 af
 Secondary = 3.09 cfs @ 12.39 hrs, Volume= 1.318 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 929.00' Surf.Area= 1.975 ac Storage= 5.062 af
 Peak Elev= 931.48' @ 12.39 hrs Surf.Area= 2.486 ac Storage= 10.663 af (5.600 af above start)

Plug-Flow detention time= 294.2 min calculated for 12.371 af (71% of inflow)
 Center-of-Mass det. time= 114.8 min (920.1 - 805.3)

Volume	Invert	Avail.Storage	Storage Description
#1	926.00'	14.650 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
926.00	1.510	0.000	0.000
928.00	1.710	3.220	3.220
930.00	2.240	3.950	7.170
931.00	2.400	2.320	9.490
933.00	2.760	5.160	14.650

Device	Routing	Invert	Outlet Devices
#1	Primary	929.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	929.50'	7.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#3	Primary	930.50'	14.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#4	Secondary	929.00'	9.0" Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=109.90 cfs @ 12.39 hrs HW=931.48' TW=0.00' (Dynamic Tailwater)

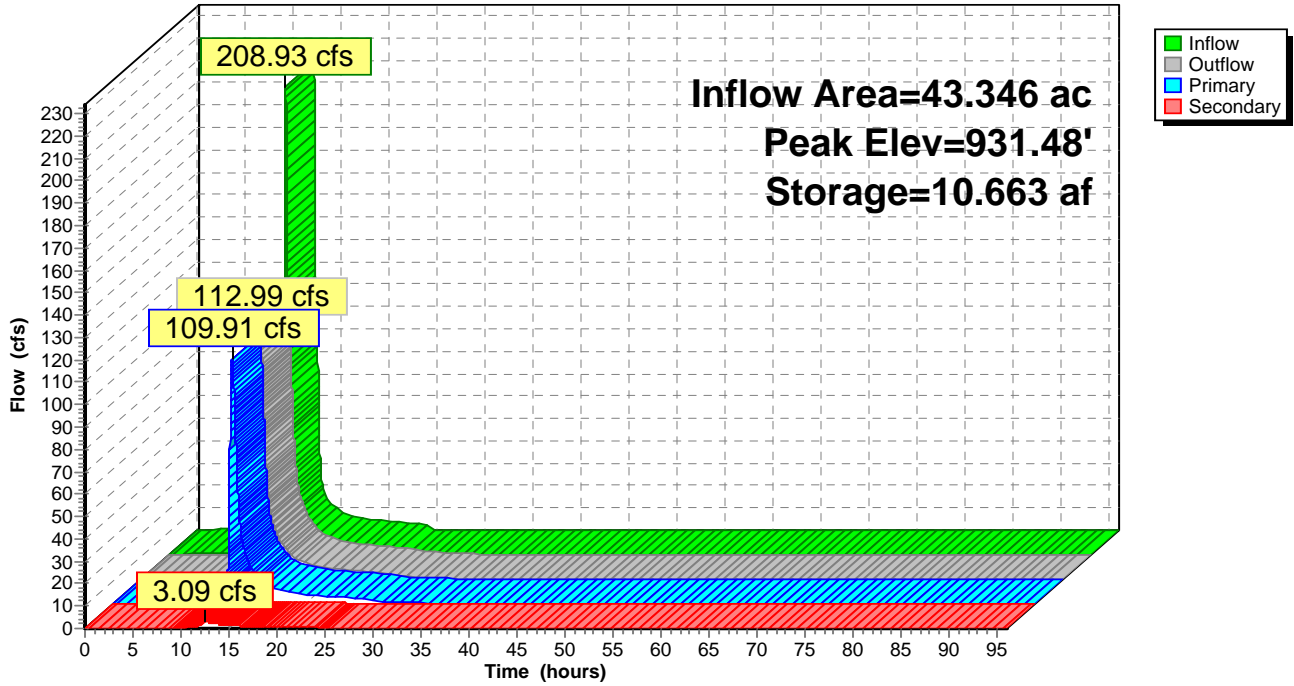
- ↑1=Orifice/Grate (Orifice Controls 5.96 cfs @ 7.58 fps)
- └2=Sharp-Crested Rectangular Weir (Weir Controls 60.16 cfs @ 4.60 fps)
- └3=Sharp-Crested Rectangular Weir (Weir Controls 43.78 cfs @ 3.24 fps)

Secondary OutFlow Max=3.09 cfs @ 12.39 hrs HW=931.48' TW=929.22' (Dynamic Tailwater)

- ↑4=Orifice/Grate (Orifice Controls 3.09 cfs @ 6.99 fps)

Pond P-5/P-6: P-5/P-6

Hydrograph



Summary for Pond TI P: Thumb Infiltration (Thumb TP load only)

Inflow Area = 48.539 ac, 11.38% Impervious, Inflow Depth = 2.41" for 100-Year event
 Inflow = 62.42 cfs @ 12.48 hrs, Volume= 9.759 af
 Outflow = 53.02 cfs @ 12.75 hrs, Volume= 6.019 af, Atten= 15%, Lag= 16.2 min
 Primary = 53.02 cfs @ 12.75 hrs, Volume= 6.019 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 903.80' @ 12.75 hrs Surf.Area= 1.000 ac Storage= 3.804 af

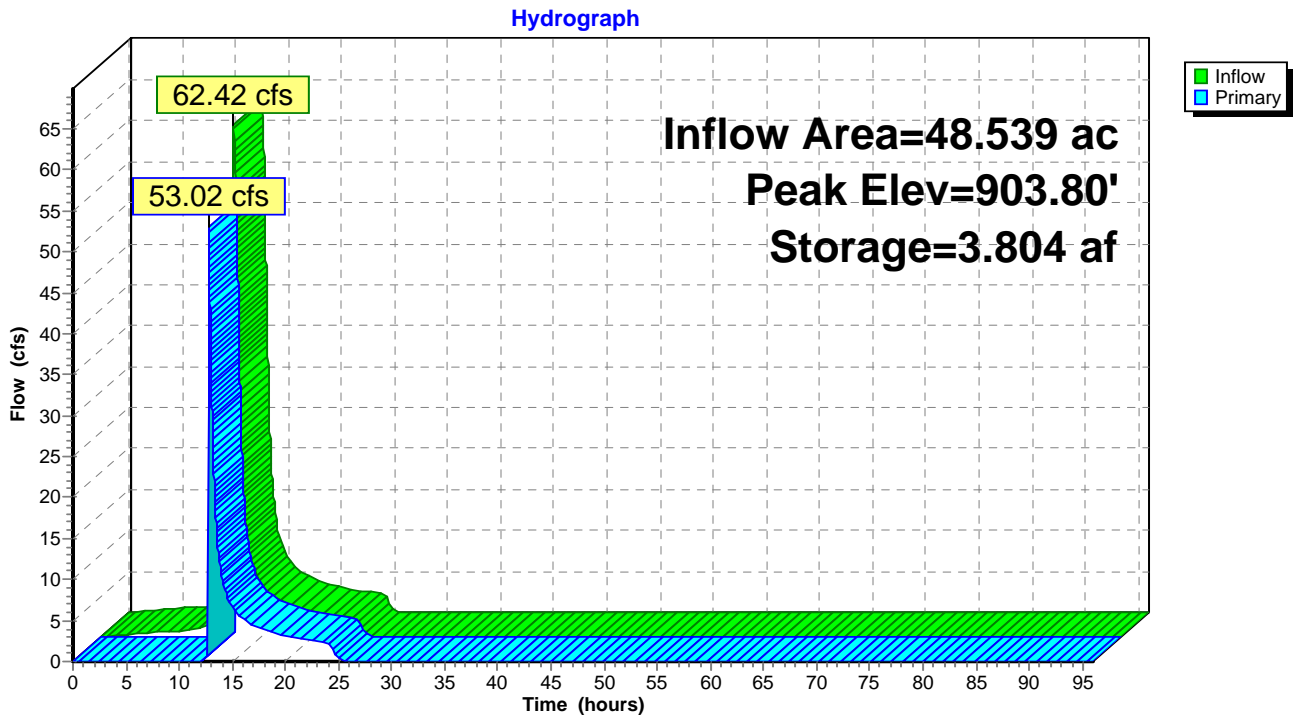
Plug-Flow detention time= 229.2 min calculated for 6.018 af (62% of inflow)
 Center-of-Mass det. time= 101.6 min (958.8 - 857.2)

Volume	Invert	Avail.Storage	Storage Description
#1	900.00'	5.000 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
900.00	1.000	0.000	0.000
901.00	1.000	1.000	1.000
902.00	1.000	1.000	2.000
903.00	1.000	1.000	3.000
904.00	1.000	1.000	4.000
905.00	1.000	1.000	5.000

Device	Routing	Invert	Outlet Devices
#1	Primary	903.74'	1,000.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) 5.0' Crest Height

Primary OutFlow Max=52.85 cfs @ 12.75 hrs HW=903.80' (Free Discharge)
 ↑1=Sharp-Crested Rectangular Weir (Weir Controls 52.85 cfs @ 0.83 fps)

Pond TI P: Thumb Infiltration (Thumb TP load only)



Summary for Pond W-1: W-1

Inflow Area = 1.000 ac, 10.00% Impervious, Inflow Depth = 21.59" for 100-Year event
 Inflow = 5.53 cfs @ 12.25 hrs, Volume= 1.799 af
 Outflow = 2.75 cfs @ 14.34 hrs, Volume= 1.799 af, Atten= 50%, Lag= 125.5 min
 Primary = 2.75 cfs @ 14.34 hrs, Volume= 1.799 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 915.28' @ 14.34 hrs Surf.Area= 0.744 ac Storage= 0.371 af

Plug-Flow detention time= 106.0 min calculated for 1.799 af (100% of inflow)
 Center-of-Mass det. time= 106.0 min (1,026.7 - 920.7)

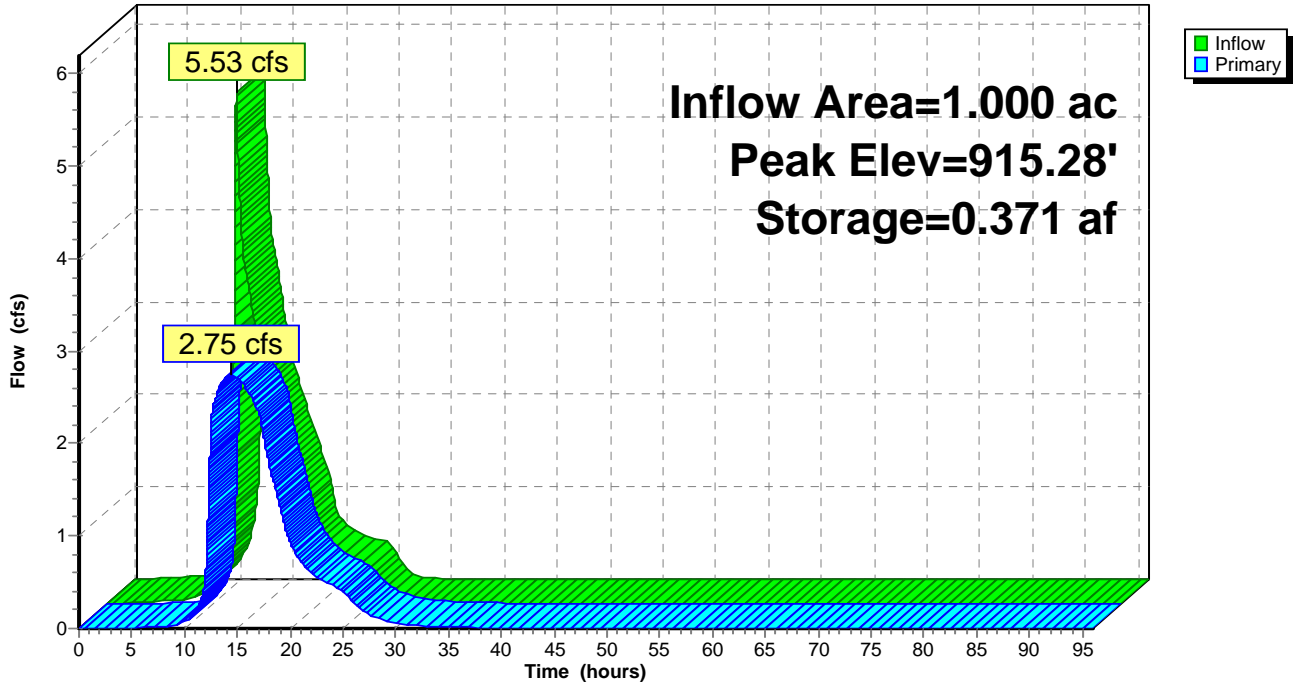
Volume	Invert	Avail.Storage	Storage Description
#1	914.75'	0.950 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
914.75	0.660	0.000	0.000
916.00	0.860	0.950	0.950

Device	Routing	Invert	Outlet Devices
#1	Primary	914.75'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=2.75 cfs @ 14.34 hrs HW=915.28' TW=0.00' (Dynamic Tailwater)
 ↑1=Orifice/Grate (Orifice Controls 2.75 cfs @ 3.50 fps)

Pond W-1: W-1

Hydrograph



Summary for Pond W-3: W-3

Inflow = 1.02 cfs @ 16.83 hrs, Volume= 1.173 af
 Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

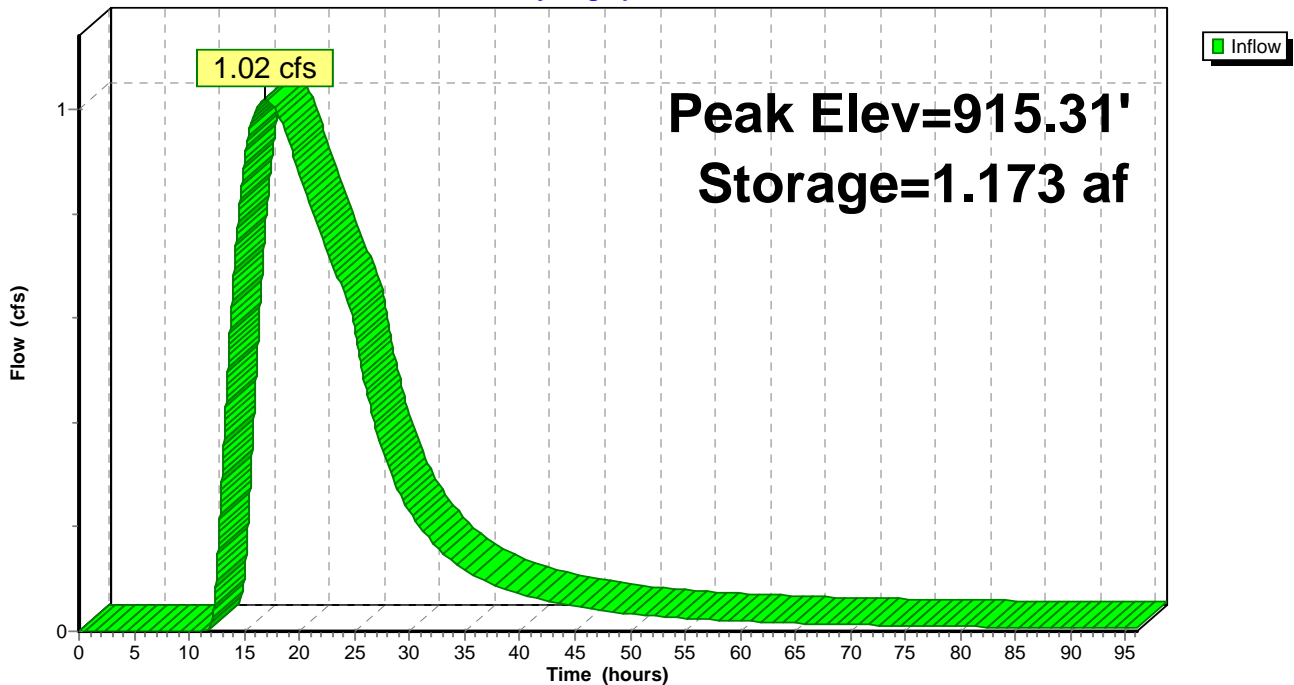
Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 915.31' @ 96.00 hrs Surf.Area= 2.133 ac Storage= 1.173 af

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description
#1	914.75'	2.680 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
914.75	2.040	0.000	0.000
915.00	2.080	0.515	0.515
916.00	2.250	2.165	2.680

Pond W-3: W-3

Hydrograph



Summary for Pond W-4: W-4

Inflow Area = 2.980 ac, 26.17% Impervious, Inflow Depth = 19.14" for 100-Year event
 Inflow = 21.33 cfs @ 12.08 hrs, Volume= 4.752 af
 Outflow = 4.11 cfs @ 15.14 hrs, Volume= 4.732 af, Atten= 81%, Lag= 183.4 min
 Primary = 4.11 cfs @ 15.14 hrs, Volume= 4.732 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 909.25' @ 15.14 hrs Surf.Area= 1.231 ac Storage= 1.261 af

Plug-Flow detention time= 245.8 min calculated for 4.732 af (100% of inflow)
 Center-of-Mass det. time= 242.2 min (1,224.2 - 982.0)

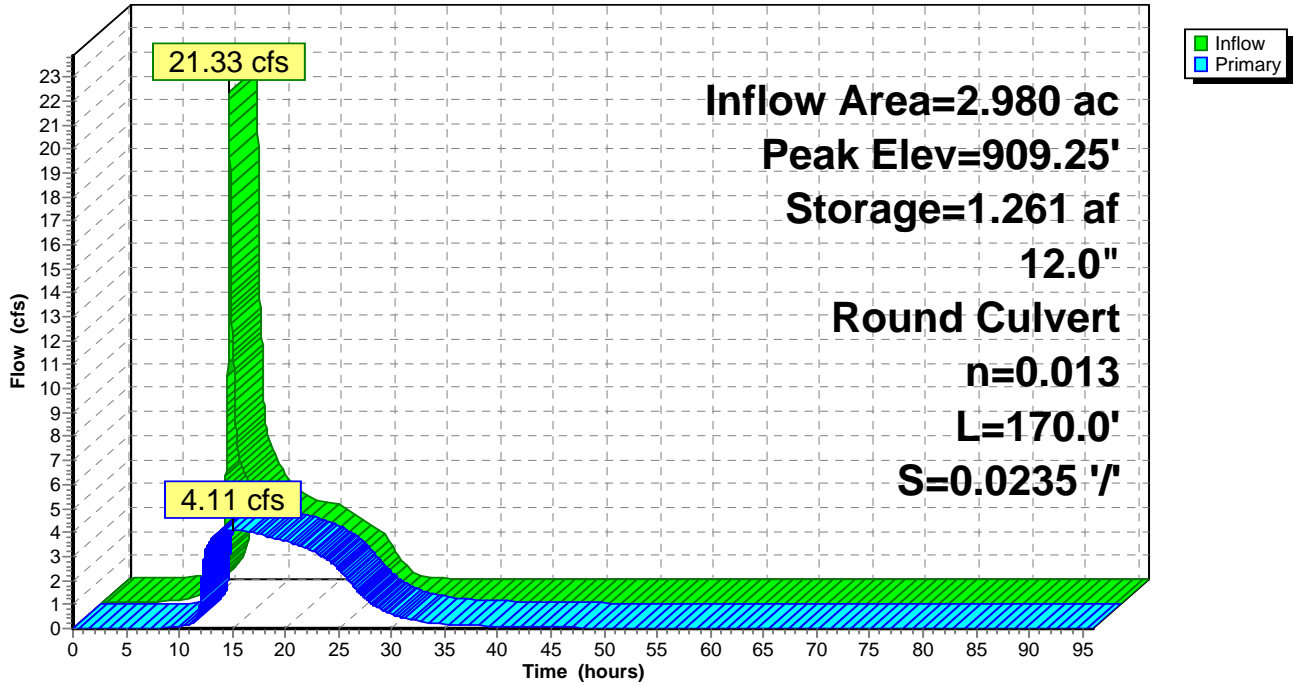
Volume	Invert	Avail.Storage	Storage Description
#1	908.00'	2.280 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
908.00	0.780	0.000	0.000
910.00	1.500	2.280	2.280

Device	Routing	Invert	Outlet Devices
#1	Primary	908.00'	12.0" Round RCP_Round 12" L= 170.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 908.00' / 904.00' S= 0.0235 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=4.11 cfs @ 15.14 hrs HW=909.25' TW=894.24' (Dynamic Tailwater)
 ↳1=RCP_Round 12" (Inlet Controls 4.11 cfs @ 5.23 fps)

Pond W-4: W-4

Hydrograph



Summary for Pond W-5: W-5

Inflow Area = 7.608 ac, 48.41% Impervious, Inflow Depth = 12.36" for 100-Year event
 Inflow = 73.62 cfs @ 12.02 hrs, Volume= 7.835 af
 Outflow = 15.85 cfs @ 13.13 hrs, Volume= 7.828 af, Atten= 78%, Lag= 66.4 min
 Primary = 15.85 cfs @ 13.13 hrs, Volume= 7.828 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 882.75' Surf.Area= 4.910 ac Storage= 3.412 af
 Peak Elev= 883.30' @ 13.13 hrs Surf.Area= 5.645 ac Storage= 6.307 af (2.894 af above start)

Plug-Flow detention time= 520.4 min calculated for 4.415 af (56% of inflow)
 Center-of-Mass det. time= 194.5 min (1,024.1 - 829.7)

Volume	Invert	Avail.Storage	Storage Description
#1	882.00'	7.390 af	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
882.00	4.190	0.000	0.000
883.00	5.150	4.670	4.670
883.49	5.950	2.720	7.390

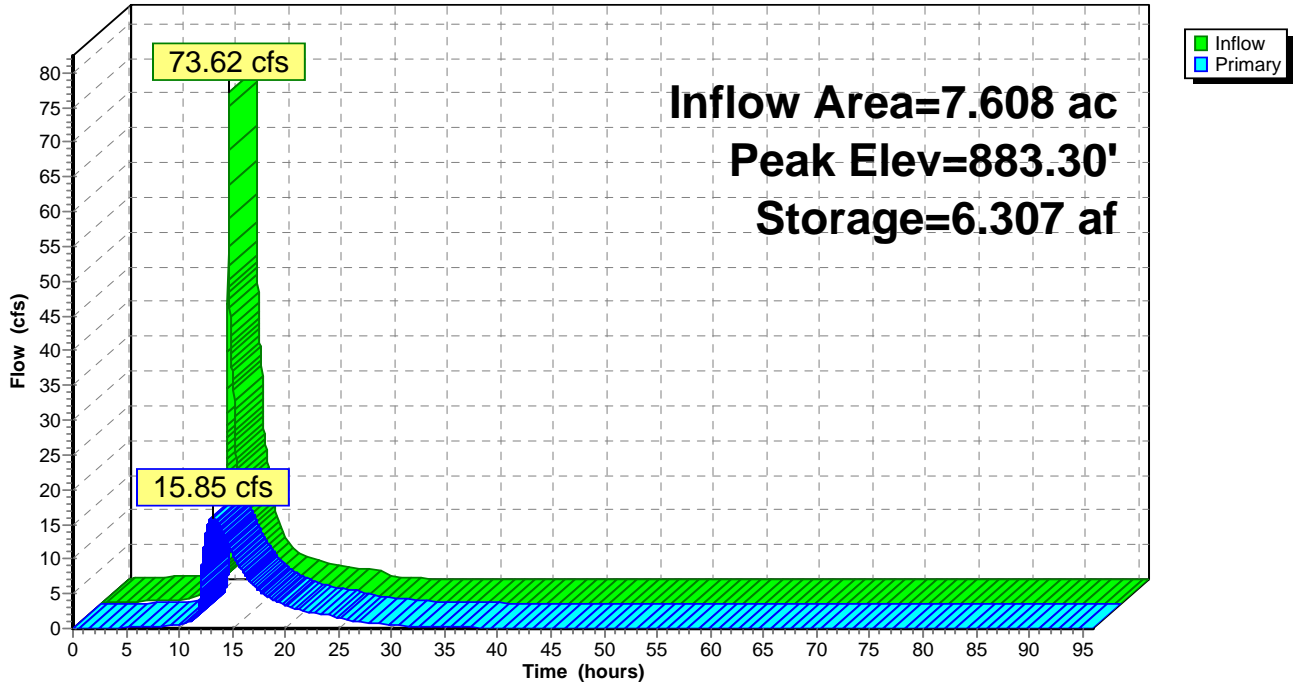
Device	Routing	Invert	Outlet Devices
#1	Primary	882.75'	6.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Primary	882.75'	6.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)

Primary OutFlow Max=15.85 cfs @ 13.13 hrs HW=883.30' TW=0.00' (Dynamic Tailwater)

- 1=Sharp-Crested Rectangular Weir (Weir Controls 7.92 cfs @ 2.43 fps)
- 2=Sharp-Crested Rectangular Weir (Weir Controls 7.92 cfs @ 2.43 fps)

Pond W-5: W-5

Hydrograph

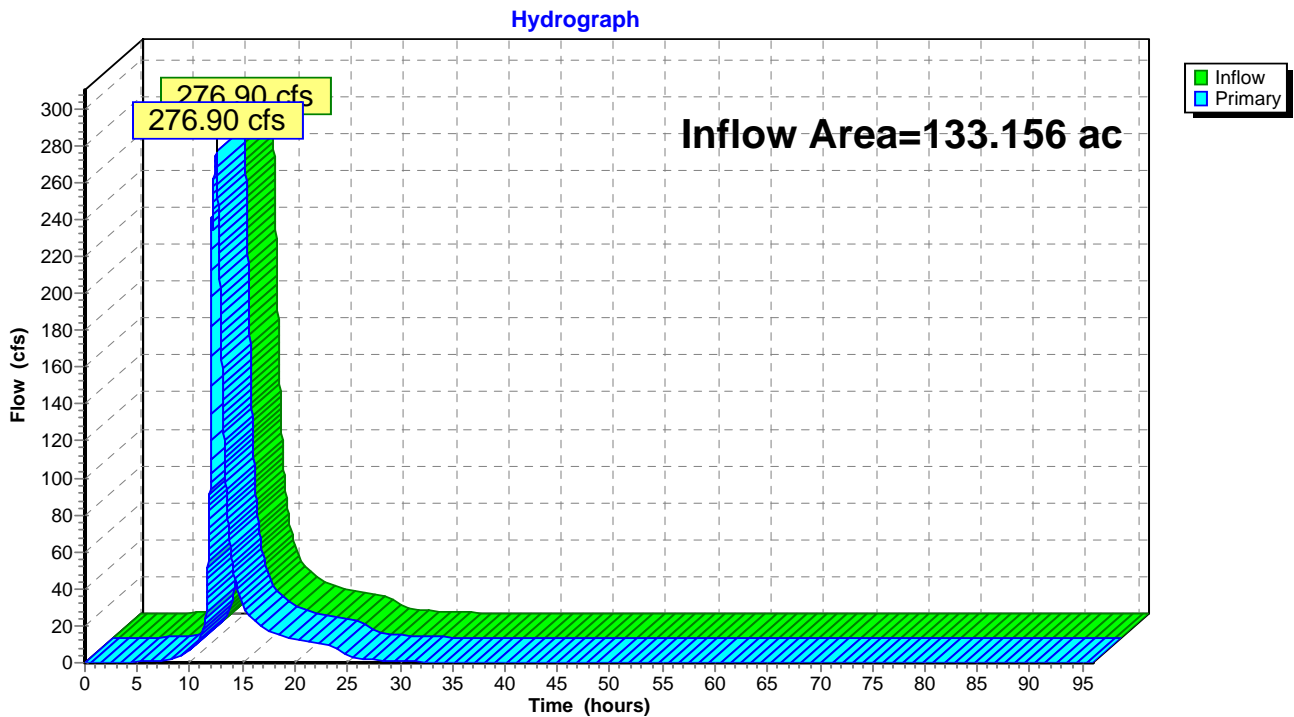


Summary for Link 53L: Sum of Outlet #2 Discharges to Round Lake

Inflow Area = 133.156 ac, 9.78% Impervious, Inflow Depth = 4.46" for 100-Year event
Inflow = 276.90 cfs @ 12.51 hrs, Volume= 49.540 af
Primary = 276.90 cfs @ 12.51 hrs, Volume= 49.540 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 53L: Sum of Outlet #2 Discharges to Round Lake

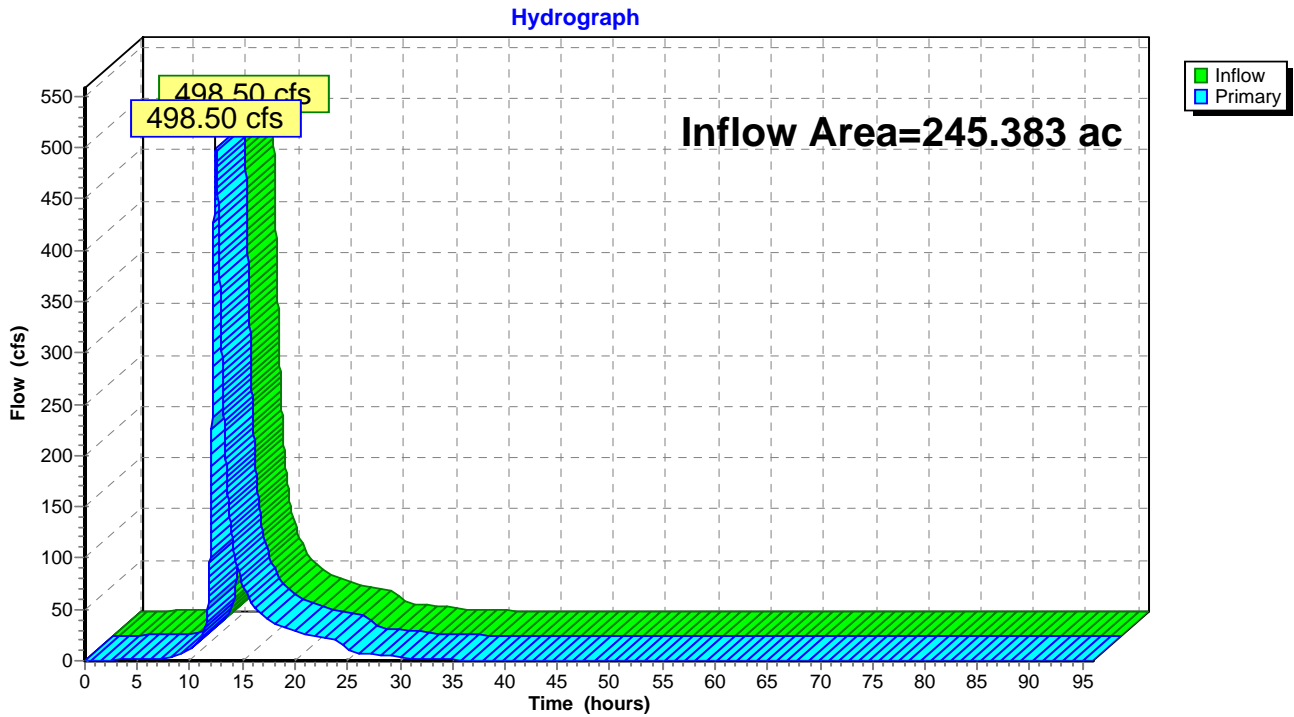


Summary for Link 54L: Sum of Discharges from P-13 and W-5

Inflow Area = 245.383 ac, 10.42% Impervious, Inflow Depth = 4.60" for 100-Year event
Inflow = 498.50 cfs @ 12.46 hrs, Volume= 94.152 af
Primary = 498.50 cfs @ 12.46 hrs, Volume= 94.152 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 54L: Sum of Discharges from P-13 and W-5



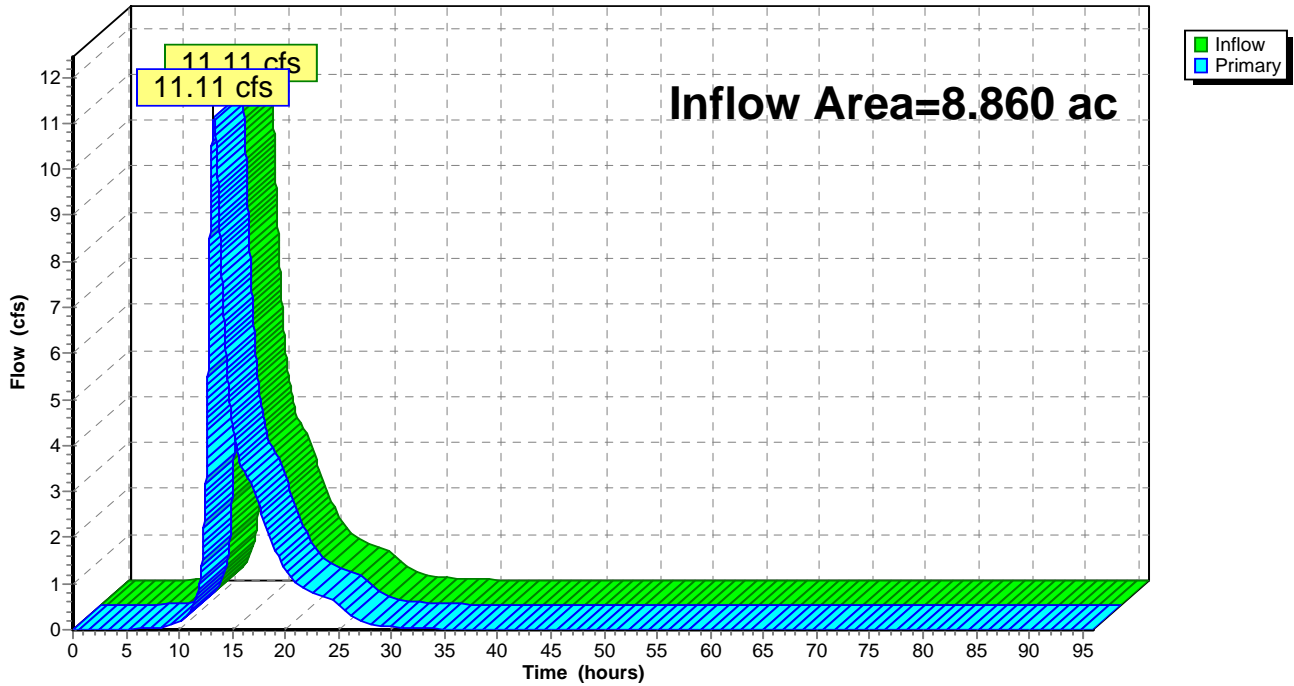
Summary for Link 55L: Sum of Outlet #1 Discharges to Round Lake

Inflow Area = 8.860 ac, 6.43% Impervious, Inflow Depth = 4.49" for 100-Year event
Inflow = 11.11 cfs @ 13.27 hrs, Volume= 3.318 af
Primary = 11.11 cfs @ 13.27 hrs, Volume= 3.318 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 55L: Sum of Outlet #1 Discharges to Round Lake

Hydrograph



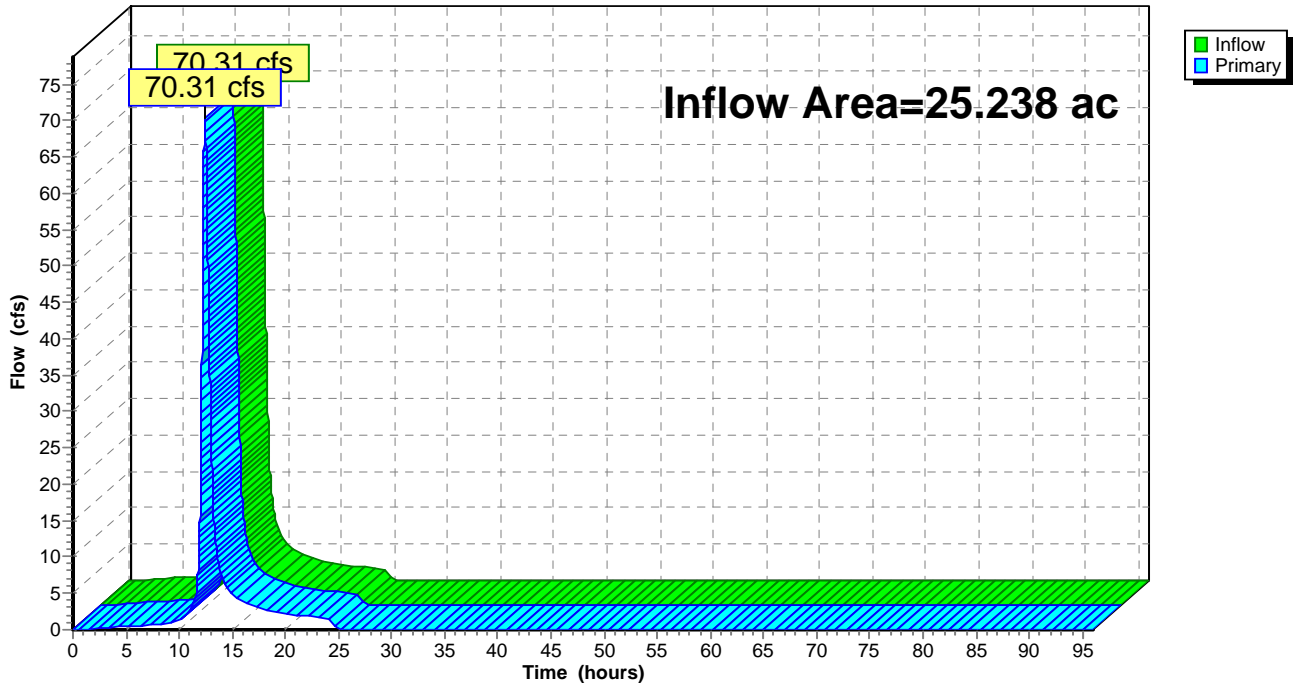
Summary for Link 57L: Outlet #3 Discharge to Round Lake

Inflow Area = 25.238 ac, 19.96% Impervious, Inflow Depth = 4.09" for 100-Year event
Inflow = 70.31 cfs @ 12.43 hrs, Volume= 8.599 af
Primary = 70.31 cfs @ 12.43 hrs, Volume= 8.599 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

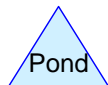
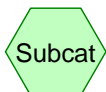
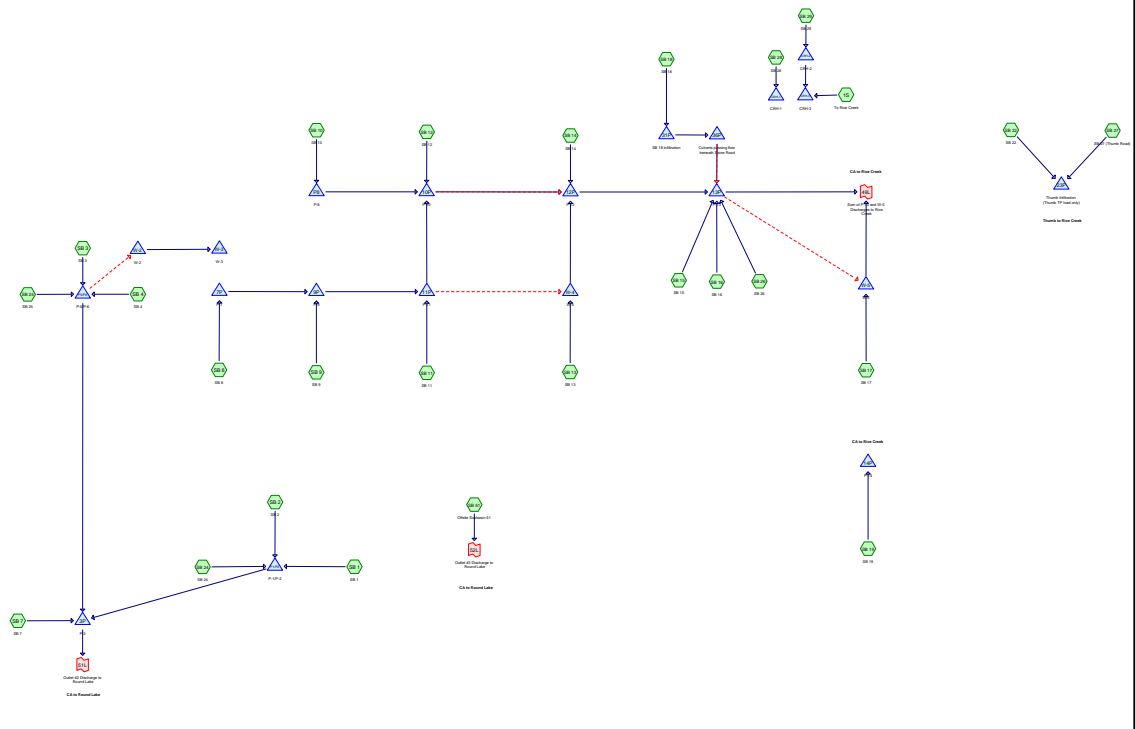
Link 57L: Outlet #3 Discharge to Round Lake

Hydrograph



Appendix C

Fully Developed Conditions Hydrology and Hydraulics Modeling (HydroCAD)



Routing Diagram for Full Buildout_HydroCAD_20150830
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Full Buildout_HydroCAD_20150830

Prepared by {enter your company name here}

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Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
5.038	98	(SB 51)
56.502	98	Impervious (SB 22, SB 27, SB 3)
20.200	65	Offsite subbasin 51 (SB 51)
7.656	49	Pervious (SB 22, SB 27)
22.050	74	Pervious (SB 3)
13.406	98	impermiabile (SB 24, SB 9)
191.729	98	impervious (1S, SB 1, SB 10, SB 12, SB 14, SB 15, SB 16, SB 18, SB 19, SB 2, SB 25, SB 26, SB 28, SB 29, SB 5, SB 7, SB 8)
6.029	100	impervious (SB 11, SB 13, SB 17, SB 4, SB 6)
17.322	74	permiabile (SB 24, SB 9)
161.530	74	pervious (1S, SB 1, SB 10, SB 11, SB 12, SB 13, SB 14, SB 15, SB 16, SB 17, SB 18, SB 19, SB 2, SB 25, SB 26, SB 28, SB 29, SB 4, SB 5, SB 6, SB 7, SB 8)
501.462	86	TOTAL AREA

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Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
501.462	Other	1S, SB 1, SB 10, SB 11, SB 12, SB 13, SB 14, SB 15, SB 16, SB 17, SB 18, SB 19, SB 2, SB 22, SB 24, SB 25, SB 26, SB 27, SB 28, SB 29, SB 3, SB 4, SB 5, SB 51, SB 6, SB 7, SB 8, SB 9
501.462		TOTAL AREA

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Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	0.000	0.000	5.038	5.038		SB 51
0.000	0.000	0.000	0.000	56.502	56.502	Impervious	SB 22, SB 27, SB 3
0.000	0.000	0.000	0.000	20.200	20.200	Offsite subbasin 51	SB 51
0.000	0.000	0.000	0.000	29.706	29.706	Pervious	SB 22, SB 27, SB 3
0.000	0.000	0.000	0.000	13.406	13.406	impermiabile	SB 24, SB 9
0.000	0.000	0.000	0.000	197.758	197.758	impervious	1S, SB 1, SB 10, SB 11, SB 12, SB 13, SB 14, SB 15, SB 16, SB 17, SB 18, SB 19, SB 2, SB 25, SB 26, SB 28, SB 29, SB 4, SB 5, SB 6, SB 7, SB 8
0.000	0.000	0.000	0.000	17.322	17.322	permiable	SB 24, SB 9
0.000	0.000	0.000	0.000	161.530	161.530	pervious	1S, SB 1, SB 10, SB 11, SB 12, SB 13, SB 14, SB 15, SB 16, SB 17, SB 18, SB 19, SB 2, SB 25, SB 26, SB 28, SB 29, SB 4, SB 5, SB 6, SB 7, SB 8
0.000	0.000	0.000	0.000	501.462	501.462	TOTAL AREA	

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Page 5

Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	4P	915.80	915.95	50.0	-0.0030	0.013	24.0	0.0	0.0
2	11P	910.00	909.00	200.0	0.0050	0.013	24.0	0.0	0.0
3	11P	910.00	909.00	200.0	0.0050	0.013	24.0	0.0	0.0
4	11P	909.00	908.00	150.0	0.0067	0.013	12.0	0.0	0.0
5	12P	893.50	893.35	30.0	0.0050	0.013	43.8	26.6	0.0
6	12P	893.50	893.35	30.0	0.0050	0.013	43.8	26.6	0.0
7	12P	893.50	893.35	30.0	0.0050	0.013	43.8	26.6	0.0
8	12P	893.50	893.35	30.0	0.0050	0.013	43.8	26.6	0.0
9	13P	883.00	882.75	100.0	0.0025	0.013	12.0	0.0	0.0
10	13P	883.00	882.75	100.0	0.0025	0.013	12.0	0.0	0.0
11	13P	883.00	882.75	100.0	0.0025	0.013	12.0	0.0	0.0
12	13P	883.00	882.75	100.0	0.0025	0.013	12.0	0.0	0.0
13	13P	883.00	882.75	100.0	0.0025	0.013	12.0	0.0	0.0
14	14P	893.00	892.75	50.0	0.0050	0.013	18.0	0.0	0.0
15	36P	887.50	886.50	100.0	0.0100	0.013	18.0	0.0	0.0
16	36P	887.50	886.50	100.0	0.0100	0.013	18.0	0.0	0.0
17	36P	887.50	886.50	100.0	0.0100	0.013	18.0	0.0	0.0
18	36P	887.50	886.50	100.0	0.0100	0.013	18.0	0.0	0.0
19	36P	887.50	886.50	100.0	0.0100	0.013	18.0	0.0	0.0
20	36P	887.50	886.50	100.0	0.0100	0.013	18.0	0.0	0.0
21	36P	887.50	886.50	100.0	0.0100	0.013	18.0	0.0	0.0
22	36P	887.50	886.50	100.0	0.0100	0.013	18.0	0.0	0.0
23	CRH-1	877.00	876.00	155.0	0.0065	0.013	24.0	0.0	0.0
24	CRH-1	877.00	876.00	155.0	0.0065	0.013	24.0	0.0	0.0
25	CRH-2	881.50	881.00	155.0	0.0032	0.013	24.0	0.0	0.0
26	CRH-2	881.50	881.00	155.0	0.0032	0.013	24.0	0.0	0.0
27	CRH-3	878.00	877.00	155.0	0.0065	0.013	24.0	0.0	0.0
28	CRH-3	878.00	877.00	155.0	0.0065	0.013	24.0	0.0	0.0
29	P8	897.00	895.94	380.0	0.0028	0.013	24.0	0.0	0.0
30	W-2	929.10	916.00	300.0	0.0437	0.013	12.0	0.0	0.0
31	W-4	908.00	904.00	170.0	0.0235	0.013	12.0	0.0	0.0

Time span=0.00-96.00 hrs, dt=0.01 hrs, 9601 points
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: To Rice Creek	Runoff Area=1.601 ac 31.98% Impervious Runoff Depth=1.37" Tc=5.7 min CN=74/98 Runoff=2.97 cfs 0.183 af
Subcatchment SB 1: SB 1	Runoff Area=52.192 ac 48.35% Impervious Runoff Depth=1.66" Tc=53.1 min CN=74/98 Runoff=44.04 cfs 7.233 af
Subcatchment SB 10: SB 10	Runoff Area=6.389 ac 7.62% Impervious Runoff Depth=0.93" Tc=7.3 min CN=74/98 Runoff=7.36 cfs 0.497 af
Subcatchment SB 11: SB 11	Runoff Area=3.293 ac 32.16% Impervious Runoff Depth=1.45" Tc=11.7 min CN=74/100 Runoff=4.75 cfs 0.397 af
Subcatchment SB 12: SB 12	Runoff Area=1.382 ac 38.71% Impervious Runoff Depth=1.49" Tc=9.5 min CN=74/98 Runoff=2.34 cfs 0.172 af
Subcatchment SB 13: SB 13	Runoff Area=2.985 ac 30.99% Impervious Runoff Depth=1.42" Tc=9.4 min CN=74/100 Runoff=4.64 cfs 0.354 af
Subcatchment SB 14: SB 14	Runoff Area=10.225 ac 42.62% Impervious Runoff Depth=1.56" Tc=4.3 min CN=74/98 Runoff=23.23 cfs 1.330 af
Subcatchment SB 15: SB 15	Runoff Area=58.564 ac 48.22% Impervious Runoff Depth=1.66" Tc=31.3 min CN=74/98 Runoff=64.93 cfs 8.104 af
Subcatchment SB 16: SB 16	Runoff Area=32.428 ac 33.53% Impervious Runoff Depth=1.40" Tc=12.1 min CN=74/98 Runoff=46.47 cfs 3.776 af
Subcatchment SB 17: SB 17	Runoff Area=7.608 ac 48.41% Impervious Runoff Depth=1.78" Tc=4.3 min CN=74/100 Runoff=18.70 cfs 1.126 af
Subcatchment SB 18: SB 18	Runoff Area=52.908 ac 84.55% Impervious Runoff Depth=2.31" Tc=33.5 min CN=74/98 Runoff=79.77 cfs 10.194 af
Subcatchment SB 19: SB 19	Runoff Area=21.198 ac 39.93% Impervious Runoff Depth=1.51" Tc=24.7 min CN=74/98 Runoff=23.92 cfs 2.671 af
Subcatchment SB 2: SB 2	Runoff Area=11.400 ac 84.29% Impervious Runoff Depth=2.31" Tc=16.6 min CN=74/98 Runoff=23.93 cfs 2.192 af
Subcatchment SB 22: SB 22	Runoff Area=41.911 ac 82.19% Impervious Runoff Depth=2.14" Tc=41.0 min CN=49/98 Runoff=52.40 cfs 7.462 af
Subcatchment SB 24: SB 24	Runoff Area=4.939 ac 98.22% Impervious Runoff Depth=2.56" Tc=7.5 min CN=74/98 Runoff=16.07 cfs 1.052 af
Subcatchment SB 25: SB 25	Runoff Area=5.012 ac 95.71% Impervious Runoff Depth=2.51" Tc=10.7 min CN=74/98 Runoff=13.90 cfs 1.049 af

Subcatchment SB 26: SB 26	Runoff Area=14.335 ac 98.27% Impervious Runoff Depth=2.56" Tc=25.4 min CN=74/98 Runoff=27.53 cfs 3.056 af
Subcatchment SB 27: SB 27 (Thumb Road)	Runoff Area=6.629 ac 97.12% Impervious Runoff Depth=2.52" Tc=27.6 min CN=49/98 Runoff=12.02 cfs 1.390 af
Subcatchment SB 28: SB 28	Runoff Area=6.955 ac 46.76% Impervious Runoff Depth=1.63" Tc=14.6 min CN=74/98 Runoff=10.87 cfs 0.947 af
Subcatchment SB 29: SB 29	Runoff Area=10.214 ac 37.73% Impervious Runoff Depth=1.47" Tc=19.1 min CN=74/98 Runoff=12.67 cfs 1.253 af
Subcatchment SB 3: SB 3	Runoff Area=37.668 ac 41.46% Impervious Runoff Depth=1.54" Tc=15.3 min CN=74/98 Runoff=54.30 cfs 4.833 af
Subcatchment SB 4: SB 4	Runoff Area=0.599 ac 19.70% Impervious Runoff Depth=1.19" Tc=5.9 min CN=74/100 Runoff=0.93 cfs 0.060 af
Subcatchment SB 5: SB 5	Runoff Area=7.853 ac 70.37% Impervious Runoff Depth=2.06" Tc=59.3 min CN=74/98 Runoff=7.70 cfs 1.347 af
Subcatchment SB 51: Offsite Subbasin 51	Runoff Area=25.238 ac 19.96% Impervious Runoff Depth=0.86" Tc=32.8 min CN=65/98 Runoff=12.44 cfs 1.805 af
Subcatchment SB 6: SB 6	Runoff Area=0.997 ac 24.47% Impervious Runoff Depth=1.29" Tc=20.3 min CN=74/100 Runoff=1.01 cfs 0.107 af
Subcatchment SB 7: SB 7	Runoff Area=21.555 ac 84.83% Impervious Runoff Depth=2.32" Tc=5.7 min CN=74/98 Runoff=68.54 cfs 4.162 af
Subcatchment SB 8: SB 8	Runoff Area=29.595 ac 30.01% Impervious Runoff Depth=1.33" Tc=47.1 min CN=74/98 Runoff=21.21 cfs 3.290 af
Subcatchment SB 9: SB 9	Runoff Area=25.789 ac 33.17% Impervious Runoff Depth=1.39" Tc=30.0 min CN=74/98 Runoff=24.30 cfs 2.989 af
Pond 3P: P-3	Peak Elev=916.51' Storage=11.001 af Inflow=90.72 cfs 17.776 af Outflow=46.80 cfs 17.759 af
Pond 4P: P-4	Peak Elev=916.23' Storage=1.025 af Inflow=7.70 cfs 1.347 af Primary=1.52 cfs 0.485 af Secondary=2.32 cfs 0.862 af Outflow=3.83 cfs 1.347 af
Pond 7P: P-7	Peak Elev=915.39' Storage=1.241 af Inflow=21.21 cfs 3.290 af Outflow=21.61 cfs 3.290 af
Pond 9P: P-9	Peak Elev=915.35' Storage=0.439 af Inflow=41.03 cfs 6.280 af Outflow=40.96 cfs 6.280 af
Pond 10P: P-10	Peak Elev=897.48' Storage=1.172 af Inflow=14.23 cfs 5.606 af Primary=11.01 cfs 5.505 af Secondary=2.42 cfs 0.100 af Outflow=13.43 cfs 5.605 af
Pond 11P: P-11	Peak Elev=910.94' Storage=6.202 af Inflow=42.67 cfs 6.677 af Primary=13.46 cfs 4.943 af Secondary=3.61 cfs 1.732 af Outflow=17.07 cfs 6.675 af

Pond 12P: P-12 Peak Elev=893.86' Storage=6.533 af Inflow=25.40 cfs 9.002 af
Outflow=12.77 cfs 8.994 af

Pond 13P: P-13 Peak Elev=884.08' Storage=6.493 af Inflow=197.71 cfs 30.805 af
Primary=165.33 cfs 29.381 af Secondary=10.39 cfs 1.421 af Outflow=175.72 cfs 30.802 af

Pond 14P: P-14 Peak Elev=892.95' Storage=5.837 af Inflow=23.92 cfs 2.671 af
Outflow=3.68 cfs 2.669 af

Pond 23P: Thumb Infiltration (Thumb TP Peak Elev=903.81' Storage=3.810 af Inflow=62.51 cfs 8.852 af
Outflow=61.16 cfs 5.112 af

Pond 31P: SB 18 Infiltration Peak Elev=903.40' Storage=3.403 af Inflow=79.77 cfs 10.194 af
Outflow=79.67 cfs 6.874 af

Pond 36P: Culverts passing flow beneath Peak Elev=887.32' Storage=0.000 af Inflow=79.67 cfs 6.874 af
Primary=79.68 cfs 6.874 af Secondary=0.00 cfs 0.000 af Outflow=79.68 cfs 6.874 af

Pond CRH-1: CRH-1 Peak Elev=877.67' Storage=0.354 af Inflow=10.87 cfs 0.947 af
Discarded=0.22 cfs 0.471 af Primary=4.58 cfs 0.476 af Outflow=4.80 cfs 0.947 af

Pond CRH-2: CRH-2 Peak Elev=882.04' Storage=0.617 af Inflow=12.67 cfs 1.253 af
Discarded=0.33 cfs 0.833 af Primary=2.29 cfs 0.420 af Outflow=2.62 cfs 1.254 af

Pond CRH-3: CRH-3 Peak Elev=878.30' Storage=0.259 af Inflow=2.97 cfs 0.603 af
Discarded=0.20 cfs 0.381 af Primary=0.97 cfs 0.222 af Outflow=1.17 cfs 0.603 af

Pond P1/P2: P-1/P-2 Peak Elev=924.94' Storage=4.760 af Inflow=54.34 cfs 10.477 af
Outflow=52.67 cfs 10.470 af

Pond P5/P6: P-5/P-6 Peak Elev=930.72' Storage=8.830 af Inflow=66.74 cfs 5.941 af
Primary=5.60 cfs 3.144 af Secondary=1.26 cfs 0.681 af Outflow=6.86 cfs 3.826 af

Pond P8: P-8 Peak Elev=897.62' Storage=0.712 af Inflow=7.36 cfs 0.497 af
24.0" Round Culvert n=0.013 L=380.0' S=0.0028 '/' Outflow=1.25 cfs 0.491 af

Pond W-1: W-1 Peak Elev=915.09' Storage=0.231 af Inflow=2.44 cfs 0.969 af
Outflow=2.00 cfs 0.969 af

Pond W-2: W-2 Peak Elev=929.37' Storage=0.413 af Inflow=1.26 cfs 0.681 af
12.0" Round Culvert n=0.013 L=300.0' S=0.0437 '/' Outflow=0.37 cfs 0.536 af

Pond W-3: W-3 Peak Elev=915.01' Storage=0.536 af Inflow=0.37 cfs 0.536 af
Outflow=0.00 cfs 0.000 af

Pond W-4: W-4 Peak Elev=908.83' Storage=0.769 af Inflow=5.28 cfs 2.086 af
12.0" Round Culvert n=0.013 L=170.0' S=0.0235 '/' Outflow=2.69 cfs 2.067 af

Pond W-5: W-5 Peak Elev=882.98' Storage=8.297 af Inflow=20.53 cfs 2.547 af
Outflow=4.37 cfs 2.540 af

Link 49L: Sum of P-13 and W-5 Discharges to Rice Creek

Inflow=168.38 cfs 31.921 af
Primary=168.38 cfs 31.921 af

Link 50L: Outlet #1 Discharge to Round Lake

Inflow=3.24 cfs 1.454 af
Primary=3.24 cfs 1.454 af

Link 51L: Outlet #2 Discharge to Round Lake

Inflow=46.80 cfs 17.759 af
Primary=46.80 cfs 17.759 af

Link 52L: Outlet #3 Discharge to Round Lake

Inflow=12.44 cfs 1.805 af
Primary=12.44 cfs 1.805 af

Total Runoff Area = 501.462 ac Runoff Volume = 73.031 af Average Runoff Depth = 1.75"
45.62% Pervious = 228.758 ac 54.38% Impervious = 272.704 ac

Summary for Subcatchment 1S: To Rice Creek

Runoff = 2.97 cfs @ 12.04 hrs, Volume= 0.183 af, Depth= 1.37"

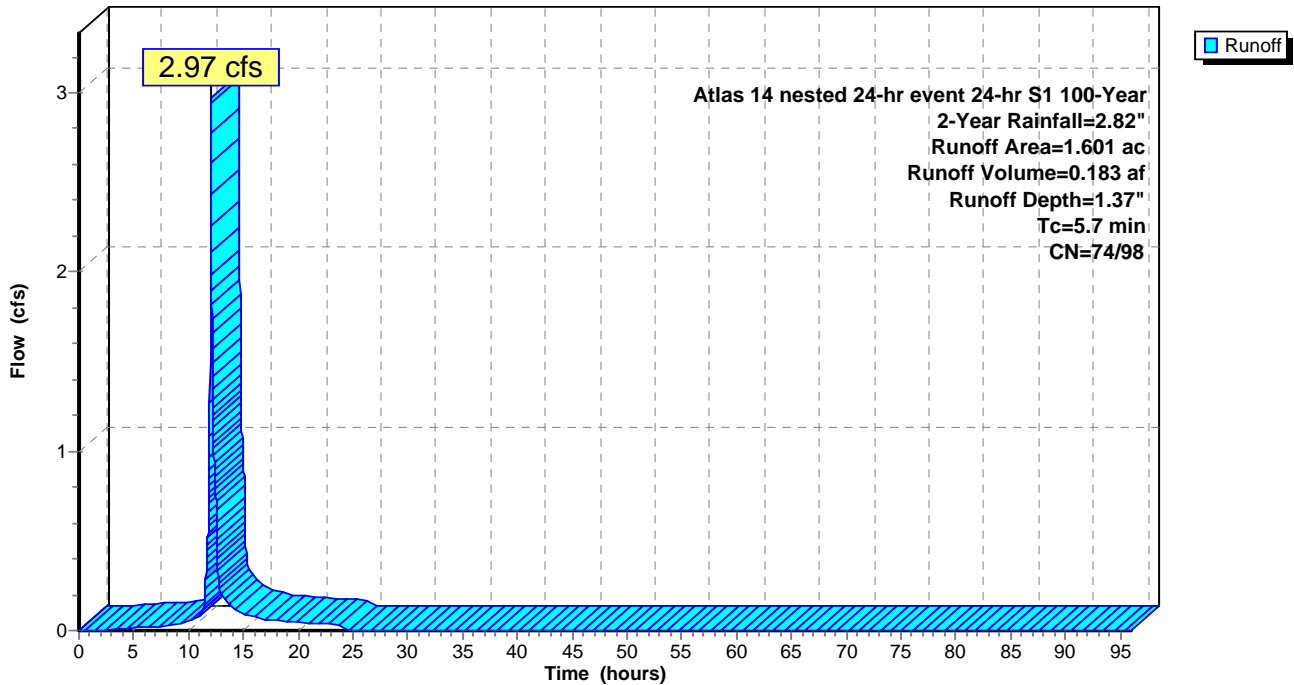
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 0.512	98	impervious
* 1.089	74	pervious
1.601	82	Weighted Average
1.089		68.02% Pervious Area
0.512		31.98% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.7					Direct Entry,

Subcatchment 1S: To Rice Creek

Hydrograph



Summary for Subcatchment SB 1: SB 1

Runoff = 44.04 cfs @ 12.69 hrs, Volume= 7.233 af, Depth= 1.66"

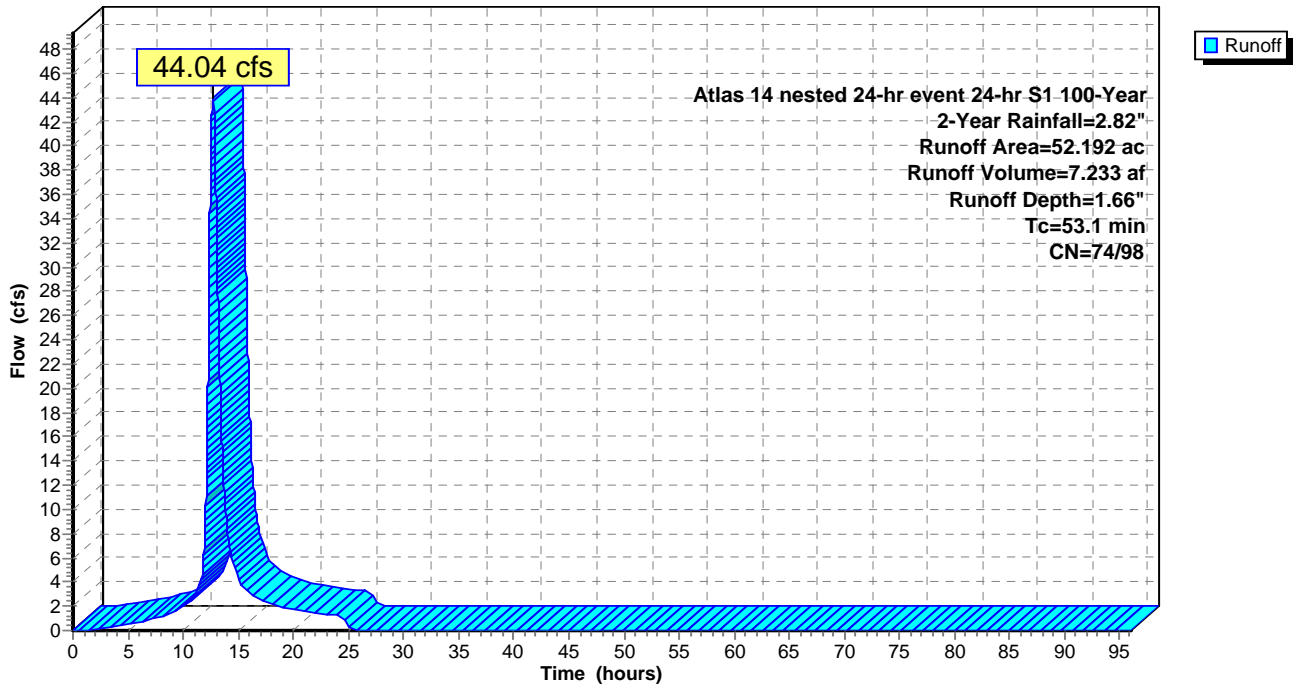
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 26.958	74	pervious
* 25.234	98	impervious
52.192	86	Weighted Average
26.958		51.65% Pervious Area
25.234		48.35% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
53.1					Direct Entry,

Subcatchment SB 1: SB 1

Hydrograph



Summary for Subcatchment SB 10: SB 10

Runoff = 7.36 cfs @ 12.06 hrs, Volume= 0.497 af, Depth= 0.93"

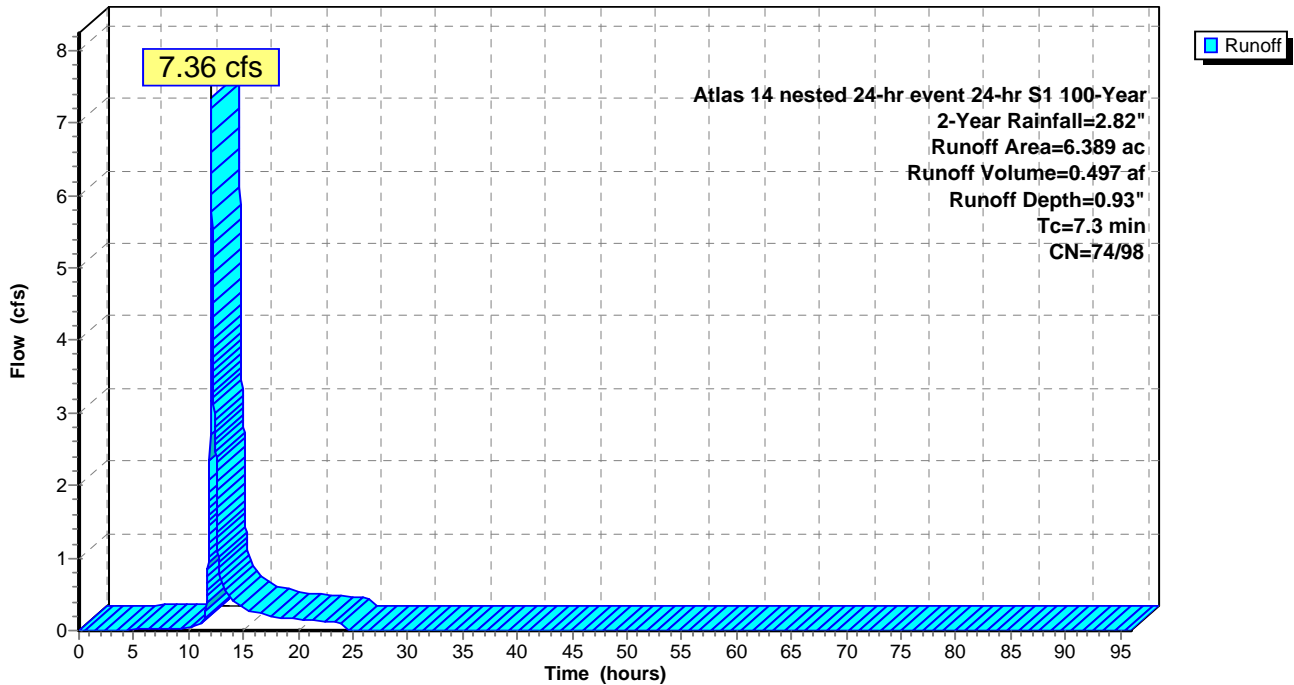
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 5.902	74	pervious
* 0.487	98	impervious
6.389	76	Weighted Average
5.902		92.38% Pervious Area
0.487		7.62% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.3					Direct Entry,

Subcatchment SB 10: SB 10

Hydrograph



Summary for Subcatchment SB 11: SB 11

Runoff = 4.75 cfs @ 12.12 hrs, Volume= 0.397 af, Depth= 1.45"

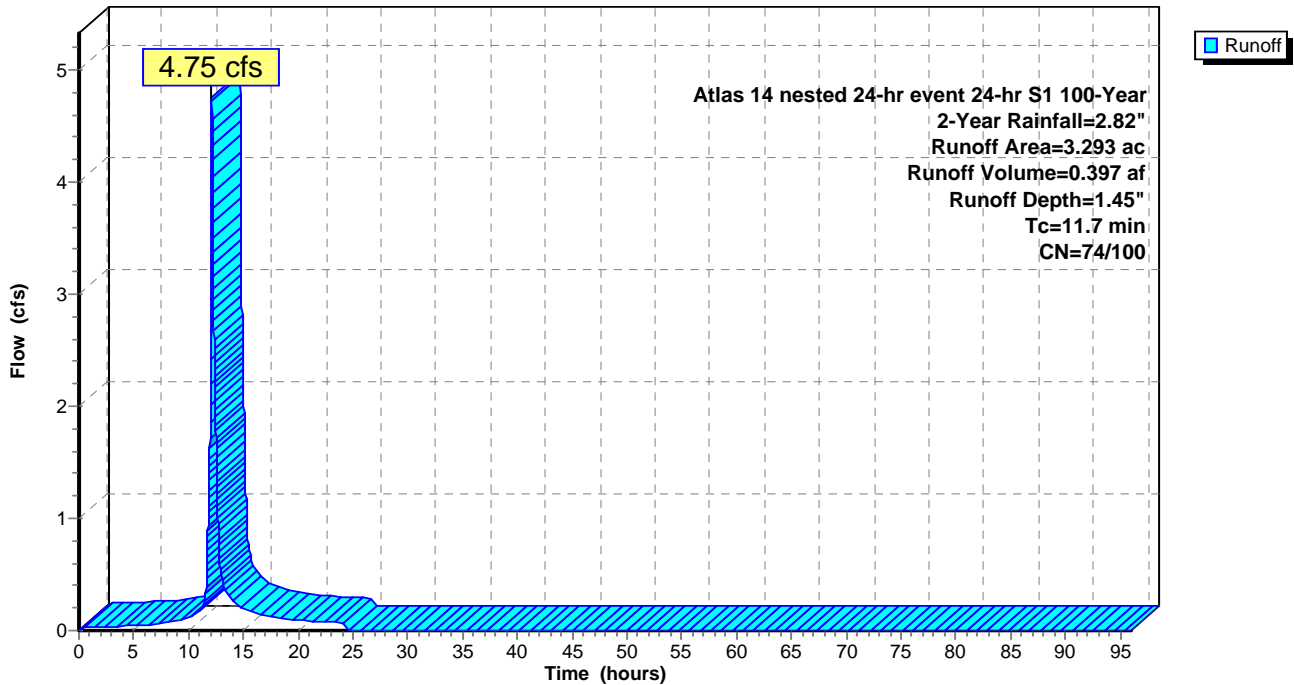
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 2.234	74	pervious
* 1.059	100	impervious
3.293	82	Weighted Average
2.234		67.84% Pervious Area
1.059		32.16% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.7					Direct Entry,

Subcatchment SB 11: SB 11

Hydrograph



Summary for Subcatchment SB 12: SB 12

Runoff = 2.34 cfs @ 12.08 hrs, Volume= 0.172 af, Depth= 1.49"

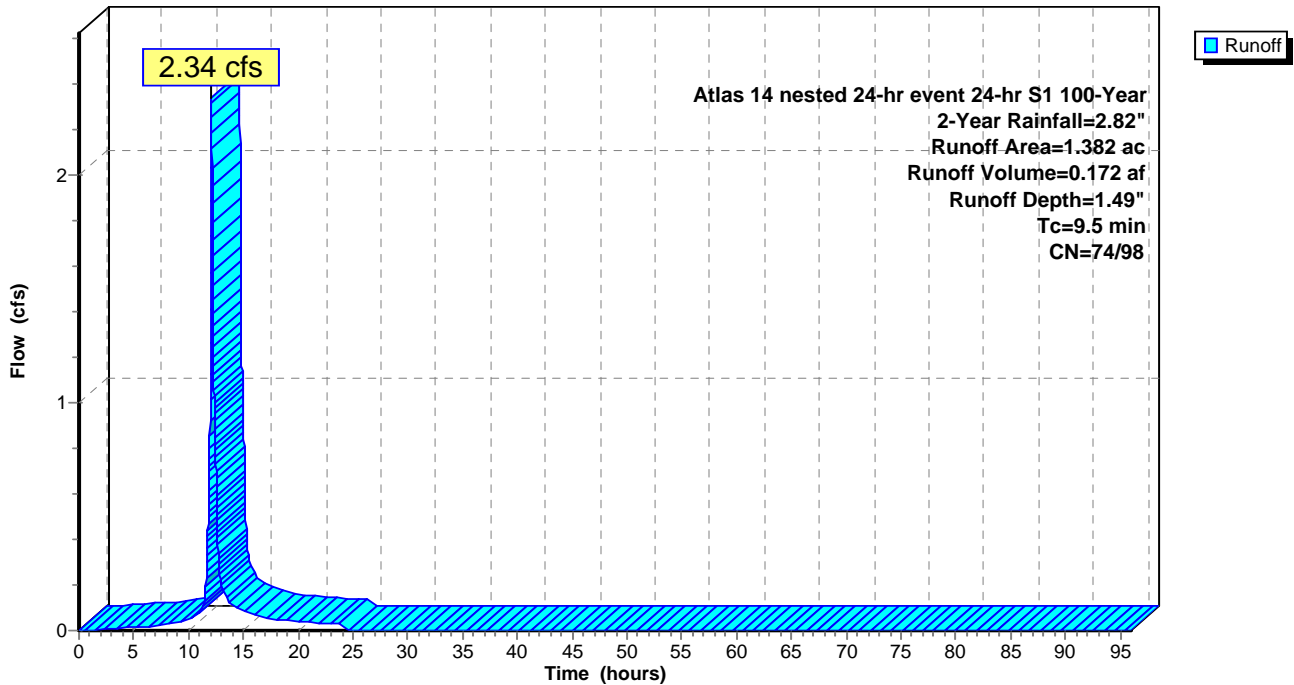
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 0.847	74	pervious
* 0.535	98	impervious
1.382	83	Weighted Average
0.847		61.29% Pervious Area
0.535		38.71% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.5					Direct Entry,

Subcatchment SB 12: SB 12

Hydrograph



Summary for Subcatchment SB 13: SB 13

Runoff = 4.64 cfs @ 12.08 hrs, Volume= 0.354 af, Depth= 1.42"

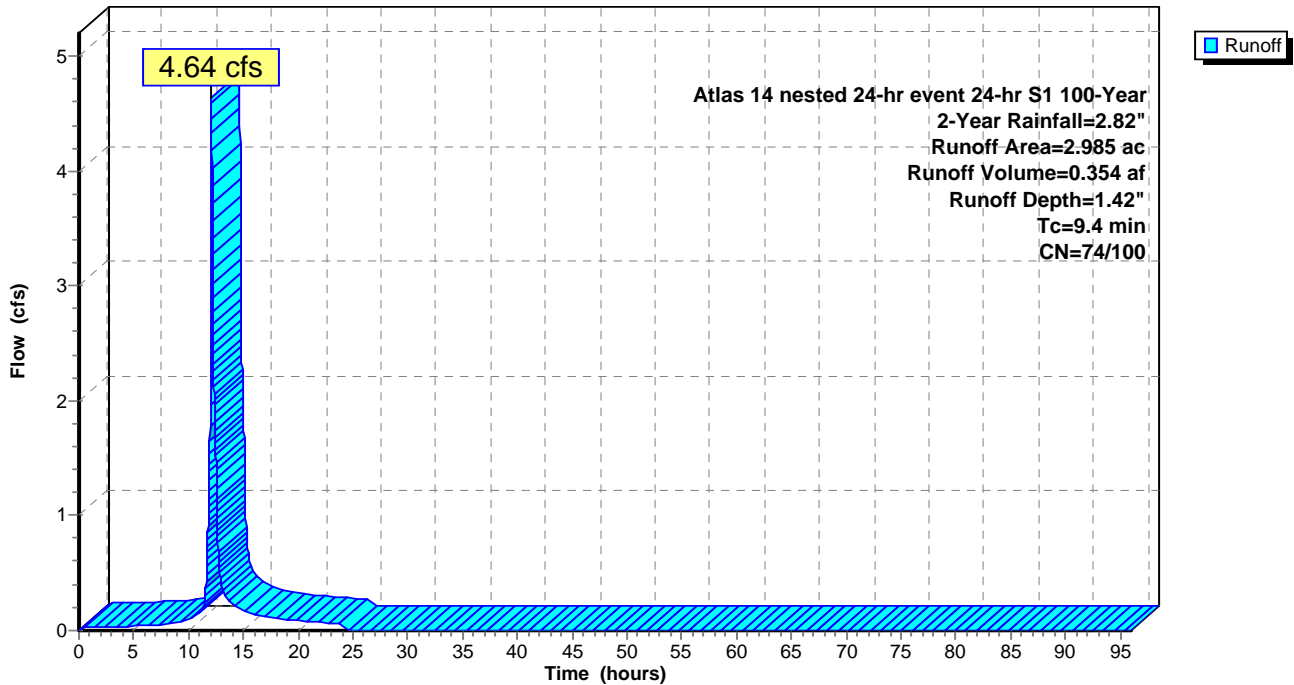
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 2.060	74	pervious
* 0.925	100	impervious
2.985	82	Weighted Average
2.060		69.01% Pervious Area
0.925		30.99% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.4					Direct Entry,

Subcatchment SB 13: SB 13

Hydrograph



Summary for Subcatchment SB 14: SB 14

Runoff = 23.23 cfs @ 12.02 hrs, Volume= 1.330 af, Depth= 1.56"

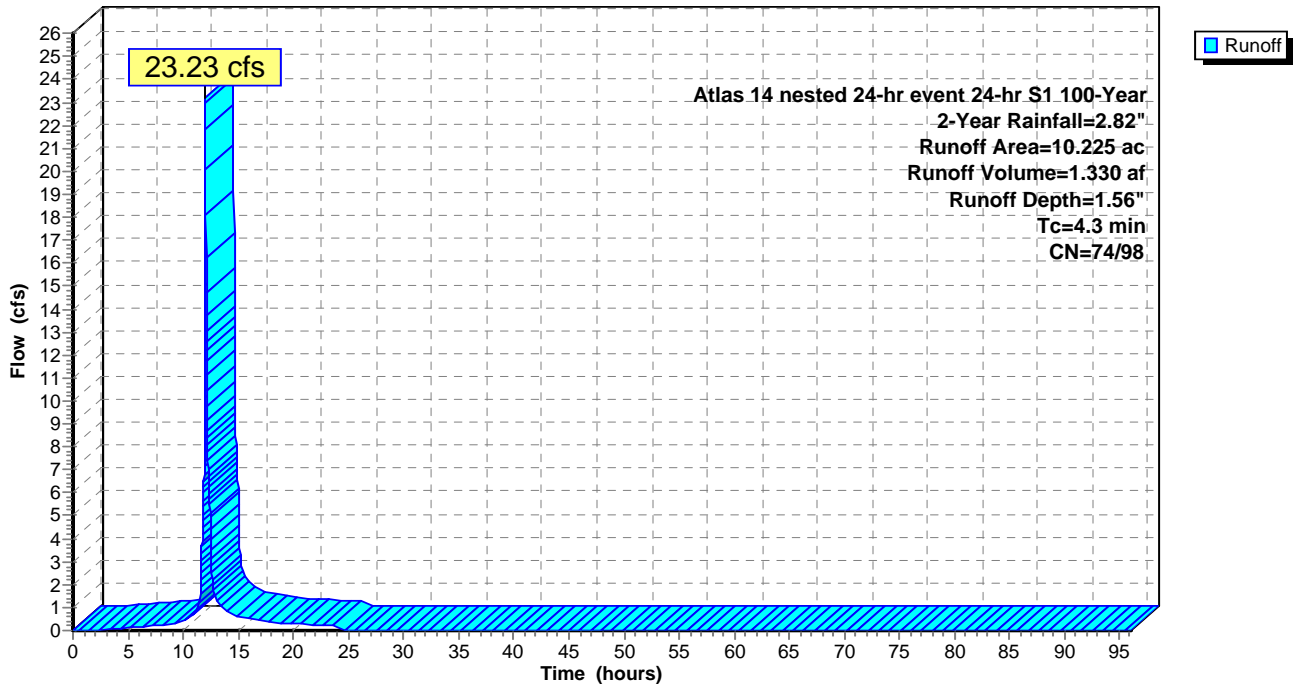
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 5.867	74	pervious
* 4.358	98	impervious
10.225	84	Weighted Average
5.867		57.38% Pervious Area
4.358		42.62% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.3					Direct Entry,

Subcatchment SB 14: SB 14

Hydrograph



Summary for Subcatchment SB 15: SB 15

Runoff = 64.93 cfs @ 12.39 hrs, Volume= 8.104 af, Depth= 1.66"

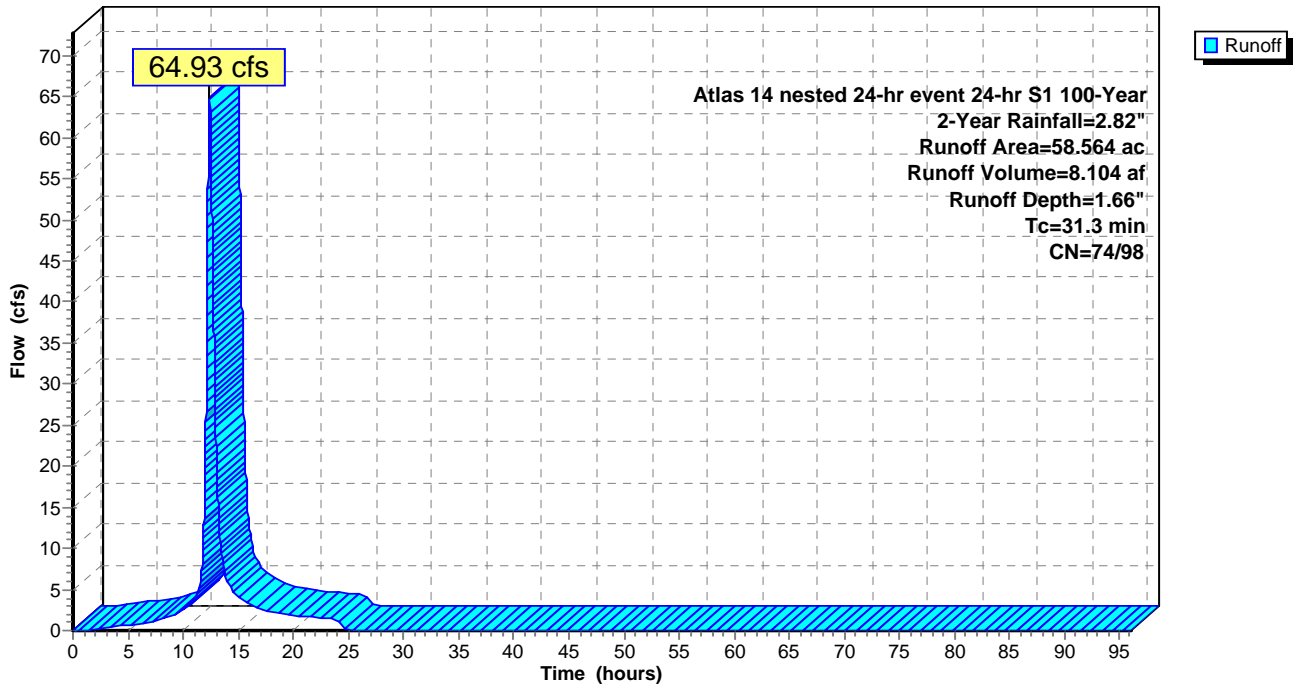
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 30.326	74	pervious
* 28.238	98	impervious
58.564	86	Weighted Average
30.326		51.78% Pervious Area
28.238		48.22% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
31.3					Direct Entry,

Subcatchment SB 15: SB 15

Hydrograph



Summary for Subcatchment SB 16: SB 16

Runoff = 46.47 cfs @ 12.12 hrs, Volume= 3.776 af, Depth= 1.40"

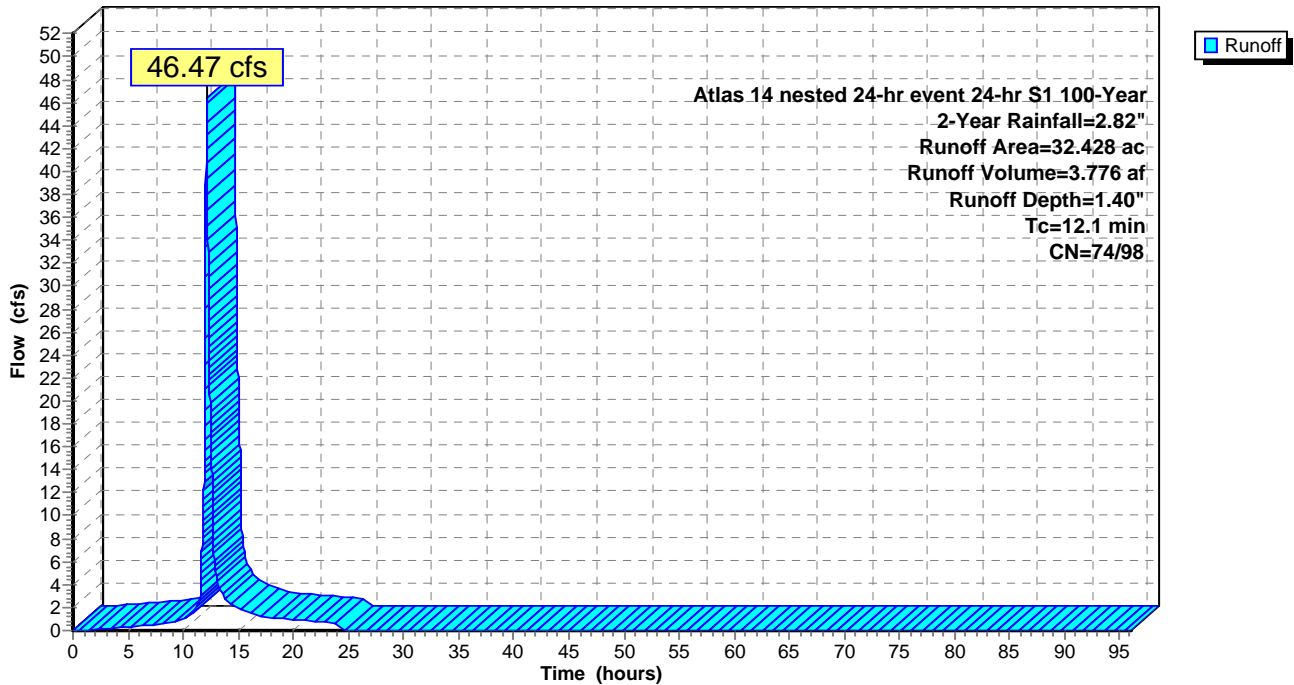
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 21.555	74	pervious
* 10.873	98	impervious
32.428	82	Weighted Average
21.555		66.47% Pervious Area
10.873		33.53% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.1					Direct Entry,

Subcatchment SB 16: SB 16

Hydrograph



Summary for Subcatchment SB 17: SB 17

Runoff = 18.70 cfs @ 12.02 hrs, Volume= 1.126 af, Depth= 1.78"

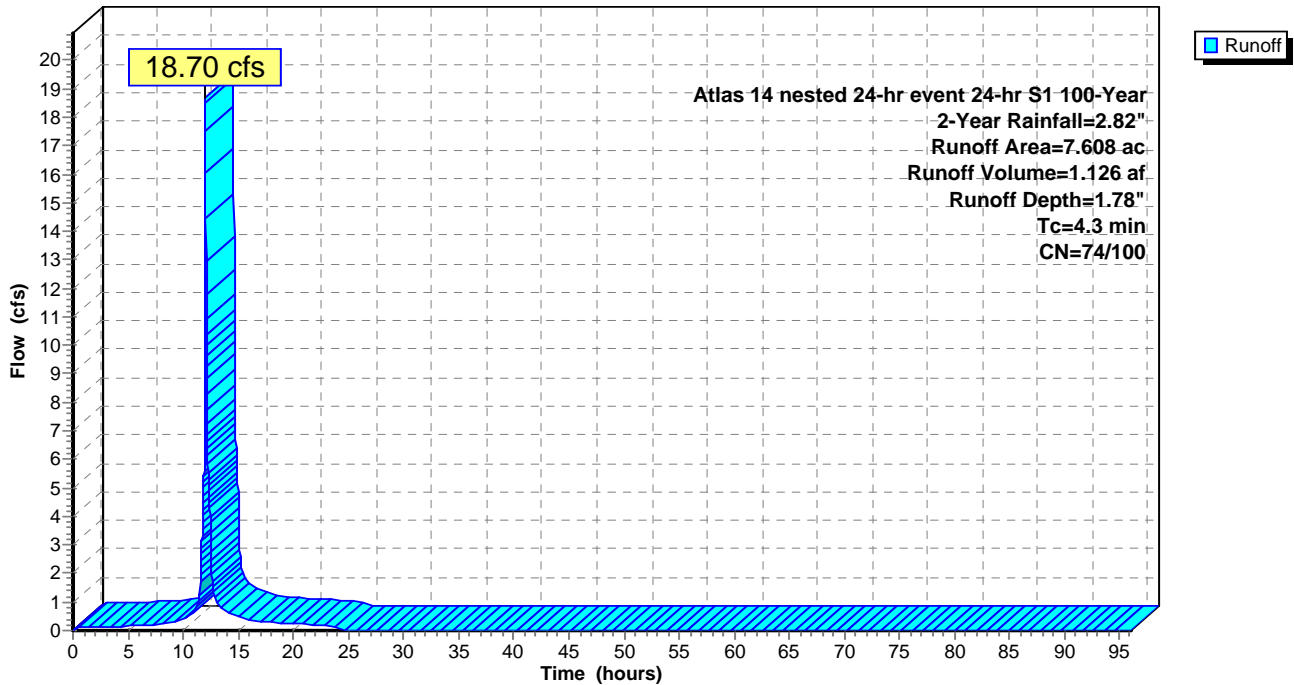
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 3.925	74	pervious
* 3.683	100	impervious
7.608	87	Weighted Average
3.925		51.59% Pervious Area
3.683		48.41% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.3					Direct Entry,

Subcatchment SB 17: SB 17

Hydrograph



Summary for Subcatchment SB 18: SB 18

Runoff = 79.77 cfs @ 12.40 hrs, Volume= 10.194 af, Depth= 2.31"

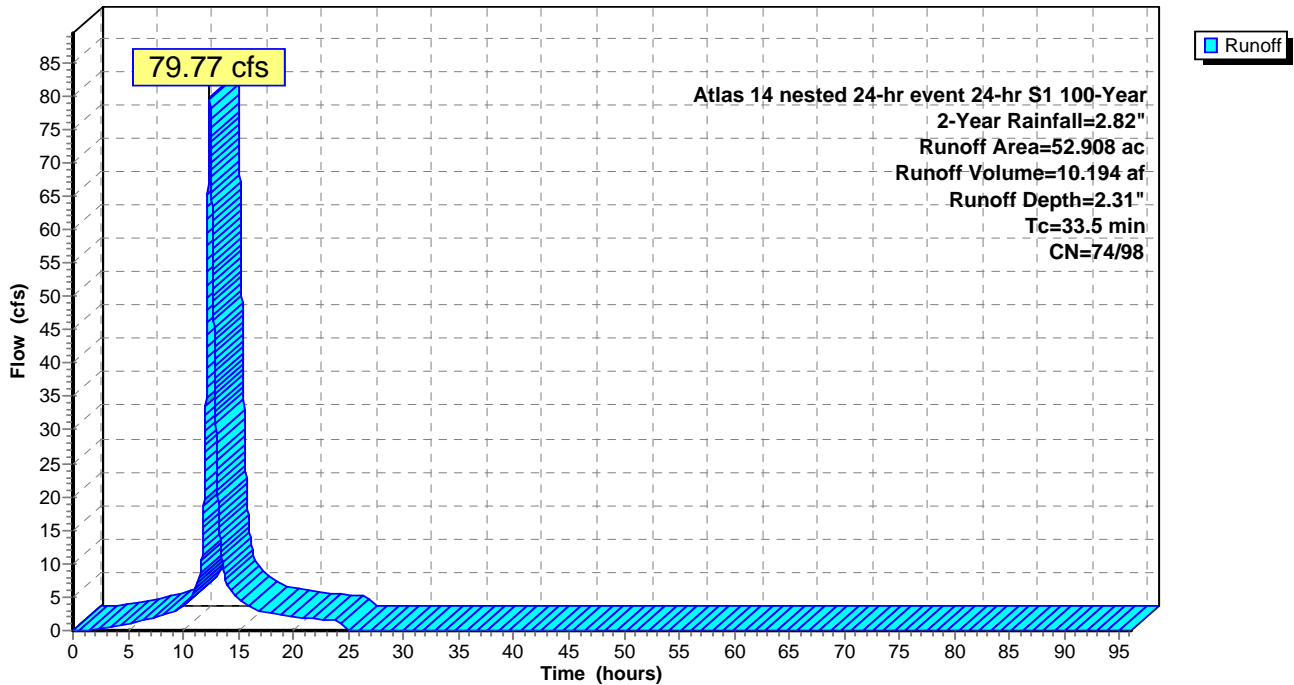
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 8.172	74	pervious
* 44.736	98	impervious
52.908	94	Weighted Average
8.172		15.45% Pervious Area
44.736		84.55% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
33.5					Direct Entry,

Subcatchment SB 18: SB 18

Hydrograph



Summary for Subcatchment SB 19: SB 19

Runoff = 23.92 cfs @ 12.30 hrs, Volume= 2.671 af, Depth= 1.51"

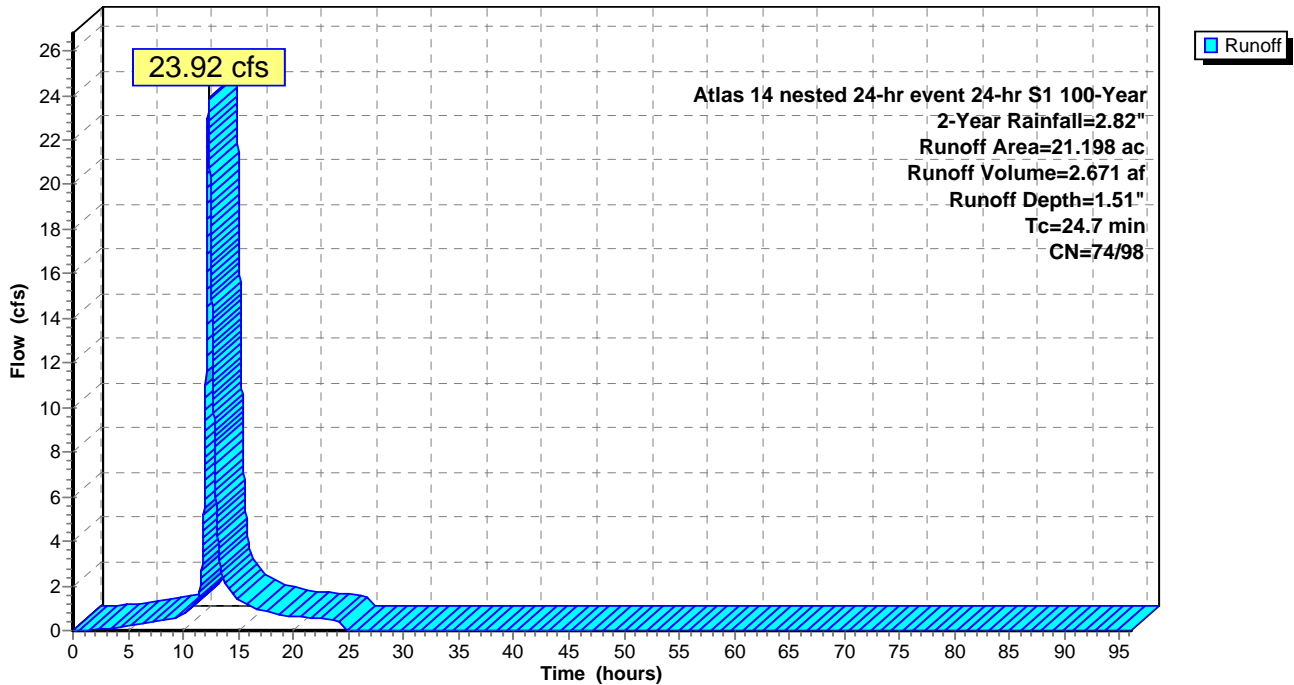
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 12.734	74	pervious
* 8.464	98	impervious
21.198	84	Weighted Average
12.734		60.07% Pervious Area
8.464		39.93% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.7					Direct Entry,

Subcatchment SB 19: SB 19

Hydrograph



Summary for Subcatchment SB 2: SB 2

Runoff = 23.93 cfs @ 12.18 hrs, Volume= 2.192 af, Depth= 2.31"

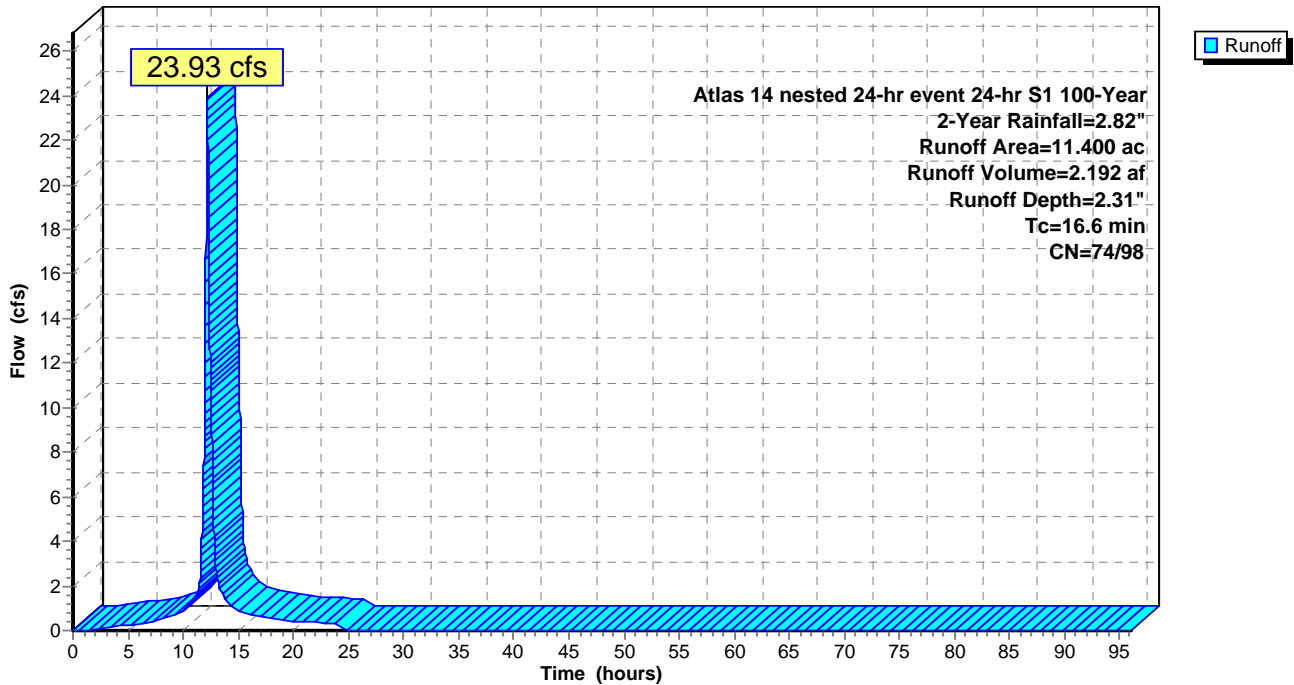
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 1.791	74	pervious
* 9.609	98	impervious
11.400	94	Weighted Average
1.791		15.71% Pervious Area
9.609		84.29% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.6					Direct Entry,

Subcatchment SB 2: SB 2

Hydrograph



Summary for Subcatchment SB 22: SB 22

Runoff = 52.40 cfs @ 12.52 hrs, Volume= 7.462 af, Depth= 2.14"

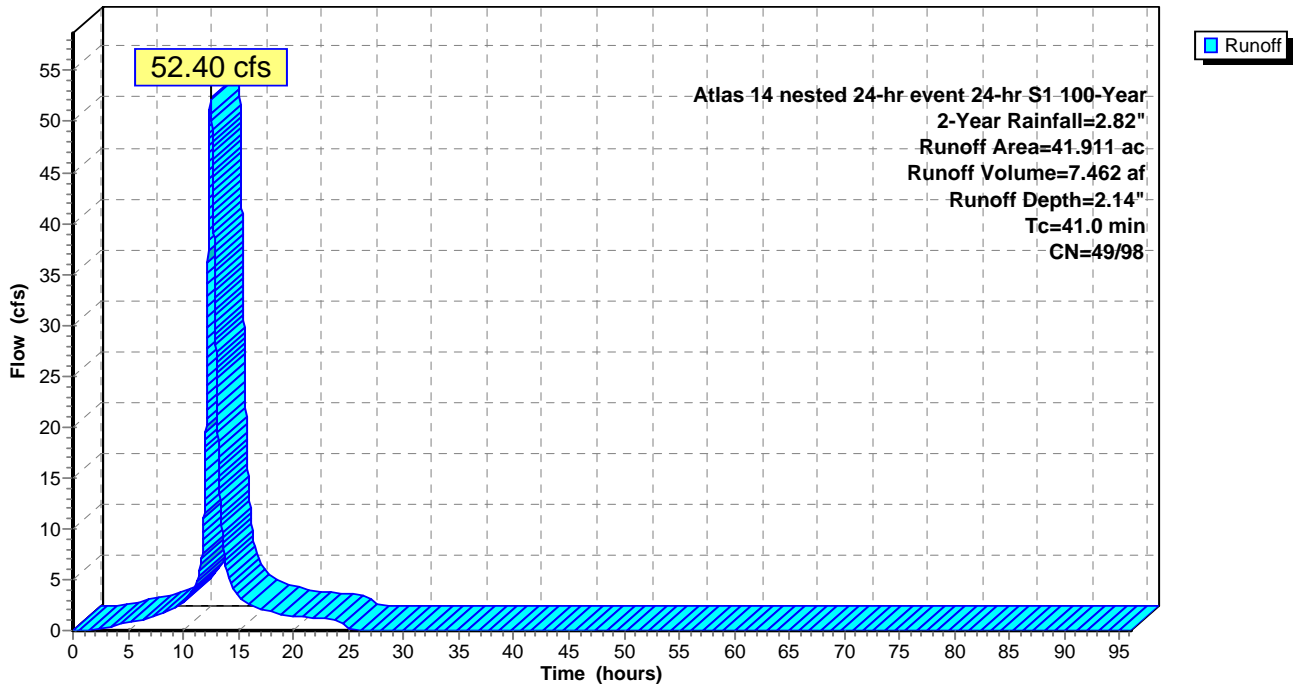
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 7.465	49	Pervious
* 34.446	98	Impervious
41.911	89	Weighted Average
7.465		17.81% Pervious Area
34.446		82.19% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
41.0					Direct Entry,

Subcatchment SB 22: SB 22

Hydrograph



Summary for Subcatchment SB 24: SB 24

Runoff = 16.07 cfs @ 12.05 hrs, Volume= 1.052 af, Depth= 2.56"

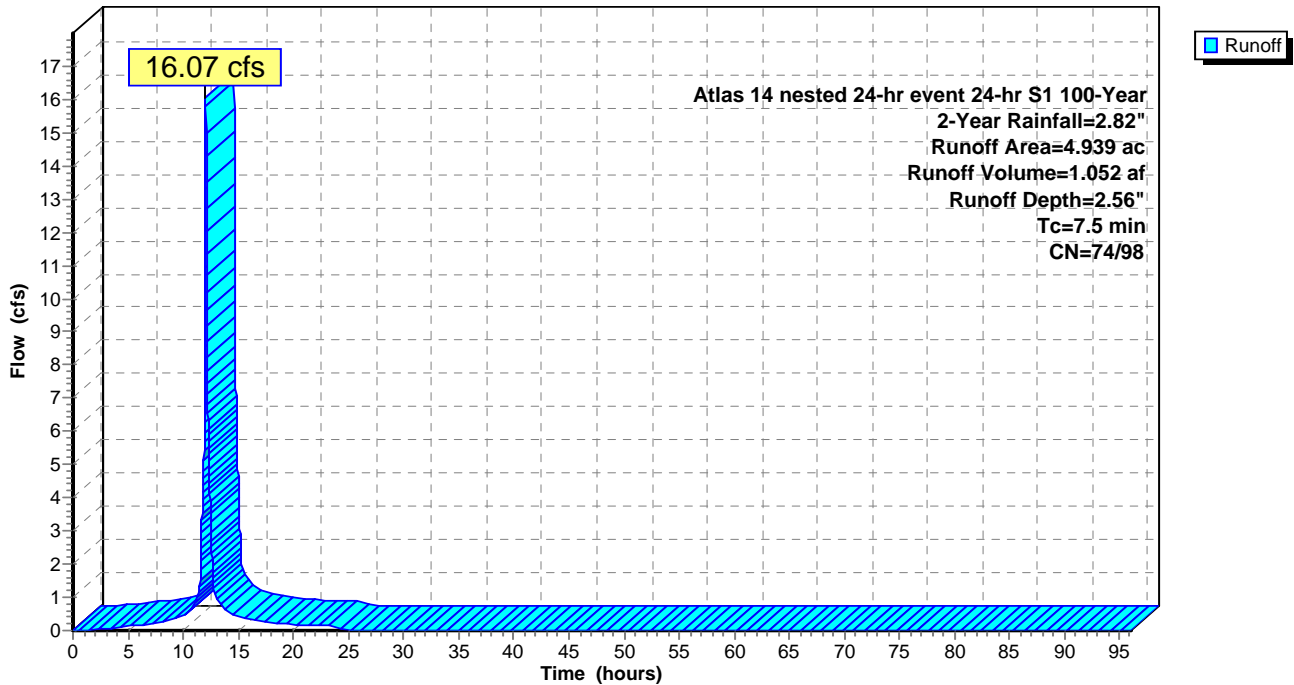
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 0.088	74	permiabile
* 4.851	98	impermiabile
4.939	98	Weighted Average
0.088		1.78% Pervious Area
4.851		98.22% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.5					Direct Entry,

Subcatchment SB 24: SB 24

Hydrograph



Summary for Subcatchment SB 25: SB 25

Runoff = 13.90 cfs @ 12.09 hrs, Volume= 1.049 af, Depth= 2.51"

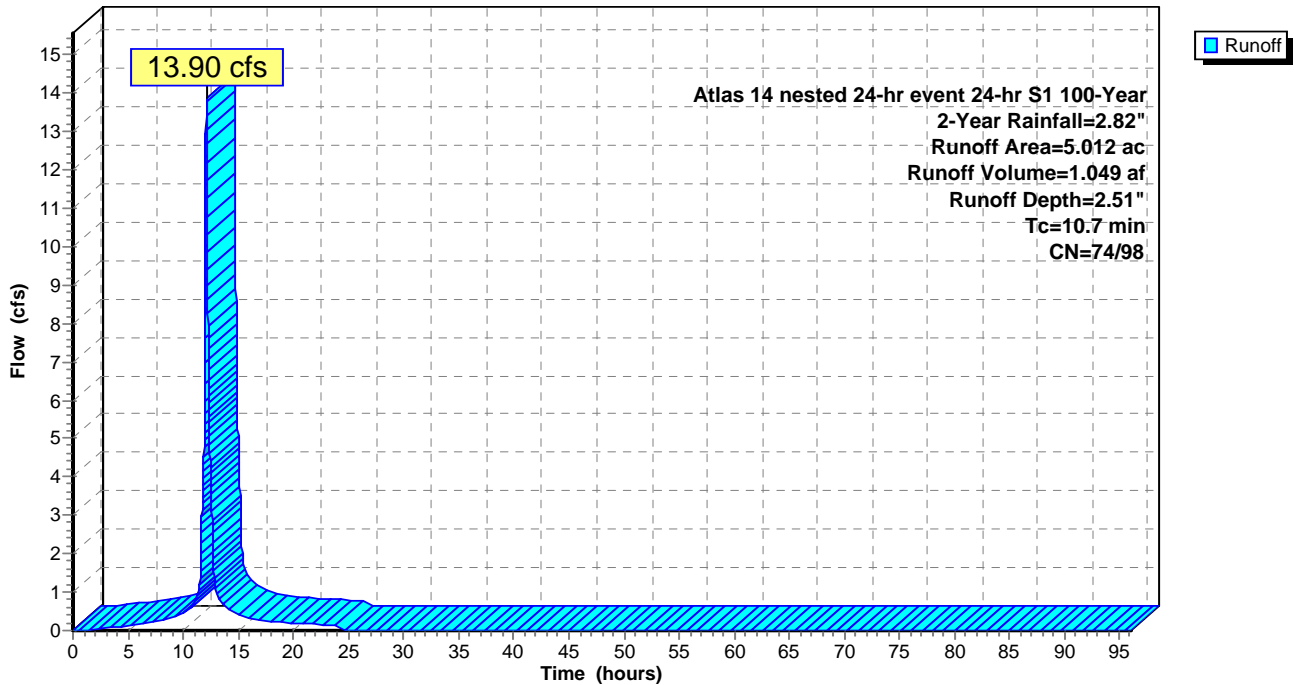
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 0.215	74	pervious
* 4.797	98	impervious
5.012	97	Weighted Average
0.215		4.29% Pervious Area
4.797		95.71% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7					Direct Entry,

Subcatchment SB 25: SB 25

Hydrograph



Summary for Subcatchment SB 26: SB 26

Runoff = 27.53 cfs @ 12.28 hrs, Volume= 3.056 af, Depth= 2.56"

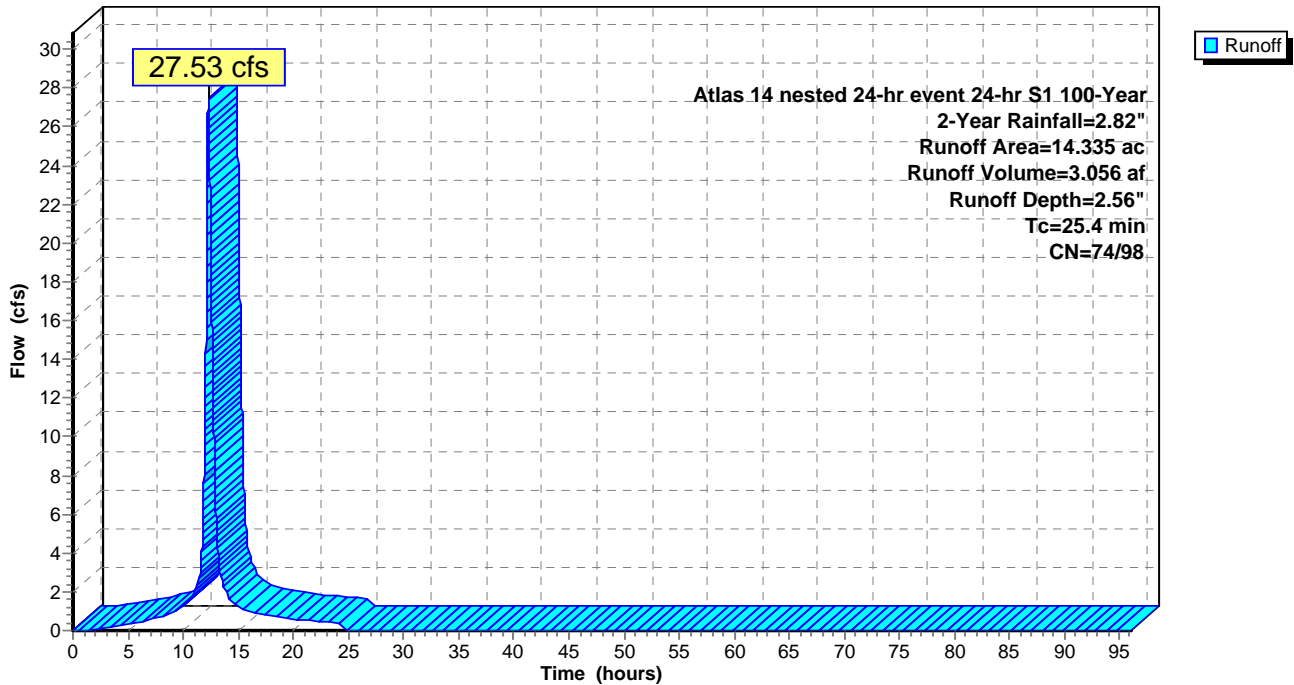
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 0.248	74	pervious
* 14.087	98	impervious
14.335	98	Weighted Average
0.248		1.73% Pervious Area
14.087		98.27% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
25.4					Direct Entry,

Subcatchment SB 26: SB 26

Hydrograph



Summary for Subcatchment SB 27: SB 27 (Thumb Road)

Runoff = 12.02 cfs @ 12.32 hrs, Volume= 1.390 af, Depth= 2.52"

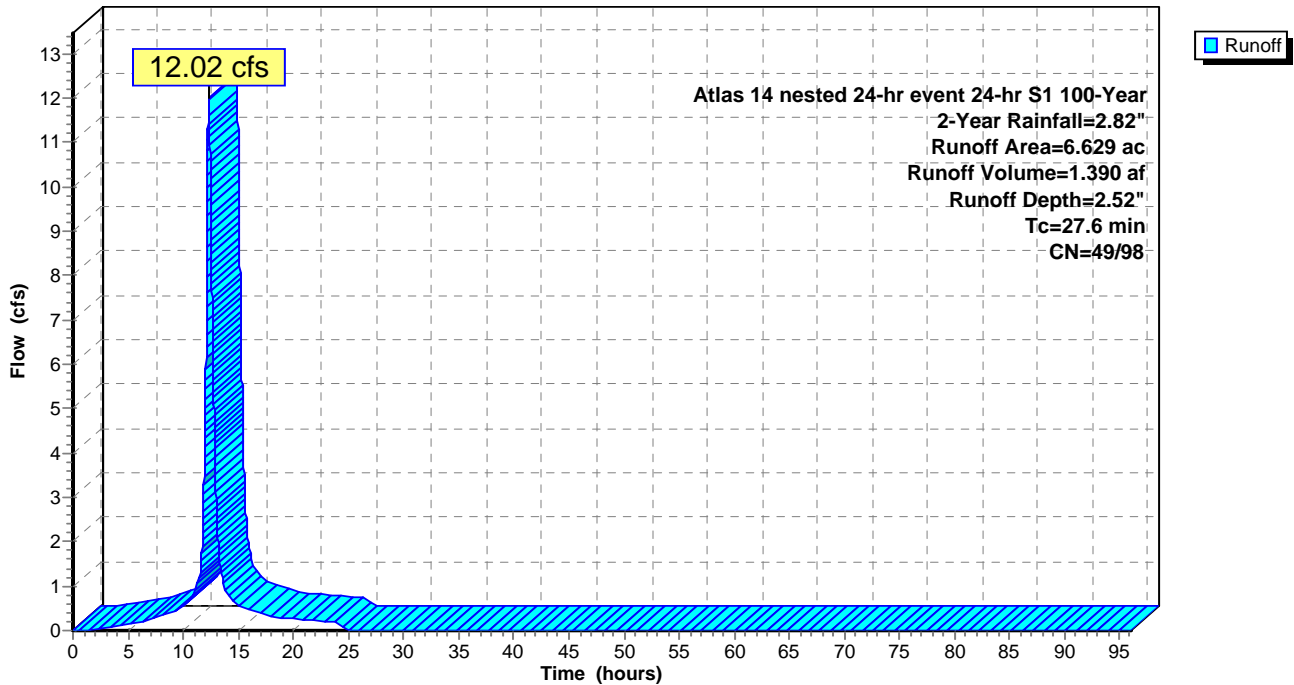
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 0.191	49	Pervious
* 6.438	98	Impervious
6.629	97	Weighted Average
0.191		2.88% Pervious Area
6.438		97.12% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.6					Direct Entry,

Subcatchment SB 27: SB 27 (Thumb Road)

Hydrograph



Summary for Subcatchment SB 28: SB 28

Runoff = 10.87 cfs @ 12.15 hrs, Volume= 0.947 af, Depth= 1.63"

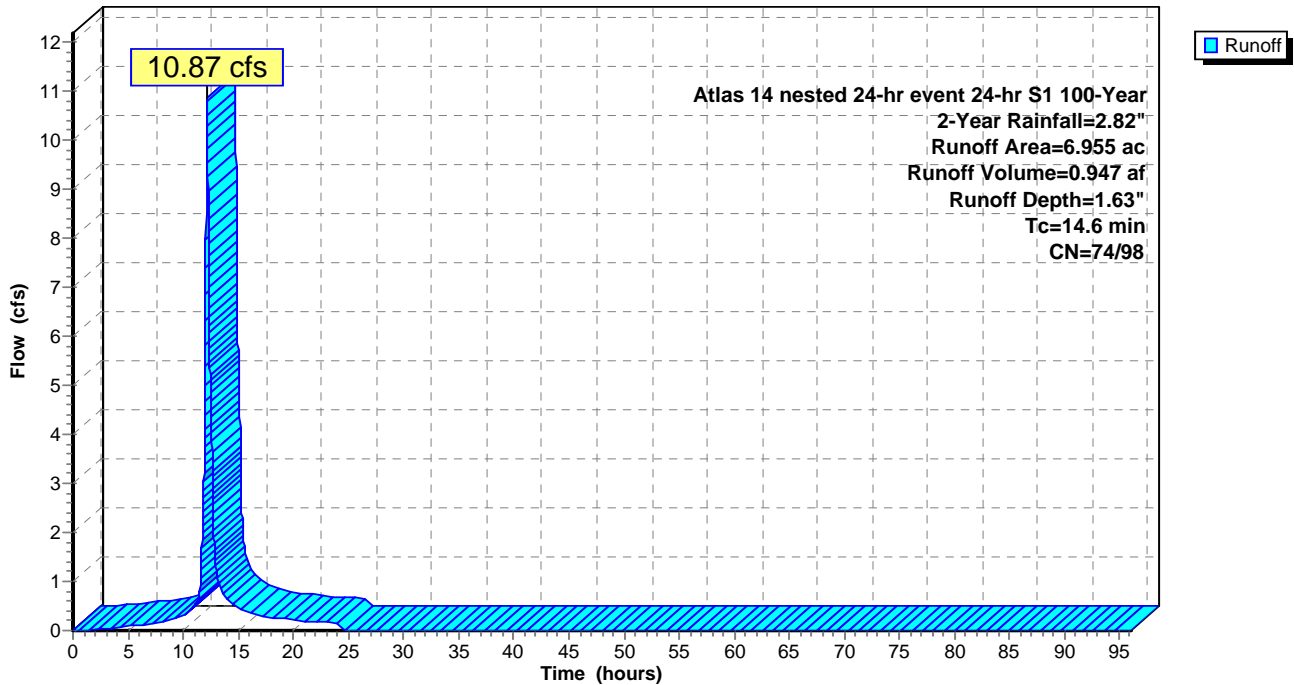
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 3.703	74	pervious
* 3.252	98	impervious
6.955	85	Weighted Average
3.703		53.24% Pervious Area
3.252		46.76% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.6					Direct Entry,

Subcatchment SB 28: SB 28

Hydrograph



Summary for Subcatchment SB 29: SB 29

Runoff = 12.67 cfs @ 12.22 hrs, Volume= 1.253 af, Depth= 1.47"

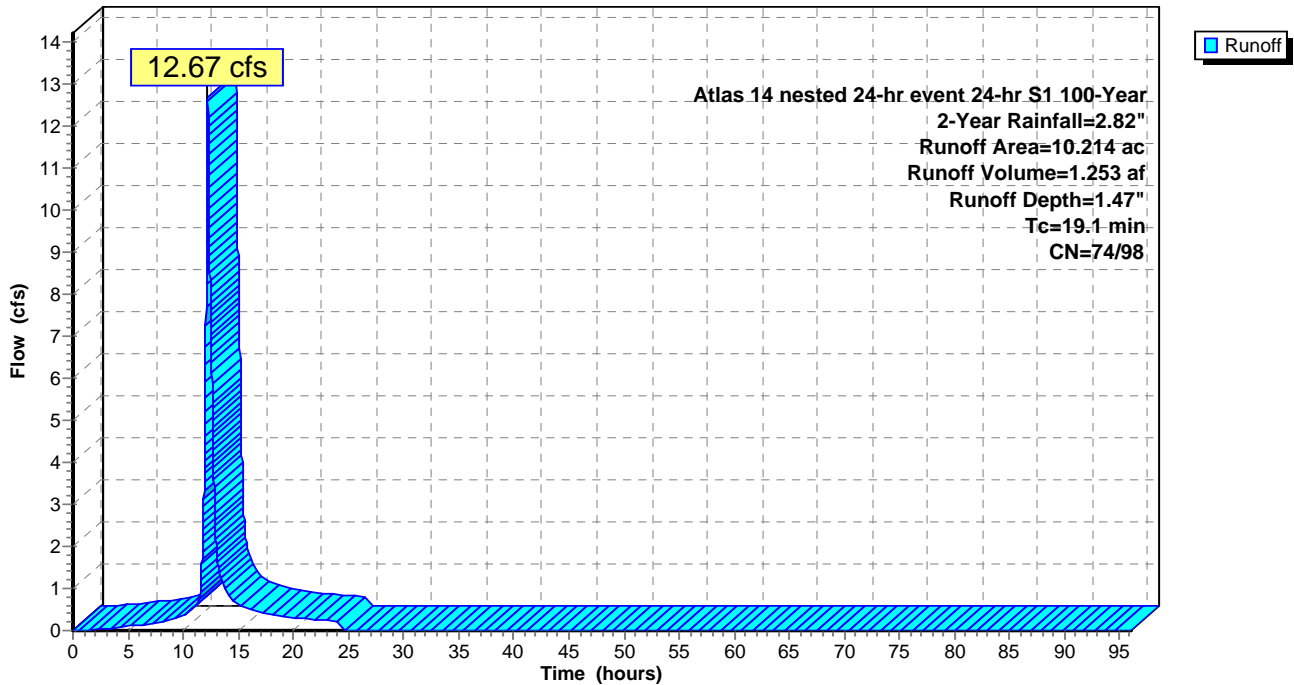
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 6.360	74	pervious
* 3.854	98	impervious
10.214	83	Weighted Average
6.360		62.27% Pervious Area
3.854		37.73% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.1					Direct Entry,

Subcatchment SB 29: SB 29

Hydrograph



Summary for Subcatchment SB 3: SB 3

Runoff = 54.30 cfs @ 12.16 hrs, Volume= 4.833 af, Depth= 1.54"

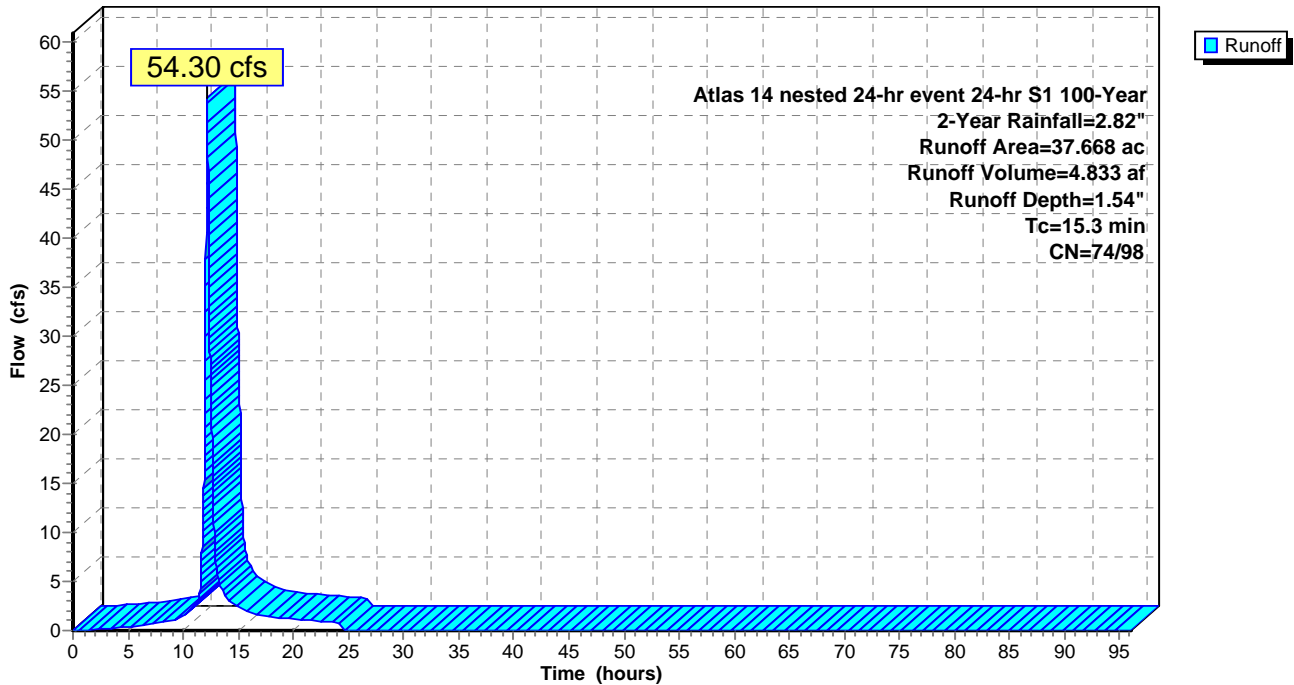
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 22.050	74	Pervious
* 15.618	98	Impervious
37.668	84	Weighted Average
22.050		58.54% Pervious Area
15.618		41.46% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.3					Direct Entry,

Subcatchment SB 3: SB 3

Hydrograph



Summary for Subcatchment SB 4: SB 4

Runoff = 0.93 cfs @ 12.04 hrs, Volume= 0.060 af, Depth= 1.19"

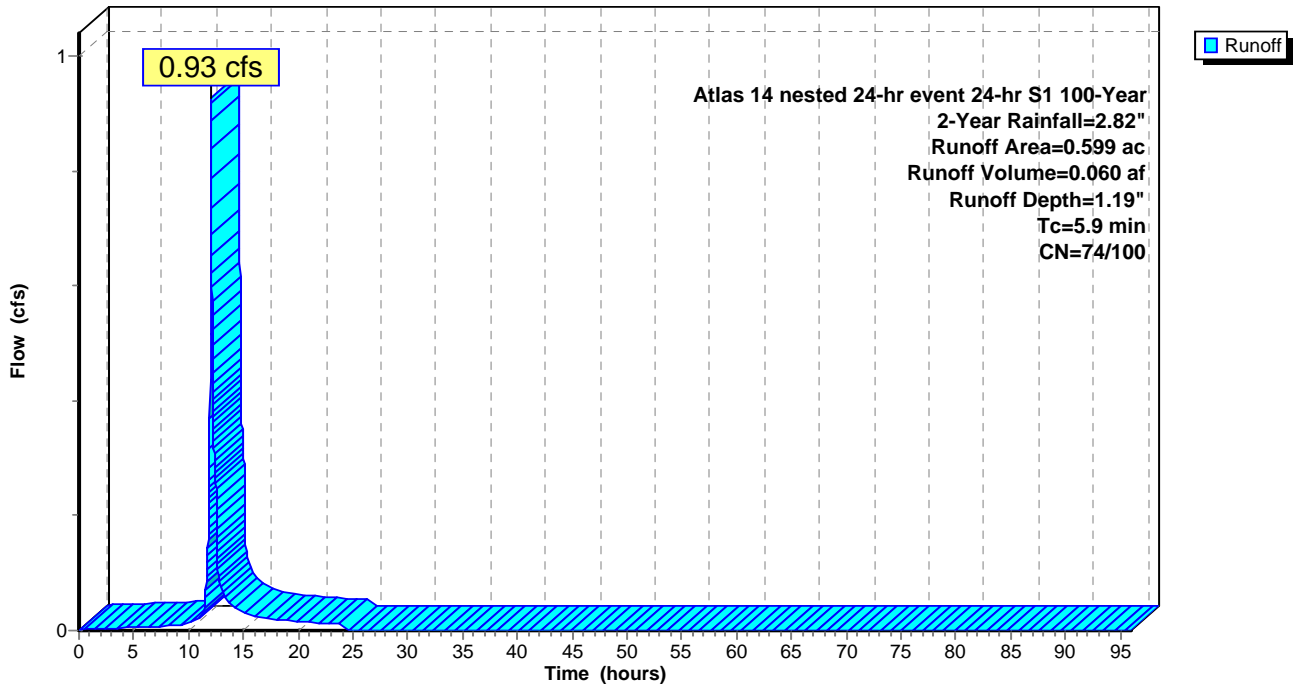
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 0.481	74	pervious
* 0.118	100	impervious
0.599	79	Weighted Average
0.481		80.30% Pervious Area
0.118		19.70% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.9					Direct Entry,

Subcatchment SB 4: SB 4

Hydrograph



Summary for Subcatchment SB 5: SB 5

Runoff = 7.70 cfs @ 12.72 hrs, Volume= 1.347 af, Depth= 2.06"

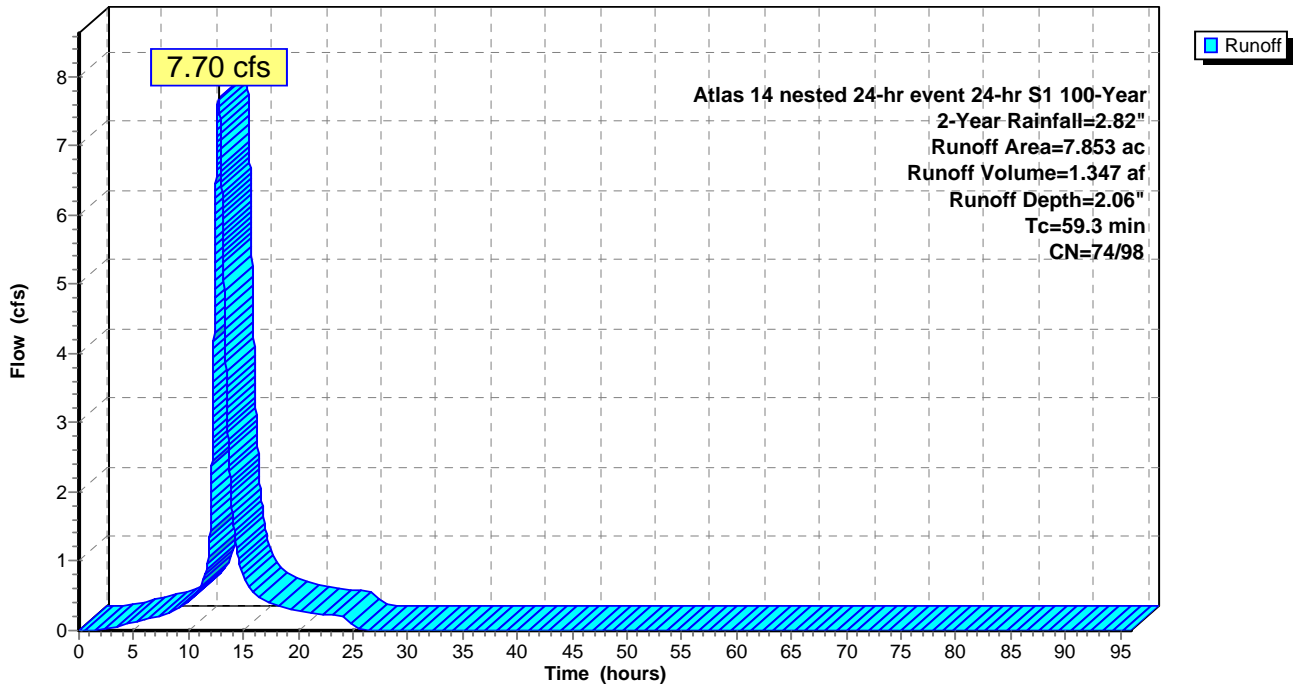
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 2.327	74	pervious
* 5.526	98	impervious
7.853	91	Weighted Average
2.327		29.63% Pervious Area
5.526		70.37% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
59.3					Direct Entry,

Subcatchment SB 5: SB 5

Hydrograph



Summary for Subcatchment SB 51: Offsite Subbasin 51

Runoff = 12.44 cfs @ 12.43 hrs, Volume= 1.805 af, Depth= 0.86"

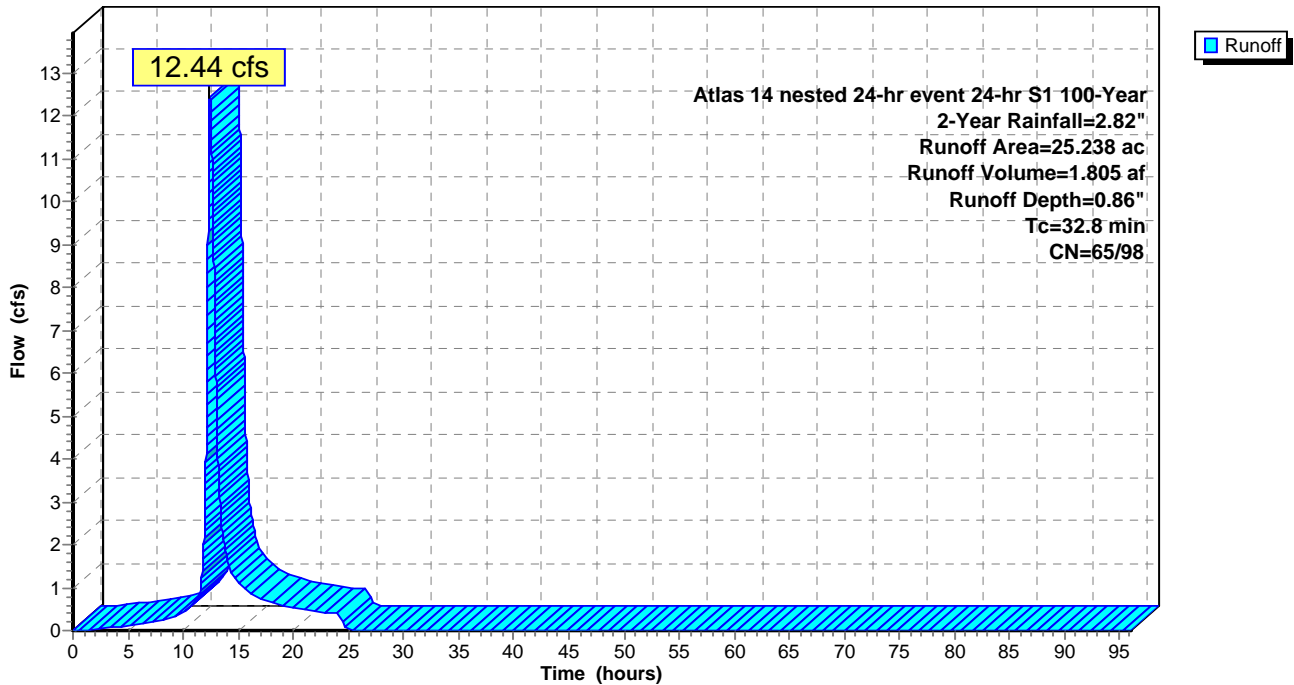
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 20.200	65	Offsite subbasin 51
* 5.038	98	
25.238	72	Weighted Average
20.200		80.04% Pervious Area
5.038		19.96% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.8					Direct Entry,

Subcatchment SB 51: Offsite Subbasin 51

Hydrograph



Summary for Subcatchment SB 6: SB 6

Runoff = 1.01 cfs @ 12.25 hrs, Volume= 0.107 af, Depth= 1.29"

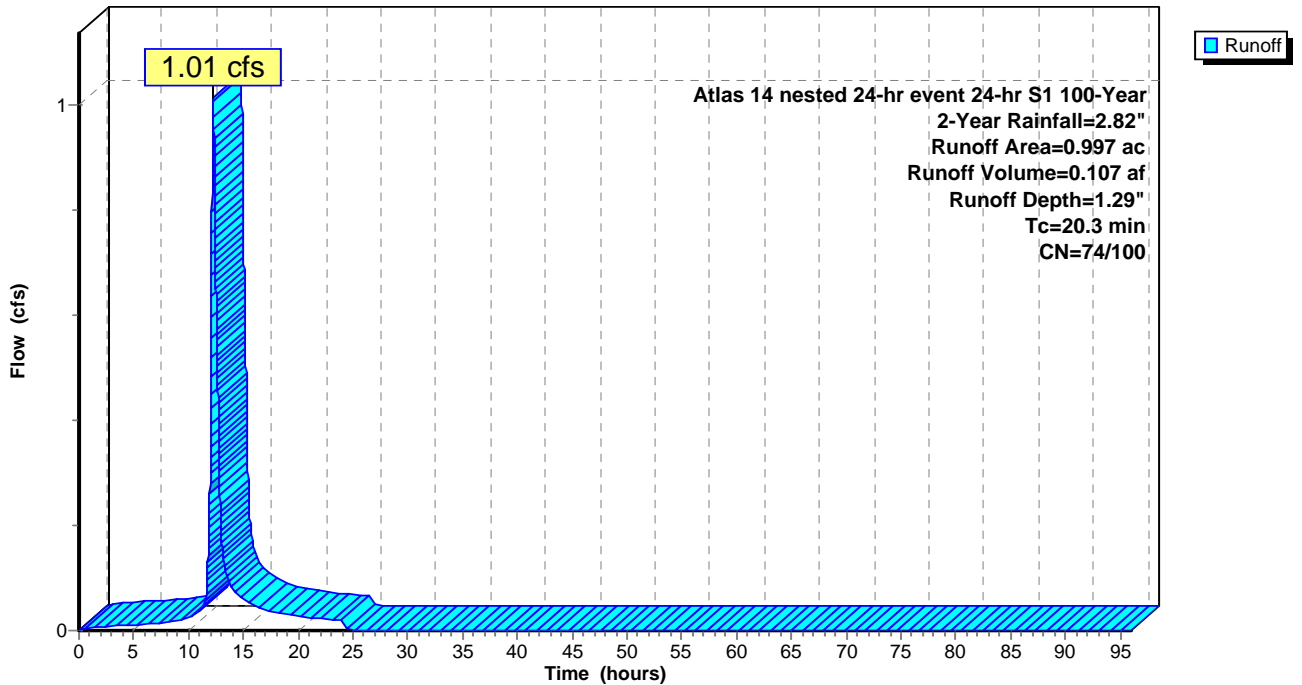
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 0.753	74	pervious
* 0.244	100	impervious
0.997	80	Weighted Average
0.753		75.53% Pervious Area
0.244		24.47% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.3					Direct Entry,

Subcatchment SB 6: SB 6

Hydrograph



Summary for Subcatchment SB 7: SB 7

Runoff = 68.54 cfs @ 12.03 hrs, Volume= 4.162 af, Depth= 2.32"

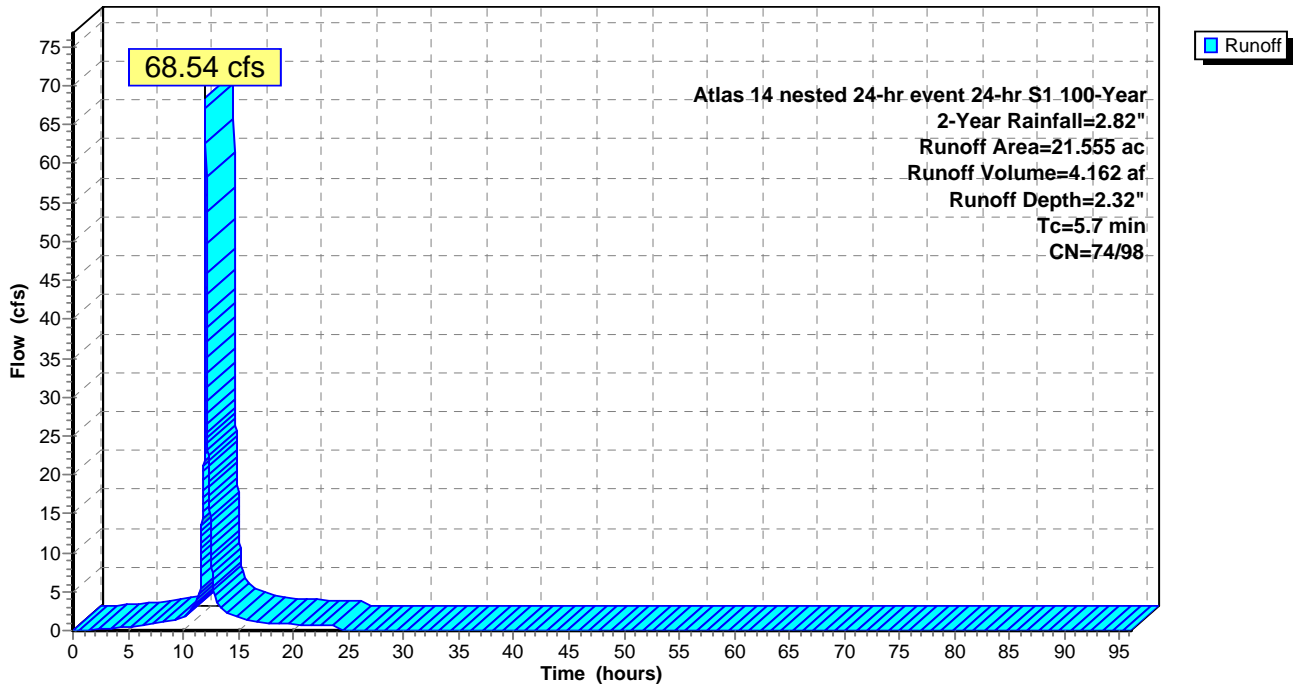
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 3.269	74	pervious
* 18.286	98	impervious
21.555	94	Weighted Average
3.269		15.17% Pervious Area
18.286		84.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.7					Direct Entry,

Subcatchment SB 7: SB 7

Hydrograph



Summary for Subcatchment SB 8: SB 8

Runoff = 21.21 cfs @ 12.62 hrs, Volume= 3.290 af, Depth= 1.33"

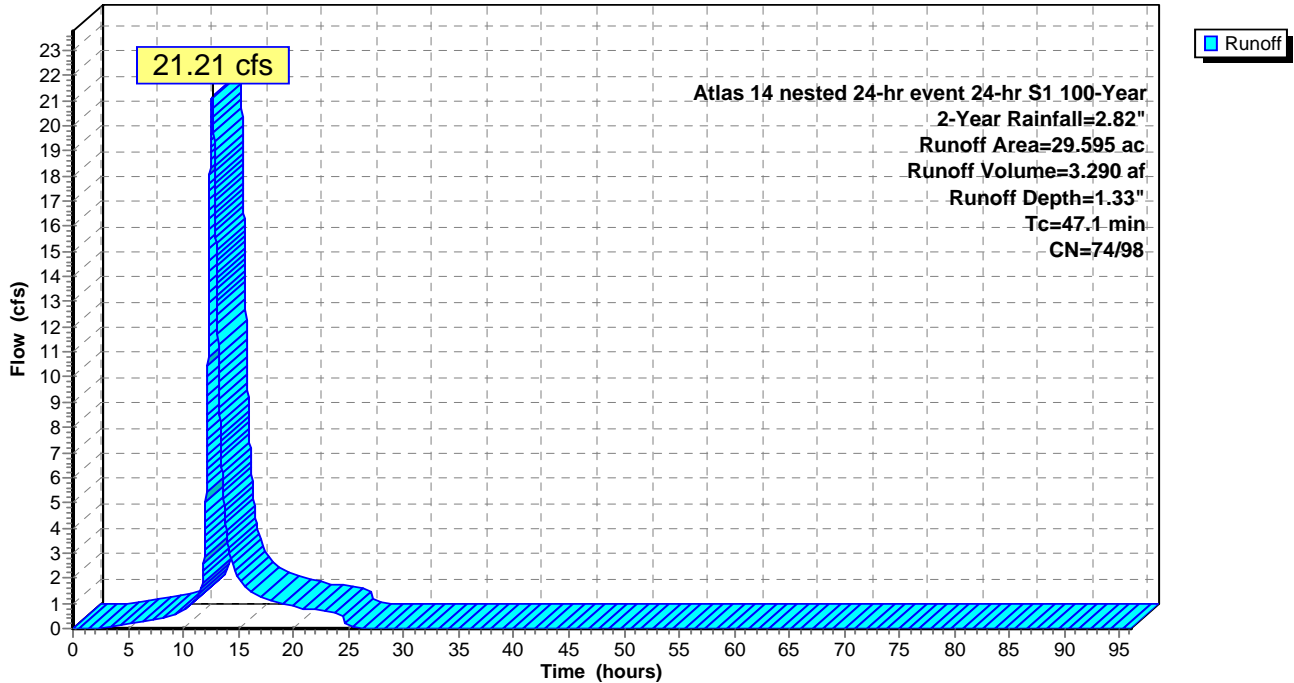
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 20.714	74	pervious
* 8.881	98	impervious
29.595	81	Weighted Average
20.714		69.99% Pervious Area
8.881		30.01% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
47.1					Direct Entry,

Subcatchment SB 8: SB 8

Hydrograph



Summary for Subcatchment SB 9: SB 9

Runoff = 24.30 cfs @ 12.37 hrs, Volume= 2.989 af, Depth= 1.39"

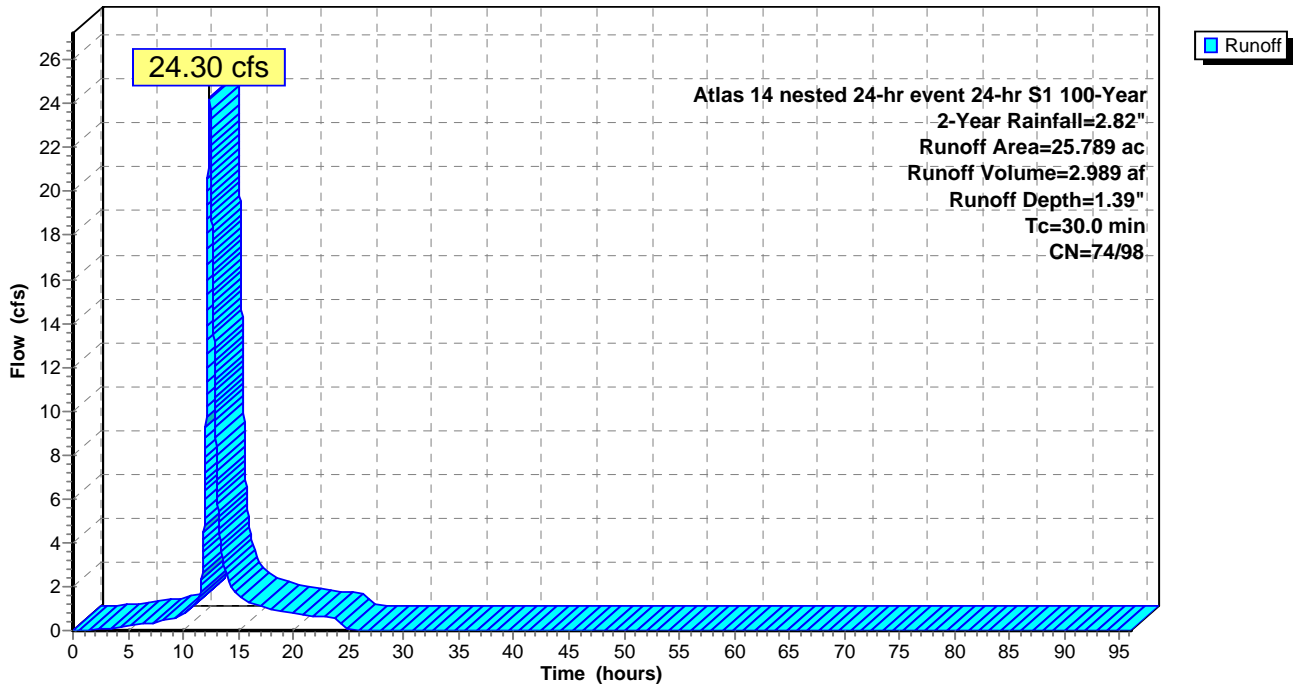
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 2-Year Rainfall=2.82"

Area (ac)	CN	Description
* 17.234	74	permiabile
* 8.555	98	impermiabile
25.789	82	Weighted Average
17.234		66.83% Pervious Area
8.555		33.17% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.0					Direct Entry,

Subcatchment SB 9: SB 9

Hydrograph



Summary for Pond 3P: P-3

Inflow Area = 133.365 ac, 58.87% Impervious, Inflow Depth = 1.60" for 2-Year event
 Inflow = 90.72 cfs @ 12.04 hrs, Volume= 17.776 af
 Outflow = 46.80 cfs @ 13.14 hrs, Volume= 17.759 af, Atten= 48%, Lag= 65.7 min
 Primary = 46.80 cfs @ 13.14 hrs, Volume= 17.759 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 914.00' Surf.Area= 1.790 ac Storage= 5.827 af
 Peak Elev= 916.51' @ 13.14 hrs Surf.Area= 2.310 ac Storage= 11.001 af (5.175 af above start)

Plug-Flow detention time= 520.8 min calculated for 11.932 af (67% of inflow)
 Center-of-Mass det. time= 182.4 min (1,098.7 - 916.3)

Volume	Invert	Avail.Storage	Storage Description
#1	909.85'	20.423 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
909.85	1.130	0.000	0.000
912.00	1.360	2.677	2.677
916.00	2.220	7.160	9.837
918.00	2.570	4.790	14.627
920.10	2.950	5.796	20.423

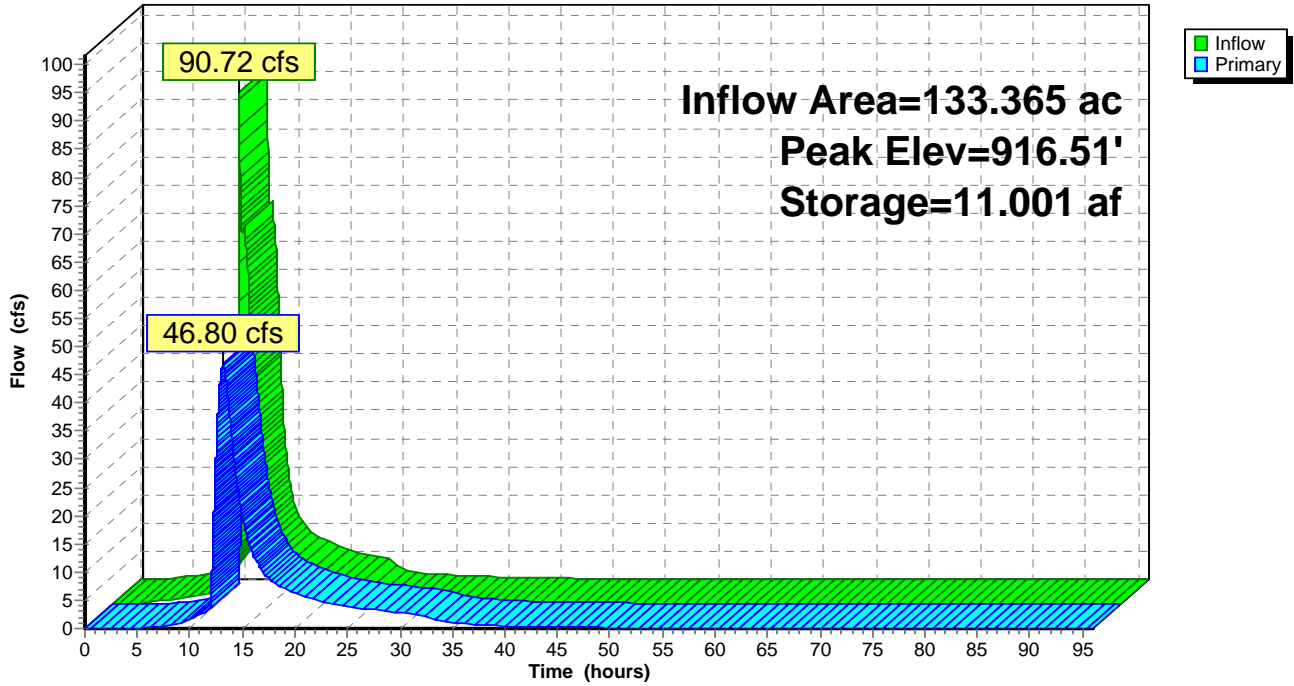
Device	Routing	Invert	Outlet Devices
#1	Primary	914.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	918.25'	10.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#3	Primary	915.00'	7.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)

Primary OutFlow Max=46.80 cfs @ 13.14 hrs HW=916.51' TW=0.00' (Dynamic Tailwater)

- 1=Orifice/Grate (Orifice Controls 6.00 cfs @ 7.63 fps)
- 2=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)
- 3=Sharp-Crested Rectangular Weir (Weir Controls 40.80 cfs @ 4.02 fps)

Pond 3P: P-3

Hydrograph



Summary for Pond 4P: P-4

Inflow Area = 7.853 ac, 70.37% Impervious, Inflow Depth = 2.06" for 2-Year event
 Inflow = 7.70 cfs @ 12.72 hrs, Volume= 1.347 af
 Outflow = 3.83 cfs @ 13.38 hrs, Volume= 1.347 af, Atten= 50%, Lag= 39.6 min
 Primary = 1.52 cfs @ 13.40 hrs, Volume= 0.485 af
 Secondary = 2.32 cfs @ 13.33 hrs, Volume= 0.862 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 915.00' Surf.Area= 0.275 ac Storage= 0.646 af
 Peak Elev= 916.23' @ 13.40 hrs Surf.Area= 0.344 ac Storage= 1.025 af (0.379 af above start)

Plug-Flow detention time= 321.5 min calculated for 0.701 af (52% of inflow)
 Center-of-Mass det. time= 59.9 min (880.2 - 820.3)

Volume	Invert	Avail.Storage	Storage Description
#1	910.90'	1.728 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
910.90	0.070	0.000	0.000
912.00	0.090	0.088	0.088
914.00	0.220	0.310	0.398
916.00	0.330	0.550	0.948
918.00	0.450	0.780	1.728

Device	Routing	Invert	Outlet Devices
#1	Primary	915.00'	6.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Secondary	915.00'	9.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	915.95'	24.0" Round RCP_Round 24" L= 50.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 915.80' / 915.95' S= -0.0030 ' /' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Primary OutFlow Max=1.52 cfs @ 13.40 hrs HW=916.23' TW=0.00' (Dynamic Tailwater)

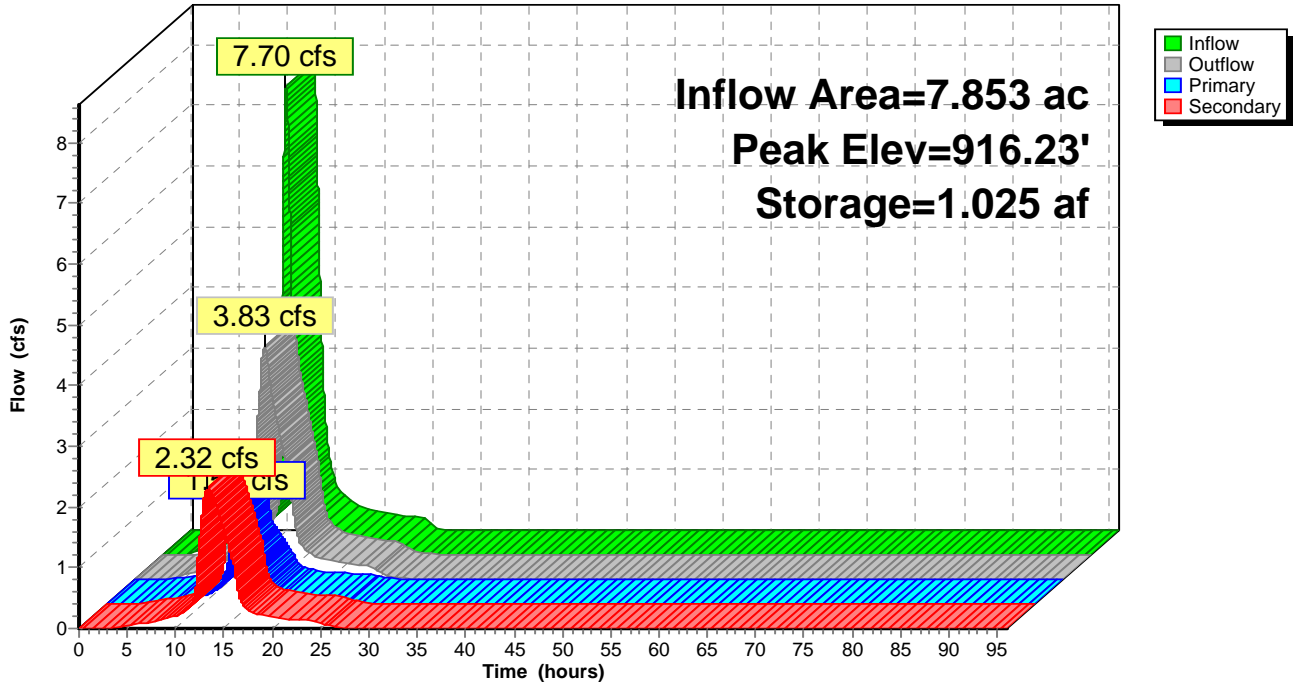
- ↑1=Orifice/Grate (Orifice Controls 1.05 cfs @ 5.34 fps)
- ↑3=RCP_Round 24" (Barrel Controls 0.47 cfs @ 1.43 fps)

Secondary OutFlow Max=2.32 cfs @ 13.33 hrs HW=916.22' TW=915.04' (Dynamic Tailwater)

- ↑2=Orifice/Grate (Orifice Controls 2.32 cfs @ 5.24 fps)

Pond 4P: P-4

Hydrograph



Summary for Pond 7P: P-7

[90] Warning: Qout>Qin may require smaller dt or Finer Routing

Inflow Area = 29.595 ac, 30.01% Impervious, Inflow Depth = 1.33" for 2-Year event
 Inflow = 21.21 cfs @ 12.62 hrs, Volume= 3.290 af
 Outflow = 21.61 cfs @ 12.68 hrs, Volume= 3.290 af, Atten= 0%, Lag= 3.6 min
 Primary = 21.61 cfs @ 12.68 hrs, Volume= 3.290 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 915.00' Surf.Area= 0.440 ac Storage= 1.062 af
 Peak Elev= 915.39' @ 12.60 hrs Surf.Area= 0.486 ac Storage= 1.241 af (0.179 af above start)

Plug-Flow detention time= 198.2 min calculated for 2.228 af (68% of inflow)
 Center-of-Mass det. time= 11.5 min (853.5 - 842.0)

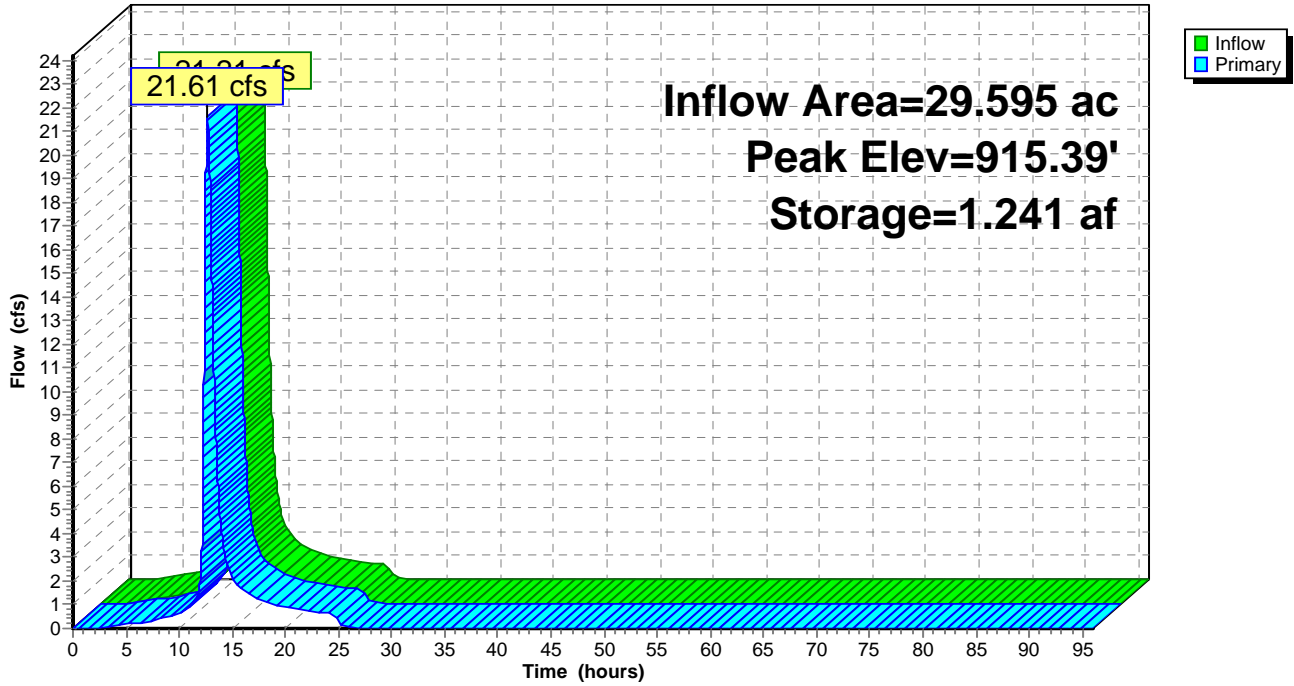
Volume	Invert	Avail.Storage	Storage Description
#1	910.95'	2.122 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
910.95	0.110	0.000	0.000
912.00	0.180	0.152	0.152
914.00	0.340	0.520	0.672
915.00	0.440	0.390	1.062
916.00	0.560	0.500	1.562
917.00	0.560	0.560	2.122

Device	Routing	Invert	Outlet Devices
#1	Primary	915.00'	75.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=21.88 cfs @ 12.68 hrs HW=915.38' TW=915.34' (Dynamic Tailwater)
 ↳ **1=Broad-Crested Rectangular Weir** (Weir Controls 21.88 cfs @ 0.76 fps)

Pond 7P: P-7

Hydrograph



Summary for Pond 9P: P-9

Inflow Area = 55.384 ac, 31.48% Impervious, Inflow Depth = 1.36" for 2-Year event
 Inflow = 41.03 cfs @ 12.51 hrs, Volume= 6.280 af
 Outflow = 40.96 cfs @ 12.53 hrs, Volume= 6.280 af, Atten= 0%, Lag= 1.3 min
 Primary = 40.96 cfs @ 12.53 hrs, Volume= 6.280 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 915.00' Surf.Area= 0.210 ac Storage= 0.353 af
 Peak Elev= 915.35' @ 12.53 hrs Surf.Area= 0.280 ac Storage= 0.439 af (0.086 af above start)

Plug-Flow detention time= 58.4 min calculated for 5.927 af (94% of inflow)
 Center-of-Mass det. time= 2.7 min (841.3 - 838.6)

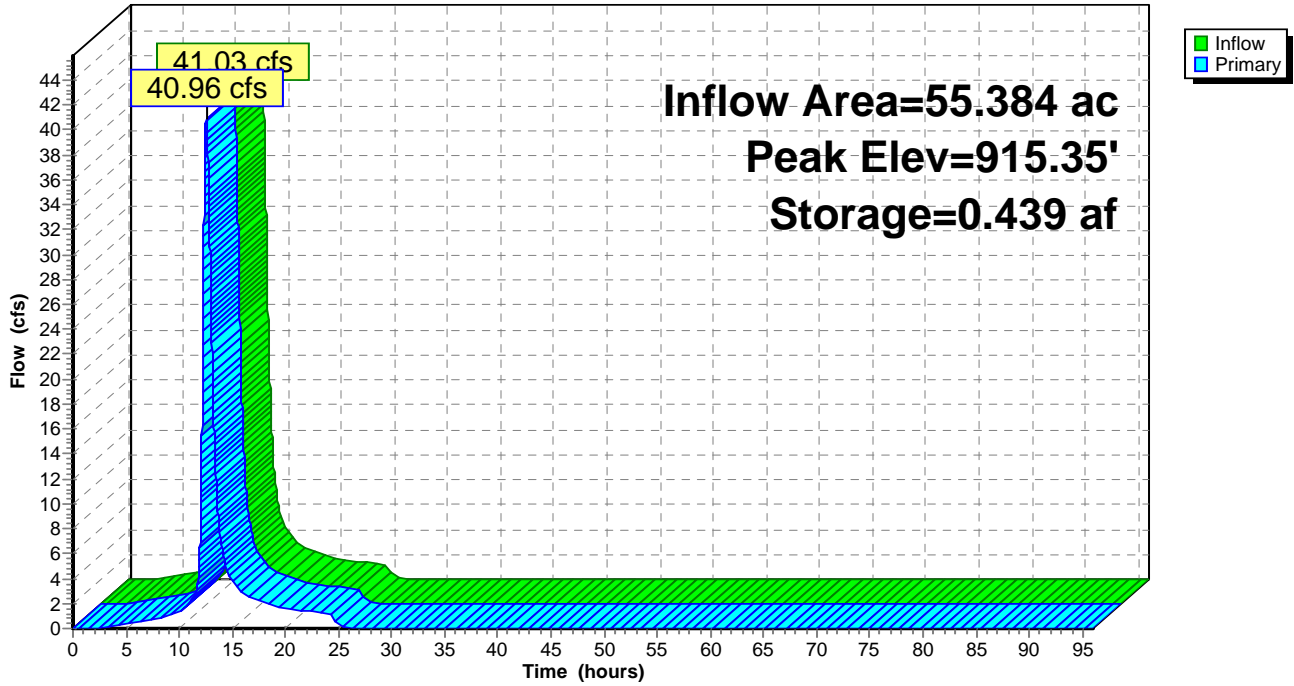
Volume	Invert	Avail.Storage	Storage Description
#1	910.50'	1.673 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
910.50	0.020	0.000	0.000
912.00	0.050	0.052	0.052
913.00	0.070	0.060	0.112
914.00	0.100	0.085	0.198
915.00	0.210	0.155	0.353
916.00	0.410	0.310	0.662
918.00	0.600	1.010	1.673

Device	Routing	Invert	Outlet Devices
#1	Primary	915.00'	80.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=40.96 cfs @ 12.53 hrs HW=915.35' TW=910.20' (Dynamic Tailwater)
 ↑1=Broad-Crested Rectangular Weir (Weir Controls 40.96 cfs @ 1.46 fps)

Pond 9P: P-9

Hydrograph



Summary for Pond 10P: P-10

[95] Warning: Outlet Device #1 rise exceeded

Inflow Area = 66.448 ac, 29.37% Impervious, Inflow Depth = 1.01" for 2-Year event
 Inflow = 14.23 cfs @ 13.20 hrs, Volume= 5.606 af
 Outflow = 13.43 cfs @ 13.49 hrs, Volume= 5.605 af, Atten= 6%, Lag= 17.0 min
 Primary = 11.01 cfs @ 13.49 hrs, Volume= 5.505 af
 Secondary = 2.42 cfs @ 13.49 hrs, Volume= 0.100 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 896.00' Surf.Area= 0.290 ac Storage= 0.700 af
 Peak Elev= 897.48' @ 13.49 hrs Surf.Area= 0.351 ac Storage= 1.172 af (0.472 af above start)

Plug-Flow detention time= 174.3 min calculated for 4.905 af (87% of inflow)
 Center-of-Mass det. time= 38.1 min (1,034.5 - 996.4)

Volume	Invert	Avail.Storage	Storage Description
#1	892.00'	1.760 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
892.00	0.120	0.000	0.000
893.00	0.140	0.130	0.130
895.00	0.190	0.330	0.460
896.00	0.290	0.240	0.700
897.00	0.330	0.310	1.010
899.00	0.420	0.750	1.760

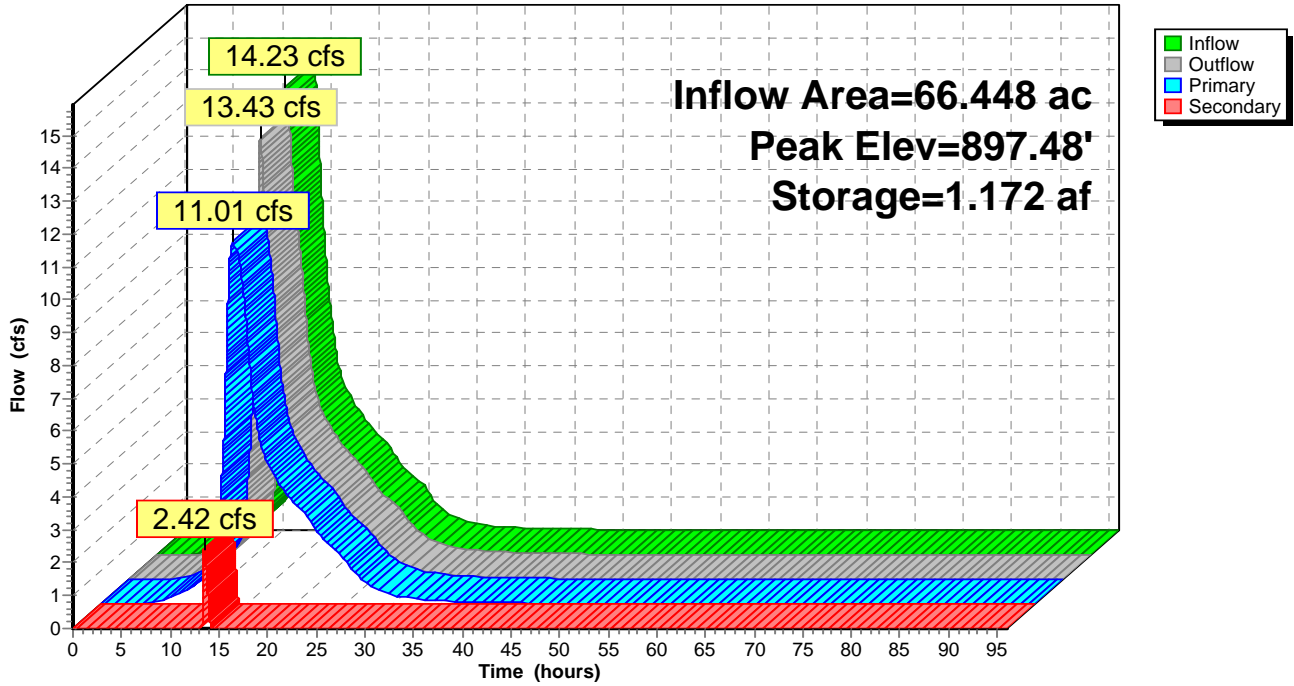
Device	Routing	Invert	Outlet Devices
#1	Primary	896.00'	2.5' long x 1.00' rise Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Secondary	897.40'	50.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=11.01 cfs @ 13.49 hrs HW=897.48' TW=893.73' (Dynamic Tailwater)
 ↖1=Sharp-Crested Rectangular Weir (Orifice Controls 11.01 cfs @ 4.79 fps)

Secondary OutFlow Max=2.42 cfs @ 13.49 hrs HW=897.48' TW=893.73' (Dynamic Tailwater)
 ↖2=Broad-Crested Rectangular Weir (Weir Controls 2.42 cfs @ 0.64 fps)

Pond 10P: P-10

Hydrograph



Summary for Pond 11P: P-11

Inflow Area = 58.677 ac, 31.52% Impervious, Inflow Depth = 1.37" for 2-Year event
 Inflow = 42.67 cfs @ 12.52 hrs, Volume= 6.677 af
 Outflow = 17.07 cfs @ 13.25 hrs, Volume= 6.675 af, Atten= 60%, Lag= 43.7 min
 Primary = 13.46 cfs @ 13.25 hrs, Volume= 4.943 af
 Secondary = 3.61 cfs @ 13.25 hrs, Volume= 1.732 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 909.00' Surf.Area= 1.210 ac Storage= 3.640 af
 Peak Elev= 910.94' @ 13.25 hrs Surf.Area= 1.433 ac Storage= 6.202 af (2.562 af above start)

Plug-Flow detention time= 498.3 min calculated for 3.035 af (45% of inflow)
 Center-of-Mass det. time= 141.9 min (979.8 - 837.9)

Volume	Invert	Avail.Storage	Storage Description
#1	905.00'	9.405 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
905.00	0.760	0.000	0.000
906.00	0.820	0.790	0.790
908.00	0.950	1.770	2.560
909.00	1.210	1.080	3.640
910.00	1.320	1.265	4.905
912.00	1.560	2.880	7.785
913.00	1.680	1.620	9.405

Device	Routing	Invert	Outlet Devices
#1	Primary	909.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	910.00'	24.0" Round RCP_Round 24" L= 200.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 910.00' / 909.00' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Primary	910.00'	24.0" Round RCP_Round 24" L= 200.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 910.00' / 909.00' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#4	Primary	912.00'	60.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88
#5	Secondary	909.00'	12.0" Round RCP_Round 12" L= 150.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 909.00' / 908.00' S= 0.0067 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=13.46 cfs @ 13.25 hrs HW=910.94' TW=897.39' (Dynamic Tailwater)

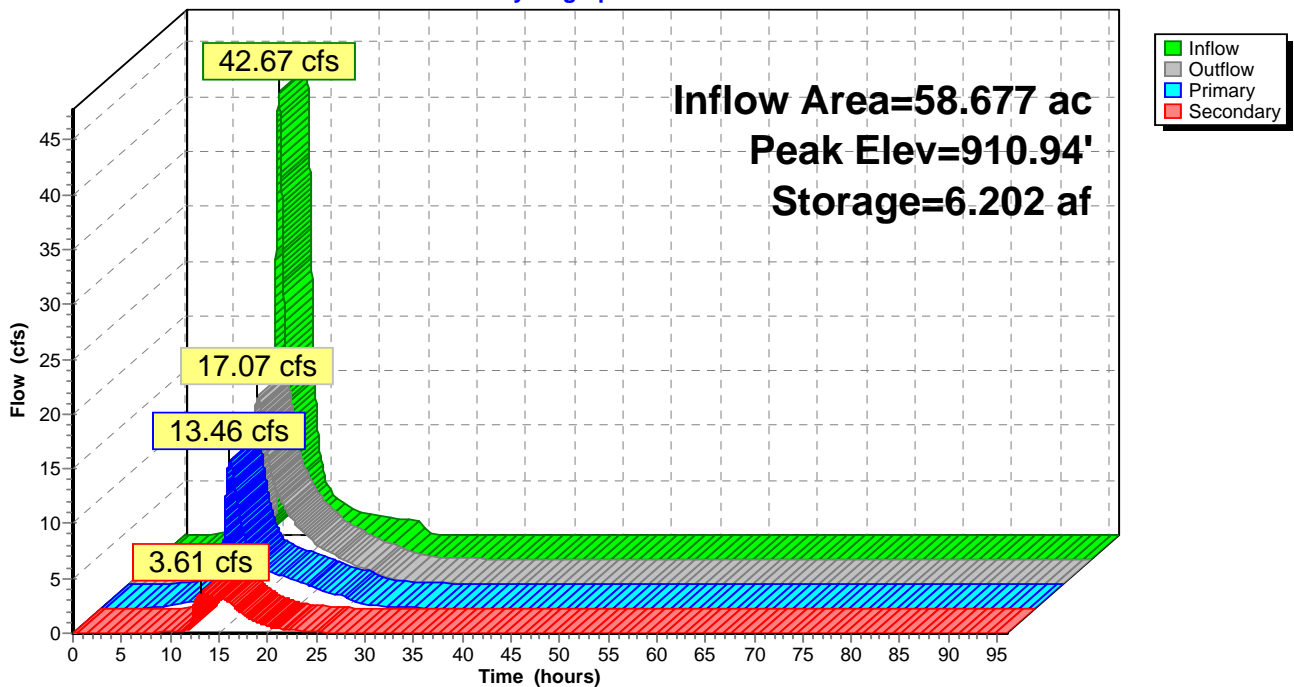
- 1=Orifice/Grate (Orifice Controls 5.27 cfs @ 6.71 fps)
- 2=RCP_Round 24" (Barrel Controls 4.09 cfs @ 4.13 fps)
- 3=RCP_Round 24" (Barrel Controls 4.09 cfs @ 4.13 fps)
- 4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Secondary OutFlow Max=3.61 cfs @ 13.25 hrs HW=910.94' TW=908.55' (Dynamic Tailwater)

- 5=RCP_Round 12" (Barrel Controls 3.61 cfs @ 4.60 fps)

Pond 11P: P-11

Hydrograph



Summary for Pond 12P: P-12

Inflow Area = 79.658 ac, 31.13% Impervious, Inflow Depth > 1.36" for 2-Year event
 Inflow = 25.40 cfs @ 12.02 hrs, Volume= 9.002 af
 Outflow = 12.77 cfs @ 14.53 hrs, Volume= 8.994 af, Atten= 50%, Lag= 150.3 min
 Primary = 12.77 cfs @ 14.53 hrs, Volume= 8.994 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 893.00' Surf.Area= 1.640 ac Storage= 5.075 af
 Peak Elev= 893.86' @ 14.53 hrs Surf.Area= 1.752 ac Storage= 6.533 af (1.458 af above start)

Plug-Flow detention time= 695.3 min calculated for 3.919 af (44% of inflow)
 Center-of-Mass det. time= 112.9 min (1,150.3 - 1,037.3)

Volume	Invert	Avail.Storage	Storage Description
#1	889.00'	10.590 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
889.00	1.070	0.000	0.000
890.00	1.150	1.110	1.110
892.00	1.330	2.480	3.590
893.00	1.640	1.485	5.075
894.00	1.770	1.705	6.780
896.00	2.040	3.810	10.590

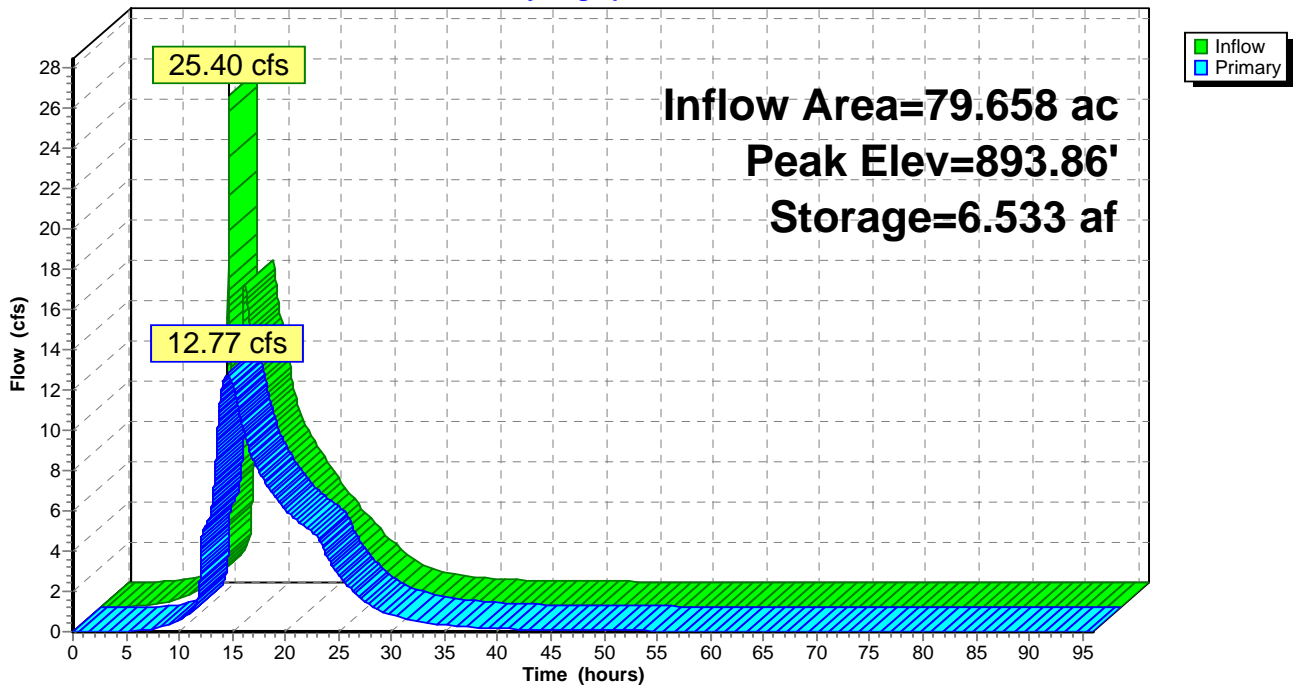
Device	Routing	Invert	Outlet Devices
#1	Primary	893.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	893.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf
#4	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf
#5	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf
#6	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf

Primary OutFlow Max=12.77 cfs @ 14.53 hrs HW=893.86' TW=883.34' (Dynamic Tailwater)

- 1=Orifice/Grate (Orifice Controls 3.51 cfs @ 4.47 fps)
- 2=Orifice/Grate (Orifice Controls 3.51 cfs @ 4.47 fps)
- 3=RCP_Arch 44x27 (Barrel Controls 1.44 cfs @ 2.43 fps)
- 4=RCP_Arch 44x27 (Barrel Controls 1.44 cfs @ 2.43 fps)
- 5=RCP_Arch 44x27 (Barrel Controls 1.44 cfs @ 2.43 fps)
- 6=RCP_Arch 44x27 (Barrel Controls 1.44 cfs @ 2.43 fps)

Pond 12P: P-12

Hydrograph



Summary for Pond 13P: P-13

Inflow Area = 237.893 ac, 51.59% Impervious, Inflow Depth = 1.55" for 2-Year event
 Inflow = 197.71 cfs @ 12.35 hrs, Volume= 30.805 af
 Outflow = 175.72 cfs @ 12.52 hrs, Volume= 30.802 af, Atten= 11%, Lag= 9.9 min
 Primary = 165.33 cfs @ 12.52 hrs, Volume= 29.381 af
 Secondary = 10.39 cfs @ 12.52 hrs, Volume= 1.421 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 883.00' Surf.Area= 1.870 ac Storage= 4.265 af
 Peak Elev= 884.08' @ 12.52 hrs Surf.Area= 2.250 ac Storage= 6.493 af (2.228 af above start)

Plug-Flow detention time= 160.9 min calculated for 26.537 af (86% of inflow)
 Center-of-Mass det. time= 18.9 min (937.7 - 918.8)

Volume	Invert	Avail.Storage	Storage Description
#1	878.00'	11.490 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
878.00	0.000	0.000	0.000
879.00	0.630	0.315	0.315
880.00	0.730	0.680	0.995
882.00	1.070	1.800	2.795
883.00	1.870	1.470	4.265
884.00	2.220	2.045	6.310
886.00	2.960	5.180	11.490

Device	Routing	Invert	Outlet Devices
#1	Primary	883.00'	55.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88
#2	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#3	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#4	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#5	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#6	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200

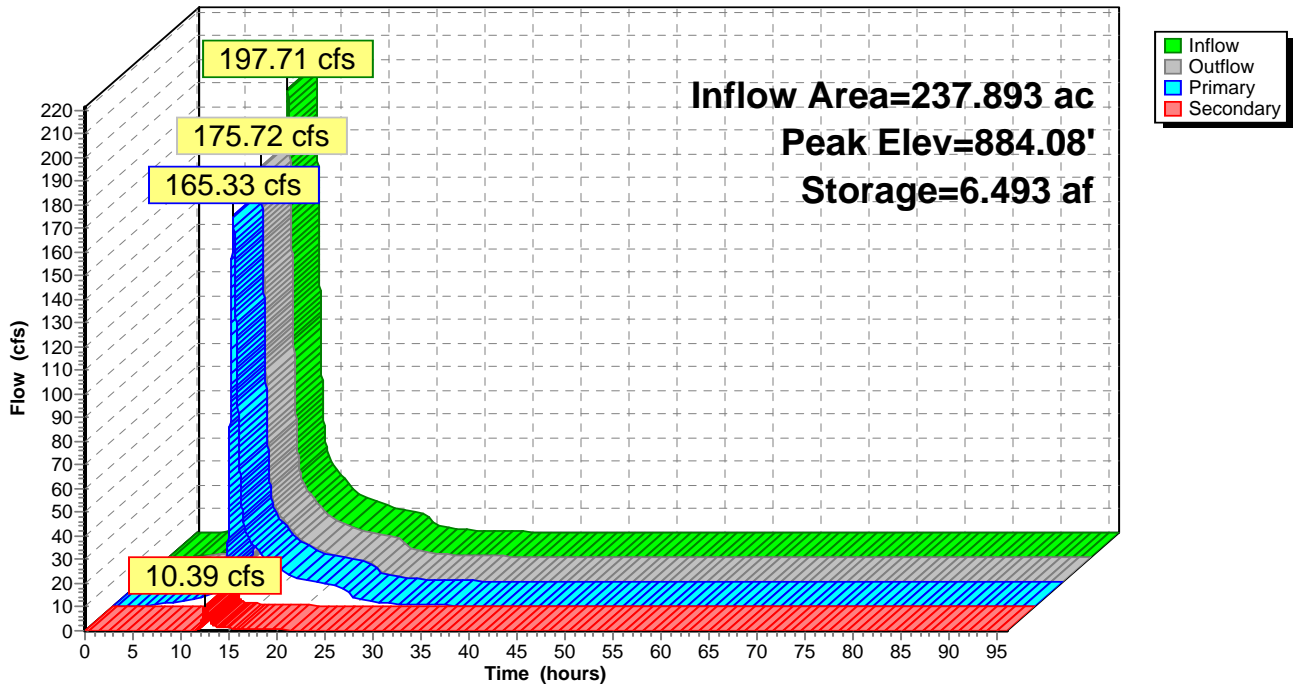
Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 1' Cc= 0.900
 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=165.31 cfs @ 12.52 hrs HW=884.08' TW=0.00' (Dynamic Tailwater)
 1=Broad-Crested Rectangular Weir (Weir Controls 165.31 cfs @ 2.78 fps)

Secondary OutFlow Max=10.39 cfs @ 12.52 hrs HW=884.08' TW=882.93' (Dynamic Tailwater)
 2=RCP_Round 12" (Barrel Controls 2.08 cfs @ 3.04 fps)
 3=RCP_Round 12" (Barrel Controls 2.08 cfs @ 3.04 fps)
 4=RCP_Round 12" (Barrel Controls 2.08 cfs @ 3.04 fps)
 5=RCP_Round 12" (Barrel Controls 2.08 cfs @ 3.04 fps)
 6=RCP_Round 12" (Barrel Controls 2.08 cfs @ 3.04 fps)

Pond 13P: P-13

Hydrograph



Summary for Pond 14P: P-14

Inflow Area = 21.198 ac, 39.93% Impervious, Inflow Depth = 1.51" for 2-Year event
 Inflow = 23.92 cfs @ 12.30 hrs, Volume= 2.671 af
 Outflow = 3.68 cfs @ 13.21 hrs, Volume= 2.669 af, Atten= 85%, Lag= 54.4 min
 Primary = 3.68 cfs @ 13.21 hrs, Volume= 2.669 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 892.00' Surf.Area= 1.380 ac Storage= 4.490 af
 Peak Elev= 892.95' @ 13.21 hrs Surf.Area= 1.465 ac Storage= 5.837 af (1.347 af above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= 239.6 min (1,049.8 - 810.2)

Volume	Invert	Avail.Storage	Storage Description
#1	888.00'	9.910 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
888.00	0.950	0.000	0.000
890.00	1.080	2.030	2.030
892.00	1.380	2.460	4.490
893.00	1.470	1.425	5.915
894.00	1.570	1.520	7.435
895.50	1.730	2.475	9.910

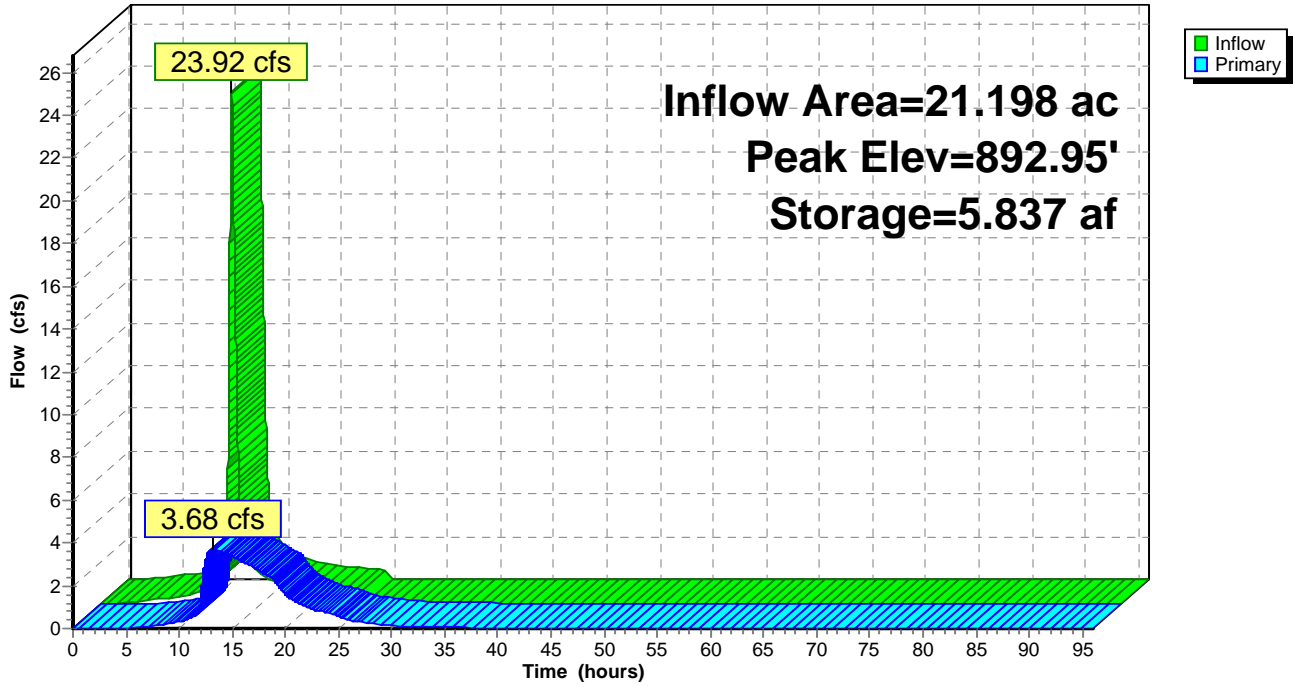
Device	Routing	Invert	Outlet Devices
#1	Primary	892.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	893.00'	18.0" Round RCP_Round 18" L= 50.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 893.00' / 892.75' S= 0.0050 1' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=3.68 cfs @ 13.21 hrs HW=892.95' (Free Discharge)

- ↑ 1=Orifice/Grate (Orifice Controls 3.68 cfs @ 4.69 fps)
- └ 2=RCP_Round 18" (Controls 0.00 cfs)

Pond 14P: P-14

Hydrograph



Summary for Pond 23P: Thumb Infiltration (Thumb TP load only)

Inflow Area = 48.540 ac, 84.23% Impervious, Inflow Depth = 2.19" for 2-Year event
 Inflow = 62.51 cfs @ 12.44 hrs, Volume= 8.852 af
 Outflow = 61.16 cfs @ 12.54 hrs, Volume= 5.112 af, Atten= 2%, Lag= 5.6 min
 Primary = 61.16 cfs @ 12.54 hrs, Volume= 5.112 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 903.81' @ 12.54 hrs Surf.Area= 1.000 ac Storage= 3.810 af

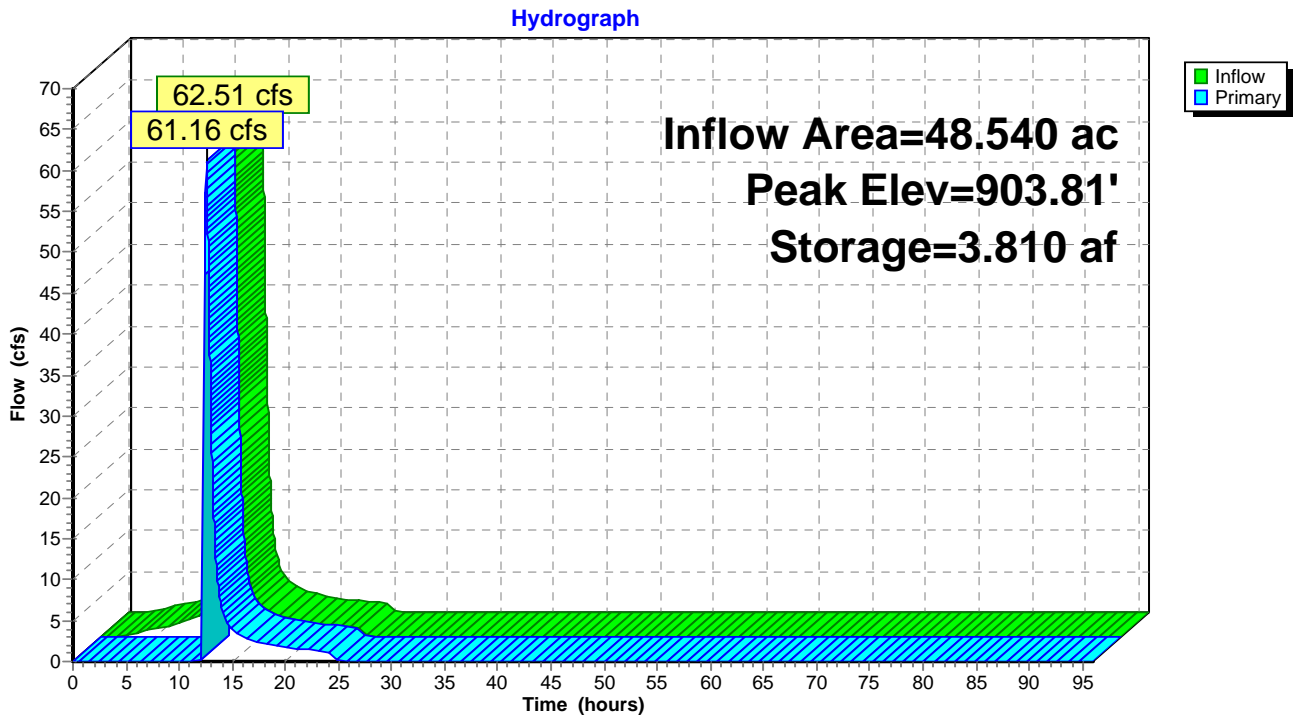
Plug-Flow detention time= 214.4 min calculated for 5.112 af (58% of inflow)
 Center-of-Mass det. time= 103.7 min (893.9 - 790.2)

Volume	Invert	Avail.Storage	Storage Description
#1	900.00'	5.000 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
900.00	1.000	0.000	0.000
901.00	1.000	1.000	1.000
902.00	1.000	1.000	2.000
903.00	1.000	1.000	3.000
904.00	1.000	1.000	4.000
905.00	1.000	1.000	5.000

Device	Routing	Invert	Outlet Devices
#1	Primary	903.74'	1,000.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) 5.0' Crest Height

Primary OutFlow Max=61.12 cfs @ 12.54 hrs HW=903.81' (Free Discharge)
 ↑1=Sharp-Crested Rectangular Weir (Weir Controls 61.12 cfs @ 0.87 fps)

Pond 23P: Thumb Infiltration (Thumb TP load only)



Summary for Pond 31P: SB 18 Infiltration

Inflow Area = 52.908 ac, 84.55% Impervious, Inflow Depth = 2.31" for 2-Year event
 Inflow = 79.77 cfs @ 12.40 hrs, Volume= 10.194 af
 Outflow = 79.67 cfs @ 12.41 hrs, Volume= 6.874 af, Atten= 0%, Lag= 0.5 min
 Primary = 79.67 cfs @ 12.41 hrs, Volume= 6.874 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 903.40' @ 12.41 hrs Surf.Area= 1.000 ac Storage= 3.403 af

Plug-Flow detention time= 178.8 min calculated for 6.873 af (67% of inflow)
 Center-of-Mass det. time= 81.2 min (871.0 - 789.8)

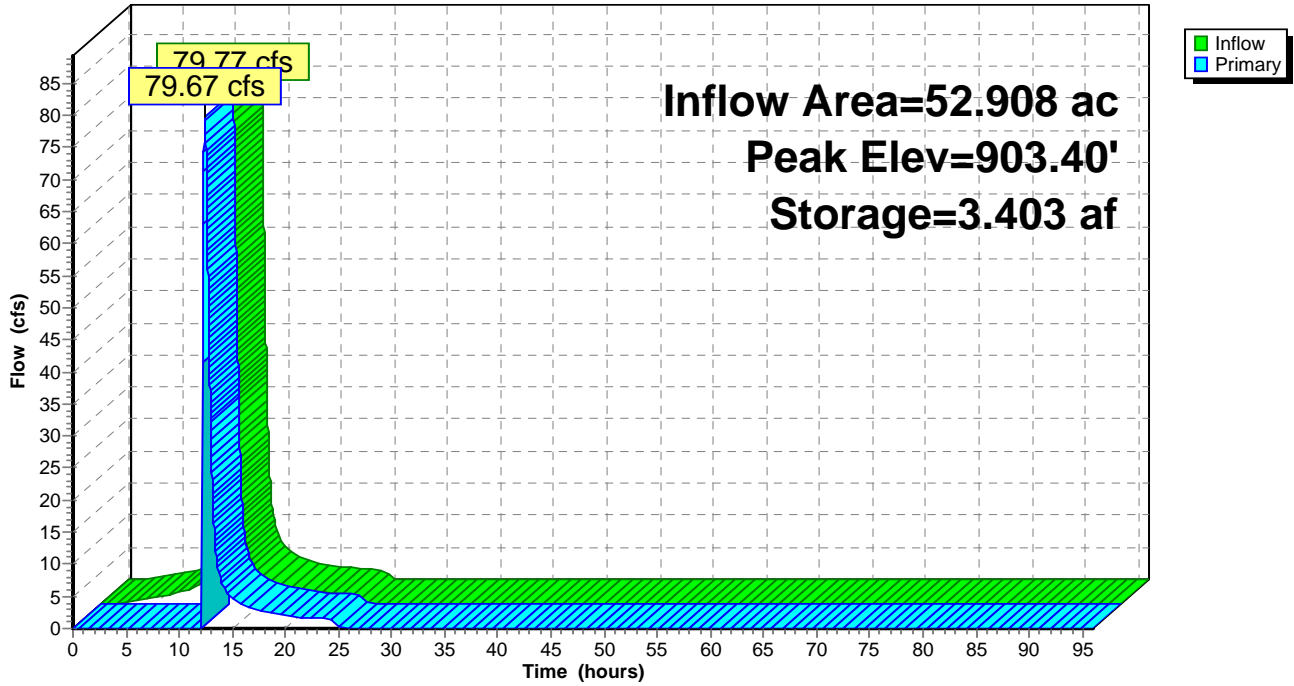
Volume	Invert	Avail.Storage	Storage Description
#1	900.00'	5.000 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
900.00	1.000	0.000	0.000
901.00	1.000	1.000	1.000
902.00	1.000	1.000	2.000
903.00	1.000	1.000	3.000
904.00	1.000	1.000	4.000
905.00	1.000	1.000	5.000

Device	Routing	Invert	Outlet Devices
#1	Primary	903.32'	1,000.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) 0.8' Crest Height

Primary OutFlow Max=79.65 cfs @ 12.41 hrs HW=903.40' TW=887.32' (Dynamic Tailwater)
 ↑1=Sharp-Crested Rectangular Weir (Weir Controls 79.65 cfs @ 0.96 fps)

Pond 31P: SB 18 Infiltration

Hydrograph



Summary for Pond 36P: Culverts passing flow beneath Spine Road

[90] Warning: Qout>Qin may require smaller dt or Finer Routing

Inflow Area = 52.908 ac, 84.55% Impervious, Inflow Depth = 1.56" for 2-Year event
 Inflow = 79.67 cfs @ 12.41 hrs, Volume= 6.874 af
 Outflow = 79.68 cfs @ 12.40 hrs, Volume= 6.874 af, Atten= 0%, Lag= 0.0 min
 Primary = 79.68 cfs @ 12.40 hrs, Volume= 6.874 af
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 887.32' @ 12.40 hrs Surf.Area= 0.002 ac Storage= 0.000 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 0.0 min (871.0 - 871.0)

Volume	Invert	Avail.Storage	Storage Description
#1	887.00'	0.025 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
887.00	0.000	0.000	0.000
887.50	0.003	0.001	0.001
890.50	0.006	0.014	0.014
892.00	0.009	0.011	0.025

Device	Routing	Invert	Outlet Devices
#1	Primary	887.00'	Special & User-Defined Head (feet) 0.00 0.10 0.20 0.30 0.40 0.50 5.00 Disch. (cfs) 0.000 25.000 50.000 75.000 100.000 125.000 125.000
#2	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#3	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#4	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#5	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#6	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#7	Secondary	887.50'	18.0" Round RCP_Round 18"

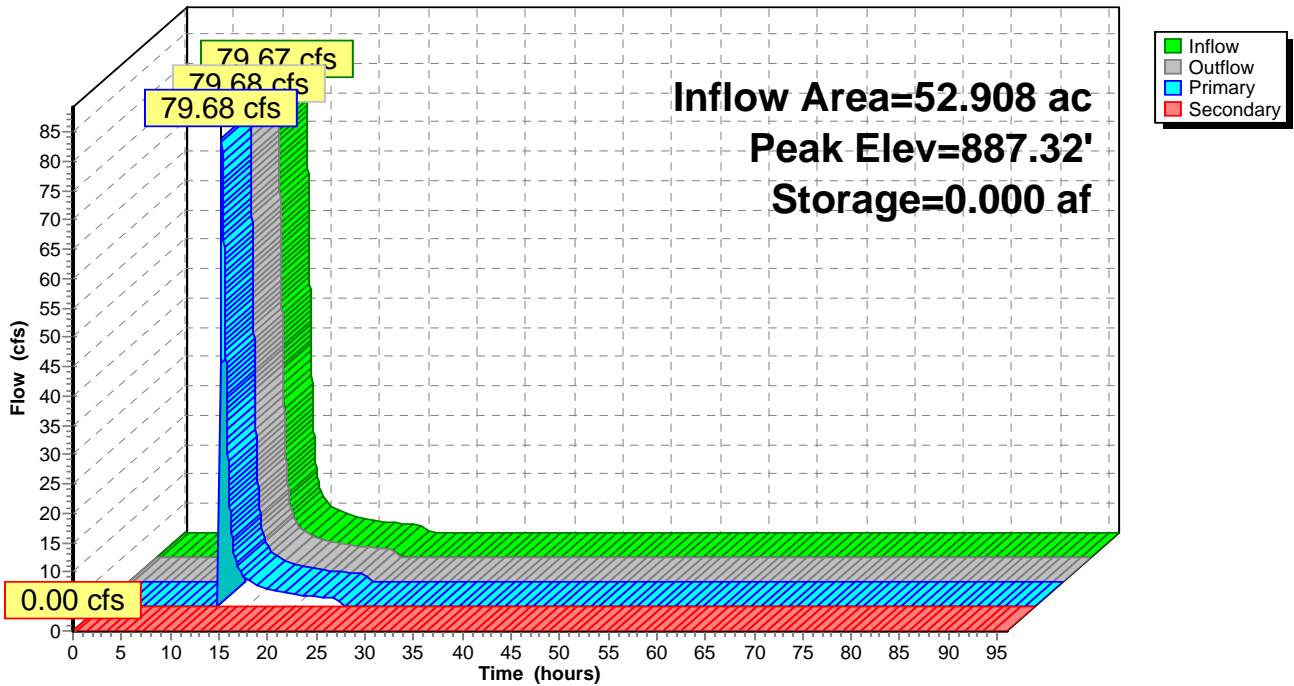
			L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#8	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#9	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=79.65 cfs @ 12.40 hrs HW=887.32' TW=884.02' (Dynamic Tailwater)
 1=Special & User-Defined (Custom Controls 79.65 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=887.00' TW=883.00' (Dynamic Tailwater)
 2=RCP_Round 18" (Controls 0.00 cfs)
 3=RCP_Round 18" (Controls 0.00 cfs)
 4=RCP_Round 18" (Controls 0.00 cfs)
 5=RCP_Round 18" (Controls 0.00 cfs)
 6=RCP_Round 18" (Controls 0.00 cfs)
 7=RCP_Round 18" (Controls 0.00 cfs)
 8=RCP_Round 18" (Controls 0.00 cfs)
 9=RCP_Round 18" (Controls 0.00 cfs)

Pond 36P: Culverts passing flow beneath Spine Road

Hydrograph



Summary for Pond CRH-1: CRH-1

Inflow Area = 6.955 ac, 46.76% Impervious, Inflow Depth = 1.63" for 2-Year event
 Inflow = 10.87 cfs @ 12.15 hrs, Volume= 0.947 af
 Outflow = 4.80 cfs @ 12.48 hrs, Volume= 0.947 af, Atten= 56%, Lag= 19.6 min
 Discarded = 0.22 cfs @ 12.48 hrs, Volume= 0.471 af
 Primary = 4.58 cfs @ 12.48 hrs, Volume= 0.476 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 877.67' @ 12.48 hrs Surf.Area= 0.275 ac Storage= 0.354 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 270.2 min (1,064.9 - 794.7)

Volume	Invert	Avail.Storage	Storage Description
#1	876.00'	0.850 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
876.00	0.150	0.000	0.000
878.00	0.300	0.450	0.450
879.00	0.500	0.400	0.850

Device	Routing	Invert	Outlet Devices
#1	Discarded	876.00'	0.800 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 0.00'
#2	Primary	877.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 877.00' / 876.00' S= 0.0065 1/1' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Primary	877.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 877.00' / 876.00' S= 0.0065 1/1' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Discarded OutFlow Max=0.22 cfs @ 12.48 hrs HW=877.67' (Free Discharge)

└─1=Exfiltration (Controls 0.22 cfs)

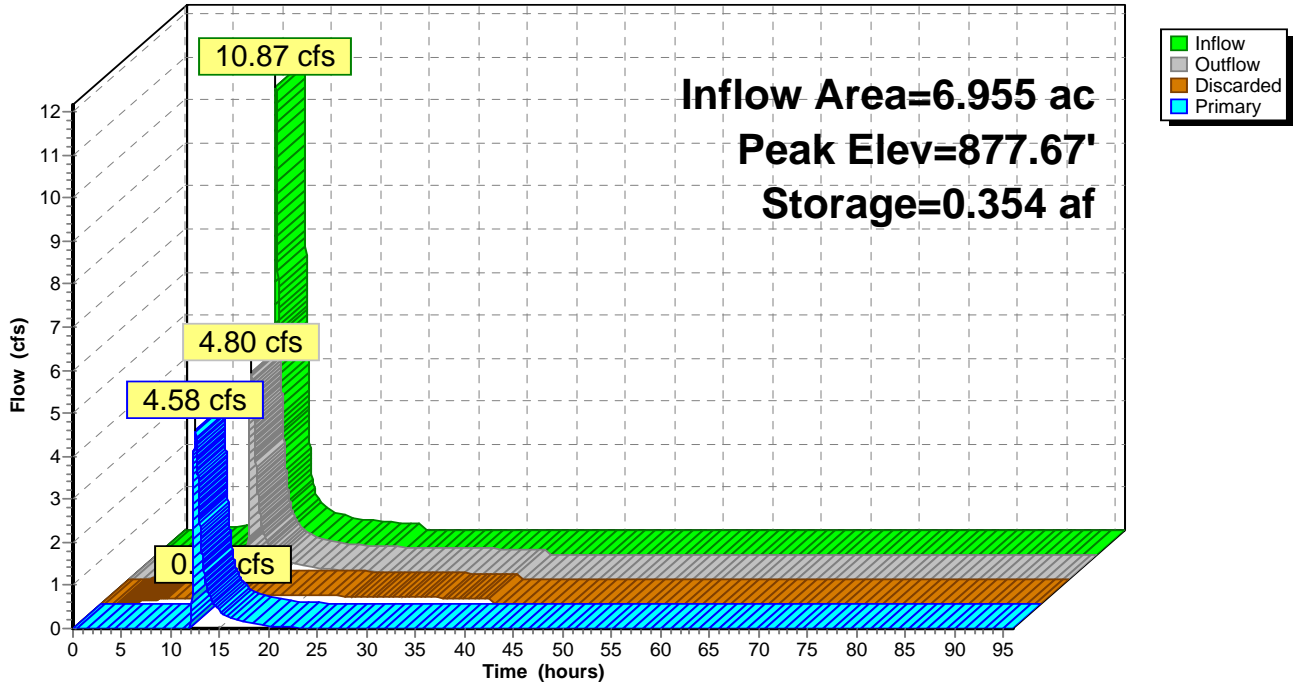
Primary OutFlow Max=4.58 cfs @ 12.48 hrs HW=877.67' (Free Discharge)

└─2=Culvert (Barrel Controls 2.29 cfs @ 3.72 fps)

└─3=Culvert (Barrel Controls 2.29 cfs @ 3.72 fps)

Pond CRH-1: CRH-1

Hydrograph



Summary for Pond CRH-2: CRH-2

Inflow Area = 10.214 ac, 37.73% Impervious, Inflow Depth = 1.47" for 2-Year event
 Inflow = 12.67 cfs @ 12.22 hrs, Volume= 1.253 af
 Outflow = 2.62 cfs @ 12.88 hrs, Volume= 1.254 af, Atten= 79%, Lag= 39.4 min
 Discarded = 0.33 cfs @ 12.88 hrs, Volume= 0.833 af
 Primary = 2.29 cfs @ 12.88 hrs, Volume= 0.420 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 882.04' @ 12.88 hrs Surf.Area= 0.404 ac Storage= 0.617 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 500.6 min (1,307.9 - 807.2)

Volume	Invert	Avail.Storage	Storage Description
#1	880.00'	1.600 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
880.00	0.200	0.000	0.000
882.00	0.400	0.600	0.600
884.00	0.600	1.000	1.600

Device	Routing	Invert	Outlet Devices
#1	Discarded	880.00'	0.800 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 0.00'
#2	Primary	881.50'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 881.50' / 881.00' S= 0.0032 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Primary	881.50'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 881.50' / 881.00' S= 0.0032 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Discarded OutFlow Max=0.33 cfs @ 12.88 hrs HW=882.04' (Free Discharge)

↑ **1=Exfiltration** (Controls 0.33 cfs)

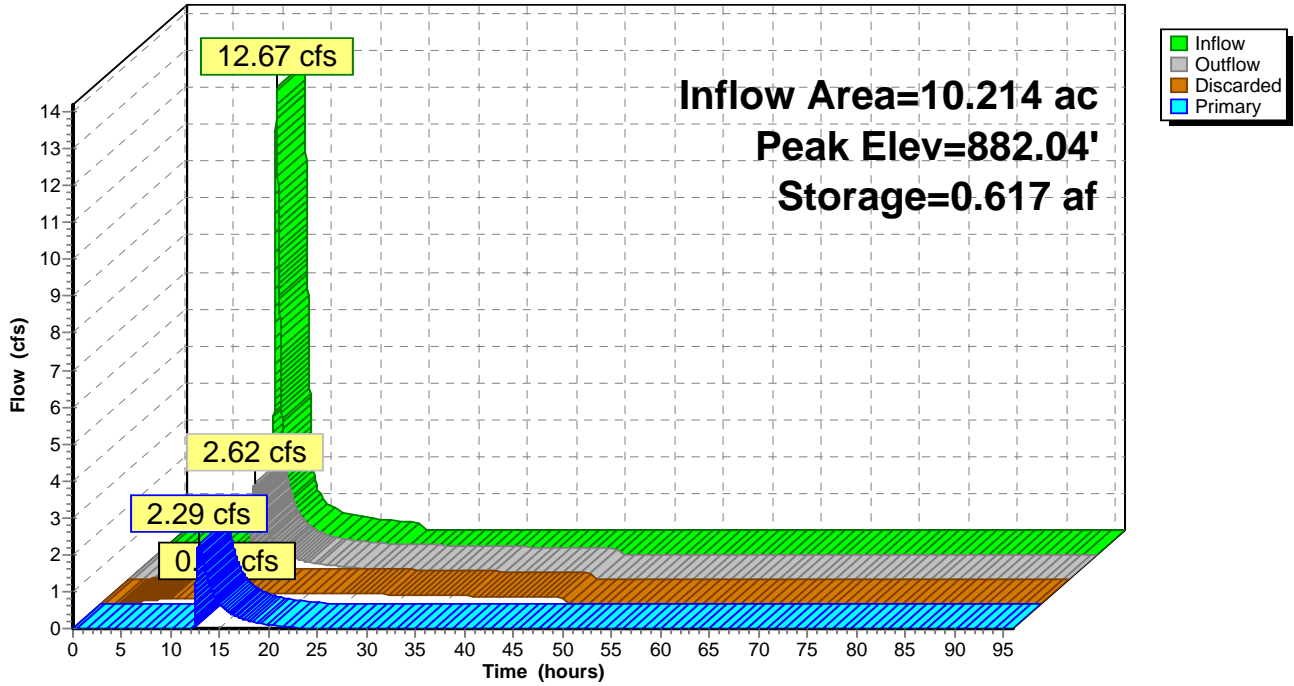
Primary OutFlow Max=2.29 cfs @ 12.88 hrs HW=882.04' TW=877.79' (Dynamic Tailwater)

↑ **2=Culvert** (Barrel Controls 1.15 cfs @ 2.51 fps)

↑ **3=Culvert** (Barrel Controls 1.15 cfs @ 2.51 fps)

Pond CRH-2: CRH-2

Hydrograph



Summary for Pond CRH-3: CRH-3

Inflow Area = 11.815 ac, 36.95% Impervious, Inflow Depth = 0.61" for 2-Year event
 Inflow = 2.97 cfs @ 12.04 hrs, Volume= 0.603 af
 Outflow = 1.17 cfs @ 14.13 hrs, Volume= 0.603 af, Atten= 61%, Lag= 125.4 min
 Discarded = 0.20 cfs @ 14.13 hrs, Volume= 0.381 af
 Primary = 0.97 cfs @ 14.13 hrs, Volume= 0.222 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 878.30' @ 14.13 hrs Surf.Area= 0.248 ac Storage= 0.259 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 351.8 min (1,209.1 - 857.3)

Volume	Invert	Avail.Storage	Storage Description
#1	877.00'	0.850 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
877.00	0.150	0.000	0.000
879.00	0.300	0.450	0.450
880.00	0.500	0.400	0.850

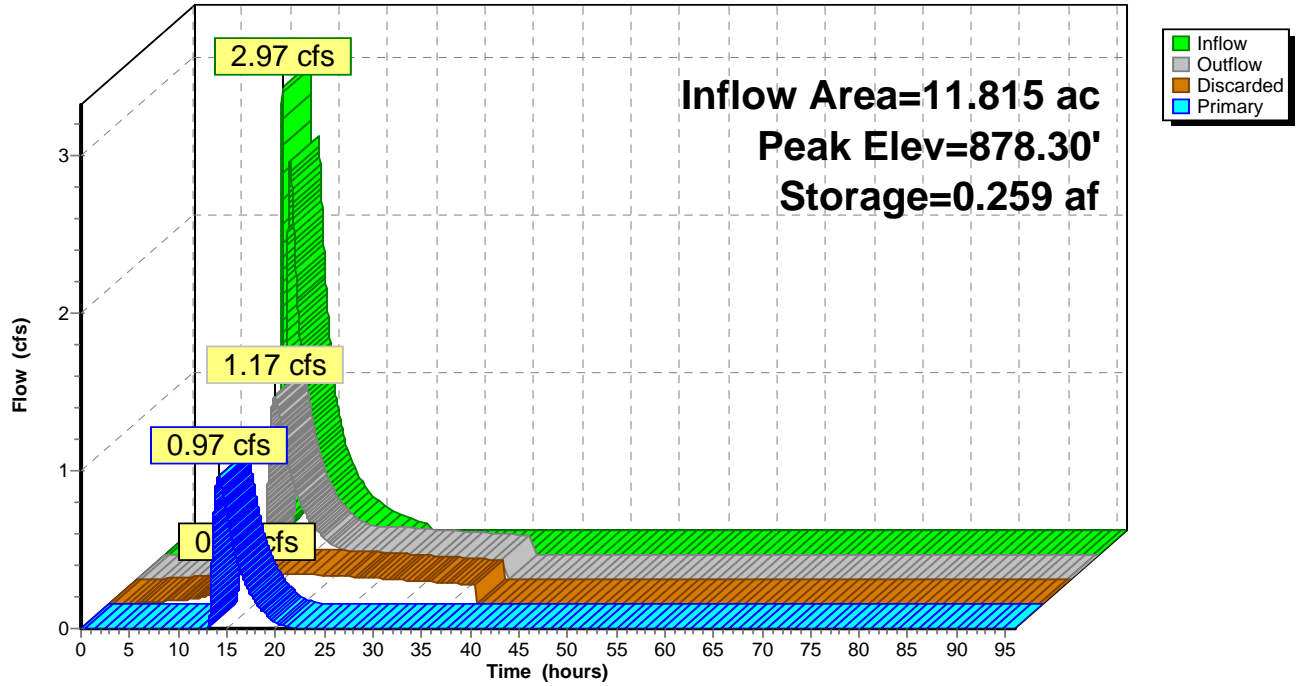
Device	Routing	Invert	Outlet Devices
#1	Discarded	877.00'	0.800 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 0.00'
#2	Primary	878.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 878.00' / 877.00' S= 0.0065 1/1' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Primary	878.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 878.00' / 877.00' S= 0.0065 1/1' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Discarded OutFlow Max=0.20 cfs @ 14.13 hrs HW=878.30' (Free Discharge)
 ↳ **1=Exfiltration** (Controls 0.20 cfs)

Primary OutFlow Max=0.97 cfs @ 14.13 hrs HW=878.30' (Free Discharge)
 ↳ **2=Culvert** (Barrel Controls 0.48 cfs @ 2.44 fps)
 ↳ **3=Culvert** (Barrel Controls 0.48 cfs @ 2.44 fps)

Pond CRH-3: CRH-3

Hydrograph



Summary for Pond P1/P2: P-1/P-2

Inflow Area = 68.531 ac, 57.92% Impervious, Inflow Depth = 1.83" for 2-Year event
 Inflow = 54.34 cfs @ 12.57 hrs, Volume= 10.477 af
 Outflow = 52.67 cfs @ 12.67 hrs, Volume= 10.470 af, Atten= 3%, Lag= 6.2 min
 Primary = 52.67 cfs @ 12.67 hrs, Volume= 10.470 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 924.00' Surf.Area= 1.270 ac Storage= 3.500 af
 Peak Elev= 924.94' @ 12.67 hrs Surf.Area= 1.411 ac Storage= 4.760 af (1.260 af above start)

Plug-Flow detention time= 302.4 min calculated for 6.969 af (67% of inflow)
 Center-of-Mass det. time= 96.8 min (907.5 - 810.7)

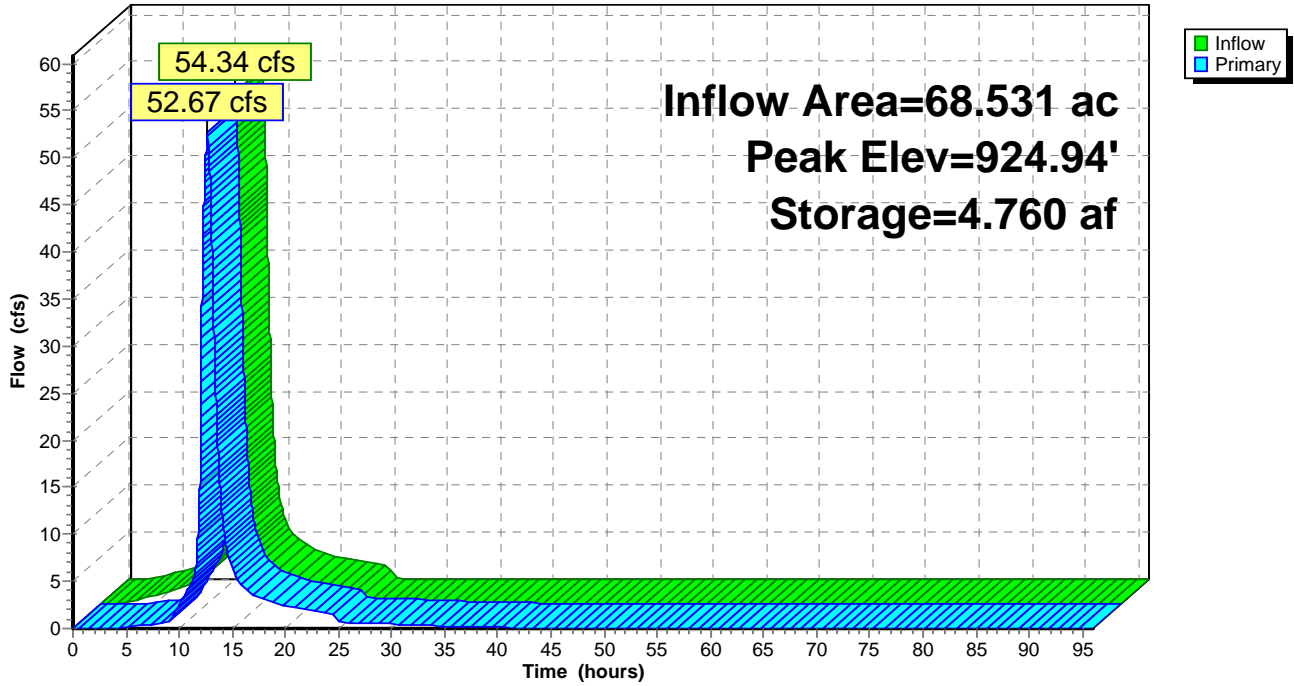
Volume	Invert	Avail.Storage	Storage Description
#1	920.00'	6.340 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
920.00	0.650	0.000	0.000
922.00	0.790	1.440	1.440
924.00	1.270	2.060	3.500
926.00	1.570	2.840	6.340

Device	Routing	Invert	Outlet Devices
#1	Primary	924.40'	40.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Primary	924.00'	6.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=52.66 cfs @ 12.67 hrs HW=924.94' TW=916.30' (Dynamic Tailwater)
 1=Sharp-Crested Rectangular Weir (Weir Controls 51.75 cfs @ 2.40 fps)
 2=Orifice/Grate (Orifice Controls 0.92 cfs @ 4.67 fps)

Pond P1/P2: P-1/P-2

Hydrograph



Summary for Pond P5/P6: P-5/P-6

Inflow Area = 43.279 ac, 47.44% Impervious, Inflow Depth = 1.65" for 2-Year event
 Inflow = 66.74 cfs @ 12.15 hrs, Volume= 5.941 af
 Outflow = 6.86 cfs @ 13.07 hrs, Volume= 3.826 af, Atten= 90%, Lag= 55.4 min
 Primary = 5.60 cfs @ 13.07 hrs, Volume= 3.144 af
 Secondary = 1.26 cfs @ 13.07 hrs, Volume= 0.681 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 929.00' Surf.Area= 1.975 ac Storage= 5.062 af
 Peak Elev= 930.72' @ 13.07 hrs Surf.Area= 2.356 ac Storage= 8.830 af (3.767 af above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= 335.9 min (1,129.6 - 793.7)

Volume	Invert	Avail.Storage	Storage Description
#1	926.00'	14.650 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
926.00	1.510	0.000	0.000
928.00	1.710	3.220	3.220
930.00	2.240	3.950	7.170
931.00	2.400	2.320	9.490
933.00	2.760	5.160	14.650

Device	Routing	Invert	Outlet Devices
#1	Primary	930.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	930.50'	7.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#3	Primary	931.50'	14.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#4	Secondary	930.00'	9.0" Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=5.60 cfs @ 13.07 hrs HW=930.72' TW=916.51' (Dynamic Tailwater)

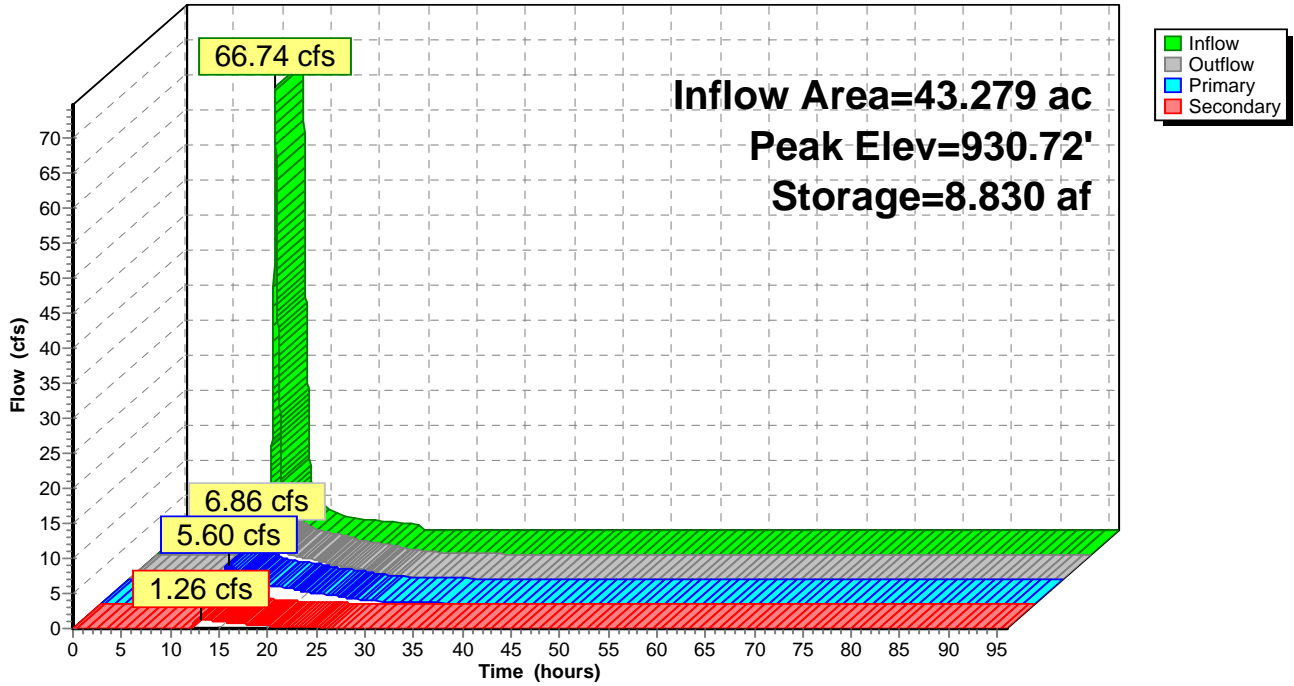
- ↑ 1=Orifice/Grate (Orifice Controls 3.21 cfs @ 4.09 fps)
- └ 2=Sharp-Crested Rectangular Weir (Weir Controls 2.38 cfs @ 1.54 fps)
- └ 3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

Secondary OutFlow Max=1.26 cfs @ 13.07 hrs HW=930.72' TW=929.06' (Dynamic Tailwater)

- ↑ 4=Orifice/Grate (Orifice Controls 1.26 cfs @ 2.89 fps)

Pond P5/P6: P-5/P-6

Hydrograph



Summary for Pond P8: P-8

Inflow Area = 6.389 ac, 7.62% Impervious, Inflow Depth = 0.93" for 2-Year event
 Inflow = 7.36 cfs @ 12.06 hrs, Volume= 0.497 af
 Outflow = 1.25 cfs @ 12.60 hrs, Volume= 0.491 af, Atten= 83%, Lag= 32.2 min
 Primary = 1.25 cfs @ 12.60 hrs, Volume= 0.491 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 897.00' Surf.Area= 0.300 ac Storage= 0.495 af
 Peak Elev= 897.62' @ 12.65 hrs Surf.Area= 0.394 ac Storage= 0.712 af (0.217 af above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= 274.4 min (1,119.7 - 845.3)

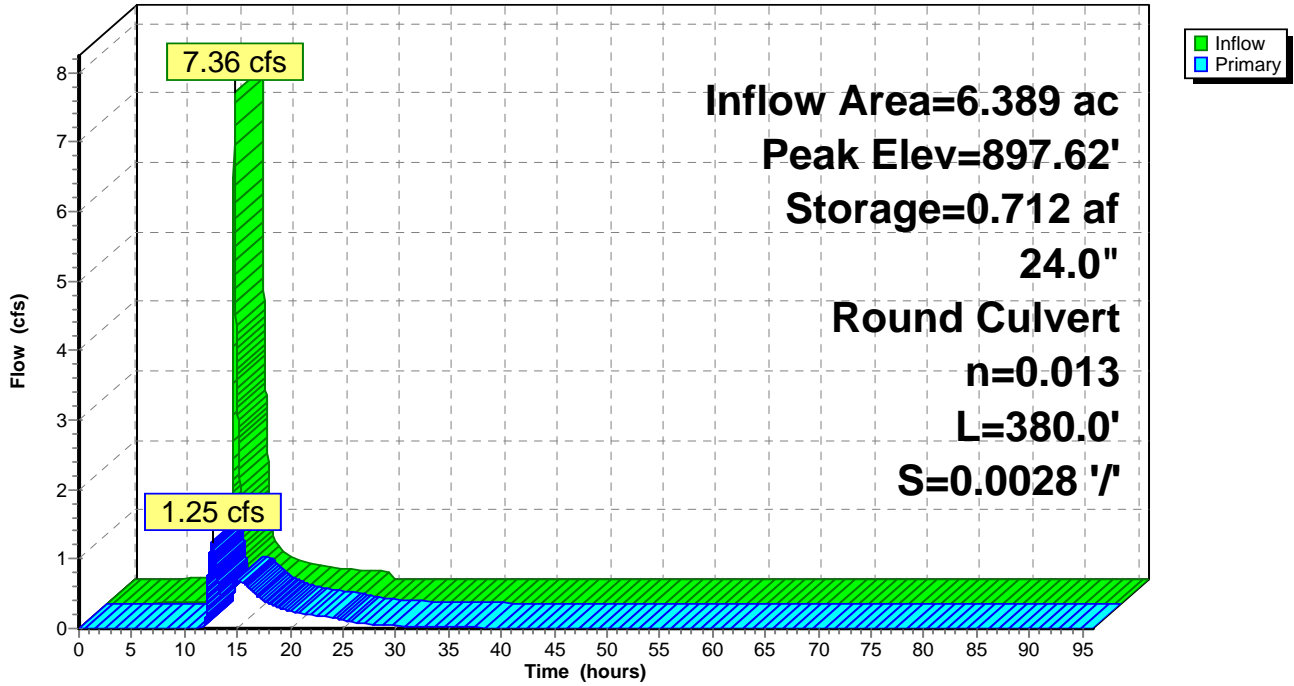
Volume	Invert	Avail.Storage	Storage Description
#1	893.00'	1.850 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
893.00	0.030	0.000	0.000
894.00	0.070	0.050	0.050
896.00	0.150	0.220	0.270
897.00	0.300	0.225	0.495
898.00	0.450	0.375	0.870
900.00	0.530	0.980	1.850

Device	Routing	Invert	Outlet Devices
#1	Primary	897.00'	24.0" Round RCP_Round 24" L= 380.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 897.00' / 895.94' S= 0.0028 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Primary OutFlow Max=1.24 cfs @ 12.60 hrs HW=897.62' TW=896.73' (Dynamic Tailwater)
 ↑1=RCP_Round 24" (Outlet Controls 1.24 cfs @ 2.23 fps)

Pond P8: P-8

Hydrograph



Summary for Pond W-1: W-1

Inflow Area = 0.997 ac, 24.47% Impervious, Inflow Depth = 11.67" for 2-Year event
 Inflow = 2.44 cfs @ 13.25 hrs, Volume= 0.969 af
 Outflow = 2.00 cfs @ 14.40 hrs, Volume= 0.969 af, Atten= 18%, Lag= 69.3 min
 Primary = 2.00 cfs @ 14.40 hrs, Volume= 0.969 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 915.09' @ 14.40 hrs Surf.Area= 0.714 ac Storage= 0.231 af

Plug-Flow detention time= 121.0 min calculated for 0.969 af (100% of inflow)
 Center-of-Mass det. time= 120.8 min (990.9 - 870.1)

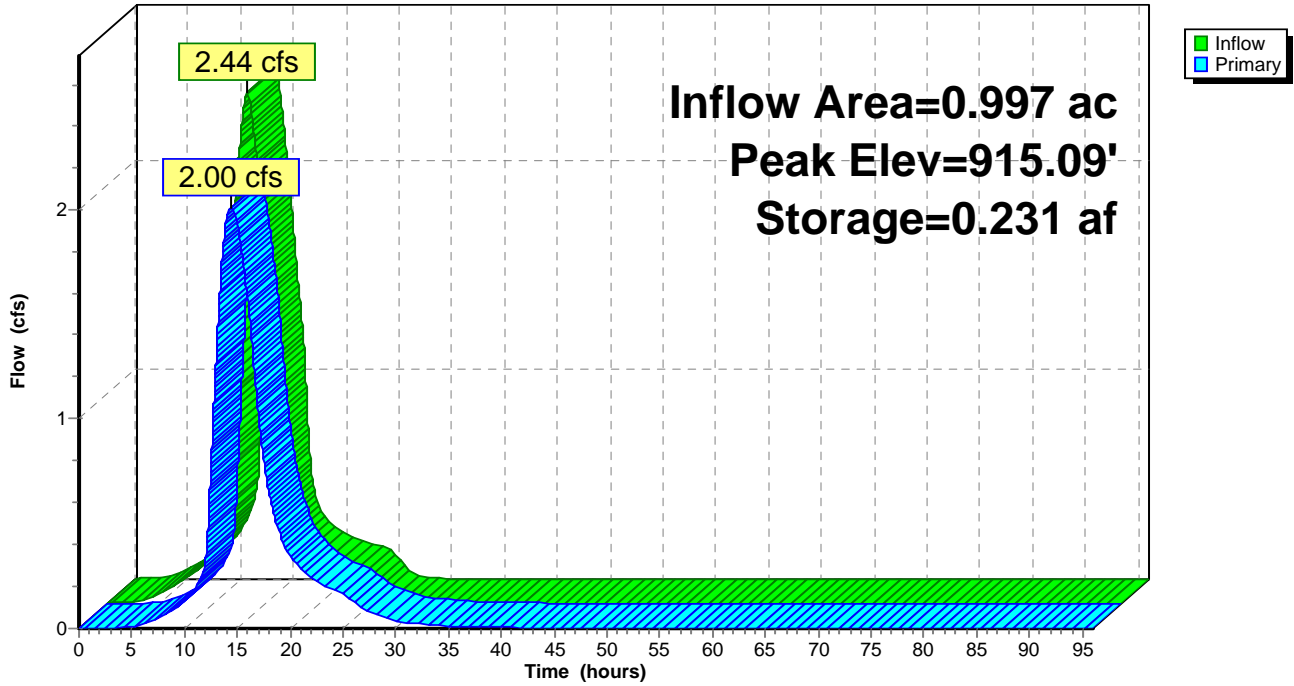
Volume	Invert	Avail.Storage	Storage Description
#1	914.75'	0.950 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
914.75	0.660	0.000	0.000
916.00	0.860	0.950	0.950

Device	Routing	Invert	Outlet Devices
#1	Primary	914.75'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=2.00 cfs @ 14.40 hrs HW=915.09' TW=0.00' (Dynamic Tailwater)
 ↑1=Orifice/Grate (Weir Controls 2.00 cfs @ 1.90 fps)

Pond W-1: W-1

Hydrograph



Summary for Pond W-2: W-2

Inflow = 1.26 cfs @ 13.07 hrs, Volume= 0.681 af
 Outflow = 0.37 cfs @ 20.19 hrs, Volume= 0.536 af, Atten= 70%, Lag= 426.8 min
 Primary = 0.37 cfs @ 20.19 hrs, Volume= 0.536 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 929.37' @ 20.19 hrs Surf.Area= 1.153 ac Storage= 0.413 af

Plug-Flow detention time= 878.1 min calculated for 0.536 af (79% of inflow)
 Center-of-Mass det. time= 760.2 min (1,809.8 - 1,049.7)

Volume	Invert	Avail.Storage	Storage Description
#1	929.00'	1.175 af	Custom Stage Data (Prismatic) Listed below (Recalc)

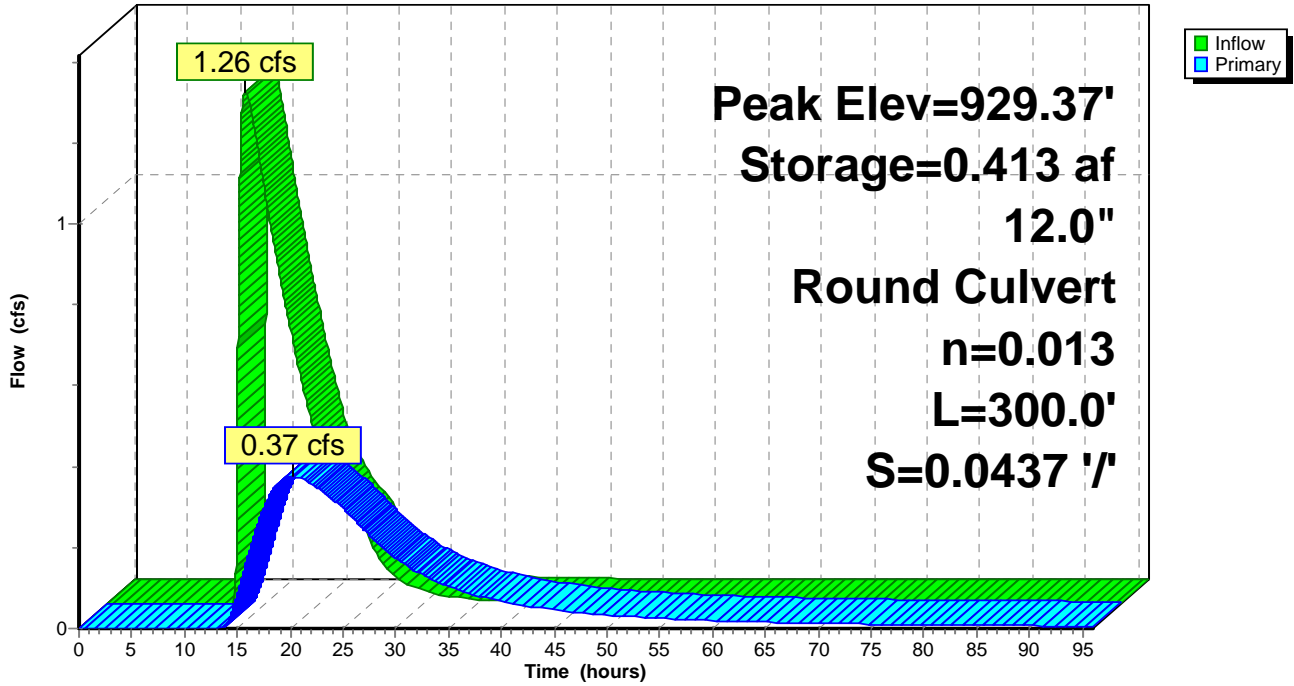
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
929.00	1.090	0.000	0.000
930.00	1.260	1.175	1.175

Device	Routing	Invert	Outlet Devices
#1	Primary	929.10'	12.0" Round RCP_Round 12" L= 300.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 929.10' / 916.00' S= 0.0437 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.37 cfs @ 20.19 hrs HW=929.37' TW=914.81' (Dynamic Tailwater)
 ↳1=RCP_Round 12" (Inlet Controls 0.37 cfs @ 2.21 fps)

Pond W-2: W-2

Hydrograph



Summary for Pond W-3: W-3

Inflow = 0.37 cfs @ 20.19 hrs, Volume= 0.536 af
 Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

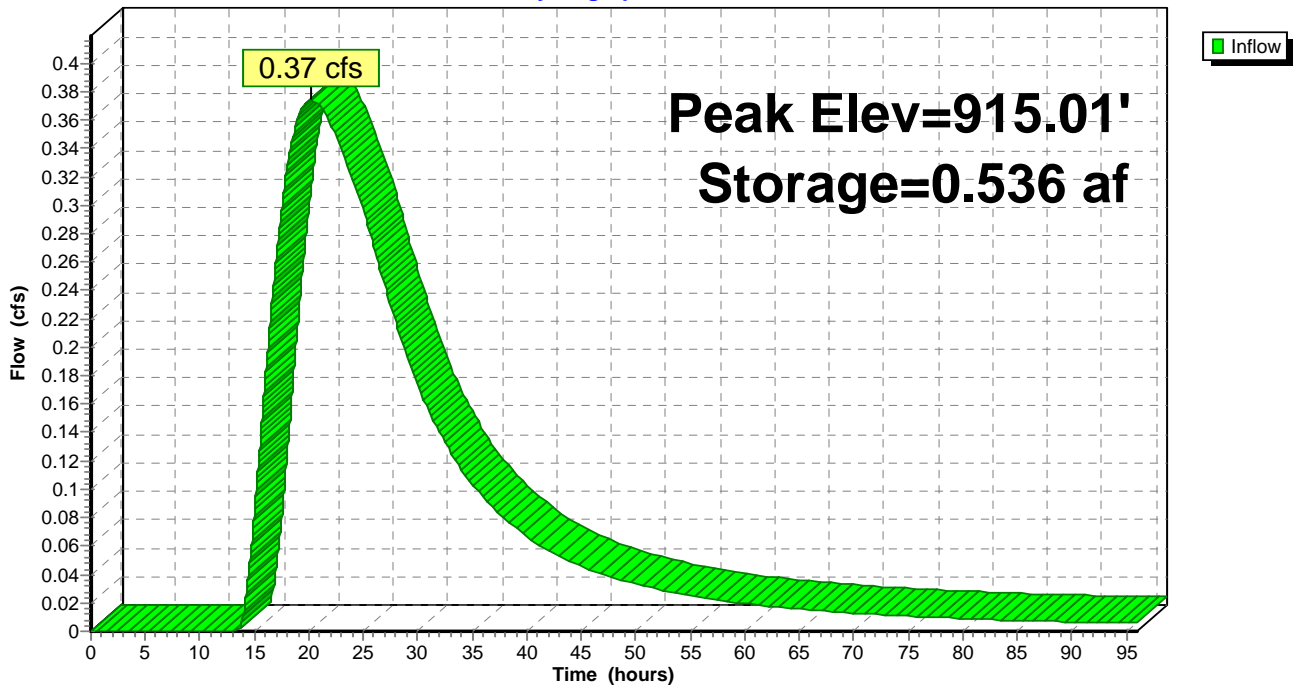
Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 915.01' @ 96.00 hrs Surf.Area= 2.082 ac Storage= 0.536 af

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description
#1	914.75'	2.680 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
914.75	2.040	0.000	0.000
915.00	2.080	0.515	0.515
916.00	2.250	2.165	2.680

Pond W-3: W-3

Hydrograph



Summary for Pond W-4: W-4

Inflow Area = 2.985 ac, 30.99% Impervious, Inflow Depth = 8.39" for 2-Year event
 Inflow = 5.28 cfs @ 12.09 hrs, Volume= 2.086 af
 Outflow = 2.69 cfs @ 16.41 hrs, Volume= 2.067 af, Atten= 49%, Lag= 259.5 min
 Primary = 2.69 cfs @ 16.41 hrs, Volume= 2.067 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 908.83' @ 16.41 hrs Surf.Area= 1.078 ac Storage= 0.769 af

Plug-Flow detention time= 292.4 min calculated for 2.067 af (99% of inflow)
 Center-of-Mass det. time= 285.3 min (1,204.8 - 919.5)

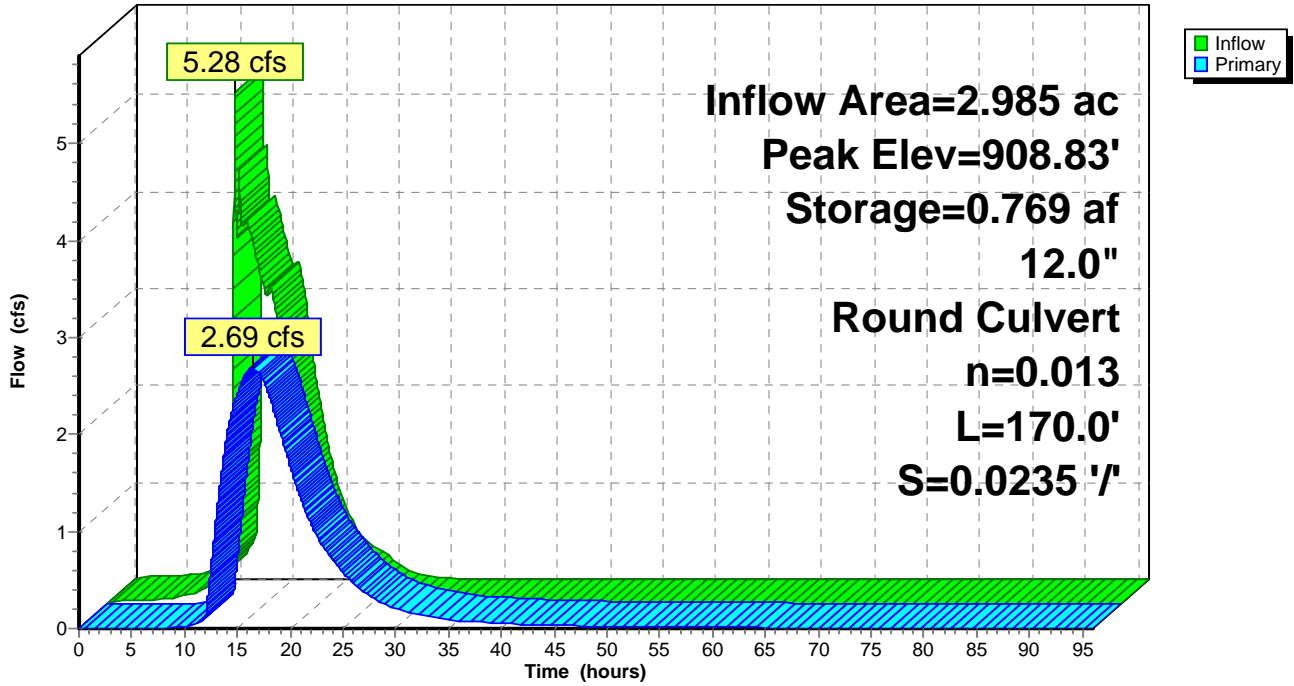
Volume	Invert	Avail.Storage	Storage Description
#1	908.00'	2.280 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
908.00	0.780	0.000	0.000
910.00	1.500	2.280	2.280

Device	Routing	Invert	Outlet Devices
#1	Primary	908.00'	12.0" Round RCP_Round 12" L= 170.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 908.00' / 904.00' S= 0.0235 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=2.69 cfs @ 16.41 hrs HW=908.83' TW=893.75' (Dynamic Tailwater)
 ↳1=RCP_Round 12" (Inlet Controls 2.69 cfs @ 3.87 fps)

Pond W-4: W-4

Hydrograph



Summary for Pond W-5: W-5

Inflow Area = 7.608 ac, 48.41% Impervious, Inflow Depth = 4.02" for 2-Year event
 Inflow = 20.53 cfs @ 12.02 hrs, Volume= 2.547 af
 Outflow = 4.37 cfs @ 13.19 hrs, Volume= 2.540 af, Atten= 79%, Lag= 70.0 min
 Primary = 4.37 cfs @ 13.19 hrs, Volume= 2.540 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 882.75' Surf.Area= 4.887 ac Storage= 7.134 af
 Peak Elev= 882.98' @ 13.19 hrs Surf.Area= 5.104 ac Storage= 8.297 af (1.163 af above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= 288.6 min (1,115.7 - 827.1)

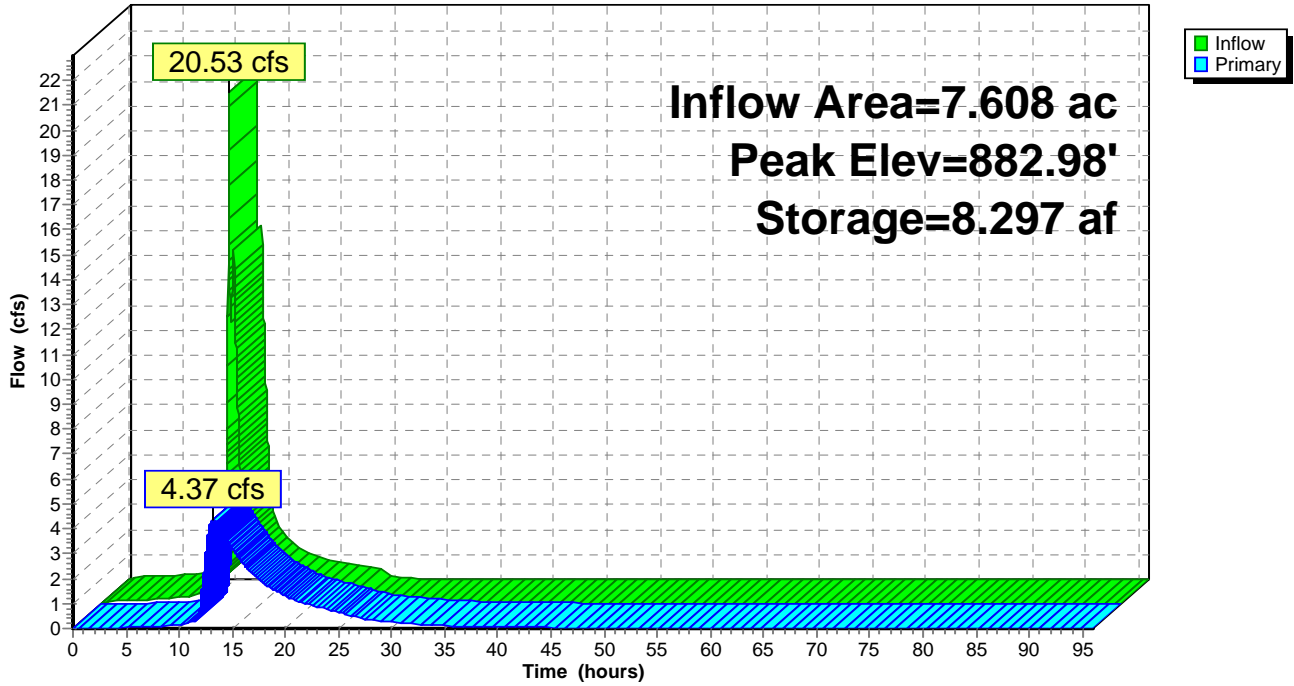
Volume	Invert	Avail.Storage	Storage Description
#1	881.00'	11.097 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
881.00	3.270	0.000	0.000
882.00	4.190	3.730	3.730
883.00	5.120	4.655	8.385
883.49	5.950	2.712	11.097

Device	Routing	Invert	Outlet Devices
#1	Primary	882.75'	6.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Primary	882.75'	6.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)

Primary OutFlow Max=4.37 cfs @ 13.19 hrs HW=882.98' TW=0.00' (Dynamic Tailwater)
 1=Sharp-Crested Rectangular Weir (Weir Controls 2.19 cfs @ 1.58 fps)
 2=Sharp-Crested Rectangular Weir (Weir Controls 2.19 cfs @ 1.58 fps)

Pond W-5: W-5

Hydrograph

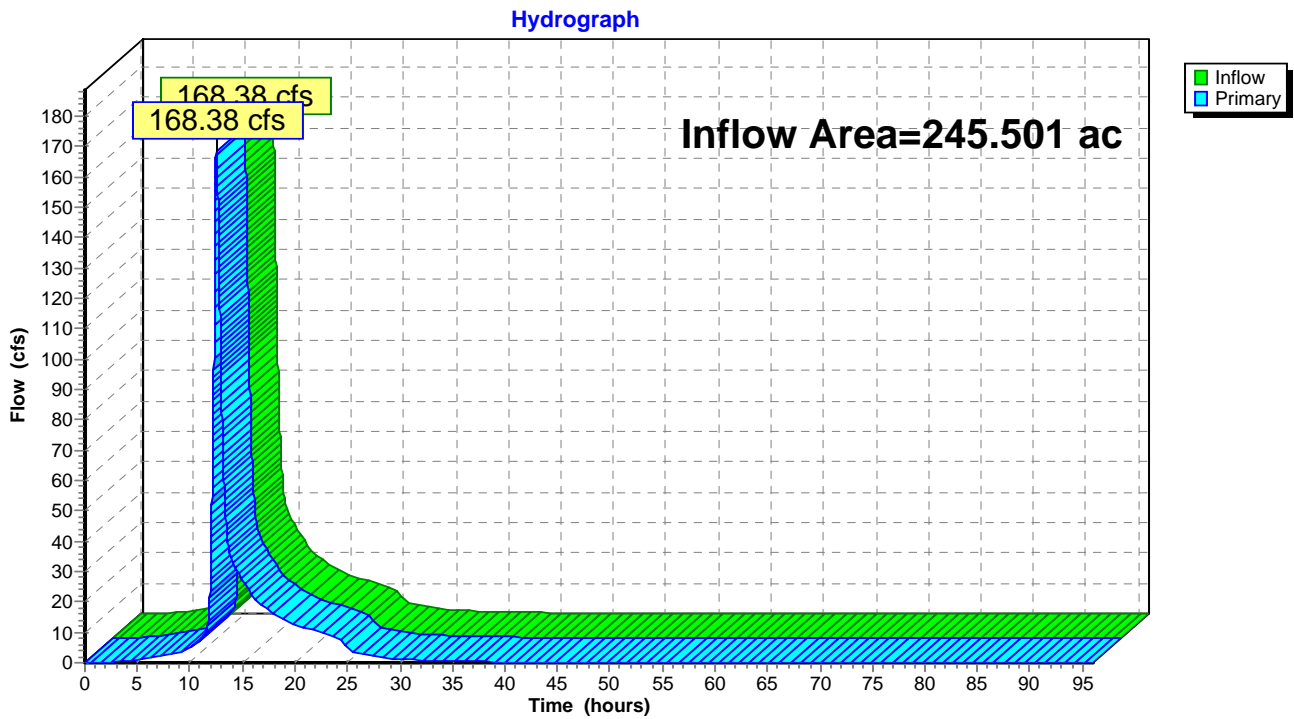


Summary for Link 49L: Sum of P-13 and W-5 Discharges to Rice Creek

Inflow Area = 245.501 ac, 51.49% Impervious, Inflow Depth = 1.56" for 2-Year event
Inflow = 168.38 cfs @ 12.52 hrs, Volume= 31.921 af
Primary = 168.38 cfs @ 12.52 hrs, Volume= 31.921 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 49L: Sum of P-13 and W-5 Discharges to Rice Creek

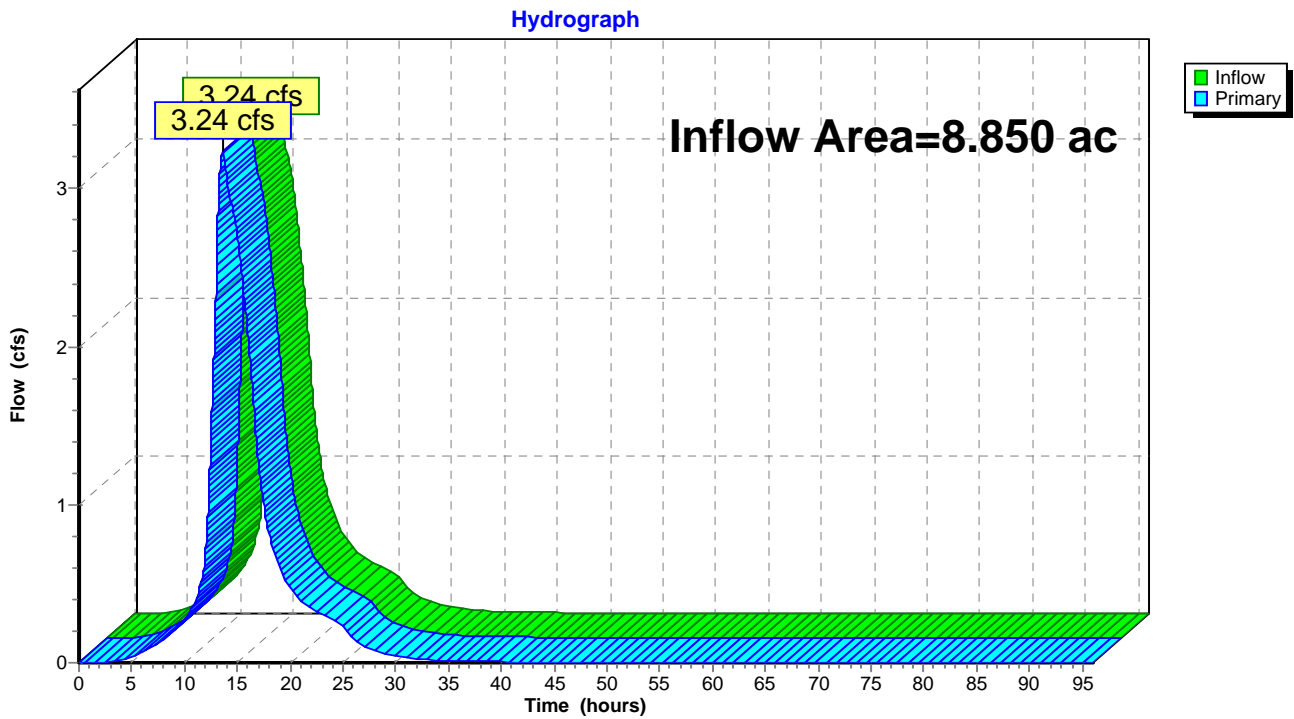


Summary for Link 50L: Outlet #1 Discharge to Round Lake

Inflow Area = 8.850 ac, 65.20% Impervious, Inflow Depth = 1.97" for 2-Year event
Inflow = 3.24 cfs @ 13.59 hrs, Volume= 1.454 af
Primary = 3.24 cfs @ 13.59 hrs, Volume= 1.454 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 50L: Outlet #1 Discharge to Round Lake



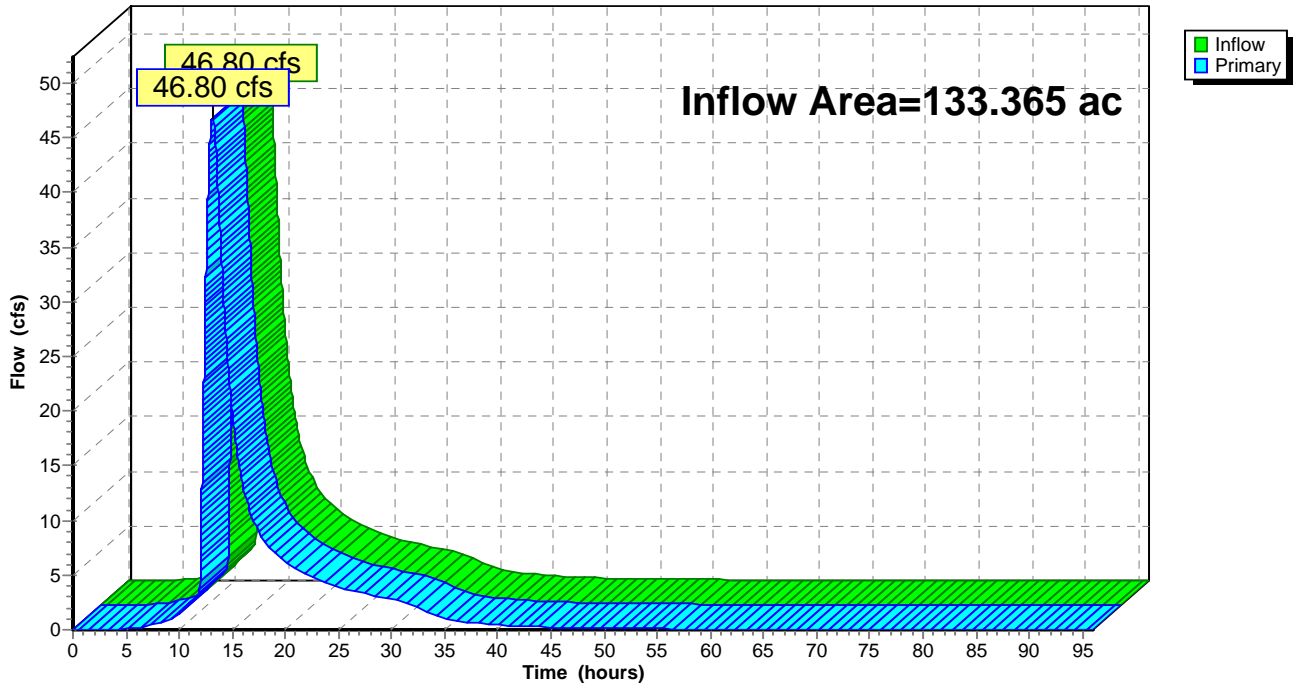
Summary for Link 51L: Outlet #2 Discharge to Round Lake

Inflow Area = 133.365 ac, 58.87% Impervious, Inflow Depth > 1.60" for 2-Year event
Inflow = 46.80 cfs @ 13.14 hrs, Volume= 17.759 af
Primary = 46.80 cfs @ 13.14 hrs, Volume= 17.759 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 51L: Outlet #2 Discharge to Round Lake

Hydrograph



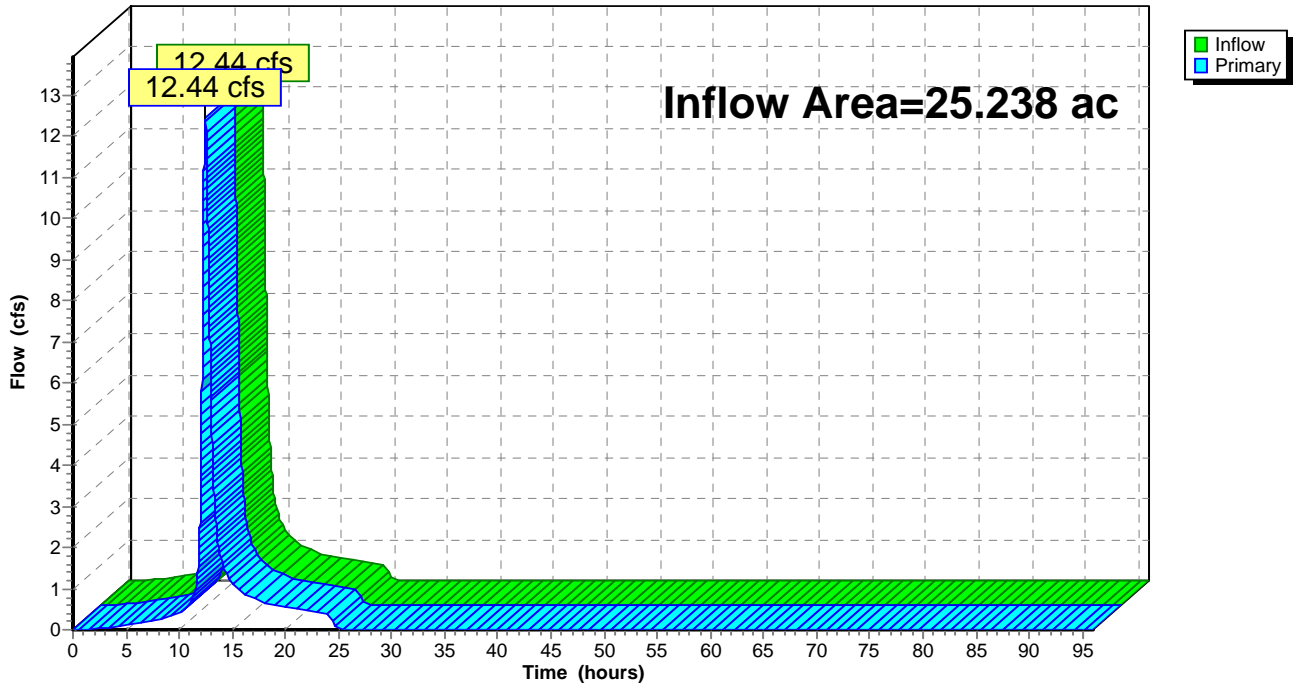
Summary for Link 52L: Outlet #3 Discharge to Round Lake

Inflow Area = 25.238 ac, 19.96% Impervious, Inflow Depth = 0.86" for 2-Year event
Inflow = 12.44 cfs @ 12.43 hrs, Volume= 1.805 af
Primary = 12.44 cfs @ 12.43 hrs, Volume= 1.805 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 52L: Outlet #3 Discharge to Round Lake

Hydrograph



Time span=0.00-96.00 hrs, dt=0.01 hrs, 9601 points
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: To Rice Creek	Runoff Area=1.601 ac 31.98% Impervious Runoff Depth=2.47" Tc=5.7 min CN=74/98 Runoff=5.55 cfs 0.330 af
Subcatchment SB 1: SB 1	Runoff Area=52.192 ac 48.35% Impervious Runoff Depth=2.83" Tc=53.1 min CN=74/98 Runoff=76.21 cfs 12.329 af
Subcatchment SB 10: SB 10	Runoff Area=6.389 ac 7.62% Impervious Runoff Depth=1.93" Tc=7.3 min CN=74/98 Runoff=16.44 cfs 1.027 af
Subcatchment SB 11: SB 11	Runoff Area=3.293 ac 32.16% Impervious Runoff Depth=2.55" Tc=11.7 min CN=74/100 Runoff=8.82 cfs 0.700 af
Subcatchment SB 12: SB 12	Runoff Area=1.382 ac 38.71% Impervious Runoff Depth=2.62" Tc=9.5 min CN=74/98 Runoff=4.23 cfs 0.302 af
Subcatchment SB 13: SB 13	Runoff Area=2.985 ac 30.99% Impervious Runoff Depth=2.52" Tc=9.4 min CN=74/100 Runoff=8.67 cfs 0.627 af
Subcatchment SB 14: SB 14	Runoff Area=10.225 ac 42.62% Impervious Runoff Depth=2.71" Tc=4.3 min CN=74/98 Runoff=41.19 cfs 2.307 af
Subcatchment SB 15: SB 15	Runoff Area=58.564 ac 48.22% Impervious Runoff Depth=2.83" Tc=31.3 min CN=74/98 Runoff=112.62 cfs 13.820 af
Subcatchment SB 16: SB 16	Runoff Area=32.428 ac 33.53% Impervious Runoff Depth=2.50" Tc=12.1 min CN=74/98 Runoff=86.24 cfs 6.769 af
Subcatchment SB 17: SB 17	Runoff Area=7.608 ac 48.41% Impervious Runoff Depth=2.95" Tc=4.3 min CN=74/100 Runoff=32.13 cfs 1.870 af
Subcatchment SB 18: SB 18	Runoff Area=52.908 ac 84.55% Impervious Runoff Depth=3.64" Tc=33.5 min CN=74/98 Runoff=124.38 cfs 16.050 af
Subcatchment SB 19: SB 19	Runoff Area=21.198 ac 39.93% Impervious Runoff Depth=2.65" Tc=24.7 min CN=74/98 Runoff=42.96 cfs 4.676 af
Subcatchment SB 2: SB 2	Runoff Area=11.400 ac 84.29% Impervious Runoff Depth=3.63" Tc=16.6 min CN=74/98 Runoff=37.33 cfs 3.453 af
Subcatchment SB 22: SB 22	Runoff Area=41.911 ac 82.19% Impervious Runoff Depth=3.34" Tc=41.0 min CN=49/98 Runoff=79.81 cfs 11.662 af
Subcatchment SB 24: SB 24	Runoff Area=4.939 ac 98.22% Impervious Runoff Depth=3.94" Tc=7.5 min CN=74/98 Runoff=24.36 cfs 1.623 af
Subcatchment SB 25: SB 25	Runoff Area=5.012 ac 95.71% Impervious Runoff Depth=3.89" Tc=10.7 min CN=74/98 Runoff=21.17 cfs 1.624 af

Subcatchment SB 26: SB 26	Runoff Area=14.335 ac 98.27% Impervious Runoff Depth=3.95" Tc=25.4 min CN=74/98 Runoff=41.75 cfs 4.713 af
Subcatchment SB 27: SB 27 (Thumb Road)	Runoff Area=6.629 ac 97.12% Impervious Runoff Depth=3.88" Tc=27.6 min CN=49/98 Runoff=18.18 cfs 2.143 af
Subcatchment SB 28: SB 28	Runoff Area=6.955 ac 46.76% Impervious Runoff Depth=2.80" Tc=14.6 min CN=74/98 Runoff=18.97 cfs 1.622 af
Subcatchment SB 29: SB 29	Runoff Area=10.214 ac 37.73% Impervious Runoff Depth=2.60" Tc=19.1 min CN=74/98 Runoff=23.01 cfs 2.212 af
Subcatchment SB 3: SB 3	Runoff Area=37.668 ac 41.46% Impervious Runoff Depth=2.68" Tc=15.3 min CN=74/98 Runoff=96.96 cfs 8.417 af
Subcatchment SB 4: SB 4	Runoff Area=0.599 ac 19.70% Impervious Runoff Depth=2.24" Tc=5.9 min CN=74/100 Runoff=1.86 cfs 0.112 af
Subcatchment SB 5: SB 5	Runoff Area=7.853 ac 70.37% Impervious Runoff Depth=3.32" Tc=59.3 min CN=74/98 Runoff=12.43 cfs 2.176 af
Subcatchment SB 51: Offsite Subbasin 51	Runoff Area=25.238 ac 19.96% Impervious Runoff Depth=1.72" Tc=32.8 min CN=65/98 Runoff=27.74 cfs 3.622 af
Subcatchment SB 6: SB 6	Runoff Area=0.997 ac 24.47% Impervious Runoff Depth=2.36" Tc=20.3 min CN=74/100 Runoff=1.96 cfs 0.196 af
Subcatchment SB 7: SB 7	Runoff Area=21.555 ac 84.83% Impervious Runoff Depth=3.65" Tc=5.7 min CN=74/98 Runoff=106.75 cfs 6.550 af
Subcatchment SB 8: SB 8	Runoff Area=29.595 ac 30.01% Impervious Runoff Depth=2.43" Tc=47.1 min CN=74/98 Runoff=39.95 cfs 5.985 af
Subcatchment SB 9: SB 9	Runoff Area=25.789 ac 33.17% Impervious Runoff Depth=2.50" Tc=30.0 min CN=74/98 Runoff=45.10 cfs 5.366 af
Pond 3P: P-3	Peak Elev=917.66' Storage=13.758 af Inflow=146.54 cfs 30.858 af Outflow=98.88 cfs 30.840 af
Pond 4P: P-4	Peak Elev=916.82' Storage=1.241 af Inflow=12.43 cfs 2.176 af Primary=4.47 cfs 0.953 af Secondary=2.78 cfs 1.223 af Outflow=7.25 cfs 2.176 af
Pond 7P: P-7	Peak Elev=915.56' Storage=1.330 af Inflow=39.95 cfs 5.985 af Outflow=40.55 cfs 5.985 af
Pond 9P: P-9	Peak Elev=915.51' Storage=0.487 af Inflow=77.38 cfs 11.351 af Outflow=77.30 cfs 11.351 af
Pond 10P: P-10	Peak Elev=897.75' Storage=1.270 af Inflow=38.10 cfs 10.812 af Primary=12.53 cfs 7.912 af Secondary=25.45 cfs 2.899 af Outflow=37.98 cfs 10.811 af
Pond 11P: P-11	Peak Elev=912.03' Storage=7.830 af Inflow=80.40 cfs 12.051 af Primary=35.82 cfs 9.489 af Secondary=4.51 cfs 2.560 af Outflow=40.33 cfs 12.049 af

Pond 12P: P-12	Peak Elev=894.35' Storage=7.405 af Inflow=45.31 cfs 16.286 af Outflow=37.09 cfs 16.278 af
Pond 13P: P-13	Peak Elev=884.60' Storage=7.718 af Inflow=327.06 cfs 54.310 af Primary=296.00 cfs 51.732 af Secondary=13.98 cfs 2.575 af Outflow=309.98 cfs 54.307 af
Pond 14P: P-14	Peak Elev=893.66' Storage=6.910 af Inflow=42.96 cfs 4.676 af Outflow=6.52 cfs 4.674 af
Pond 23P: Thumb Infiltration (Thumb TP	Peak Elev=903.83' Storage=3.834 af Inflow=95.01 cfs 13.805 af Outflow=94.93 cfs 10.065 af
Pond 31P: SB 18 Infiltration	Peak Elev=903.43' Storage=3.432 af Inflow=124.38 cfs 16.050 af Outflow=124.27 cfs 12.730 af
Pond 36P: Culverts passing flow	Peak Elev=887.50' Storage=0.001 af Inflow=124.27 cfs 12.730 af Primary=124.27 cfs 12.730 af Secondary=0.00 cfs 0.000 af Outflow=124.27 cfs 12.730 af
Pond CRH-1: CRH-1	Peak Elev=878.12' Storage=0.489 af Inflow=18.97 cfs 1.622 af Discarded=0.26 cfs 0.512 af Primary=11.80 cfs 1.110 af Outflow=12.06 cfs 1.622 af
Pond CRH-2: CRH-2	Peak Elev=882.67' Storage=0.888 af Inflow=23.01 cfs 2.212 af Discarded=0.38 cfs 0.904 af Primary=9.82 cfs 1.308 af Outflow=10.19 cfs 2.212 af
Pond CRH-3: CRH-3	Peak Elev=878.90' Storage=0.421 af Inflow=10.84 cfs 1.637 af Discarded=0.24 cfs 0.449 af Primary=7.98 cfs 1.188 af Outflow=8.22 cfs 1.637 af
Pond P1/P2: P-1/P-2	Peak Elev=925.17' Storage=5.093 af Inflow=92.00 cfs 17.405 af Outflow=89.60 cfs 17.397 af
Pond P5/P6: P-5/P-6	Peak Elev=931.49' Storage=10.676 af Inflow=116.17 cfs 10.153 af Primary=26.37 cfs 6.911 af Secondary=2.24 cfs 1.126 af Outflow=28.61 cfs 8.037 af
Pond P8: P-8	Peak Elev=898.11' Storage=0.922 af Inflow=16.44 cfs 1.027 af 24.0" Round Culvert n=0.013 L=380.0' S=0.0028 '/' Outflow=3.63 cfs 1.021 af
Pond W-1: W-1	Peak Elev=915.16' Storage=0.286 af Inflow=3.34 cfs 1.419 af Outflow=2.43 cfs 1.419 af
Pond W-2: W-2	Peak Elev=929.49' Storage=0.559 af Inflow=2.24 cfs 1.126 af 12.0" Round Culvert n=0.013 L=300.0' S=0.0437 '/' Outflow=0.77 cfs 0.979 af
Pond W-3: W-3	Peak Elev=915.22' Storage=0.979 af Inflow=0.77 cfs 0.979 af Outflow=0.00 cfs 0.000 af
Pond W-4: W-4	Peak Elev=908.99' Storage=0.949 af Inflow=10.44 cfs 3.187 af 12.0" Round Culvert n=0.013 L=170.0' S=0.0235 '/' Outflow=3.32 cfs 3.168 af
Pond W-5: W-5	Peak Elev=883.11' Storage=8.975 af Inflow=37.78 cfs 4.445 af Outflow=8.48 cfs 4.438 af

Link 49L: Sum of P-13 and W-5 Discharges to Rice Creek

Inflow=302.16 cfs 56.170 af
Primary=302.16 cfs 56.170 af

Link 50L: Outlet #1 Discharge to Round Lake

Inflow=6.78 cfs 2.372 af
Primary=6.78 cfs 2.372 af

Link 51L: Outlet #2 Discharge to Round Lake

Inflow=98.88 cfs 30.840 af
Primary=98.88 cfs 30.840 af

Link 52L: Outlet #3 Discharge to Round Lake

Inflow=27.74 cfs 3.622 af
Primary=27.74 cfs 3.622 af

Total Runoff Area = 501.462 ac Runoff Volume = 122.282 af Average Runoff Depth = 2.93"
45.62% Pervious = 228.758 ac 54.38% Impervious = 272.704 ac

Summary for Subcatchment 1S: To Rice Creek

Runoff = 5.55 cfs @ 12.04 hrs, Volume= 0.330 af, Depth= 2.47"

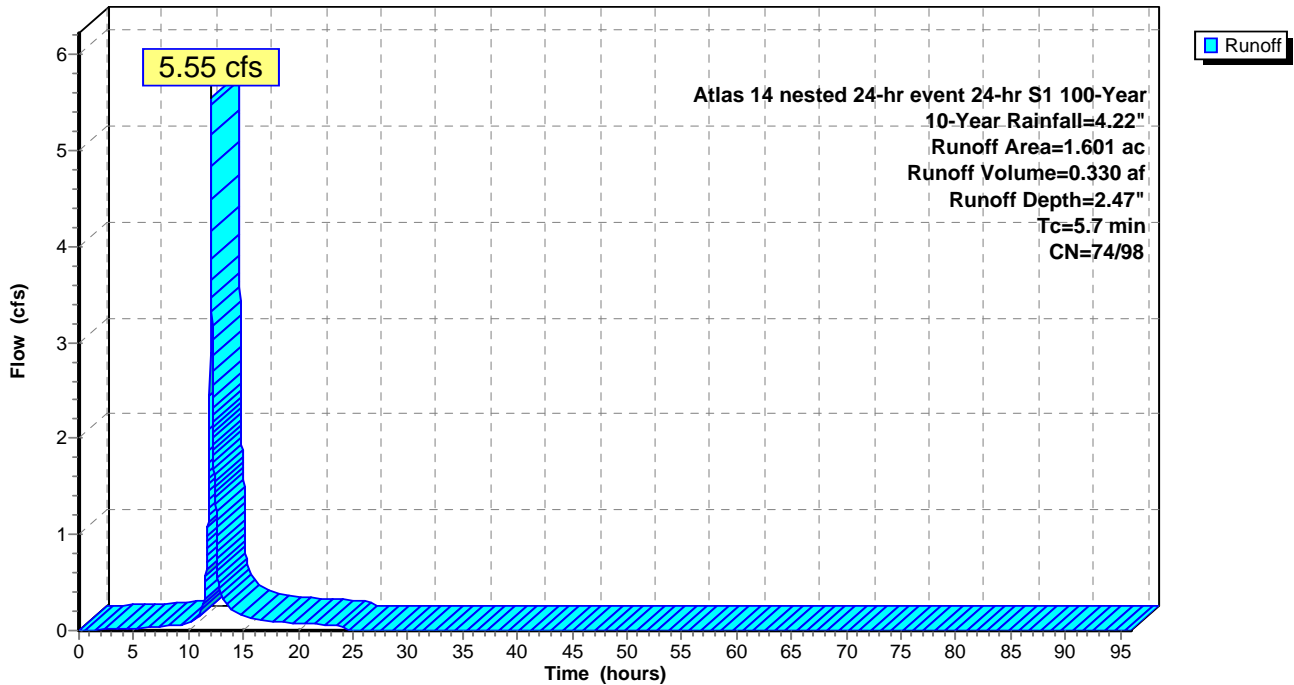
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 0.512	98	impervious
* 1.089	74	pervious
1.601	82	Weighted Average
1.089		68.02% Pervious Area
0.512		31.98% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.7					Direct Entry,

Subcatchment 1S: To Rice Creek

Hydrograph



Summary for Subcatchment SB 1: SB 1

Runoff = 76.21 cfs @ 12.69 hrs, Volume= 12.329 af, Depth= 2.83"

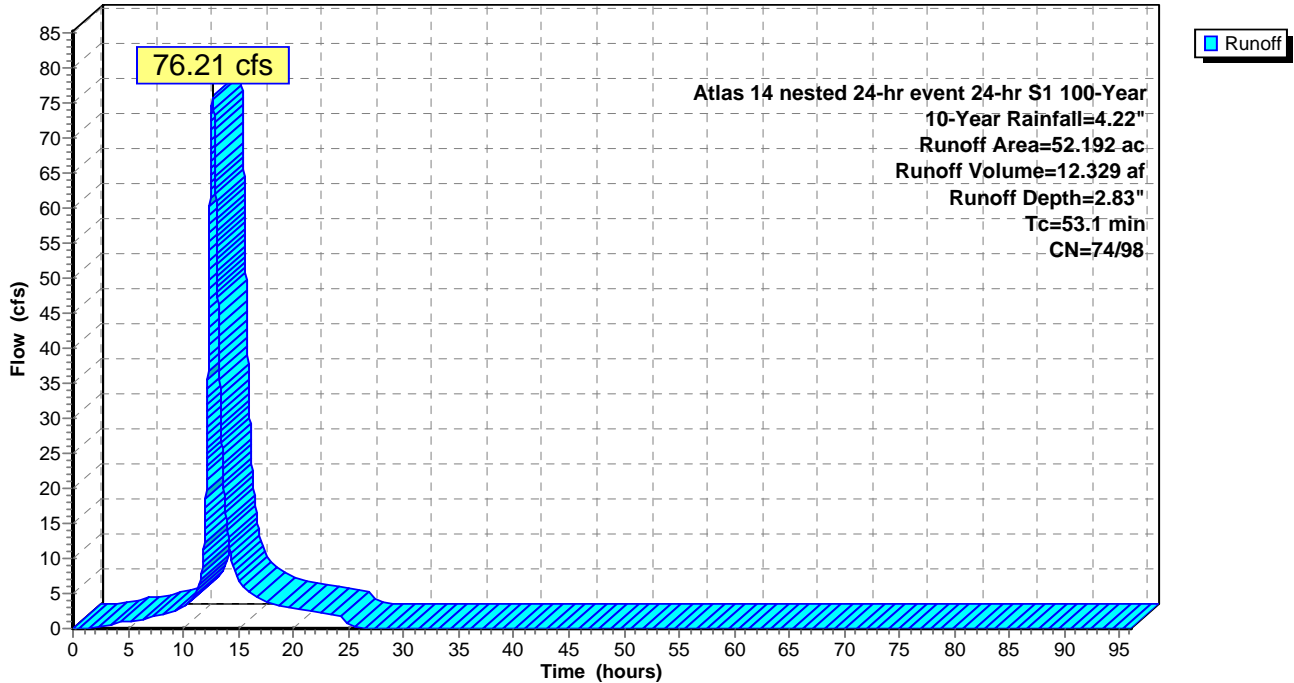
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 26.958	74	pervious
* 25.234	98	impervious
52.192	86	Weighted Average
26.958		51.65% Pervious Area
25.234		48.35% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
53.1					Direct Entry,

Subcatchment SB 1: SB 1

Hydrograph



Summary for Subcatchment SB 10: SB 10

Runoff = 16.44 cfs @ 12.06 hrs, Volume= 1.027 af, Depth= 1.93"

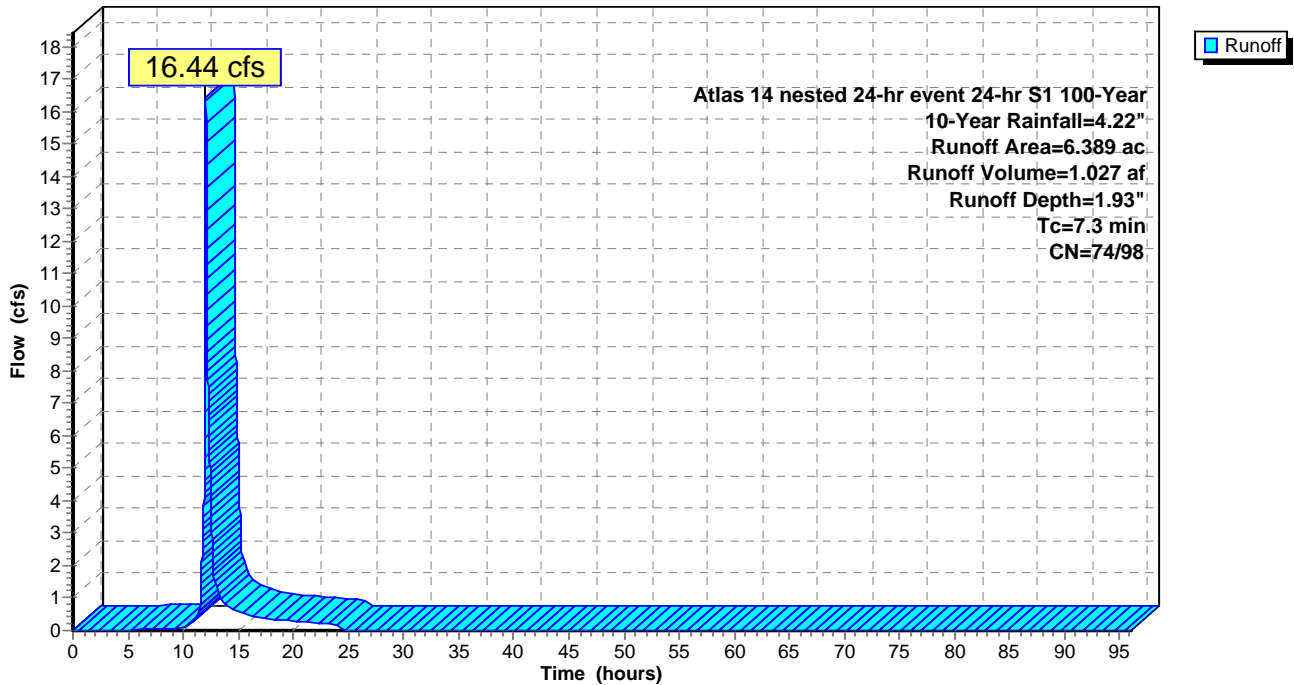
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 5.902	74	pervious
* 0.487	98	impervious
6.389	76	Weighted Average
5.902		92.38% Pervious Area
0.487		7.62% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.3					Direct Entry,

Subcatchment SB 10: SB 10

Hydrograph



Summary for Subcatchment SB 11: SB 11

Runoff = 8.82 cfs @ 12.11 hrs, Volume= 0.700 af, Depth= 2.55"

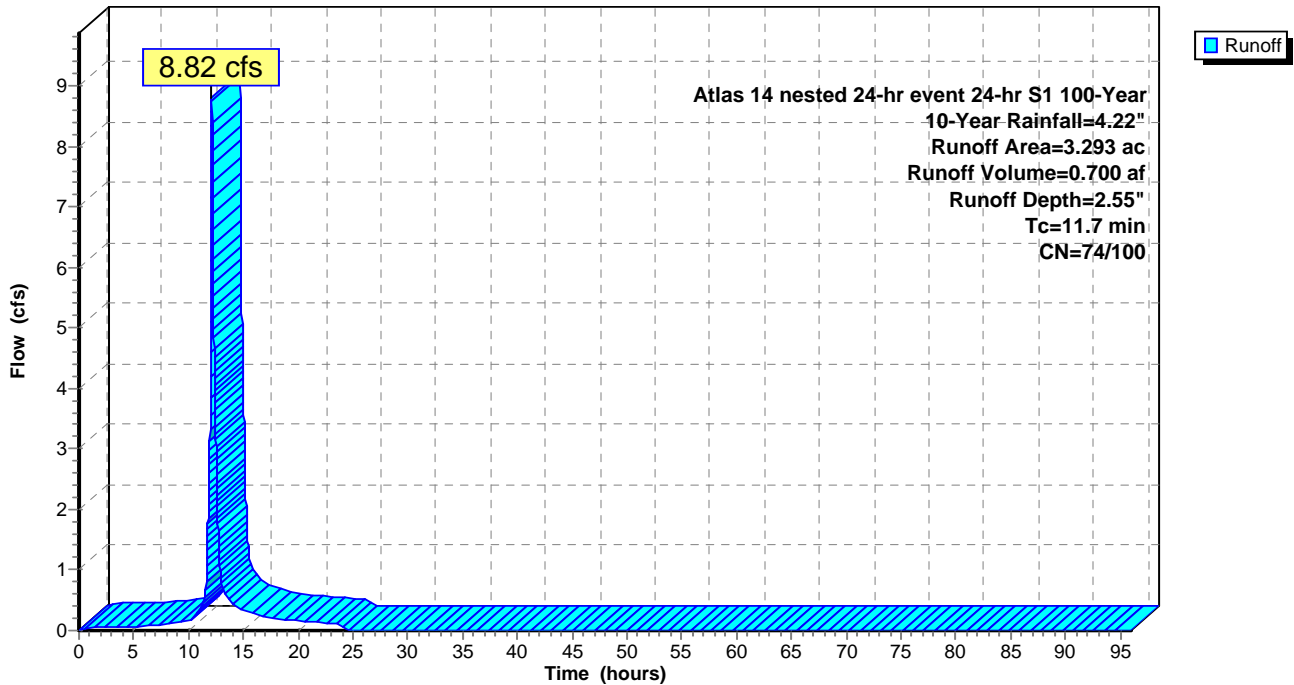
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 2.234	74	pervious
* 1.059	100	impervious
3.293	82	Weighted Average
2.234		67.84% Pervious Area
1.059		32.16% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.7					Direct Entry,

Subcatchment SB 11: SB 11

Hydrograph



Summary for Subcatchment SB 12: SB 12

Runoff = 4.23 cfs @ 12.08 hrs, Volume= 0.302 af, Depth= 2.62"

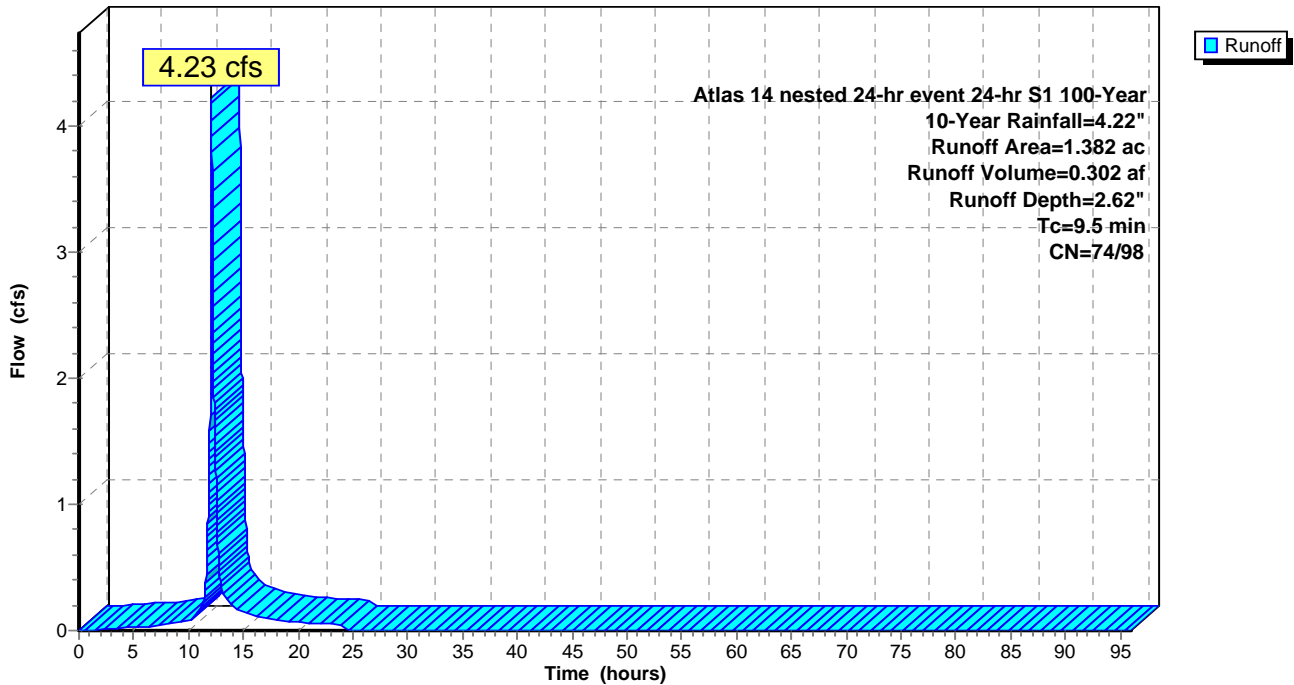
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 0.847	74	pervious
* 0.535	98	impervious
1.382	83	Weighted Average
0.847		61.29% Pervious Area
0.535		38.71% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.5					Direct Entry,

Subcatchment SB 12: SB 12

Hydrograph



Summary for Subcatchment SB 13: SB 13

Runoff = 8.67 cfs @ 12.08 hrs, Volume= 0.627 af, Depth= 2.52"

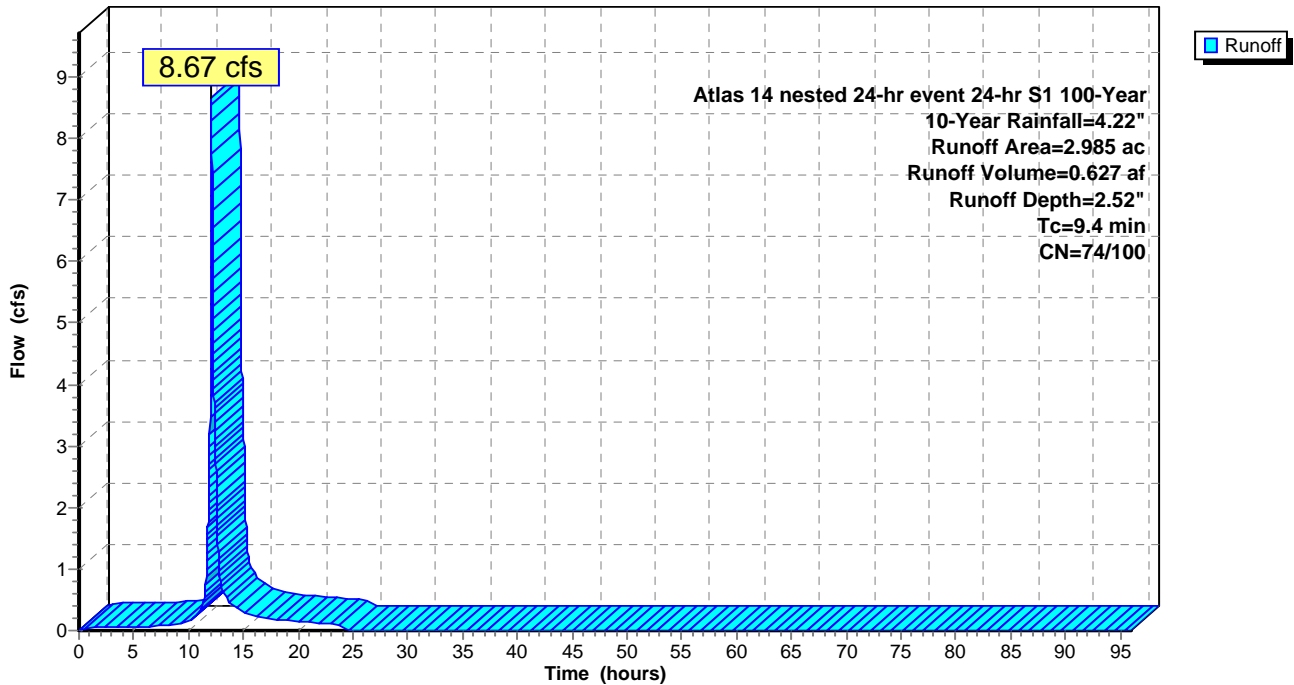
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 2.060	74	pervious
* 0.925	100	impervious
2.985	82	Weighted Average
2.060		69.01% Pervious Area
0.925		30.99% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.4					Direct Entry,

Subcatchment SB 13: SB 13

Hydrograph



Summary for Subcatchment SB 14: SB 14

Runoff = 41.19 cfs @ 12.02 hrs, Volume= 2.307 af, Depth= 2.71"

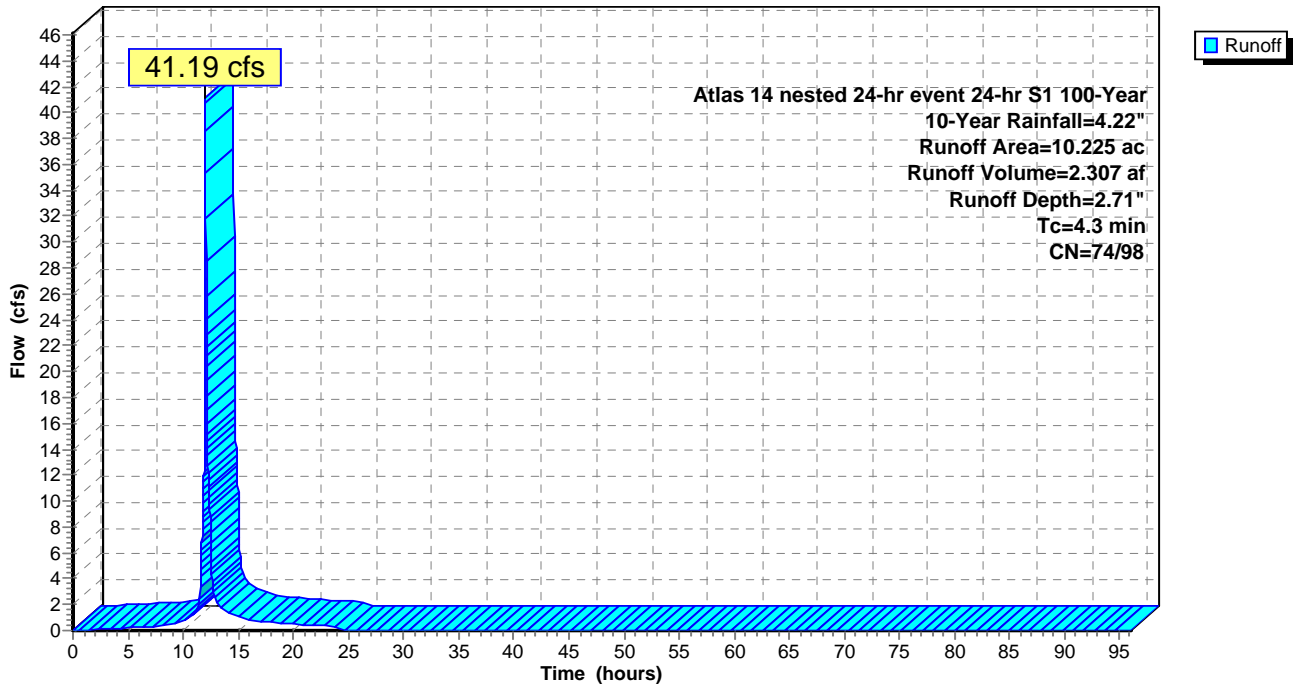
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 5.867	74	pervious
* 4.358	98	impervious
10.225	84	Weighted Average
5.867		57.38% Pervious Area
4.358		42.62% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.3					Direct Entry,

Subcatchment SB 14: SB 14

Hydrograph



Summary for Subcatchment SB 15: SB 15

Runoff = 112.62 cfs @ 12.38 hrs, Volume= 13.820 af, Depth= 2.83"

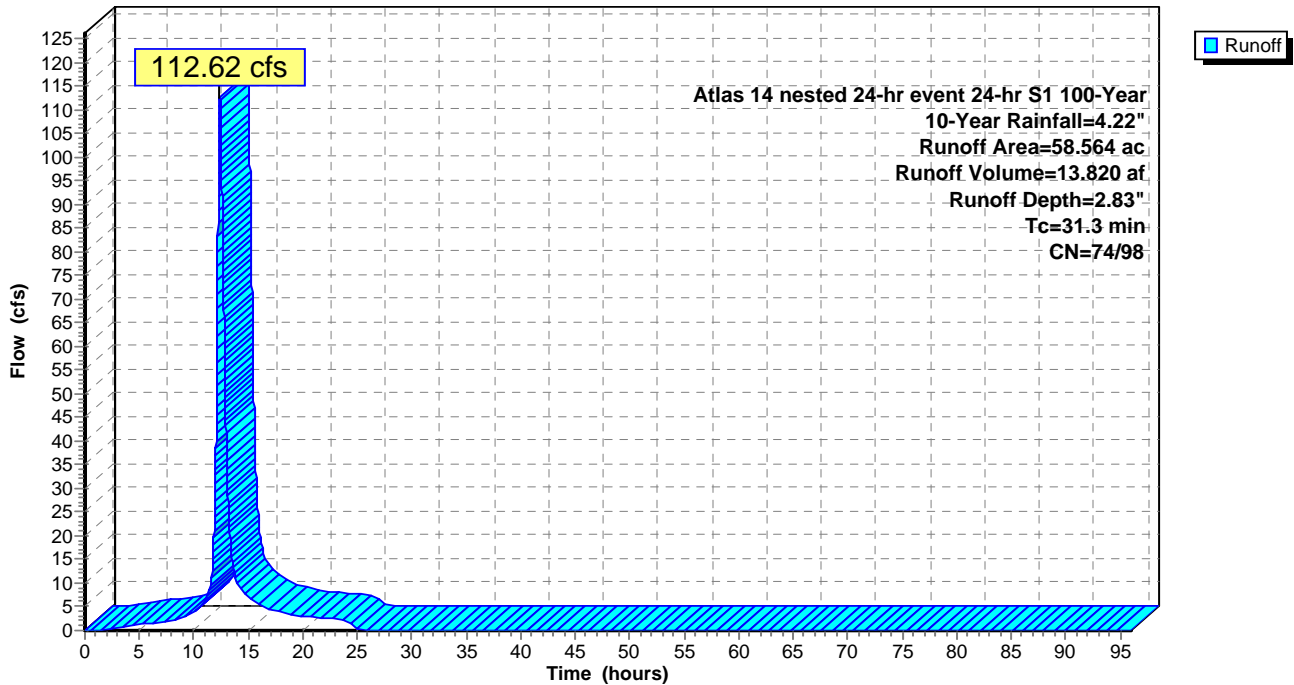
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 30.326	74	pervious
* 28.238	98	impervious
58.564	86	Weighted Average
30.326		51.78% Pervious Area
28.238		48.22% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
31.3					Direct Entry,

Subcatchment SB 15: SB 15

Hydrograph



Summary for Subcatchment SB 16: SB 16

Runoff = 86.24 cfs @ 12.12 hrs, Volume= 6.769 af, Depth= 2.50"

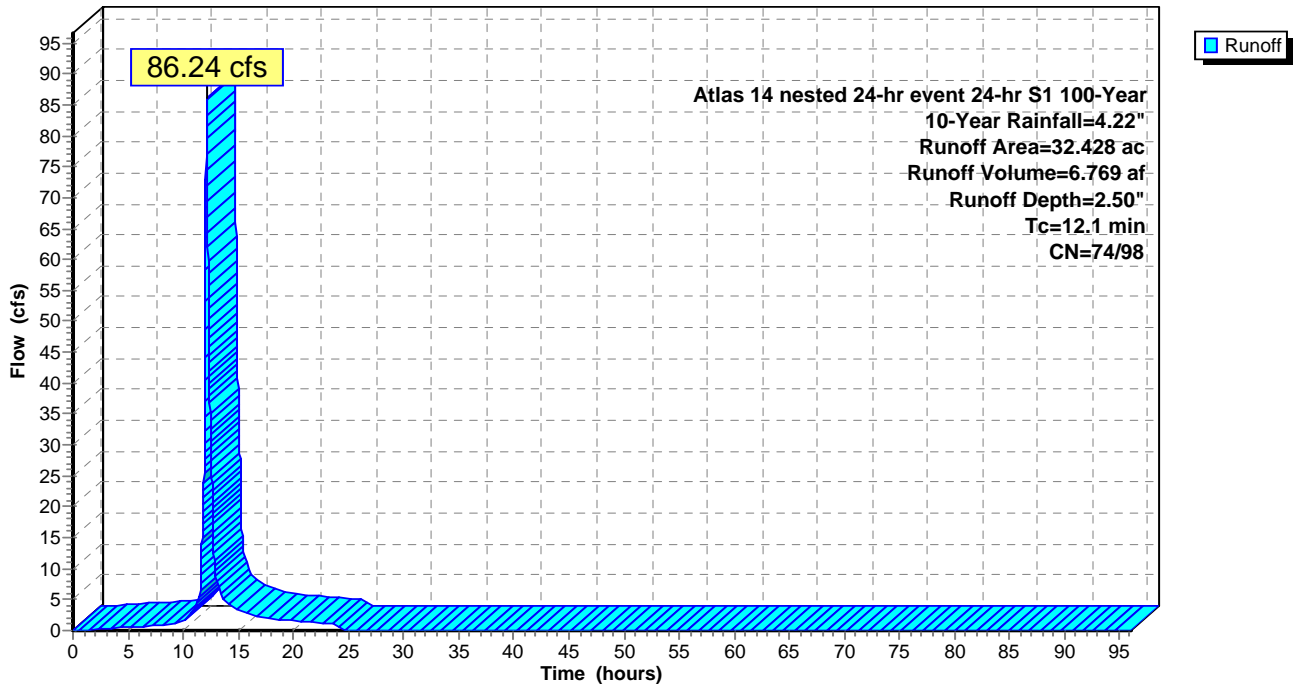
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 21.555	74	pervious
* 10.873	98	impervious
32.428	82	Weighted Average
21.555		66.47% Pervious Area
10.873		33.53% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.1					Direct Entry,

Subcatchment SB 16: SB 16

Hydrograph



Summary for Subcatchment SB 17: SB 17

Runoff = 32.13 cfs @ 12.02 hrs, Volume= 1.870 af, Depth= 2.95"

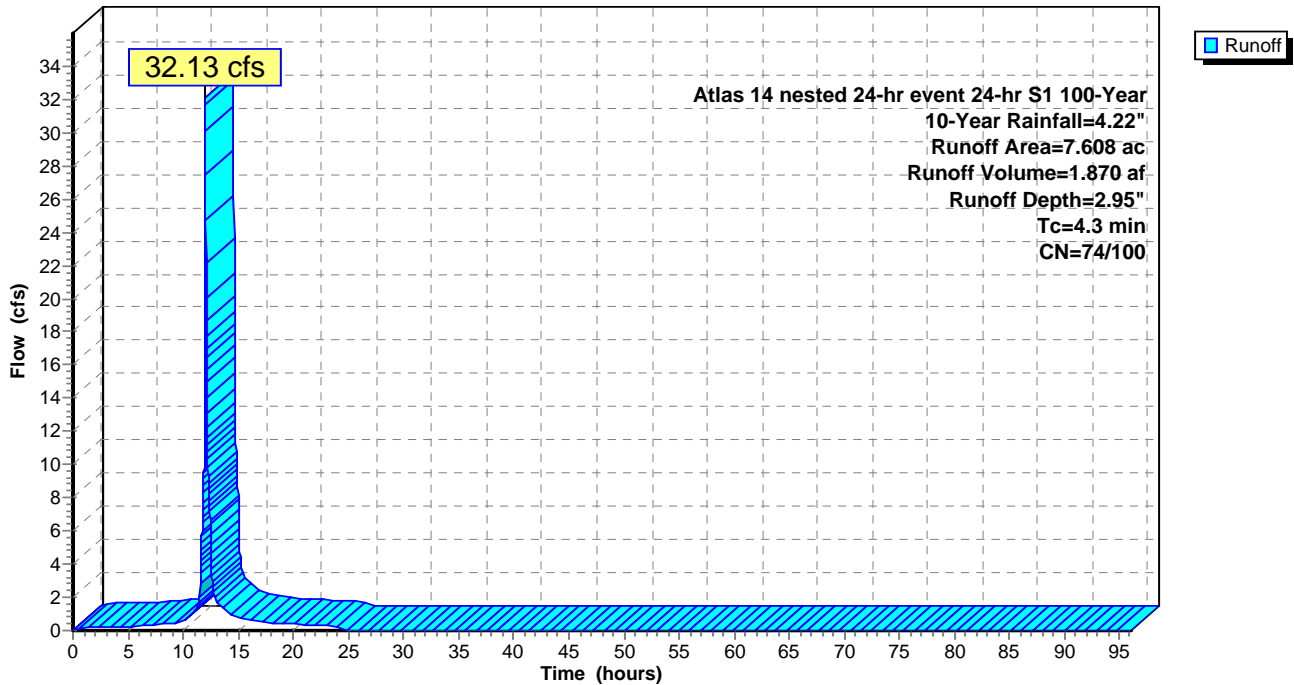
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 3.925	74	pervious
* 3.683	100	impervious
7.608	87	Weighted Average
3.925		51.59% Pervious Area
3.683		48.41% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.3					Direct Entry,

Subcatchment SB 17: SB 17

Hydrograph



Summary for Subcatchment SB 18: SB 18

Runoff = 124.38 cfs @ 12.40 hrs, Volume= 16.050 af, Depth= 3.64"

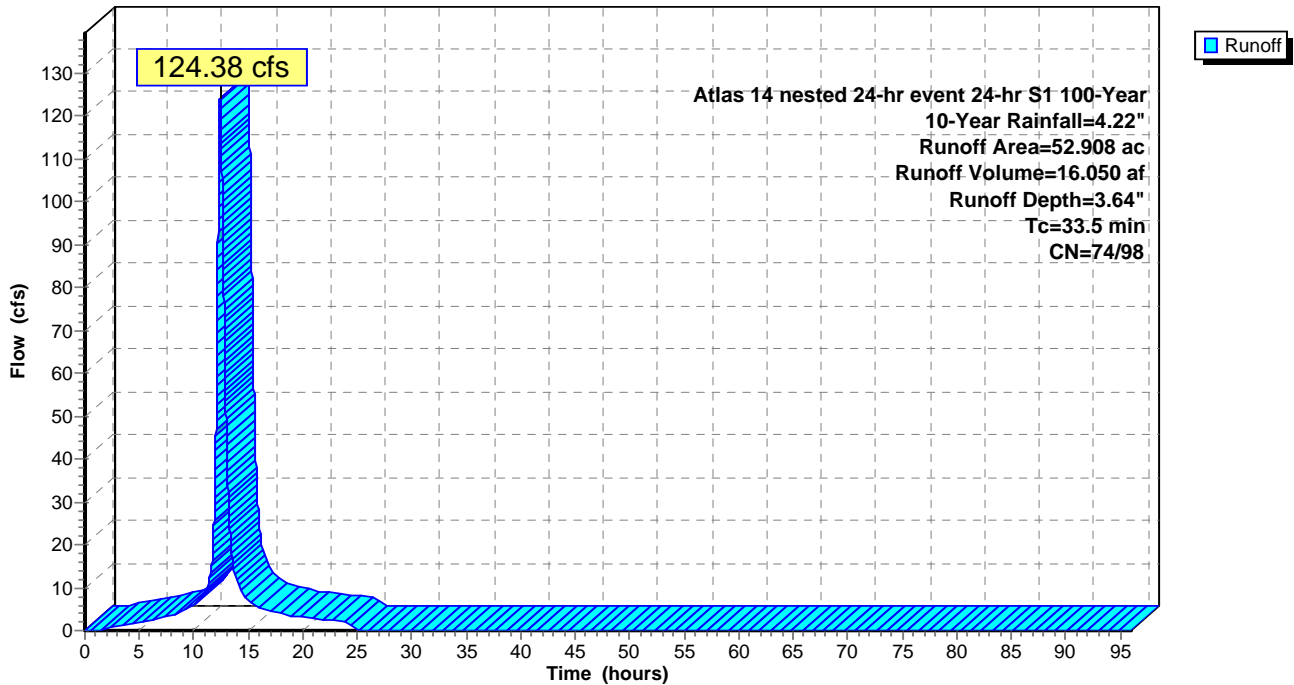
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 8.172	74	pervious
* 44.736	98	impervious
52.908	94	Weighted Average
8.172		15.45% Pervious Area
44.736		84.55% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
33.5					Direct Entry,

Subcatchment SB 18: SB 18

Hydrograph



Summary for Subcatchment SB 19: SB 19

Runoff = 42.96 cfs @ 12.30 hrs, Volume= 4.676 af, Depth= 2.65"

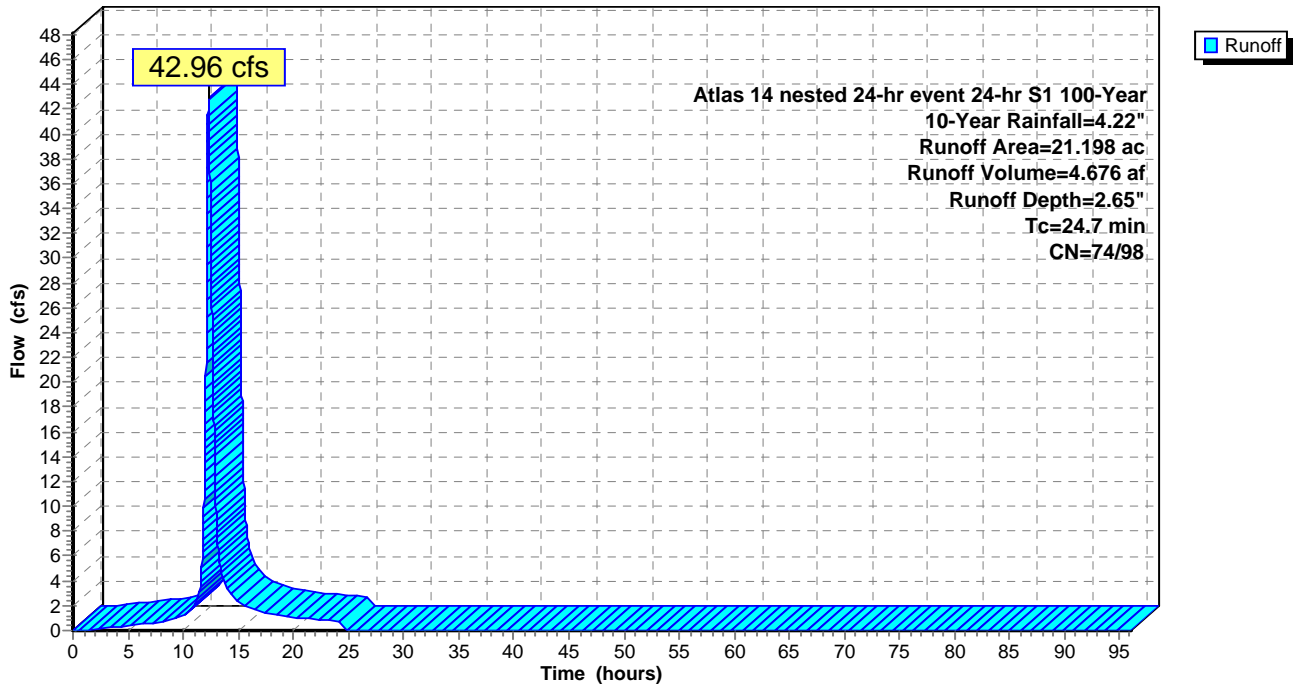
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 12.734	74	pervious
* 8.464	98	impervious
21.198	84	Weighted Average
12.734		60.07% Pervious Area
8.464		39.93% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.7					Direct Entry,

Subcatchment SB 19: SB 19

Hydrograph



Summary for Subcatchment SB 2: SB 2

Runoff = 37.33 cfs @ 12.17 hrs, Volume= 3.453 af, Depth= 3.63"

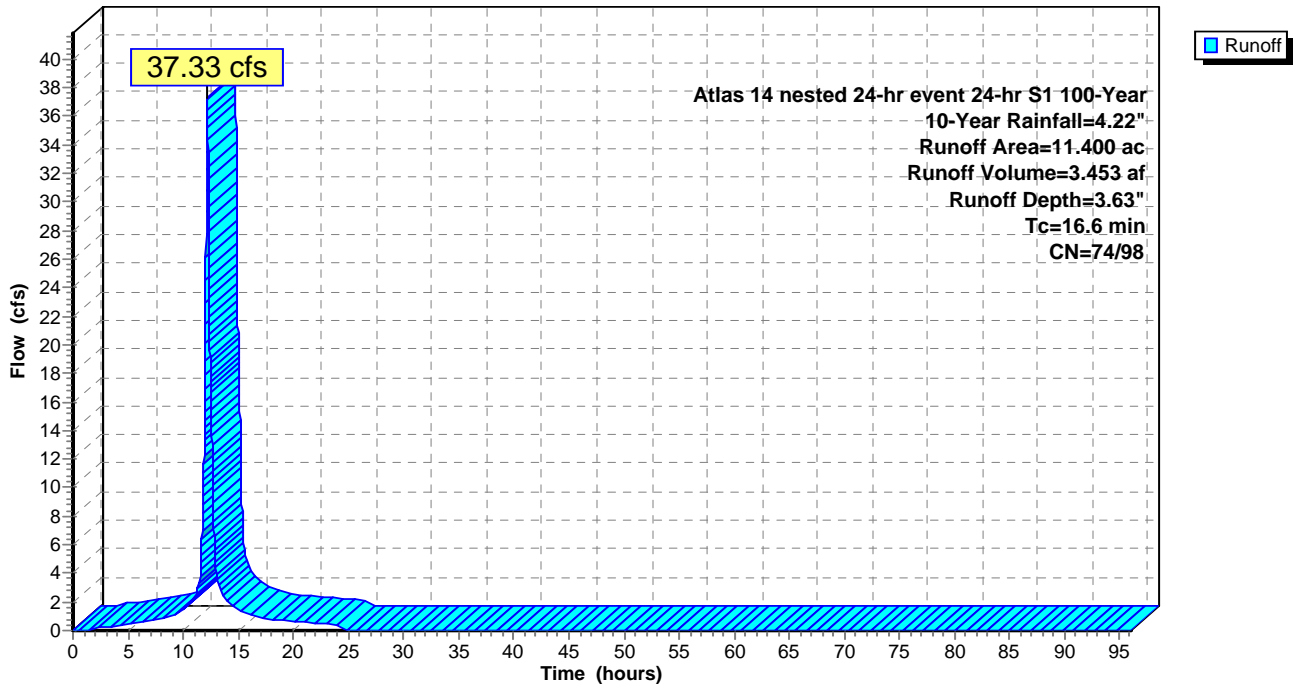
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 1.791	74	pervious
* 9.609	98	impervious
11.400	94	Weighted Average
1.791		15.71% Pervious Area
9.609		84.29% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.6					Direct Entry,

Subcatchment SB 2: SB 2

Hydrograph



Summary for Subcatchment SB 22: SB 22

Runoff = 79.81 cfs @ 12.52 hrs, Volume= 11.662 af, Depth= 3.34"

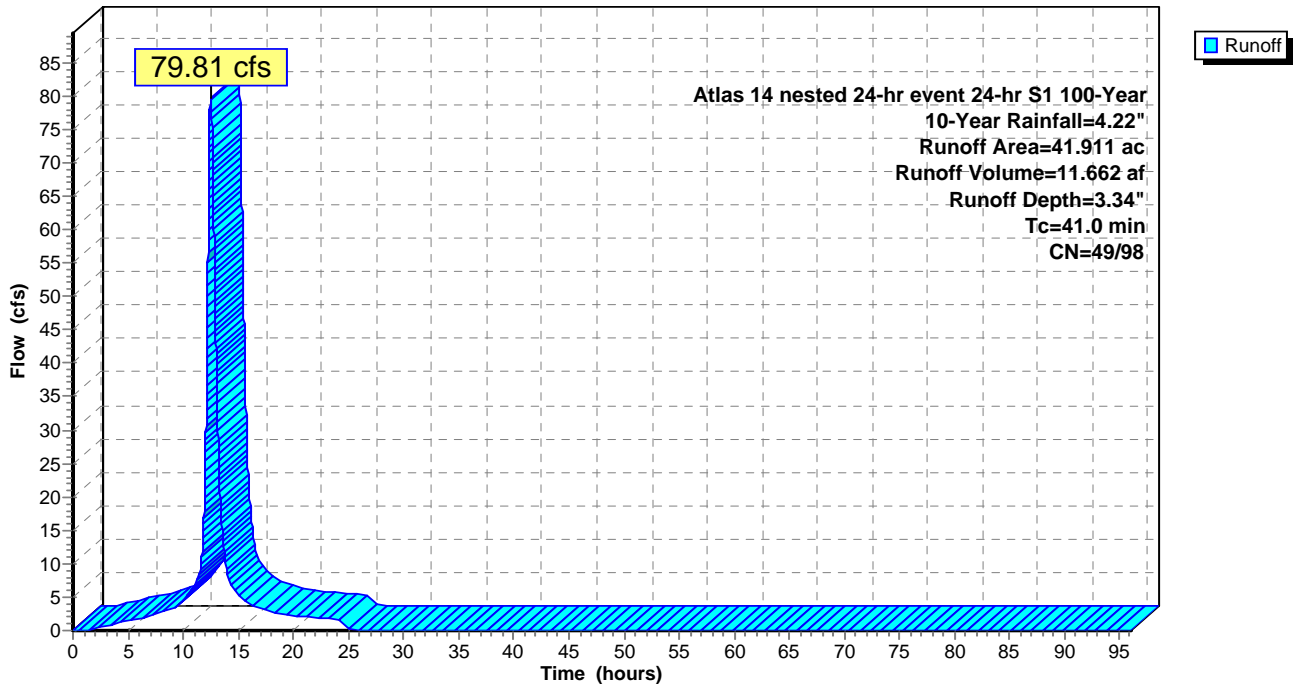
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 7.465	49	Pervious
* 34.446	98	Impervious
41.911	89	Weighted Average
7.465		17.81% Pervious Area
34.446		82.19% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
41.0					Direct Entry,

Subcatchment SB 22: SB 22

Hydrograph



Summary for Subcatchment SB 24: SB 24

Runoff = 24.36 cfs @ 12.05 hrs, Volume= 1.623 af, Depth= 3.94"

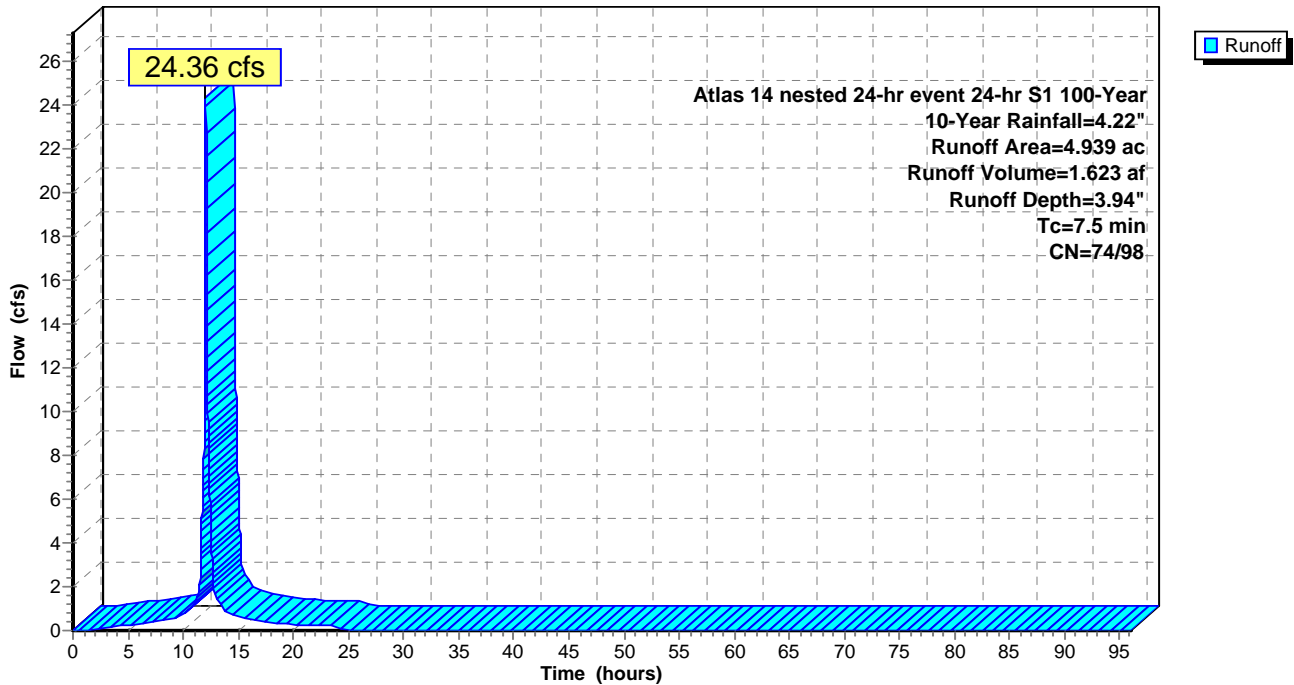
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 0.088	74	permiabile
* 4.851	98	impermiabile
4.939	98	Weighted Average
0.088		1.78% Pervious Area
4.851		98.22% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.5					Direct Entry,

Subcatchment SB 24: SB 24

Hydrograph



Summary for Subcatchment SB 25: SB 25

Runoff = 21.17 cfs @ 12.09 hrs, Volume= 1.624 af, Depth= 3.89"

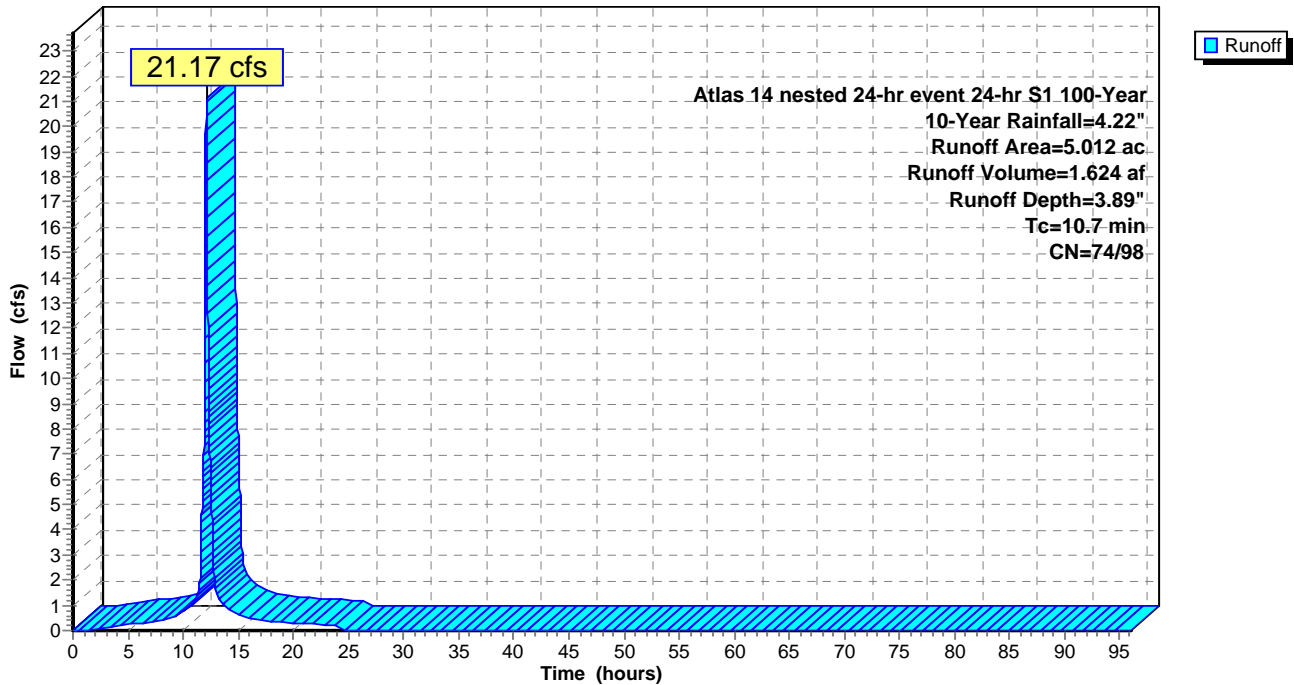
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 0.215	74	pervious
* 4.797	98	impervious
5.012	97	Weighted Average
0.215		4.29% Pervious Area
4.797		95.71% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7					Direct Entry,

Subcatchment SB 25: SB 25

Hydrograph



Summary for Subcatchment SB 26: SB 26

Runoff = 41.75 cfs @ 12.28 hrs, Volume= 4.713 af, Depth= 3.95"

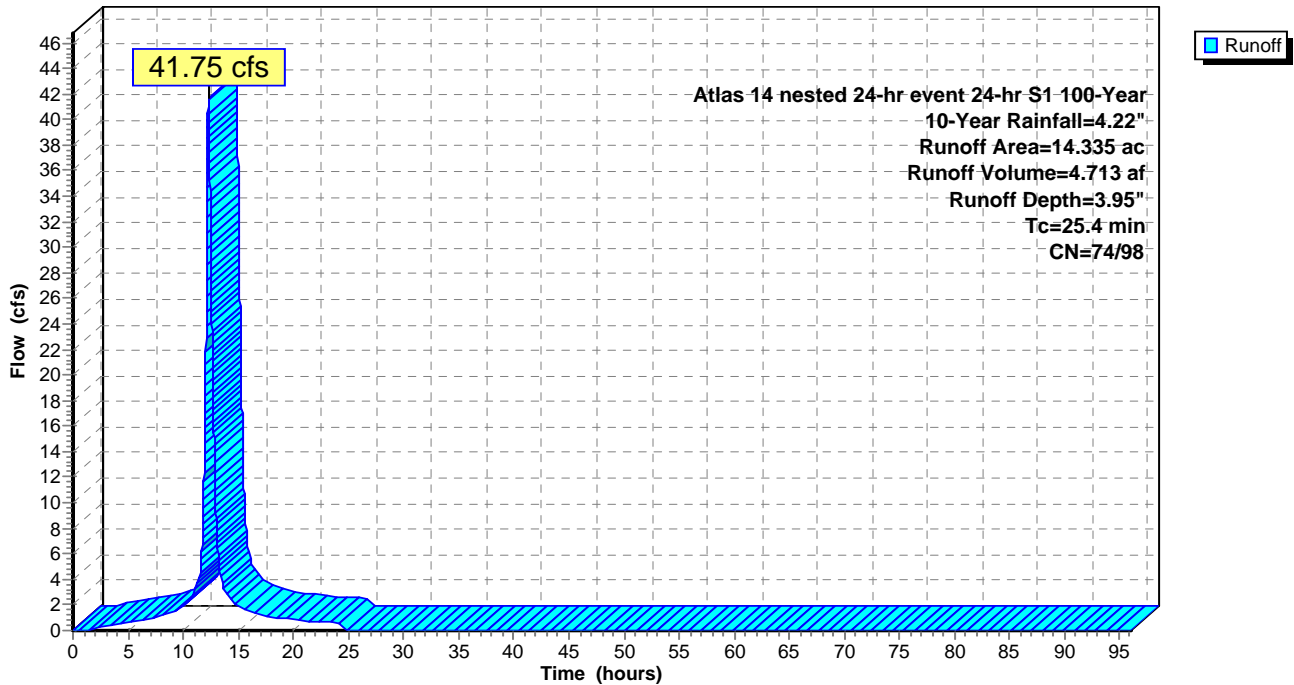
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 0.248	74	pervious
* 14.087	98	impervious
14.335	98	Weighted Average
0.248		1.73% Pervious Area
14.087		98.27% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
25.4					Direct Entry,

Subcatchment SB 26: SB 26

Hydrograph



Summary for Subcatchment SB 27: SB 27 (Thumb Road)

Runoff = 18.18 cfs @ 12.31 hrs, Volume= 2.143 af, Depth= 3.88"

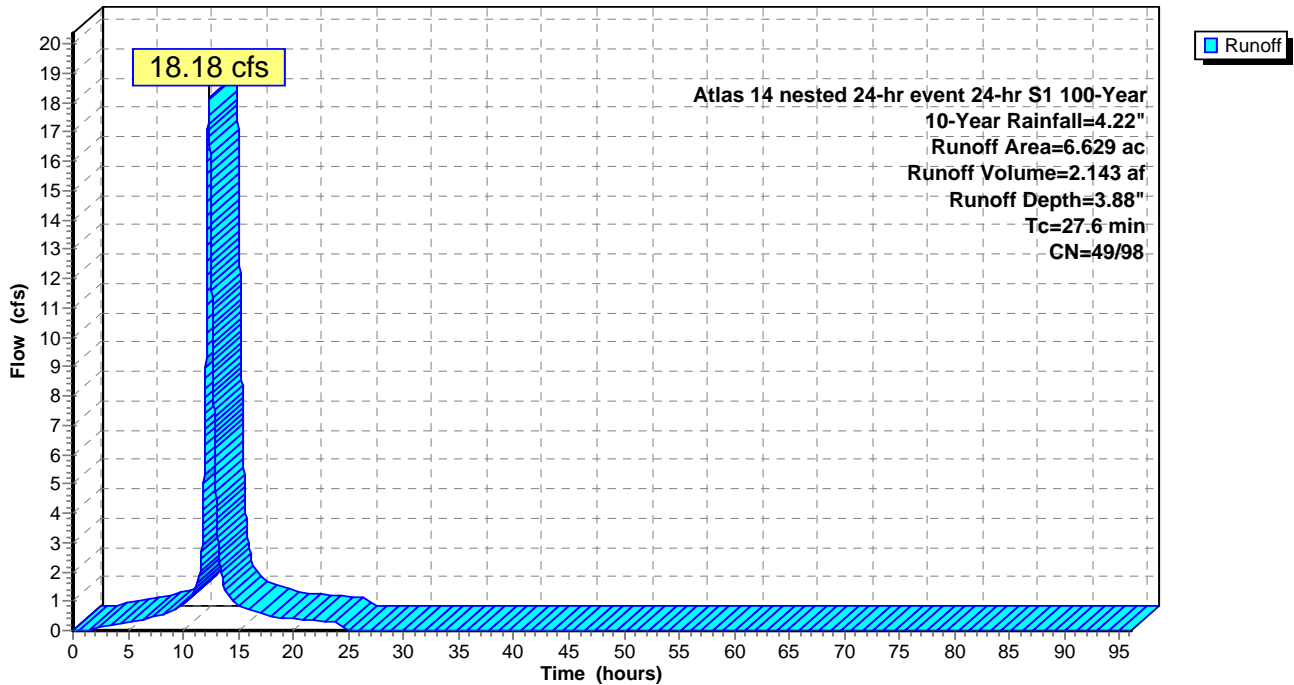
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 0.191	49	Pervious
* 6.438	98	Impervious
6.629	97	Weighted Average
0.191		2.88% Pervious Area
6.438		97.12% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.6					Direct Entry,

Subcatchment SB 27: SB 27 (Thumb Road)

Hydrograph



Summary for Subcatchment SB 28: SB 28

Runoff = 18.97 cfs @ 12.15 hrs, Volume= 1.622 af, Depth= 2.80"

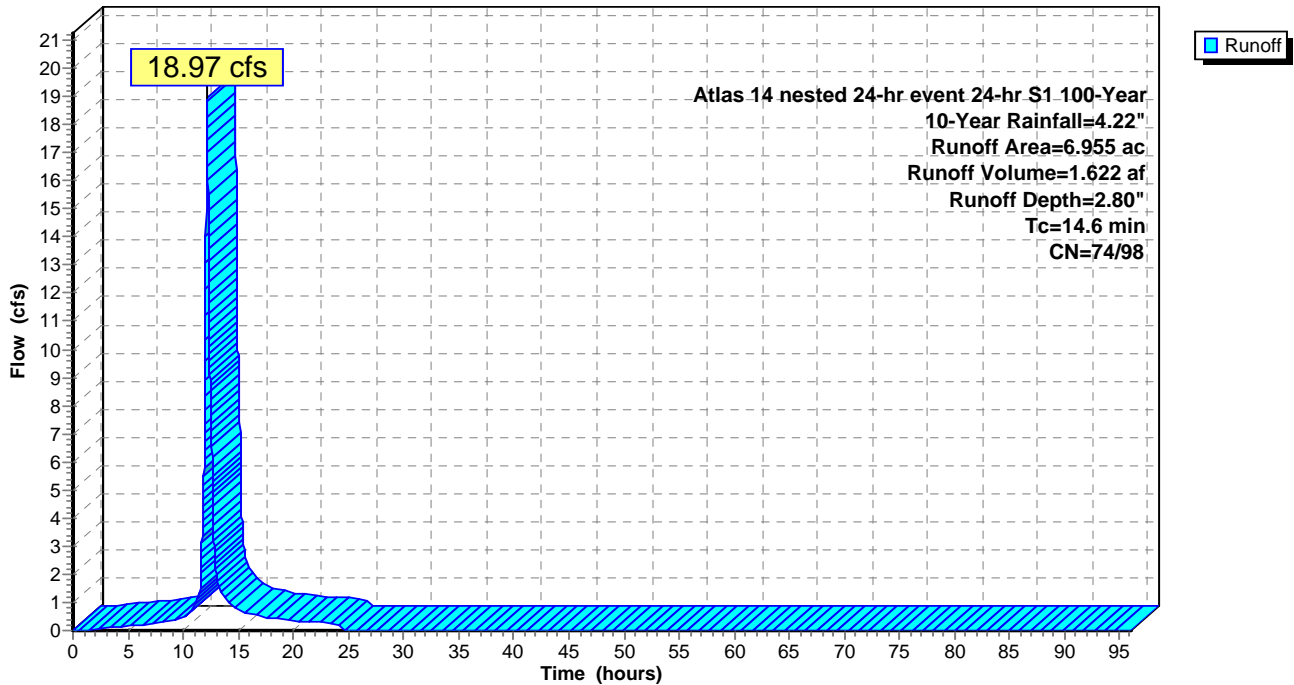
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 3.703	74	pervious
* 3.252	98	impervious
6.955	85	Weighted Average
3.703		53.24% Pervious Area
3.252		46.76% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.6					Direct Entry,

Subcatchment SB 28: SB 28

Hydrograph



Summary for Subcatchment SB 29: SB 29

Runoff = 23.01 cfs @ 12.22 hrs, Volume= 2.212 af, Depth= 2.60"

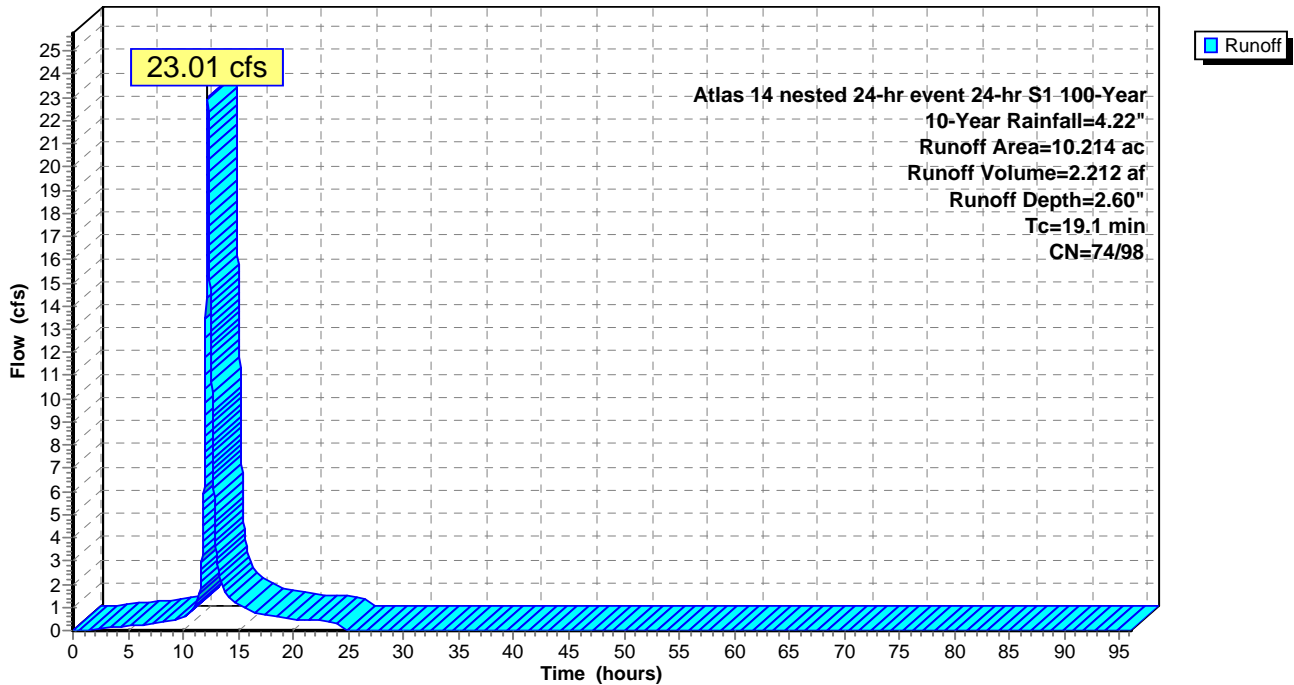
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 6.360	74	pervious
* 3.854	98	impervious
10.214	83	Weighted Average
6.360		62.27% Pervious Area
3.854		37.73% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.1					Direct Entry,

Subcatchment SB 29: SB 29

Hydrograph



Summary for Subcatchment SB 3: SB 3

Runoff = 96.96 cfs @ 12.16 hrs, Volume= 8.417 af, Depth= 2.68"

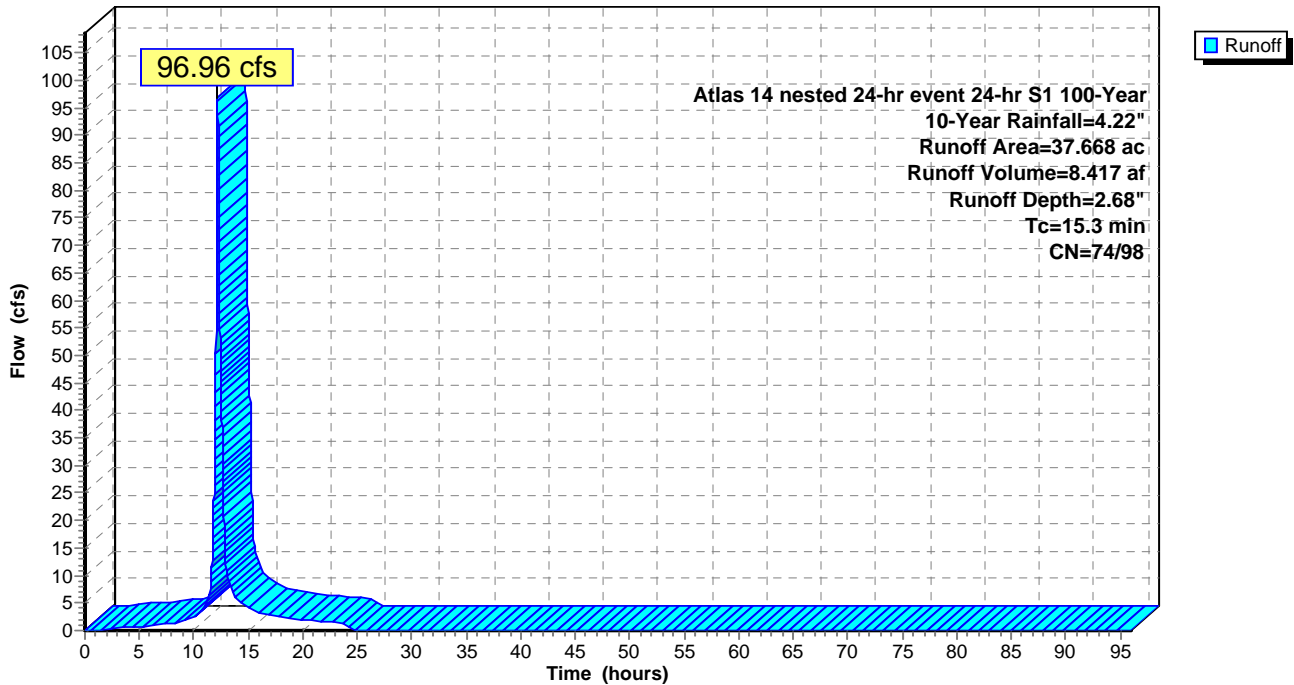
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 22.050	74	Pervious
* 15.618	98	Impervious
37.668	84	Weighted Average
22.050		58.54% Pervious Area
15.618		41.46% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.3					Direct Entry,

Subcatchment SB 3: SB 3

Hydrograph



Summary for Subcatchment SB 4: SB 4

Runoff = 1.86 cfs @ 12.04 hrs, Volume= 0.112 af, Depth= 2.24"

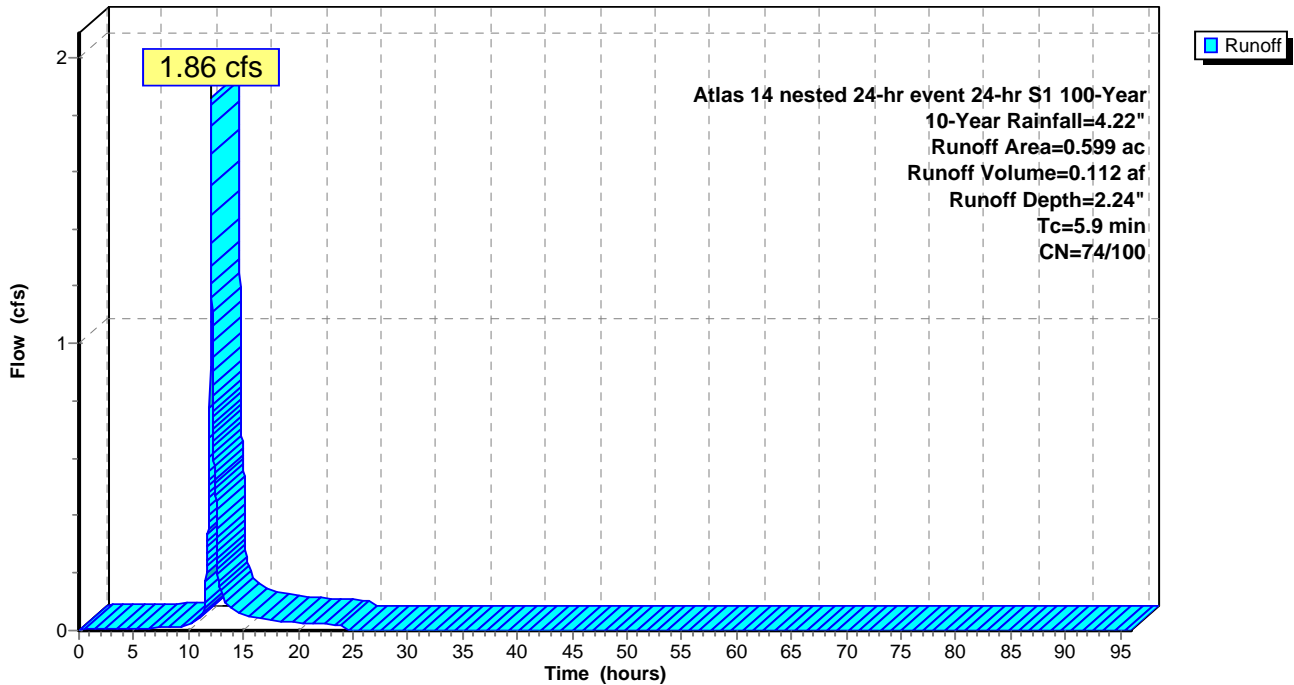
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 0.481	74	pervious
* 0.118	100	impervious
0.599	79	Weighted Average
0.481		80.30% Pervious Area
0.118		19.70% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.9					Direct Entry,

Subcatchment SB 4: SB 4

Hydrograph



Summary for Subcatchment SB 5: SB 5

Runoff = 12.43 cfs @ 12.72 hrs, Volume= 2.176 af, Depth= 3.32"

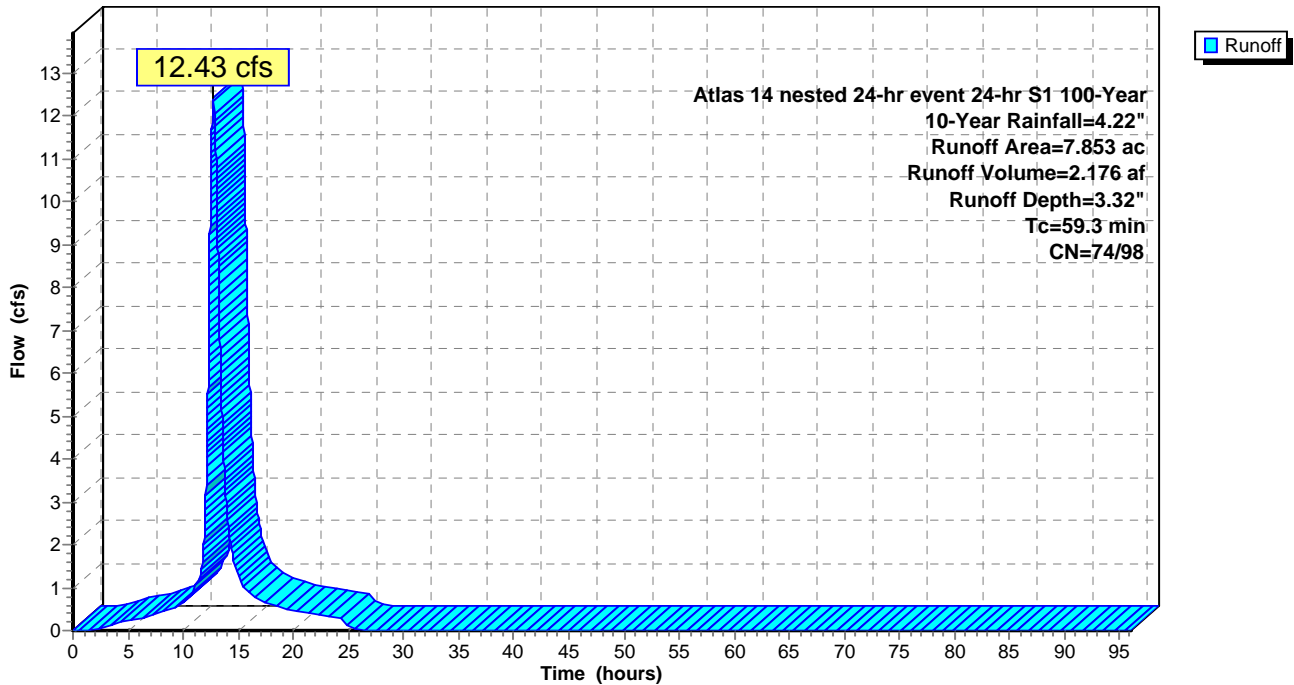
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 2.327	74	pervious
* 5.526	98	impervious
7.853	91	Weighted Average
2.327		29.63% Pervious Area
5.526		70.37% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
59.3					Direct Entry,

Subcatchment SB 5: SB 5

Hydrograph



Summary for Subcatchment SB 51: Offsite Subbasin 51

Runoff = 27.74 cfs @ 12.43 hrs, Volume= 3.622 af, Depth= 1.72"

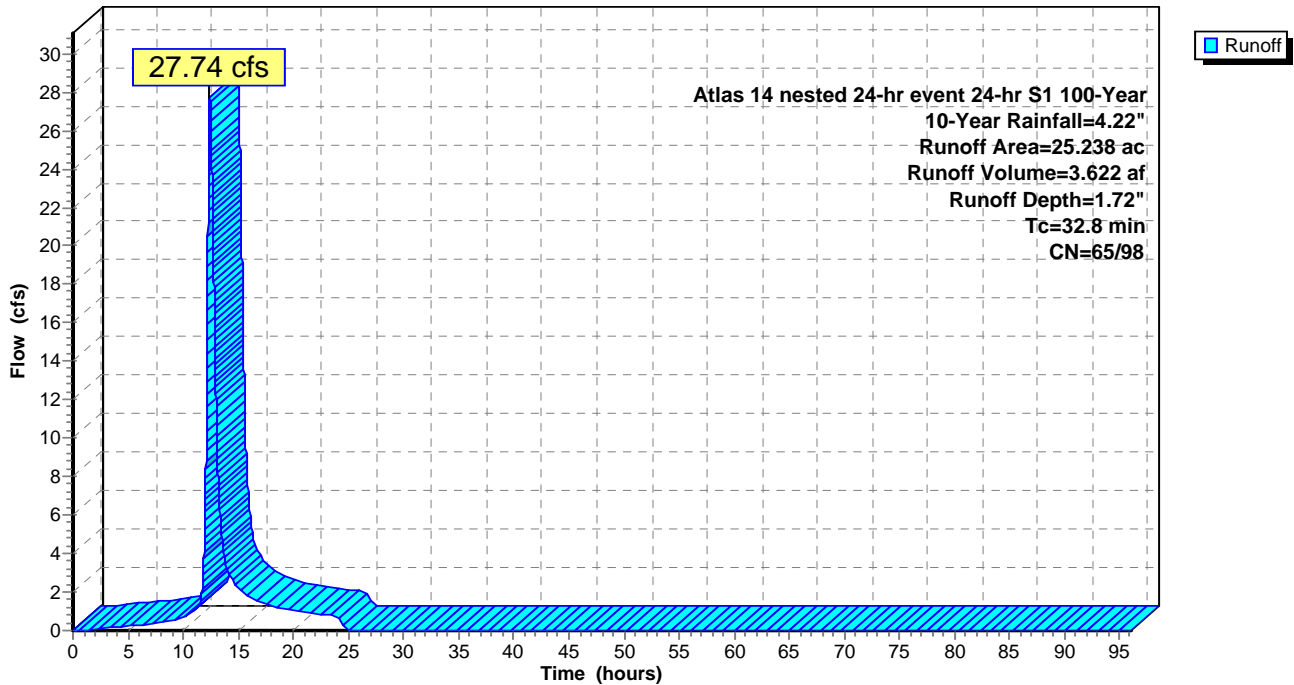
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 20.200	65	Offsite subbasin 51
* 5.038	98	
25.238	72	Weighted Average
20.200		80.04% Pervious Area
5.038		19.96% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.8					Direct Entry,

Subcatchment SB 51: Offsite Subbasin 51

Hydrograph



Summary for Subcatchment SB 6: SB 6

Runoff = 1.96 cfs @ 12.24 hrs, Volume= 0.196 af, Depth= 2.36"

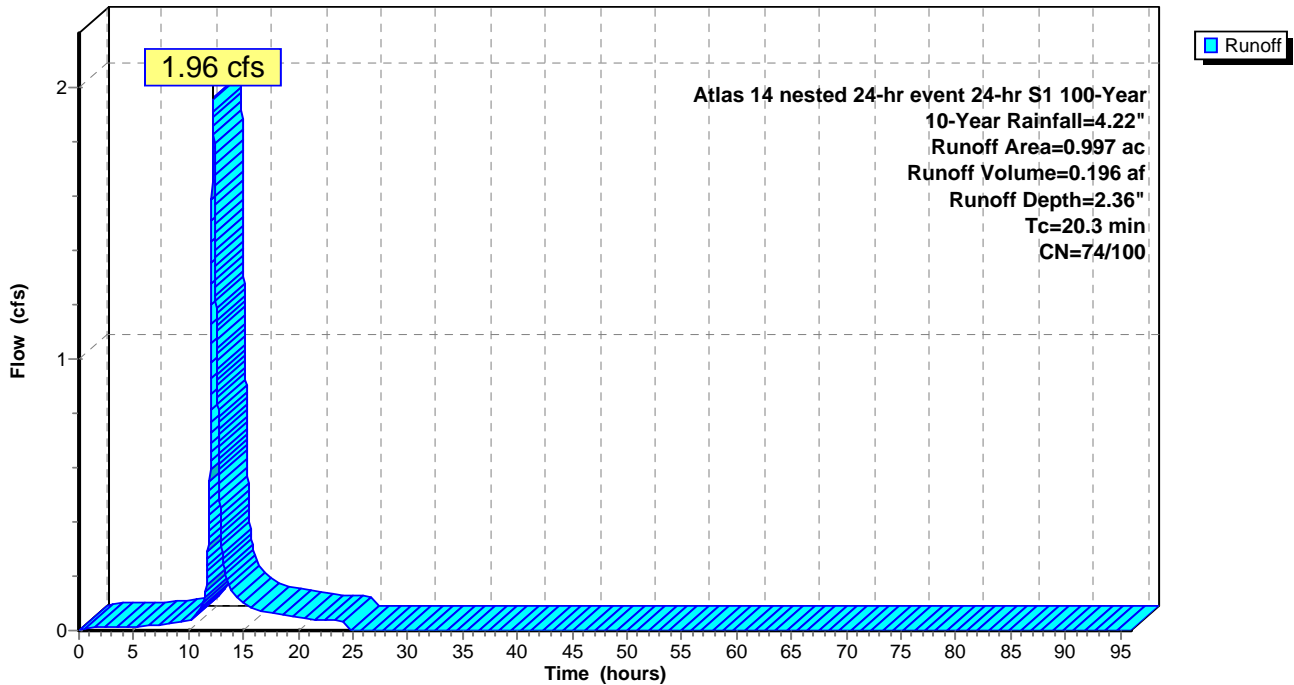
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 0.753	74	pervious
* 0.244	100	impervious
0.997	80	Weighted Average
0.753		75.53% Pervious Area
0.244		24.47% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.3					Direct Entry,

Subcatchment SB 6: SB 6

Hydrograph



Summary for Subcatchment SB 7: SB 7

Runoff = 106.75 cfs @ 12.03 hrs, Volume= 6.550 af, Depth= 3.65"

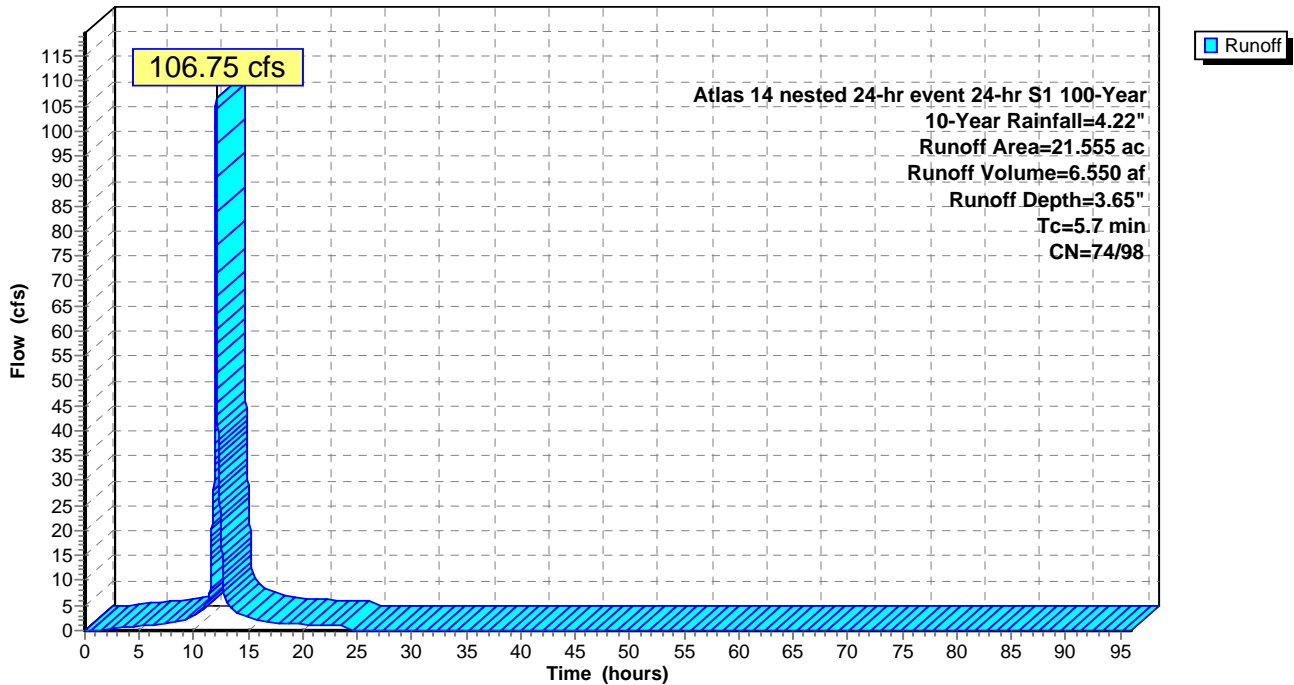
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 3.269	74	pervious
* 18.286	98	impervious
21.555	94	Weighted Average
3.269		15.17% Pervious Area
18.286		84.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.7					Direct Entry,

Subcatchment SB 7: SB 7

Hydrograph



Summary for Subcatchment SB 8: SB 8

Runoff = 39.95 cfs @ 12.61 hrs, Volume= 5.985 af, Depth= 2.43"

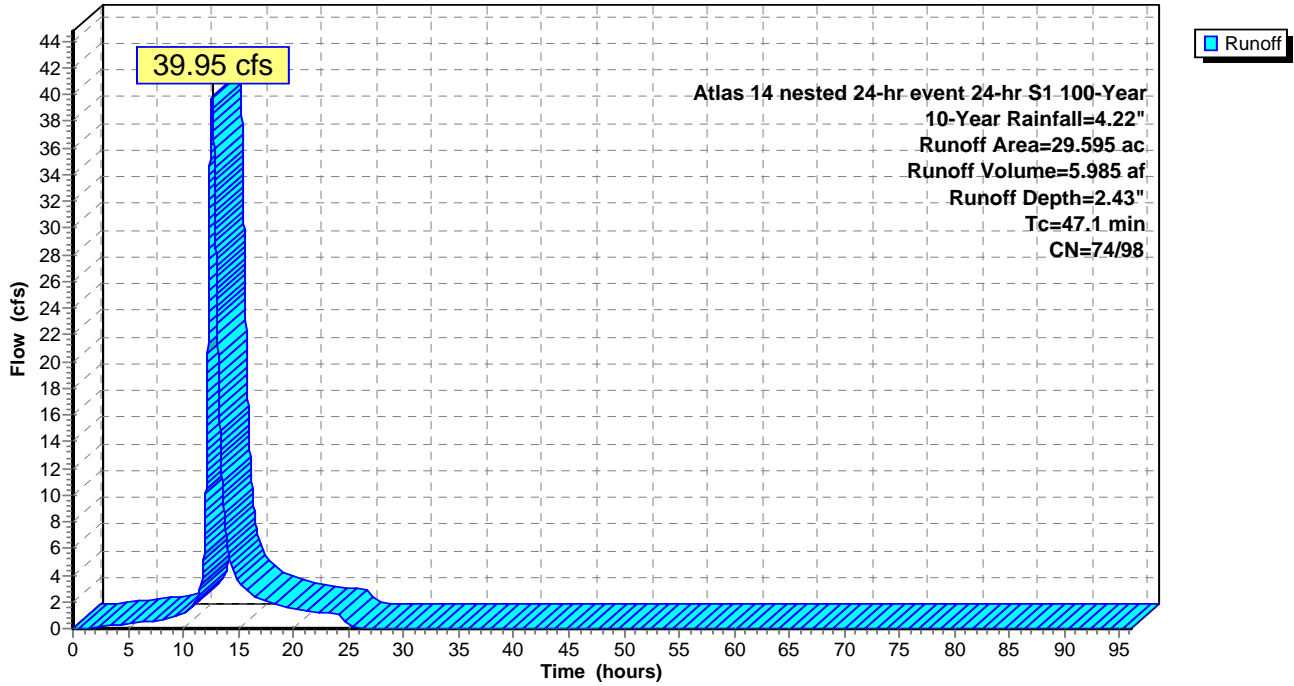
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 20.714	74	pervious
* 8.881	98	impervious
29.595	81	Weighted Average
20.714		69.99% Pervious Area
8.881		30.01% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
47.1					Direct Entry,

Subcatchment SB 8: SB 8

Hydrograph



Summary for Subcatchment SB 9: SB 9

Runoff = 45.10 cfs @ 12.37 hrs, Volume= 5.366 af, Depth= 2.50"

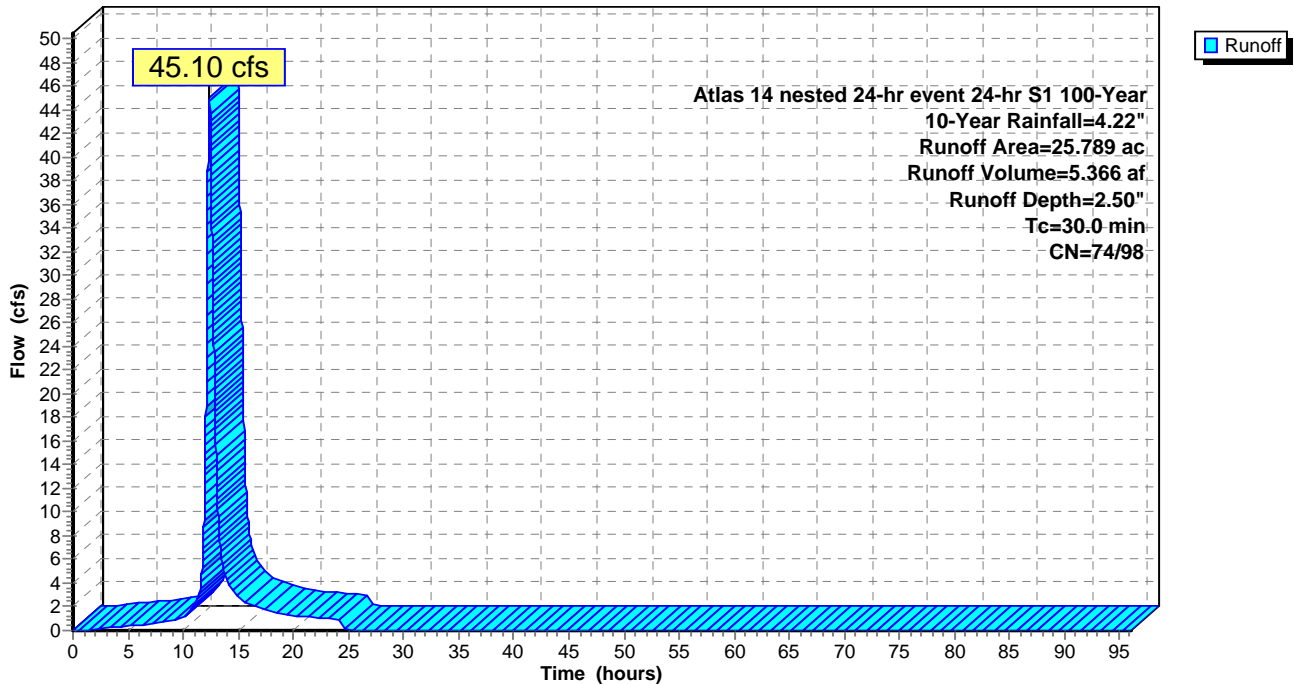
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year 10-Year Rainfall=4.22"

Area (ac)	CN	Description
* 17.234	74	permiabile
* 8.555	98	impermiabile
25.789	82	Weighted Average
17.234		66.83% Pervious Area
8.555		33.17% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.0					Direct Entry,

Subcatchment SB 9: SB 9

Hydrograph



Summary for Pond 3P: P-3

Inflow Area = 133.365 ac, 58.87% Impervious, Inflow Depth = 2.78" for 10-Year event
 Inflow = 146.54 cfs @ 12.04 hrs, Volume= 30.858 af
 Outflow = 98.88 cfs @ 13.04 hrs, Volume= 30.840 af, Atten= 33%, Lag= 60.0 min
 Primary = 98.88 cfs @ 13.04 hrs, Volume= 30.840 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 914.00' Surf.Area= 1.790 ac Storage= 5.827 af
 Peak Elev= 917.66' @ 13.04 hrs Surf.Area= 2.510 ac Storage= 13.758 af (7.931 af above start)

Plug-Flow detention time= 319.6 min calculated for 25.011 af (81% of inflow)
 Center-of-Mass det. time= 134.6 min (1,014.4 - 879.8)

Volume	Invert	Avail.Storage	Storage Description
#1	909.85'	20.423 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
909.85	1.130	0.000	0.000
912.00	1.360	2.677	2.677
916.00	2.220	7.160	9.837
918.00	2.570	4.790	14.627
920.10	2.950	5.796	20.423

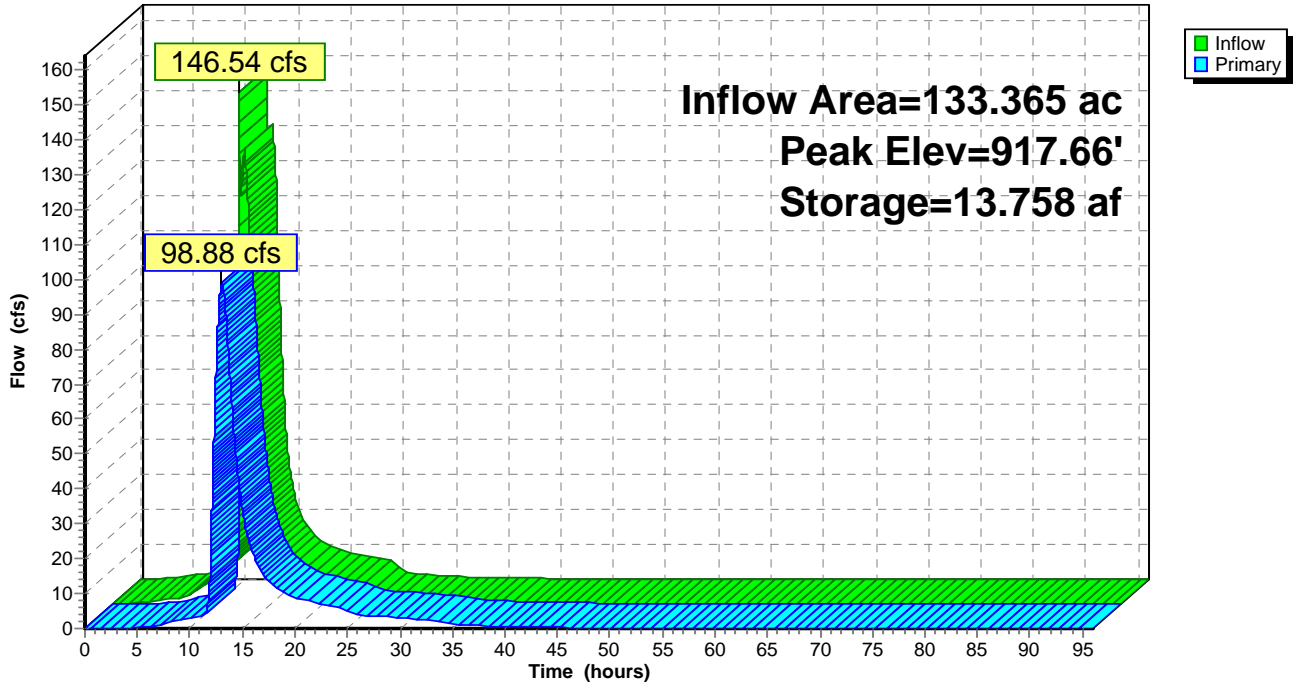
Device	Routing	Invert	Outlet Devices
#1	Primary	914.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	918.25'	10.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#3	Primary	915.00'	7.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)

Primary OutFlow Max=98.88 cfs @ 13.04 hrs HW=917.66' TW=0.00' (Dynamic Tailwater)

- 1=Orifice/Grate (Orifice Controls 7.23 cfs @ 9.21 fps)
- 2=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)
- 3=Sharp-Crested Rectangular Weir (Weir Controls 91.65 cfs @ 5.33 fps)

Pond 3P: P-3

Hydrograph



Summary for Pond 4P: P-4

Inflow Area = 7.853 ac, 70.37% Impervious, Inflow Depth = 3.32" for 10-Year event
 Inflow = 12.43 cfs @ 12.72 hrs, Volume= 2.176 af
 Outflow = 7.25 cfs @ 13.29 hrs, Volume= 2.176 af, Atten= 42%, Lag= 33.8 min
 Primary = 4.47 cfs @ 13.29 hrs, Volume= 0.953 af
 Secondary = 2.78 cfs @ 13.26 hrs, Volume= 1.223 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 915.00' Surf.Area= 0.275 ac Storage= 0.646 af
 Peak Elev= 916.82' @ 13.29 hrs Surf.Area= 0.379 ac Storage= 1.241 af (0.595 af above start)

Plug-Flow detention time= 243.0 min calculated for 1.530 af (70% of inflow)
 Center-of-Mass det. time= 58.7 min (873.0 - 814.3)

Volume	Invert	Avail.Storage	Storage Description
#1	910.90'	1.728 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
910.90	0.070	0.000	0.000
912.00	0.090	0.088	0.088
914.00	0.220	0.310	0.398
916.00	0.330	0.550	0.948
918.00	0.450	0.780	1.728

Device	Routing	Invert	Outlet Devices
#1	Primary	915.00'	6.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Secondary	915.00'	9.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	915.95'	24.0" Round RCP_Round 24" L= 50.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 915.80' / 915.95' S= -0.0030 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Primary OutFlow Max=4.47 cfs @ 13.29 hrs HW=916.82' TW=0.00' (Dynamic Tailwater)

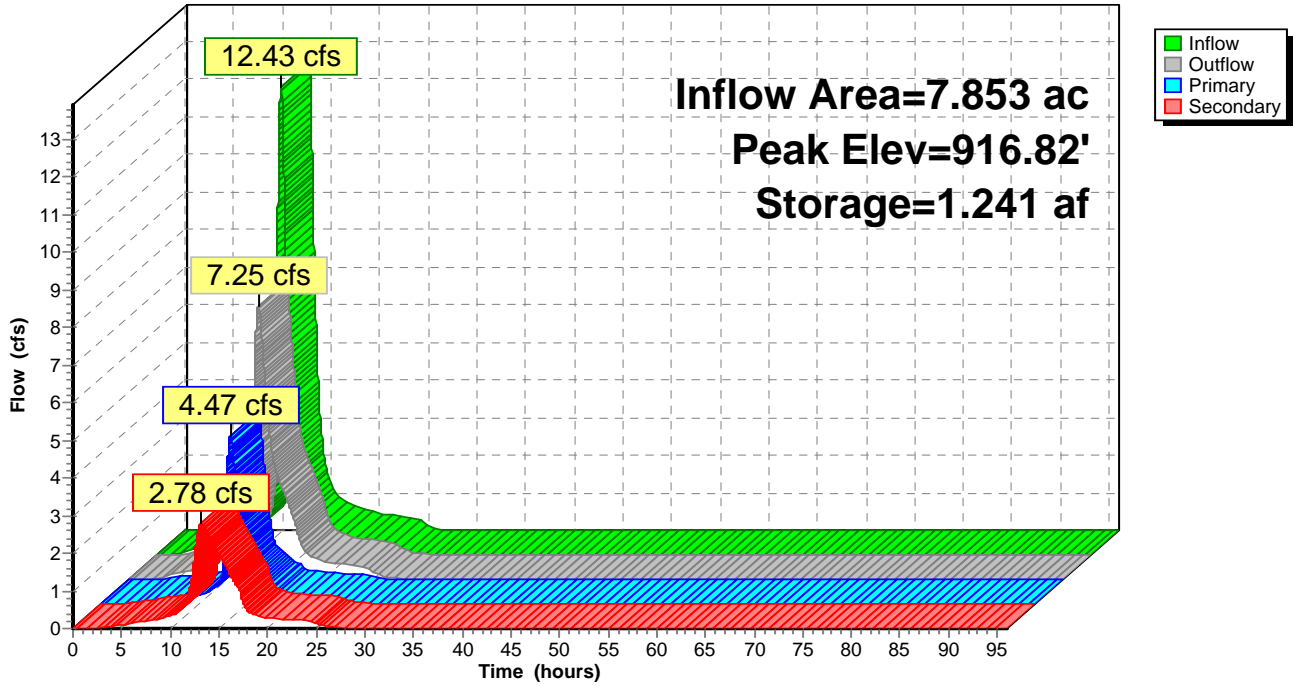
- ↑1=Orifice/Grate (Orifice Controls 1.28 cfs @ 6.50 fps)
- ↑3=RCP_Round 24" (Barrel Controls 3.20 cfs @ 2.87 fps)

Secondary OutFlow Max=2.78 cfs @ 13.26 hrs HW=916.82' TW=915.12' (Dynamic Tailwater)

- ↑2=Orifice/Grate (Orifice Controls 2.78 cfs @ 6.29 fps)

Pond 4P: P-4

Hydrograph



Summary for Pond 7P: P-7

[90] Warning: Qout>Qin may require smaller dt or Finer Routing

Inflow Area = 29.595 ac, 30.01% Impervious, Inflow Depth = 2.43" for 10-Year event
 Inflow = 39.95 cfs @ 12.61 hrs, Volume= 5.985 af
 Outflow = 40.55 cfs @ 12.66 hrs, Volume= 5.985 af, Atten= 0%, Lag= 2.6 min
 Primary = 40.55 cfs @ 12.66 hrs, Volume= 5.985 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 915.00' Surf.Area= 0.440 ac Storage= 1.062 af
 Peak Elev= 915.56' @ 12.56 hrs Surf.Area= 0.508 ac Storage= 1.330 af (0.268 af above start)

Plug-Flow detention time= 132.8 min calculated for 4.922 af (82% of inflow)
 Center-of-Mass det. time= 9.3 min (845.0 - 835.7)

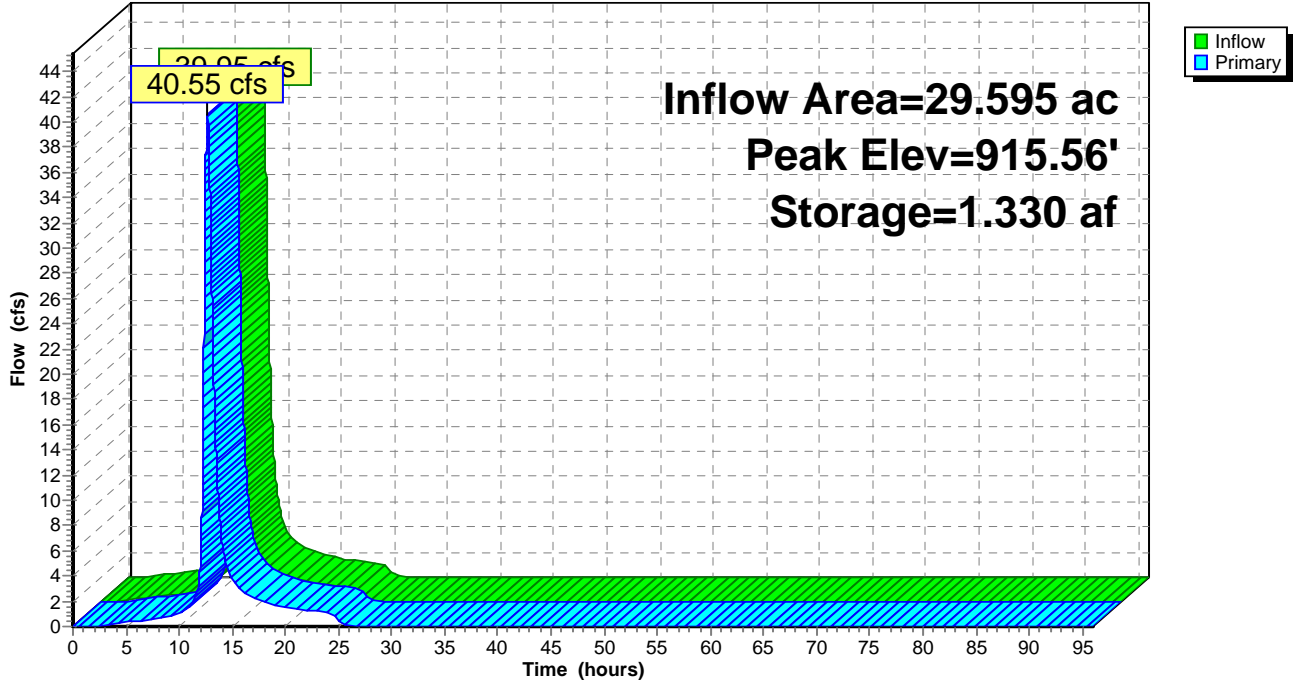
Volume	Invert	Avail.Storage	Storage Description
#1	910.95'	2.122 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
910.95	0.110	0.000	0.000
912.00	0.180	0.152	0.152
914.00	0.340	0.520	0.672
915.00	0.440	0.390	1.062
916.00	0.560	0.500	1.562
917.00	0.560	0.560	2.122

Device	Routing	Invert	Outlet Devices
#1	Primary	915.00'	75.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=41.07 cfs @ 12.66 hrs HW=915.56' TW=915.50' (Dynamic Tailwater)
 ↑1=Broad-Crested Rectangular Weir (Weir Controls 41.07 cfs @ 0.98 fps)

Pond 7P: P-7

Hydrograph



Summary for Pond 9P: P-9

Inflow Area = 55.384 ac, 31.48% Impervious, Inflow Depth = 2.46" for 10-Year event
 Inflow = 77.38 cfs @ 12.50 hrs, Volume= 11.351 af
 Outflow = 77.30 cfs @ 12.51 hrs, Volume= 11.351 af, Atten= 0%, Lag= 0.6 min
 Primary = 77.30 cfs @ 12.51 hrs, Volume= 11.351 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 915.00' Surf.Area= 0.210 ac Storage= 0.353 af
 Peak Elev= 915.51' @ 12.51 hrs Surf.Area= 0.313 ac Storage= 0.487 af (0.135 af above start)

Plug-Flow detention time= 36.8 min calculated for 10.998 af (97% of inflow)
 Center-of-Mass det. time= 2.2 min (833.7 - 831.5)

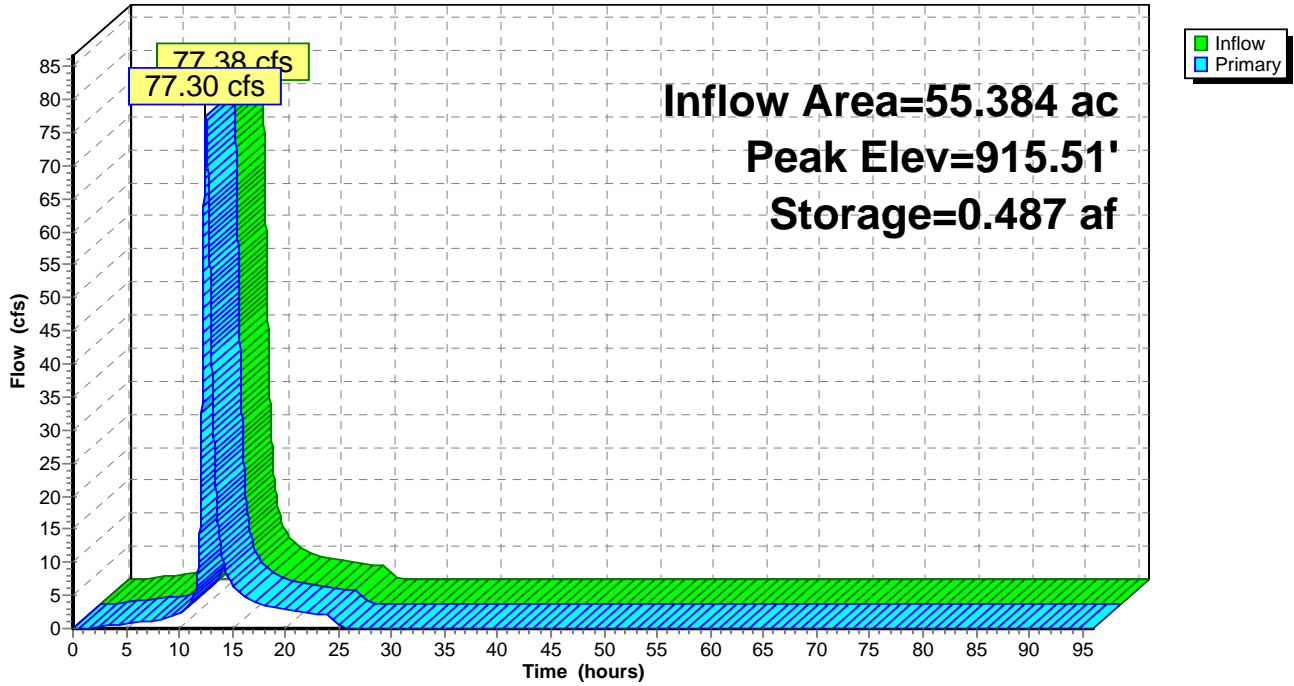
Volume	Invert	Avail.Storage	Storage Description
#1	910.50'	1.673 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
910.50	0.020	0.000	0.000
912.00	0.050	0.052	0.052
913.00	0.070	0.060	0.112
914.00	0.100	0.085	0.198
915.00	0.210	0.155	0.353
916.00	0.410	0.310	0.662
918.00	0.600	1.010	1.673

Device	Routing	Invert	Outlet Devices
#1	Primary	915.00'	80.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=77.29 cfs @ 12.51 hrs HW=915.51' TW=911.10' (Dynamic Tailwater)
 ↑1=Broad-Crested Rectangular Weir (Weir Controls 77.29 cfs @ 1.88 fps)

Pond 9P: P-9

Hydrograph



Summary for Pond 10P: P-10

[95] Warning: Outlet Device #1 rise exceeded

Inflow Area = 66.448 ac, 29.37% Impervious, Inflow Depth = 1.95" for 10-Year event
 Inflow = 38.10 cfs @ 13.07 hrs, Volume= 10.812 af
 Outflow = 37.98 cfs @ 13.11 hrs, Volume= 10.811 af, Atten= 0%, Lag= 2.1 min
 Primary = 12.53 cfs @ 13.11 hrs, Volume= 7.912 af
 Secondary = 25.45 cfs @ 13.11 hrs, Volume= 2.899 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 896.00' Surf.Area= 0.290 ac Storage= 0.700 af
 Peak Elev= 897.75' @ 13.11 hrs Surf.Area= 0.364 ac Storage= 1.270 af (0.570 af above start)

Plug-Flow detention time= 102.7 min calculated for 10.110 af (94% of inflow)
 Center-of-Mass det. time= 26.9 min (971.5 - 944.5)

Volume	Invert	Avail.Storage	Storage Description
#1	892.00'	1.760 af	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
892.00	0.120	0.000	0.000
893.00	0.140	0.130	0.130
895.00	0.190	0.330	0.460
896.00	0.290	0.240	0.700
897.00	0.330	0.310	1.010
899.00	0.420	0.750	1.760

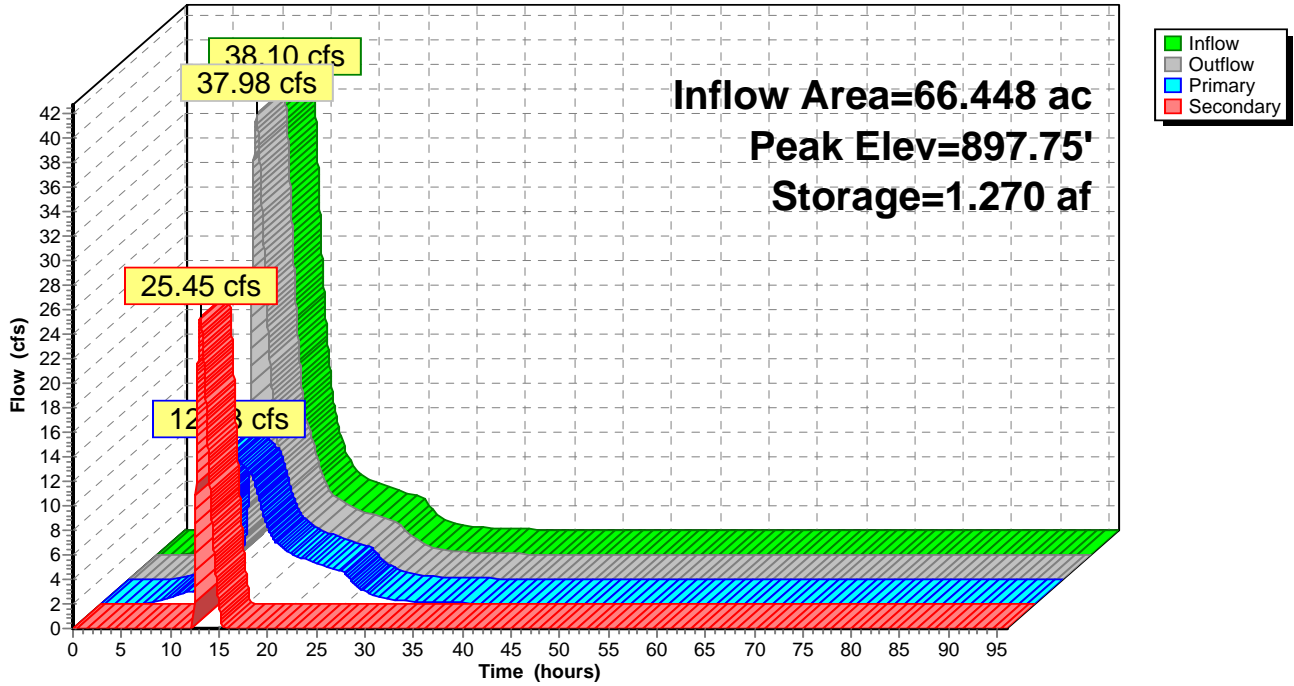
Device	Routing	Invert	Outlet Devices
#1	Primary	896.00'	2.5' long x 1.00' rise Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Secondary	897.40'	50.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=12.53 cfs @ 13.11 hrs HW=897.75' TW=894.24' (Dynamic Tailwater)
 ↖1=Sharp-Crested Rectangular Weir (Orifice Controls 12.53 cfs @ 5.45 fps)

Secondary OutFlow Max=25.45 cfs @ 13.11 hrs HW=897.75' TW=894.24' (Dynamic Tailwater)
 ↖2=Broad-Crested Rectangular Weir (Weir Controls 25.45 cfs @ 1.46 fps)

Pond 10P: P-10

Hydrograph



Summary for Pond 11P: P-11

Inflow Area = 58.677 ac, 31.52% Impervious, Inflow Depth = 2.46" for 10-Year event
 Inflow = 80.40 cfs @ 12.50 hrs, Volume= 12.051 af
 Outflow = 40.33 cfs @ 13.08 hrs, Volume= 12.049 af, Atten= 50%, Lag= 34.7 min
 Primary = 35.82 cfs @ 13.08 hrs, Volume= 9.489 af
 Secondary = 4.51 cfs @ 13.08 hrs, Volume= 2.560 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 909.00' Surf.Area= 1.210 ac Storage= 3.640 af
 Peak Elev= 912.03' @ 13.08 hrs Surf.Area= 1.563 ac Storage= 7.830 af (4.190 af above start)

Plug-Flow detention time= 315.8 min calculated for 8.408 af (70% of inflow)
 Center-of-Mass det. time= 118.1 min (949.0 - 830.9)

Volume	Invert	Avail.Storage	Storage Description
#1	905.00'	9.405 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
905.00	0.760	0.000	0.000
906.00	0.820	0.790	0.790
908.00	0.950	1.770	2.560
909.00	1.210	1.080	3.640
910.00	1.320	1.265	4.905
912.00	1.560	2.880	7.785
913.00	1.680	1.620	9.405

Device	Routing	Invert	Outlet Devices
#1	Primary	909.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	910.00'	24.0" Round RCP_Round 24" L= 200.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 910.00' / 909.00' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Primary	910.00'	24.0" Round RCP_Round 24" L= 200.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 910.00' / 909.00' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#4	Primary	912.00'	60.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88
#5	Secondary	909.00'	12.0" Round RCP_Round 12" L= 150.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 909.00' / 908.00' S= 0.0067 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=35.82 cfs @ 13.08 hrs HW=912.03' TW=897.75' (Dynamic Tailwater)

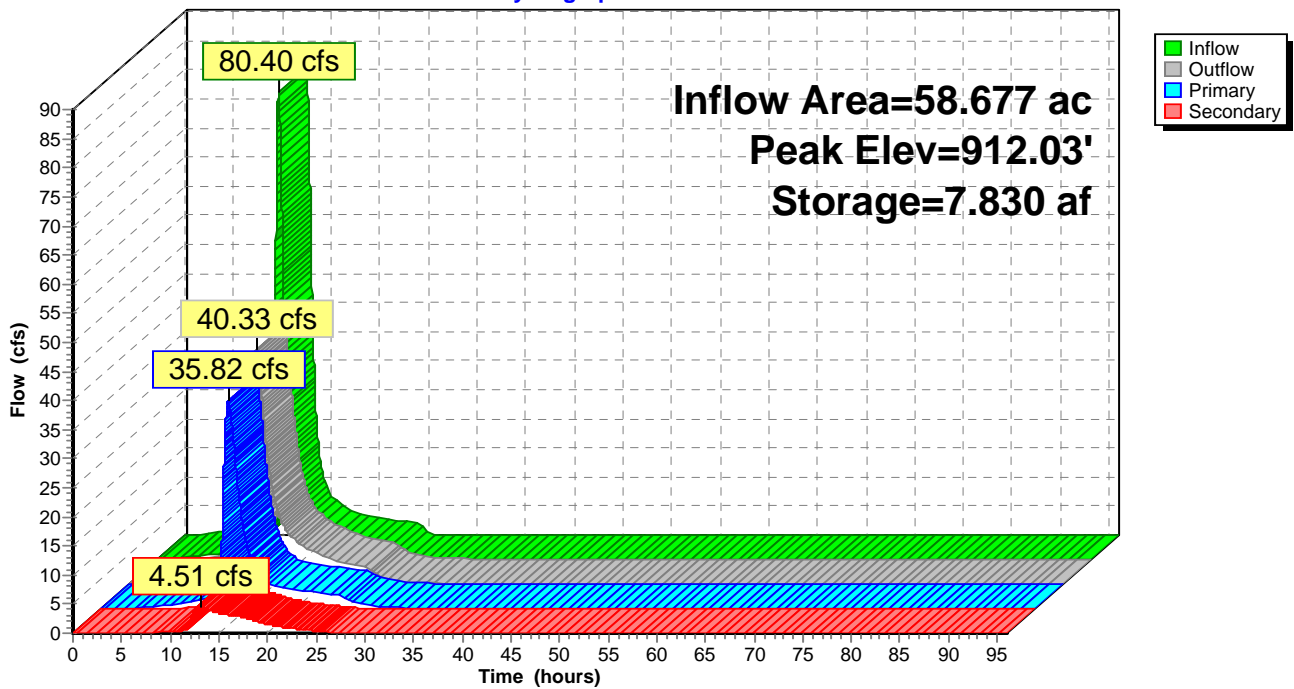
- 1=Orifice/Grate (Orifice Controls 6.58 cfs @ 8.38 fps)
- 2=RCP_Round 24" (Barrel Controls 14.27 cfs @ 5.57 fps)
- 3=RCP_Round 24" (Barrel Controls 14.27 cfs @ 5.57 fps)
- 4=Broad-Crested Rectangular Weir (Weir Controls 0.69 cfs @ 0.40 fps)

Secondary OutFlow Max=4.51 cfs @ 13.08 hrs HW=912.03' TW=908.76' (Dynamic Tailwater)

- 5=RCP_Round 12" (Barrel Controls 4.51 cfs @ 5.75 fps)

Pond 11P: P-11

Hydrograph



Summary for Pond 12P: P-12

Inflow Area = 79.658 ac, 31.13% Impervious, Inflow Depth > 2.45" for 10-Year event
 Inflow = 45.31 cfs @ 12.02 hrs, Volume= 16.286 af
 Outflow = 37.09 cfs @ 13.56 hrs, Volume= 16.278 af, Atten= 18%, Lag= 92.1 min
 Primary = 37.09 cfs @ 13.56 hrs, Volume= 16.278 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 893.00' Surf.Area= 1.640 ac Storage= 5.075 af
 Peak Elev= 894.35' @ 13.56 hrs Surf.Area= 1.817 ac Storage= 7.405 af (2.330 af above start)

Plug-Flow detention time= 392.1 min calculated for 11.202 af (69% of inflow)
 Center-of-Mass det. time= 86.6 min (1,075.0 - 988.5)

Volume	Invert	Avail.Storage	Storage Description
#1	889.00'	10.590 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
889.00	1.070	0.000	0.000
890.00	1.150	1.110	1.110
892.00	1.330	2.480	3.590
893.00	1.640	1.485	5.075
894.00	1.770	1.705	6.780
896.00	2.040	3.810	10.590

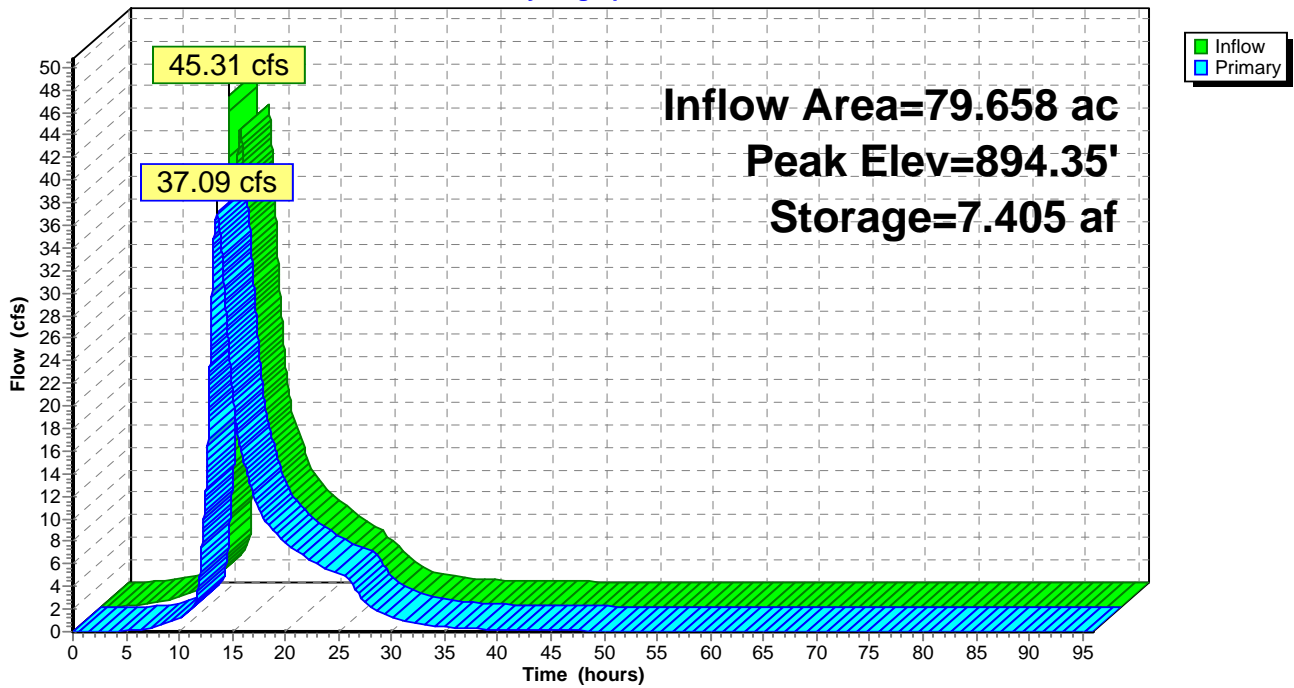
Device	Routing	Invert	Outlet Devices
#1	Primary	893.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	893.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf
#4	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf
#5	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf
#6	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf

Primary OutFlow Max=37.09 cfs @ 13.56 hrs HW=894.35' TW=883.66' (Dynamic Tailwater)

- 1=Orifice/Grate (Orifice Controls 4.39 cfs @ 5.59 fps)
- 2=Orifice/Grate (Orifice Controls 4.39 cfs @ 5.59 fps)
- 3=RCP_Arch 44x27 (Barrel Controls 7.08 cfs @ 3.78 fps)
- 4=RCP_Arch 44x27 (Barrel Controls 7.08 cfs @ 3.78 fps)
- 5=RCP_Arch 44x27 (Barrel Controls 7.08 cfs @ 3.78 fps)
- 6=RCP_Arch 44x27 (Barrel Controls 7.08 cfs @ 3.78 fps)

Pond 12P: P-12

Hydrograph



Summary for Pond 13P: P-13

Inflow Area = 237.893 ac, 51.59% Impervious, Inflow Depth = 2.74" for 10-Year event
 Inflow = 327.06 cfs @ 12.35 hrs, Volume= 54.310 af
 Outflow = 309.98 cfs @ 12.46 hrs, Volume= 54.307 af, Atten= 5%, Lag= 6.5 min
 Primary = 296.00 cfs @ 12.46 hrs, Volume= 51.732 af
 Secondary = 13.98 cfs @ 12.46 hrs, Volume= 2.575 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 883.00' Surf.Area= 1.870 ac Storage= 4.265 af
 Peak Elev= 884.60' @ 12.46 hrs Surf.Area= 2.443 ac Storage= 7.718 af (3.453 af above start)

Plug-Flow detention time= 104.7 min calculated for 50.037 af (92% of inflow)
 Center-of-Mass det. time= 15.3 min (906.8 - 891.6)

Volume	Invert	Avail.Storage	Storage Description
#1	878.00'	11.490 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
878.00	0.000	0.000	0.000
879.00	0.630	0.315	0.315
880.00	0.730	0.680	0.995
882.00	1.070	1.800	2.795
883.00	1.870	1.470	4.265
884.00	2.220	2.045	6.310
886.00	2.960	5.180	11.490

Device	Routing	Invert	Outlet Devices
#1	Primary	883.00'	55.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88
#2	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 1' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#3	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 1' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#4	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 1' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#5	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 1' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#6	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200

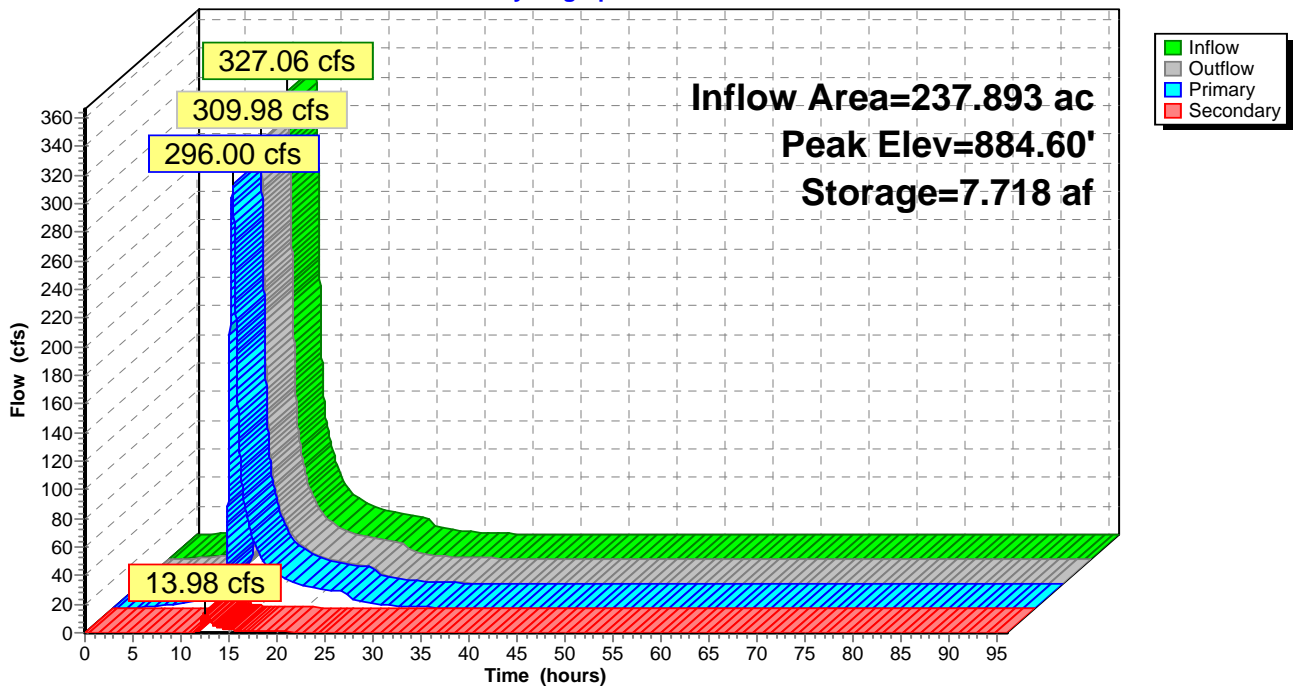
Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 1' Cc= 0.900
 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=295.98 cfs @ 12.46 hrs HW=884.60' TW=0.00' (Dynamic Tailwater)
 1=Broad-Crested Rectangular Weir (Weir Controls 295.98 cfs @ 3.36 fps)

Secondary OutFlow Max=13.98 cfs @ 12.46 hrs HW=884.60' TW=883.04' (Dynamic Tailwater)
 2=RCP_Round 12" (Barrel Controls 2.80 cfs @ 3.56 fps)
 3=RCP_Round 12" (Barrel Controls 2.80 cfs @ 3.56 fps)
 4=RCP_Round 12" (Barrel Controls 2.80 cfs @ 3.56 fps)
 5=RCP_Round 12" (Barrel Controls 2.80 cfs @ 3.56 fps)
 6=RCP_Round 12" (Barrel Controls 2.80 cfs @ 3.56 fps)

Pond 13P: P-13

Hydrograph



Summary for Pond 14P: P-14

Inflow Area = 21.198 ac, 39.93% Impervious, Inflow Depth = 2.65" for 10-Year event
 Inflow = 42.96 cfs @ 12.30 hrs, Volume= 4.676 af
 Outflow = 6.52 cfs @ 13.18 hrs, Volume= 4.674 af, Atten= 85%, Lag= 53.4 min
 Primary = 6.52 cfs @ 13.18 hrs, Volume= 4.674 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 892.00' Surf.Area= 1.380 ac Storage= 4.490 af
 Peak Elev= 893.66' @ 13.18 hrs Surf.Area= 1.536 ac Storage= 6.910 af (2.420 af above start)

Plug-Flow detention time= 1,732.7 min calculated for 0.184 af (4% of inflow)
 Center-of-Mass det. time= 259.6 min (1,064.4 - 804.9)

Volume	Invert	Avail.Storage	Storage Description
#1	888.00'	9.910 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
888.00	0.950	0.000	0.000
890.00	1.080	2.030	2.030
892.00	1.380	2.460	4.490
893.00	1.470	1.425	5.915
894.00	1.570	1.520	7.435
895.50	1.730	2.475	9.910

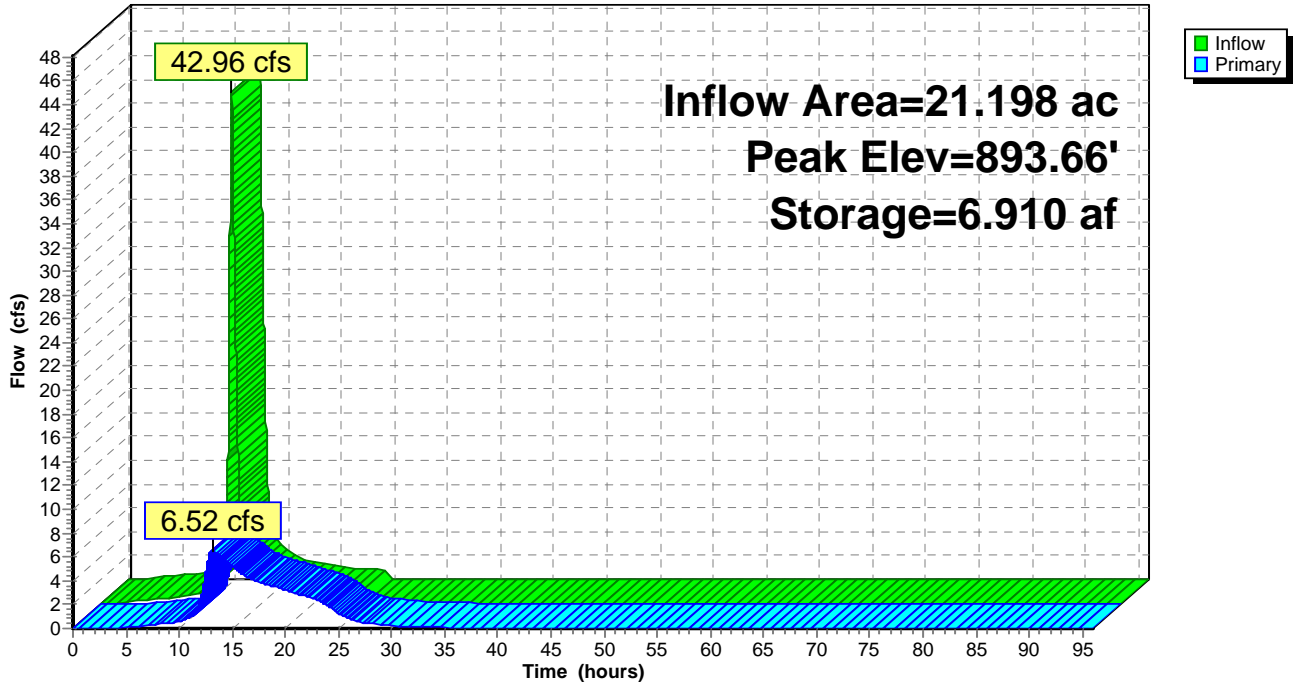
Device	Routing	Invert	Outlet Devices
#1	Primary	892.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	893.00'	18.0" Round RCP_Round 18" L= 50.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 893.00' / 892.75' S= 0.0050 1' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=6.52 cfs @ 13.18 hrs HW=893.66' (Free Discharge)

- 1=Orifice/Grate (Orifice Controls 4.88 cfs @ 6.21 fps)
- 2=RCP_Round 18" (Barrel Controls 1.65 cfs @ 3.23 fps)

Pond 14P: P-14

Hydrograph



Summary for Pond 23P: Thumb Infiltration (Thumb TP load only)

Inflow Area = 48.540 ac, 84.23% Impervious, Inflow Depth = 3.41" for 10-Year event
 Inflow = 95.01 cfs @ 12.44 hrs, Volume= 13.805 af
 Outflow = 94.93 cfs @ 12.45 hrs, Volume= 10.065 af, Atten= 0%, Lag= 0.7 min
 Primary = 94.93 cfs @ 12.45 hrs, Volume= 10.065 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 903.83' @ 12.45 hrs Surf.Area= 1.000 ac Storage= 3.834 af

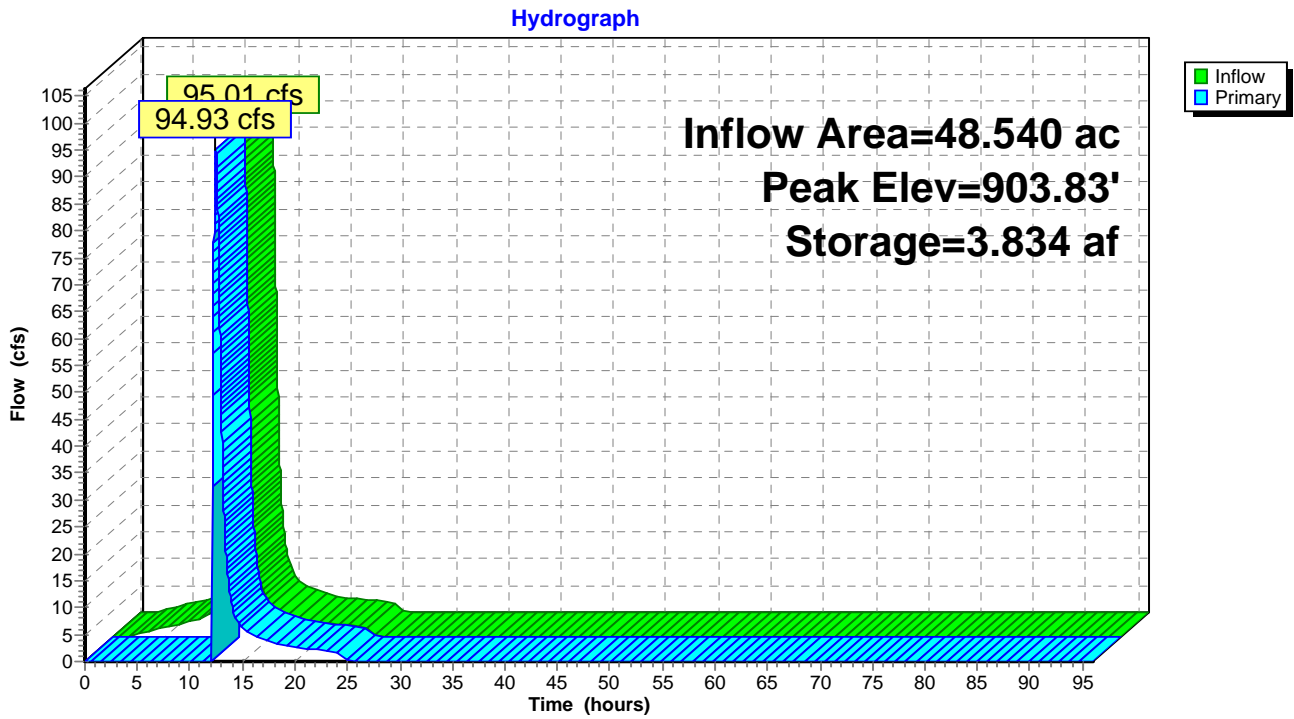
Plug-Flow detention time= 168.5 min calculated for 10.065 af (73% of inflow)
 Center-of-Mass det. time= 77.0 min (861.0 - 784.1)

Volume	Invert	Avail.Storage	Storage Description
#1	900.00'	5.000 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
900.00	1.000	0.000	0.000
901.00	1.000	1.000	1.000
902.00	1.000	1.000	2.000
903.00	1.000	1.000	3.000
904.00	1.000	1.000	4.000
905.00	1.000	1.000	5.000

Device	Routing	Invert	Outlet Devices
#1	Primary	903.74'	1,000.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) 5.0' Crest Height

Primary OutFlow Max=94.91 cfs @ 12.45 hrs HW=903.83' (Free Discharge)
 ↑1=Sharp-Crested Rectangular Weir (Weir Controls 94.91 cfs @ 1.01 fps)

Pond 23P: Thumb Infiltration (Thumb TP load only)



Summary for Pond 31P: SB 18 Infiltration

Inflow Area = 52.908 ac, 84.55% Impervious, Inflow Depth = 3.64" for 10-Year event
 Inflow = 124.38 cfs @ 12.40 hrs, Volume= 16.050 af
 Outflow = 124.27 cfs @ 12.40 hrs, Volume= 12.730 af, Atten= 0%, Lag= 0.4 min
 Primary = 124.27 cfs @ 12.40 hrs, Volume= 12.730 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 903.43' @ 12.40 hrs Surf.Area= 1.000 ac Storage= 3.432 af

Plug-Flow detention time= 144.2 min calculated for 12.730 af (79% of inflow)
 Center-of-Mass det. time= 63.9 min (846.6 - 782.8)

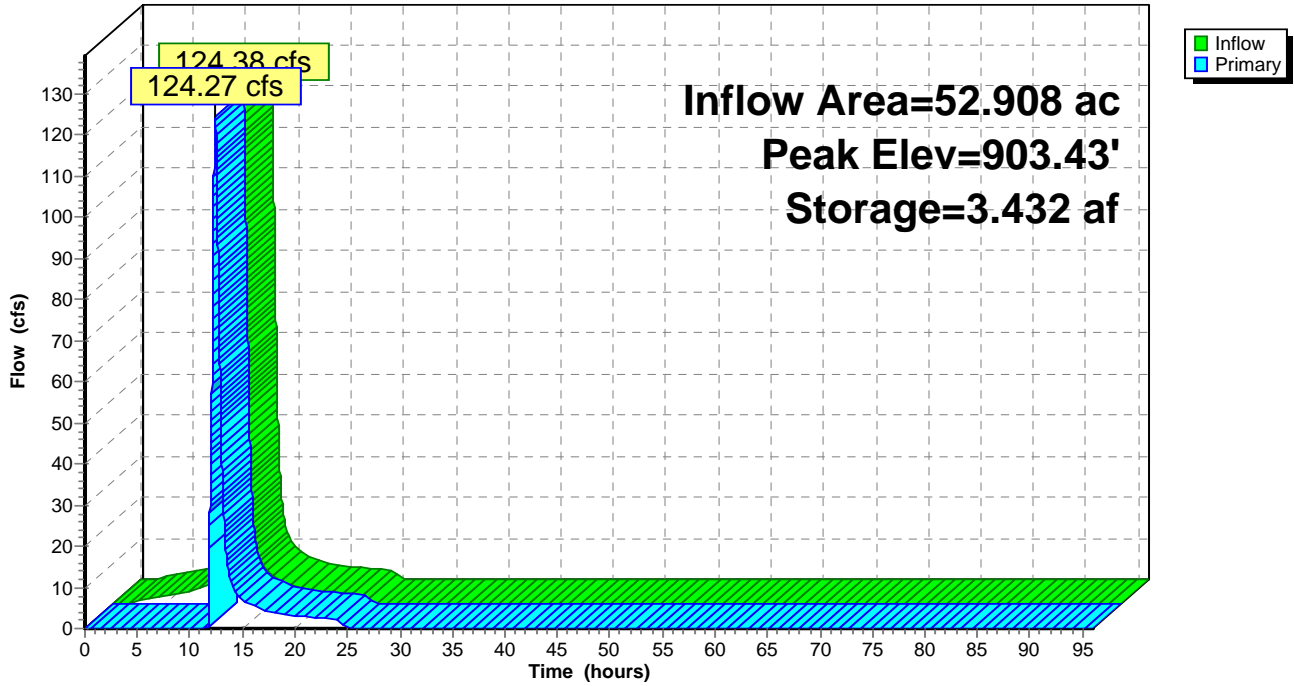
Volume	Invert	Avail.Storage	Storage Description
#1	900.00'	5.000 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
900.00	1.000	0.000	0.000
901.00	1.000	1.000	1.000
902.00	1.000	1.000	2.000
903.00	1.000	1.000	3.000
904.00	1.000	1.000	4.000
905.00	1.000	1.000	5.000

Device	Routing	Invert	Outlet Devices
#1	Primary	903.32'	1,000.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) 0.8' Crest Height

Primary OutFlow Max=124.22 cfs @ 12.40 hrs HW=903.43' TW=887.50' (Dynamic Tailwater)
 ↑1=Sharp-Crested Rectangular Weir (Weir Controls 124.22 cfs @ 1.11 fps)

Pond 31P: SB 18 Infiltration

Hydrograph



Summary for Pond 36P: Culverts passing flow beneath Spine Road

Inflow Area = 52.908 ac, 84.55% Impervious, Inflow Depth = 2.89" for 10-Year event
 Inflow = 124.27 cfs @ 12.40 hrs, Volume= 12.730 af
 Outflow = 124.27 cfs @ 12.40 hrs, Volume= 12.730 af, Atten= 0%, Lag= 0.0 min
 Primary = 124.27 cfs @ 12.40 hrs, Volume= 12.730 af
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 887.50' @ 12.40 hrs Surf.Area= 0.003 ac Storage= 0.001 af

Plug-Flow detention time= 0.0 min calculated for 12.728 af (100% of inflow)
 Center-of-Mass det. time= 0.0 min (846.6 - 846.6)

Volume	Invert	Avail.Storage	Storage Description
#1	887.00'	0.025 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
887.00	0.000	0.000	0.000
887.50	0.003	0.001	0.001
890.50	0.006	0.014	0.014
892.00	0.009	0.011	0.025

Device	Routing	Invert	Outlet Devices
#1	Primary	887.00'	Special & User-Defined Head (feet) 0.00 0.10 0.20 0.30 0.40 0.50 5.00 Disch. (cfs) 0.000 25.000 50.000 75.000 100.000 125.000 125.000
#2	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#3	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#4	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#5	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#6	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#7	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900

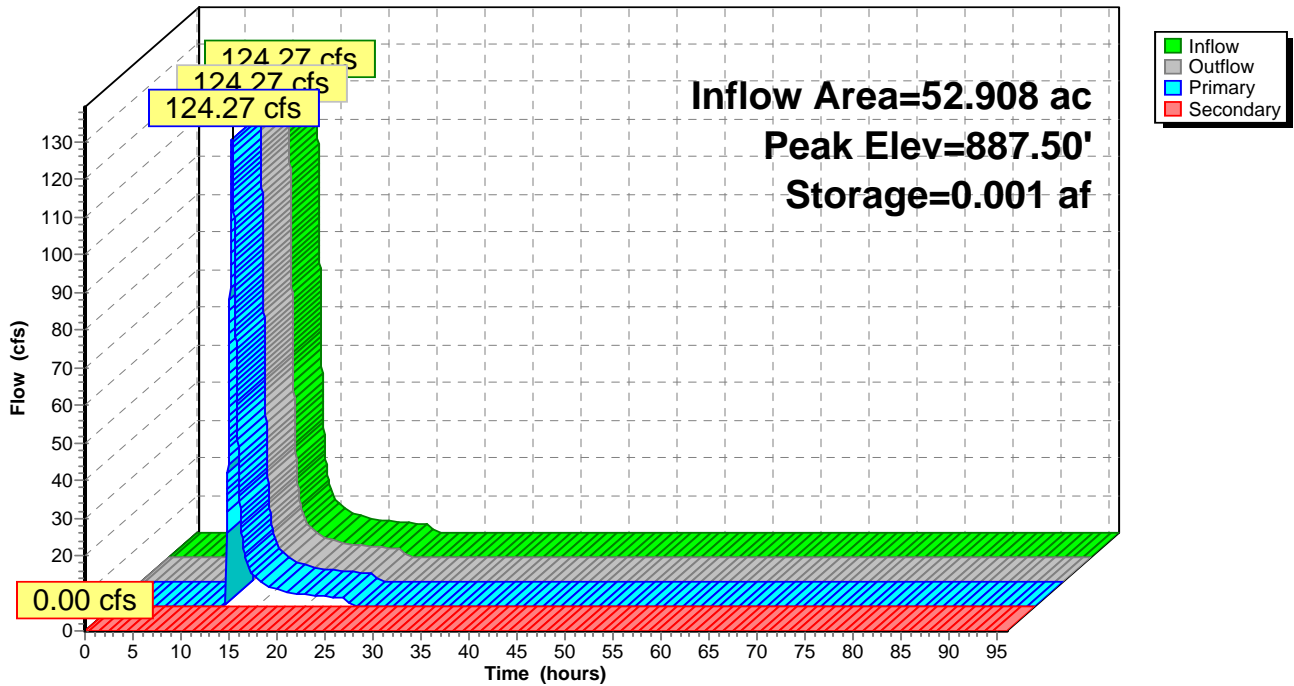
- #8 Secondary 887.50' n= 0.013, Flow Area= 1.77 sf **18.0" Round RCP_Round 18"**
 L= 100.0' RCP, groove end w/headwall, Ke= 0.200
 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1' Cc= 0.900
- #9 Secondary 887.50' n= 0.013, Flow Area= 1.77 sf **18.0" Round RCP_Round 18"**
 L= 100.0' RCP, groove end w/headwall, Ke= 0.200
 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1' Cc= 0.900

Primary OutFlow Max=124.22 cfs @ 12.40 hrs HW=887.50' TW=884.59' (Dynamic Tailwater)
 1=Special & User-Defined (Custom Controls 124.22 cfs)

- Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=887.00' TW=883.00' (Dynamic Tailwater)
- 2=RCP_Round 18" (Controls 0.00 cfs)
 - 3=RCP_Round 18" (Controls 0.00 cfs)
 - 4=RCP_Round 18" (Controls 0.00 cfs)
 - 5=RCP_Round 18" (Controls 0.00 cfs)
 - 6=RCP_Round 18" (Controls 0.00 cfs)
 - 7=RCP_Round 18" (Controls 0.00 cfs)
 - 8=RCP_Round 18" (Controls 0.00 cfs)
 - 9=RCP_Round 18" (Controls 0.00 cfs)

Pond 36P: Culverts passing flow beneath Spine Road

Hydrograph



Summary for Pond CRH-1: CRH-1

Inflow Area = 6.955 ac, 46.76% Impervious, Inflow Depth = 2.80" for 10-Year event
 Inflow = 18.97 cfs @ 12.15 hrs, Volume= 1.622 af
 Outflow = 12.06 cfs @ 12.33 hrs, Volume= 1.622 af, Atten= 36%, Lag= 10.6 min
 Discarded = 0.26 cfs @ 12.33 hrs, Volume= 0.512 af
 Primary = 11.80 cfs @ 12.33 hrs, Volume= 1.110 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 878.12' @ 12.33 hrs Surf.Area= 0.325 ac Storage= 0.489 af

Plug-Flow detention time= 182.4 min calculated for 1.622 af (100% of inflow)
 Center-of-Mass det. time= 182.5 min (971.9 - 789.5)

Volume	Invert	Avail.Storage	Storage Description
#1	876.00'	0.850 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
876.00	0.150	0.000	0.000
878.00	0.300	0.450	0.450
879.00	0.500	0.400	0.850

Device	Routing	Invert	Outlet Devices
#1	Discarded	876.00'	0.800 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 0.00'
#2	Primary	877.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 877.00' / 876.00' S= 0.0065 1/8" Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Primary	877.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 877.00' / 876.00' S= 0.0065 1/8" Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Discarded OutFlow Max=0.26 cfs @ 12.33 hrs HW=878.12' (Free Discharge)

↳ **1=Exfiltration** (Controls 0.26 cfs)

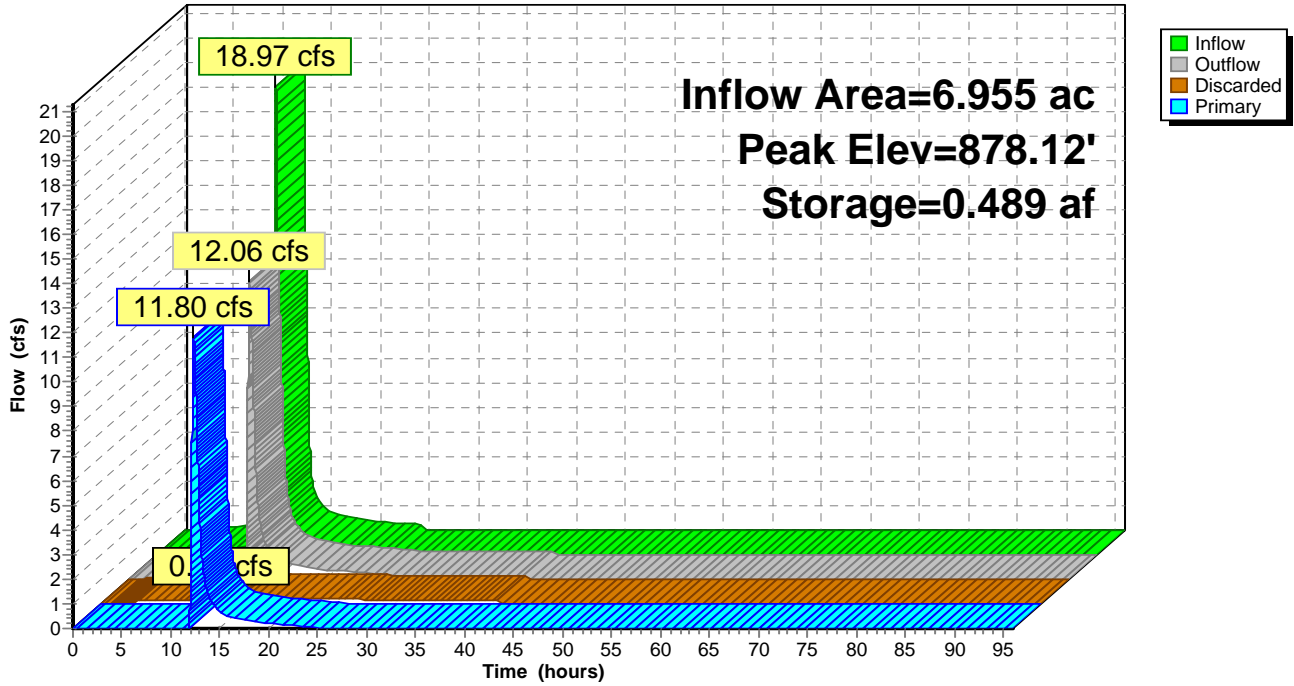
Primary OutFlow Max=11.80 cfs @ 12.33 hrs HW=878.12' (Free Discharge)

↳ **2=Culvert** (Barrel Controls 5.90 cfs @ 4.69 fps)

↳ **3=Culvert** (Barrel Controls 5.90 cfs @ 4.69 fps)

Pond CRH-1: CRH-1

Hydrograph



Summary for Pond CRH-2: CRH-2

Inflow Area = 10.214 ac, 37.73% Impervious, Inflow Depth = 2.60" for 10-Year event
 Inflow = 23.01 cfs @ 12.22 hrs, Volume= 2.212 af
 Outflow = 10.19 cfs @ 12.62 hrs, Volume= 2.212 af, Atten= 56%, Lag= 24.1 min
 Discarded = 0.38 cfs @ 12.62 hrs, Volume= 0.904 af
 Primary = 9.82 cfs @ 12.62 hrs, Volume= 1.308 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 882.67' @ 12.62 hrs Surf.Area= 0.467 ac Storage= 0.888 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 323.8 min (1,125.6 - 801.7)

Volume	Invert	Avail.Storage	Storage Description
#1	880.00'	1.600 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
880.00	0.200	0.000	0.000
882.00	0.400	0.600	0.600
884.00	0.600	1.000	1.600

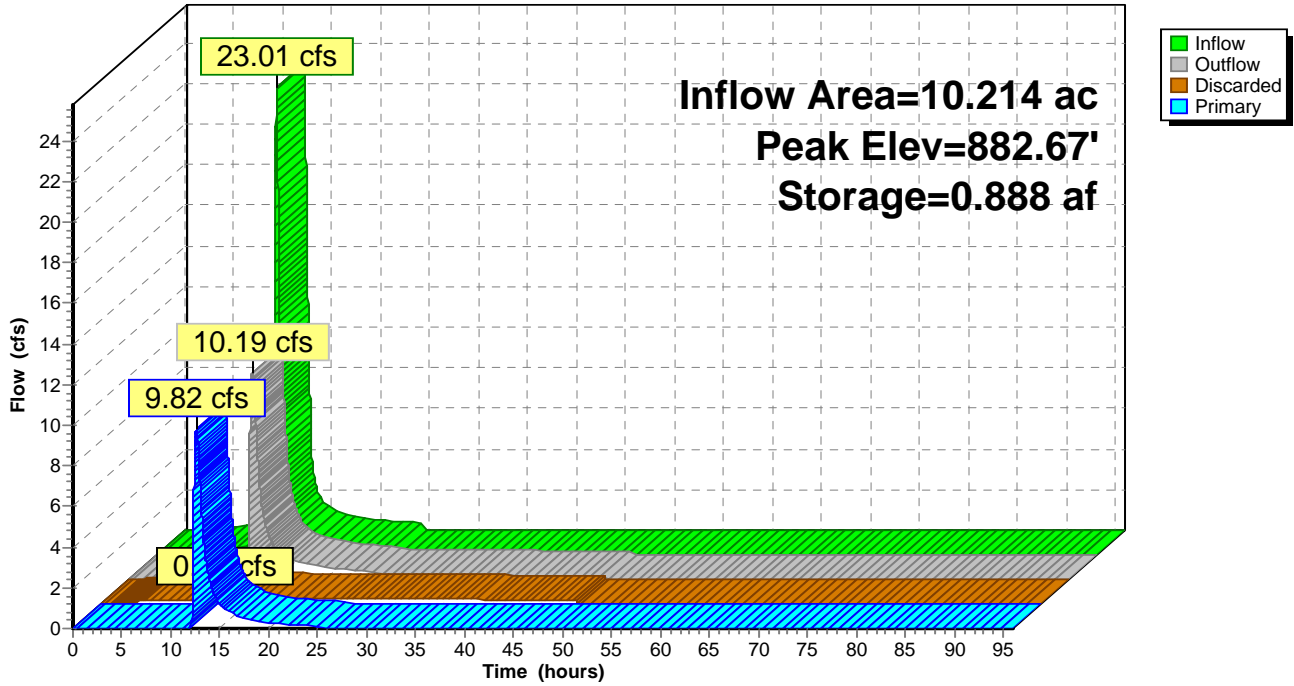
Device	Routing	Invert	Outlet Devices
#1	Discarded	880.00'	0.800 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 0.00'
#2	Primary	881.50'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 881.50' / 881.00' S= 0.0032 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Primary	881.50'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 881.50' / 881.00' S= 0.0032 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Discarded OutFlow Max=0.38 cfs @ 12.62 hrs HW=882.67' (Free Discharge)
 ↳1=Exfiltration (Controls 0.38 cfs)

Primary OutFlow Max=9.82 cfs @ 12.62 hrs HW=882.67' TW=878.71' (Dynamic Tailwater)
 ↳2=Culvert (Barrel Controls 4.91 cfs @ 3.72 fps)
 ↳3=Culvert (Barrel Controls 4.91 cfs @ 3.72 fps)

Pond CRH-2: CRH-2

Hydrograph



Summary for Pond CRH-3: CRH-3

Inflow Area = 11.815 ac, 36.95% Impervious, Inflow Depth = 1.66" for 10-Year event
 Inflow = 10.84 cfs @ 12.54 hrs, Volume= 1.637 af
 Outflow = 8.22 cfs @ 12.91 hrs, Volume= 1.637 af, Atten= 24%, Lag= 22.1 min
 Discarded = 0.24 cfs @ 12.91 hrs, Volume= 0.449 af
 Primary = 7.98 cfs @ 12.91 hrs, Volume= 1.188 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 878.90' @ 12.91 hrs Surf.Area= 0.293 ac Storage= 0.421 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 170.2 min (1,018.1 - 847.9)

Volume	Invert	Avail.Storage	Storage Description
#1	877.00'	0.850 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
877.00	0.150	0.000	0.000
879.00	0.300	0.450	0.450
880.00	0.500	0.400	0.850

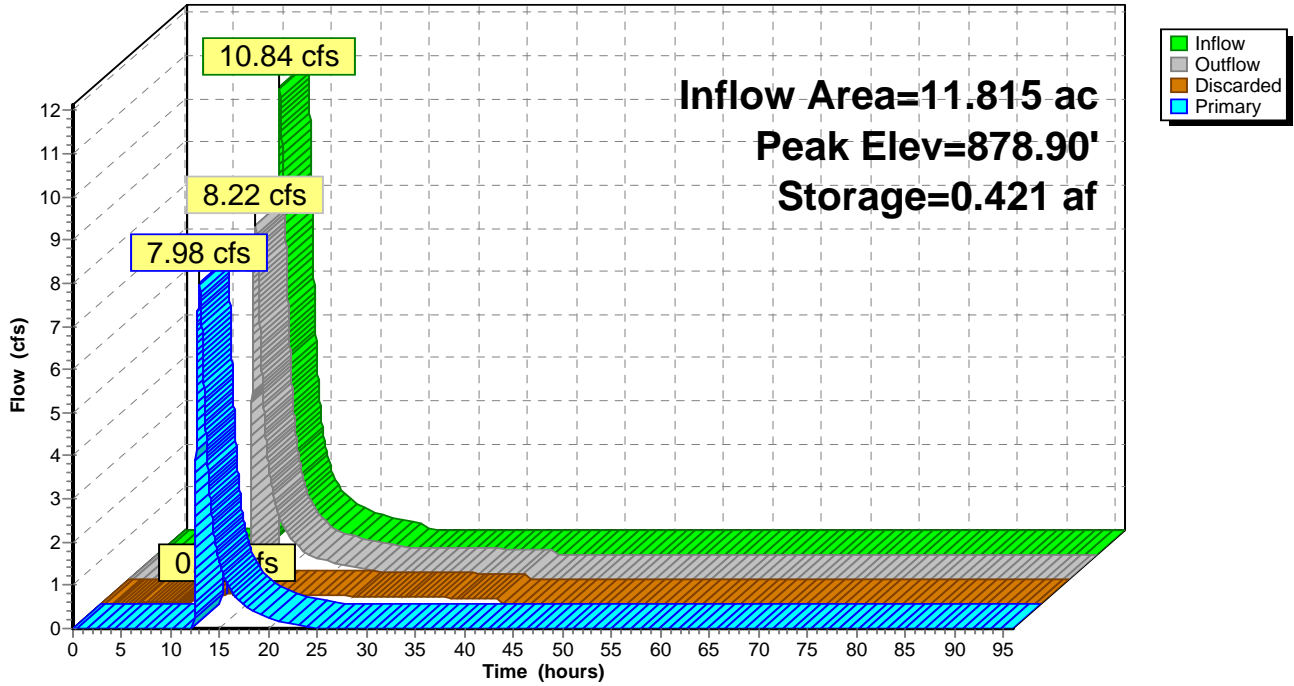
Device	Routing	Invert	Outlet Devices
#1	Discarded	877.00'	0.800 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 0.00'
#2	Primary	878.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 878.00' / 877.00' S= 0.0065 1/1' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Primary	878.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 878.00' / 877.00' S= 0.0065 1/1' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Discarded OutFlow Max=0.24 cfs @ 12.91 hrs HW=878.90' (Free Discharge)
 ↳ **1=Exfiltration** (Controls 0.24 cfs)

Primary OutFlow Max=7.98 cfs @ 12.91 hrs HW=878.90' (Free Discharge)
 ↳ **2=Culvert** (Barrel Controls 3.99 cfs @ 4.27 fps)
 ↳ **3=Culvert** (Barrel Controls 3.99 cfs @ 4.27 fps)

Pond CRH-3: CRH-3

Hydrograph



Summary for Pond P1/P2: P-1/P-2

Inflow Area = 68.531 ac, 57.92% Impervious, Inflow Depth = 3.05" for 10-Year event
 Inflow = 92.00 cfs @ 12.57 hrs, Volume= 17.405 af
 Outflow = 89.60 cfs @ 12.67 hrs, Volume= 17.397 af, Atten= 3%, Lag= 5.9 min
 Primary = 89.60 cfs @ 12.67 hrs, Volume= 17.397 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 924.00' Surf.Area= 1.270 ac Storage= 3.500 af
 Peak Elev= 925.17' @ 12.67 hrs Surf.Area= 1.446 ac Storage= 5.093 af (1.593 af above start)

Plug-Flow detention time= 203.9 min calculated for 13.896 af (80% of inflow)
 Center-of-Mass det. time= 66.2 min (872.2 - 806.0)

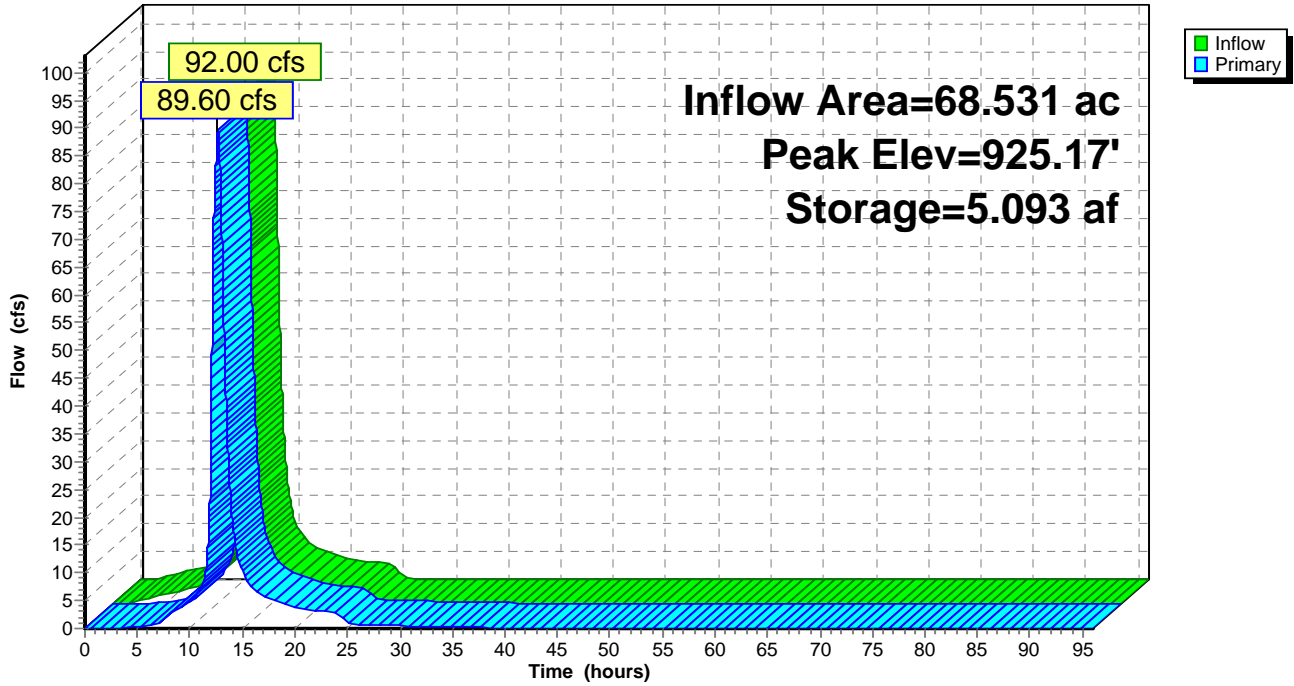
Volume	Invert	Avail.Storage	Storage Description
#1	920.00'	6.340 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
920.00	0.650	0.000	0.000
922.00	0.790	1.440	1.440
924.00	1.270	2.060	3.500
926.00	1.570	2.840	6.340

Device	Routing	Invert	Outlet Devices
#1	Primary	924.40'	40.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Primary	924.00'	6.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=89.60 cfs @ 12.67 hrs HW=925.17' TW=917.41' (Dynamic Tailwater)
 1=Sharp-Crested Rectangular Weir (Weir Controls 88.57 cfs @ 2.88 fps)
 2=Orifice/Grate (Orifice Controls 1.02 cfs @ 5.22 fps)

Pond P1/P2: P-1/P-2

Hydrograph



Summary for Pond P5/P6: P-5/P-6

Inflow Area = 43.279 ac, 47.44% Impervious, Inflow Depth = 2.82" for 10-Year event
 Inflow = 116.17 cfs @ 12.15 hrs, Volume= 10.153 af
 Outflow = 28.61 cfs @ 12.70 hrs, Volume= 8.037 af, Atten= 75%, Lag= 32.8 min
 Primary = 26.37 cfs @ 12.70 hrs, Volume= 6.911 af
 Secondary = 2.24 cfs @ 12.70 hrs, Volume= 1.126 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 929.00' Surf.Area= 1.975 ac Storage= 5.062 af
 Peak Elev= 931.49' @ 12.70 hrs Surf.Area= 2.487 ac Storage= 10.676 af (5.614 af above start)

Plug-Flow detention time= 735.4 min calculated for 2.974 af (29% of inflow)
 Center-of-Mass det. time= 230.7 min (1,019.3 - 788.7)

Volume	Invert	Avail.Storage	Storage Description
#1	926.00'	14.650 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
926.00	1.510	0.000	0.000
928.00	1.710	3.220	3.220
930.00	2.240	3.950	7.170
931.00	2.400	2.320	9.490
933.00	2.760	5.160	14.650

Device	Routing	Invert	Outlet Devices
#1	Primary	930.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	930.50'	7.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#3	Primary	931.50'	14.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#4	Secondary	930.00'	9.0" Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=26.37 cfs @ 12.70 hrs HW=931.49' TW=917.45' (Dynamic Tailwater)

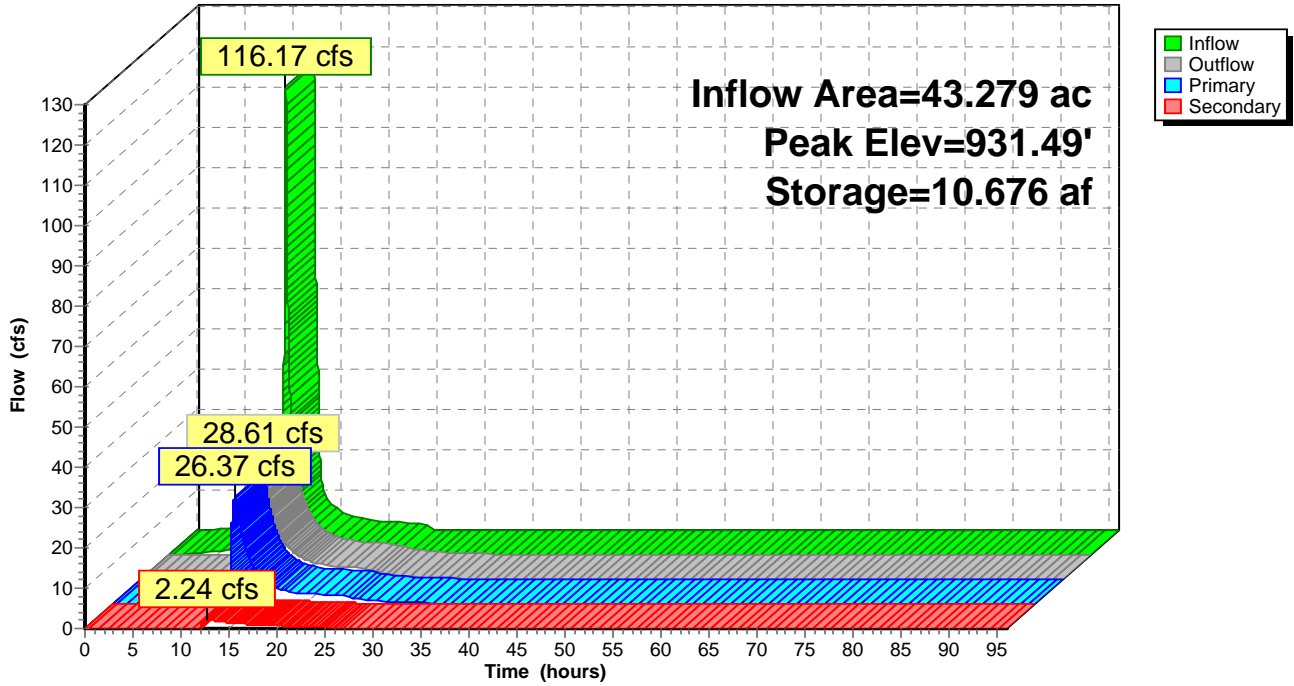
- ↑ 1=Orifice/Grate (Orifice Controls 4.61 cfs @ 5.87 fps)
- └ 2=Sharp-Crested Rectangular Weir (Weir Controls 21.76 cfs @ 3.25 fps)
- └ 3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

Secondary OutFlow Max=2.24 cfs @ 12.70 hrs HW=931.49' TW=929.09' (Dynamic Tailwater)

- ↑ 4=Orifice/Grate (Orifice Controls 2.24 cfs @ 5.07 fps)

Pond P5/P6: P-5/P-6

Hydrograph



Summary for Pond P8: P-8

Inflow Area = 6.389 ac, 7.62% Impervious, Inflow Depth = 1.93" for 10-Year event
 Inflow = 16.44 cfs @ 12.06 hrs, Volume= 1.027 af
 Outflow = 3.63 cfs @ 12.39 hrs, Volume= 1.021 af, Atten= 78%, Lag= 19.7 min
 Primary = 3.63 cfs @ 12.39 hrs, Volume= 1.021 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 897.00' Surf.Area= 0.300 ac Storage= 0.495 af
 Peak Elev= 898.11' @ 12.62 hrs Surf.Area= 0.455 ac Storage= 0.922 af (0.427 af above start)

Plug-Flow detention time= 533.7 min calculated for 0.526 af (51% of inflow)
 Center-of-Mass det. time= 193.8 min (1,023.9 - 830.1)

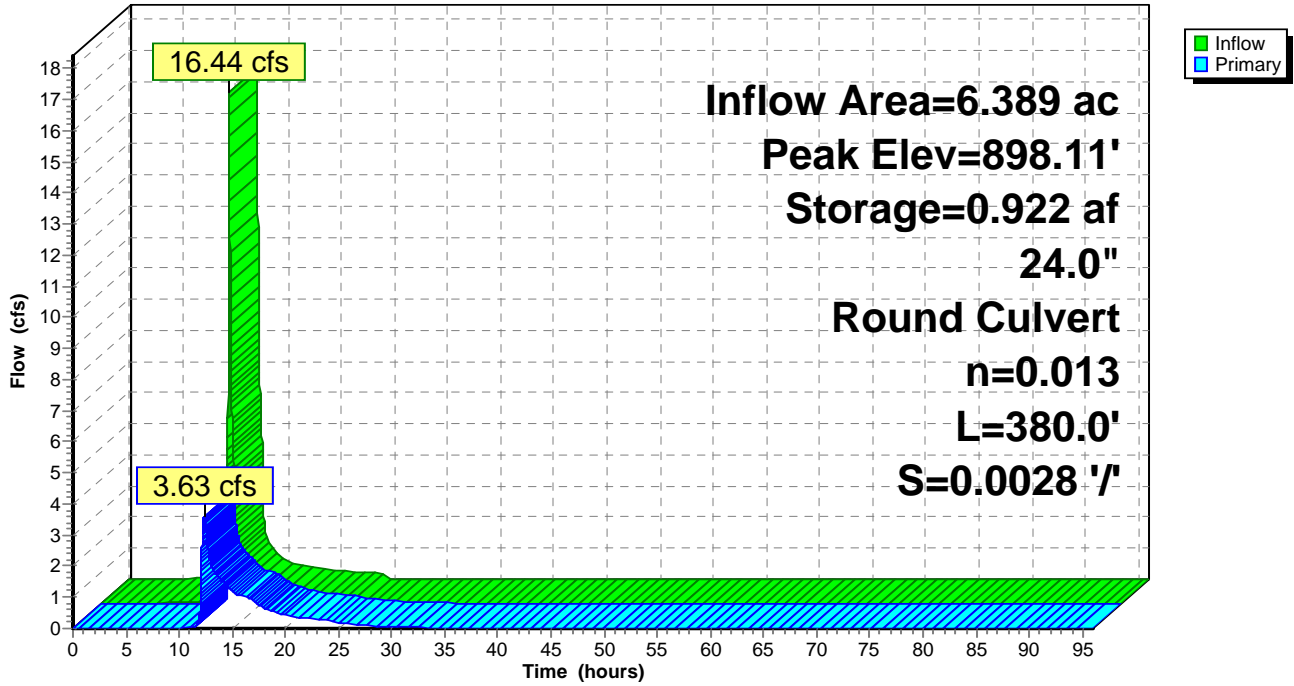
Volume	Invert	Avail.Storage	Storage Description
#1	893.00'	1.850 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
893.00	0.030	0.000	0.000
894.00	0.070	0.050	0.050
896.00	0.150	0.220	0.270
897.00	0.300	0.225	0.495
898.00	0.450	0.375	0.870
900.00	0.530	0.980	1.850

Device	Routing	Invert	Outlet Devices
#1	Primary	897.00'	24.0" Round RCP_Round 24" L= 380.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 897.00' / 895.94' S= 0.0028 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Primary OutFlow Max=3.60 cfs @ 12.39 hrs HW=898.06' TW=897.03' (Dynamic Tailwater)
 ↑1=RCP_Round 24" (Outlet Controls 3.60 cfs @ 3.08 fps)

Pond P8: P-8

Hydrograph



Summary for Pond W-1: W-1

Inflow Area = 0.997 ac, 24.47% Impervious, Inflow Depth = 17.08" for 10-Year event
 Inflow = 3.34 cfs @ 12.27 hrs, Volume= 1.419 af
 Outflow = 2.43 cfs @ 14.34 hrs, Volume= 1.419 af, Atten= 27%, Lag= 124.1 min
 Primary = 2.43 cfs @ 14.34 hrs, Volume= 1.419 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 915.16' @ 14.34 hrs Surf.Area= 0.726 ac Storage= 0.286 af

Plug-Flow detention time= 110.1 min calculated for 1.418 af (100% of inflow)
 Center-of-Mass det. time= 110.1 min (980.3 - 870.2)

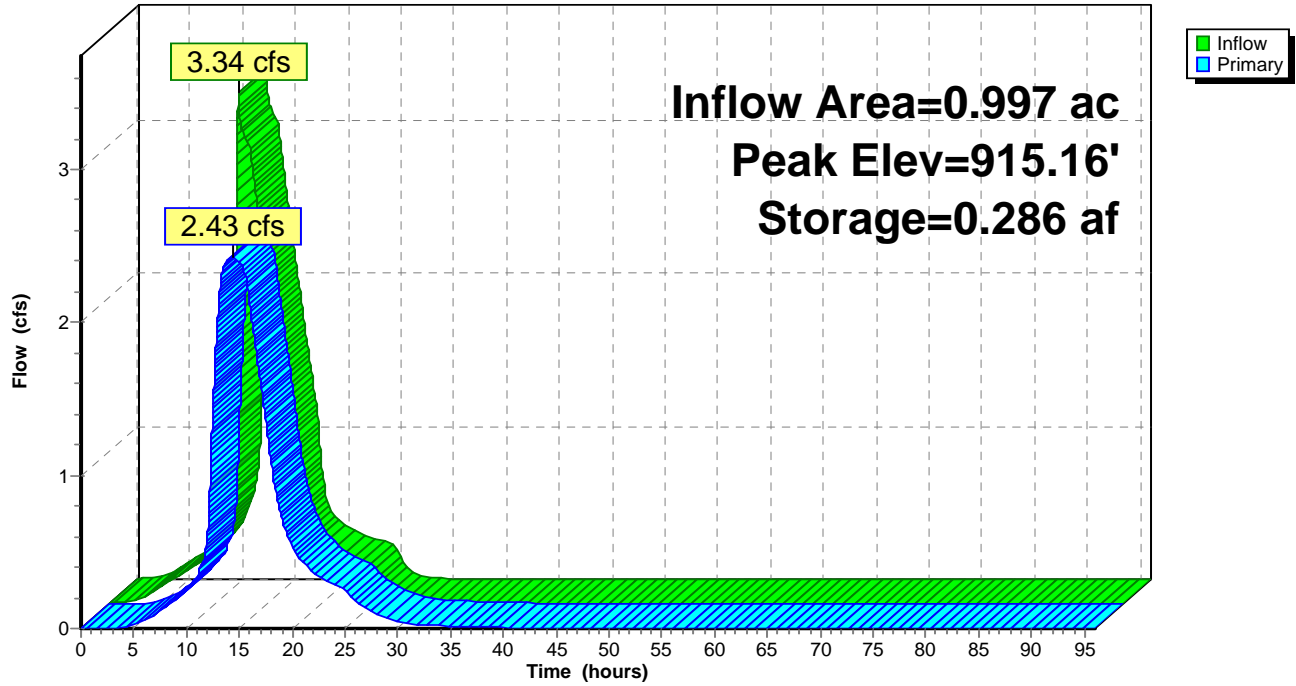
Volume	Invert	Avail.Storage	Storage Description
#1	914.75'	0.950 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
914.75	0.660	0.000	0.000
916.00	0.860	0.950	0.950

Device	Routing	Invert	Outlet Devices
#1	Primary	914.75'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=2.43 cfs @ 14.34 hrs HW=915.16' TW=0.00' (Dynamic Tailwater)
 ↑1=Orifice/Grate (Orifice Controls 2.43 cfs @ 3.09 fps)

Pond W-1: W-1

Hydrograph



Summary for Pond W-2: W-2

Inflow = 2.24 cfs @ 12.70 hrs, Volume= 1.126 af
 Outflow = 0.77 cfs @ 19.38 hrs, Volume= 0.979 af, Atten= 66%, Lag= 401.0 min
 Primary = 0.77 cfs @ 19.38 hrs, Volume= 0.979 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 929.49' @ 19.38 hrs Surf.Area= 1.174 ac Storage= 0.559 af

Plug-Flow detention time= 656.6 min calculated for 0.979 af (87% of inflow)
 Center-of-Mass det. time= 576.0 min (1,617.9 - 1,042.0)

Volume	Invert	Avail.Storage	Storage Description
#1	929.00'	1.175 af	Custom Stage Data (Prismatic) Listed below (Recalc)

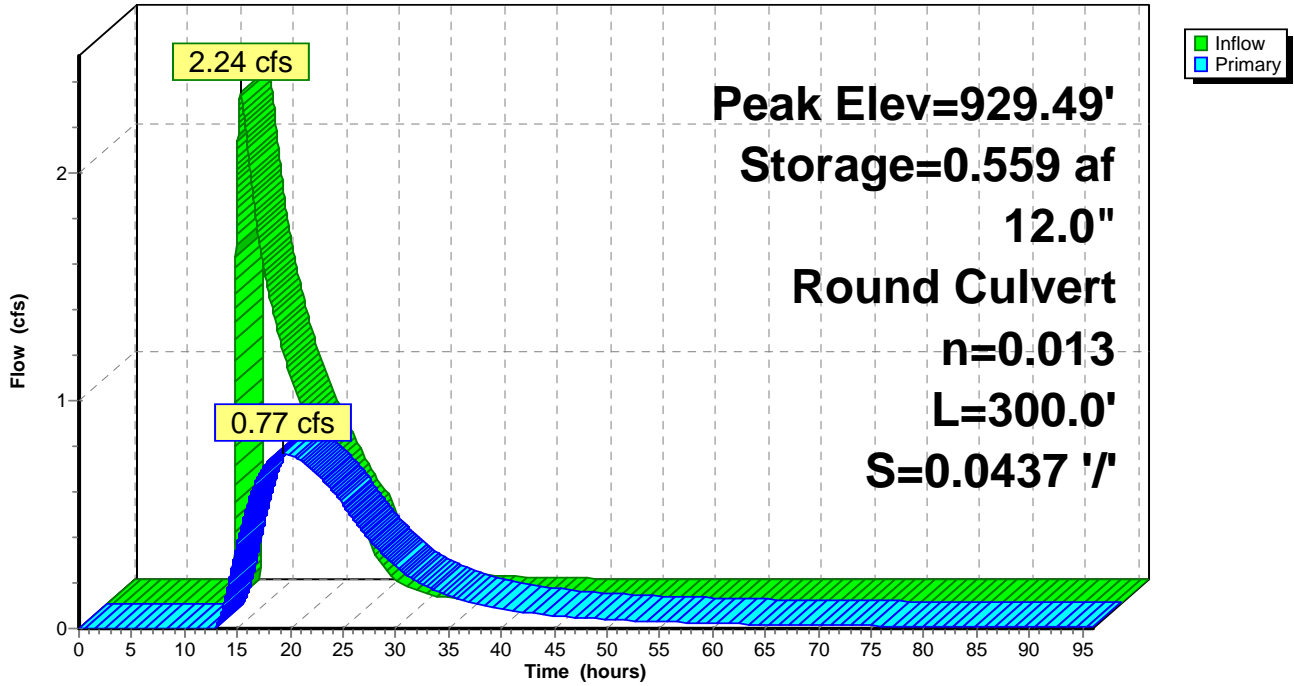
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
929.00	1.090	0.000	0.000
930.00	1.260	1.175	1.175

Device	Routing	Invert	Outlet Devices
#1	Primary	929.10'	12.0" Round RCP_Round 12" L= 300.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 929.10' / 916.00' S= 0.0437 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.77 cfs @ 19.38 hrs HW=929.49' TW=914.88' (Dynamic Tailwater)
 ↳1=RCP_Round 12" (Inlet Controls 0.77 cfs @ 2.67 fps)

Pond W-2: W-2

Hydrograph



Summary for Pond W-3: W-3

Inflow = 0.77 cfs @ 19.38 hrs, Volume= 0.979 af
 Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

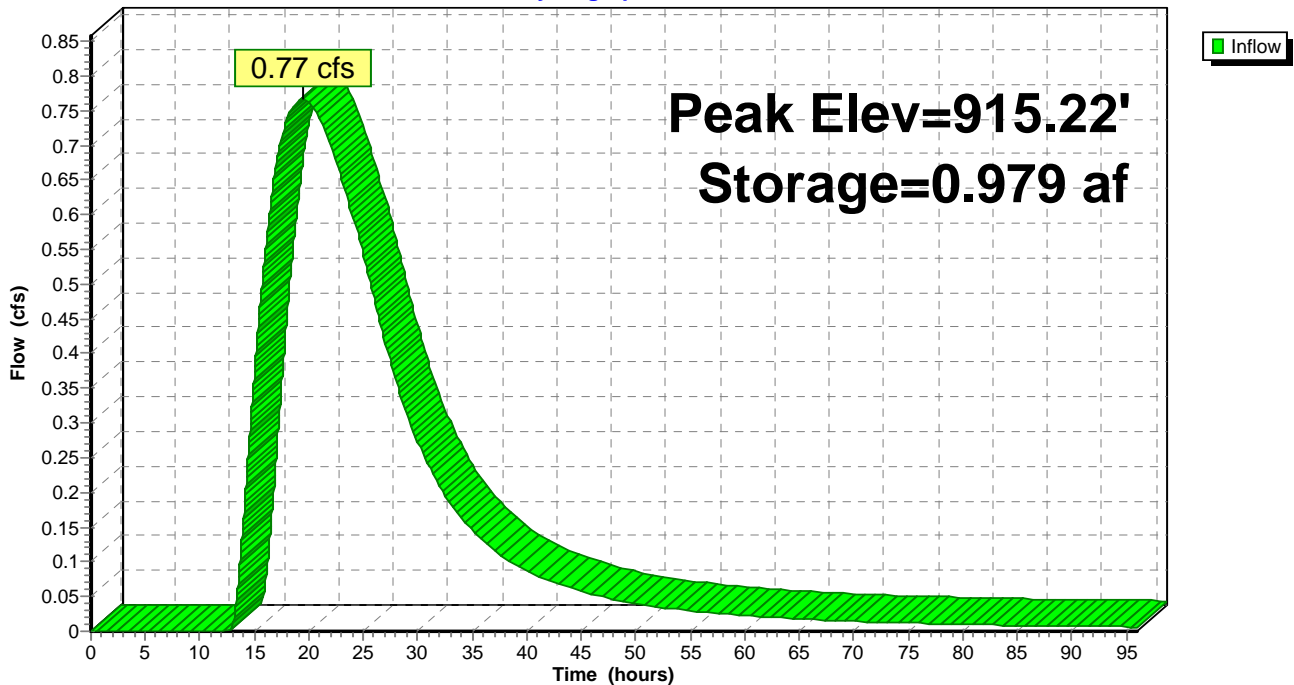
Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 915.22' @ 96.00 hrs Surf.Area= 2.118 ac Storage= 0.979 af

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description
#1	914.75'	2.680 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
914.75	2.040	0.000	0.000
915.00	2.080	0.515	0.515
916.00	2.250	2.165	2.680

Pond W-3: W-3

Hydrograph



Summary for Pond W-4: W-4

Inflow Area = 2.985 ac, 30.99% Impervious, Inflow Depth = 12.81" for 10-Year event
 Inflow = 10.44 cfs @ 12.09 hrs, Volume= 3.187 af
 Outflow = 3.32 cfs @ 16.14 hrs, Volume= 3.168 af, Atten= 68%, Lag= 243.0 min
 Primary = 3.32 cfs @ 16.14 hrs, Volume= 3.168 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 908.99' @ 16.14 hrs Surf.Area= 1.137 ac Storage= 0.949 af

Plug-Flow detention time= 259.4 min calculated for 3.168 af (99% of inflow)
 Center-of-Mass det. time= 254.5 min (1,195.6 - 941.1)

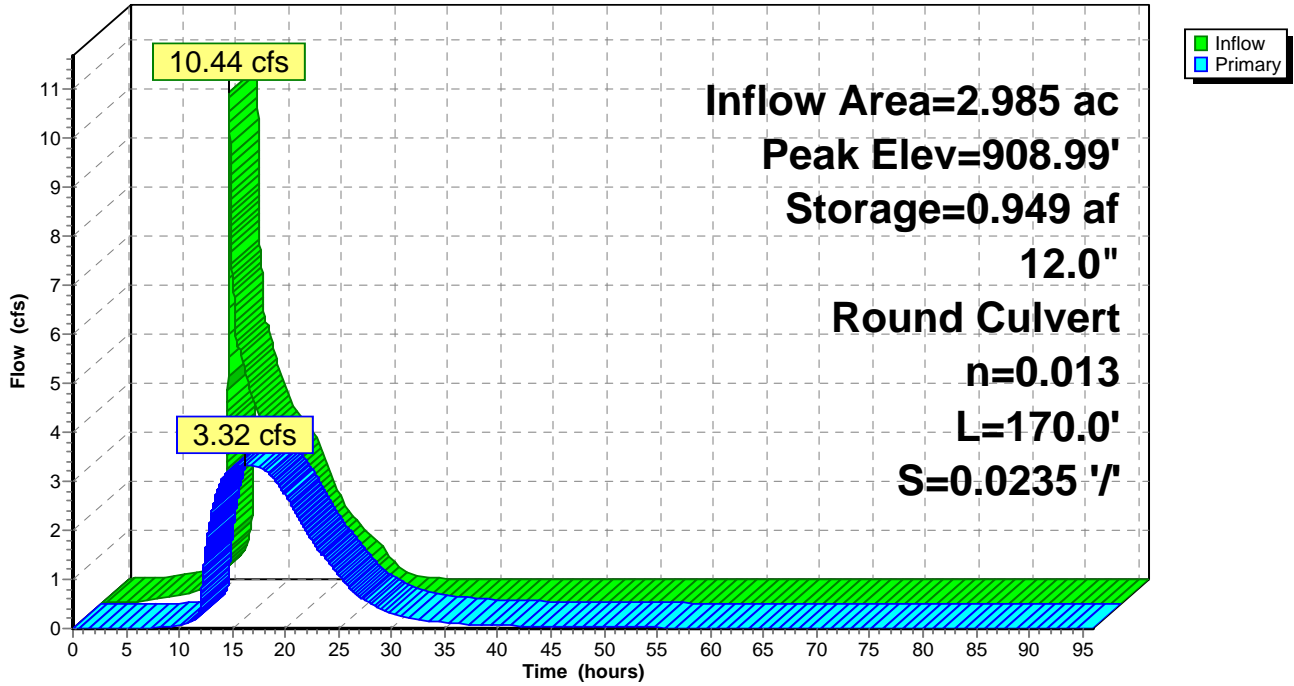
Volume	Invert	Avail.Storage	Storage Description
#1	908.00'	2.280 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
908.00	0.780	0.000	0.000
910.00	1.500	2.280	2.280

Device	Routing	Invert	Outlet Devices
#1	Primary	908.00'	12.0" Round RCP_Round 12" L= 170.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 908.00' / 904.00' S= 0.0235 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=3.32 cfs @ 16.14 hrs HW=908.99' TW=893.91' (Dynamic Tailwater)
 ↳1=RCP_Round 12" (Inlet Controls 3.32 cfs @ 4.24 fps)

Pond W-4: W-4

Hydrograph



Summary for Pond W-5: W-5

Inflow Area = 7.608 ac, 48.41% Impervious, Inflow Depth = 7.01" for 10-Year event
 Inflow = 37.78 cfs @ 12.02 hrs, Volume= 4.445 af
 Outflow = 8.48 cfs @ 13.20 hrs, Volume= 4.438 af, Atten= 78%, Lag= 70.8 min
 Primary = 8.48 cfs @ 13.20 hrs, Volume= 4.438 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 882.75' Surf.Area= 4.887 ac Storage= 7.134 af
 Peak Elev= 883.11' @ 13.20 hrs Surf.Area= 5.311 ac Storage= 8.975 af (1.841 af above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= 235.1 min (1,062.3 - 827.2)

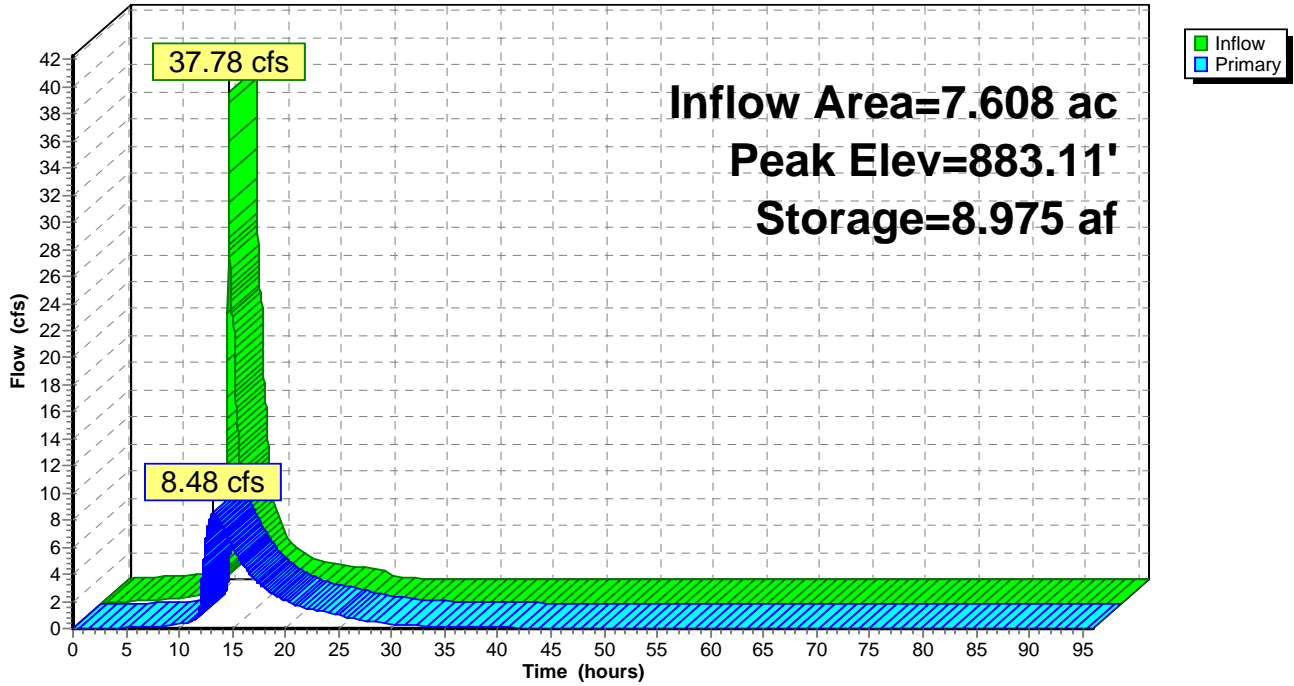
Volume	Invert	Avail.Storage	Storage Description
#1	881.00'	11.097 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
881.00	3.270	0.000	0.000
882.00	4.190	3.730	3.730
883.00	5.120	4.655	8.385
883.49	5.950	2.712	11.097

Device	Routing	Invert	Outlet Devices
#1	Primary	882.75'	6.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Primary	882.75'	6.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)

Primary OutFlow Max=8.48 cfs @ 13.20 hrs HW=883.11' TW=0.00' (Dynamic Tailwater)
 1=Sharp-Crested Rectangular Weir (Weir Controls 4.24 cfs @ 1.97 fps)
 2=Sharp-Crested Rectangular Weir (Weir Controls 4.24 cfs @ 1.97 fps)

Pond W-5: W-5

Hydrograph



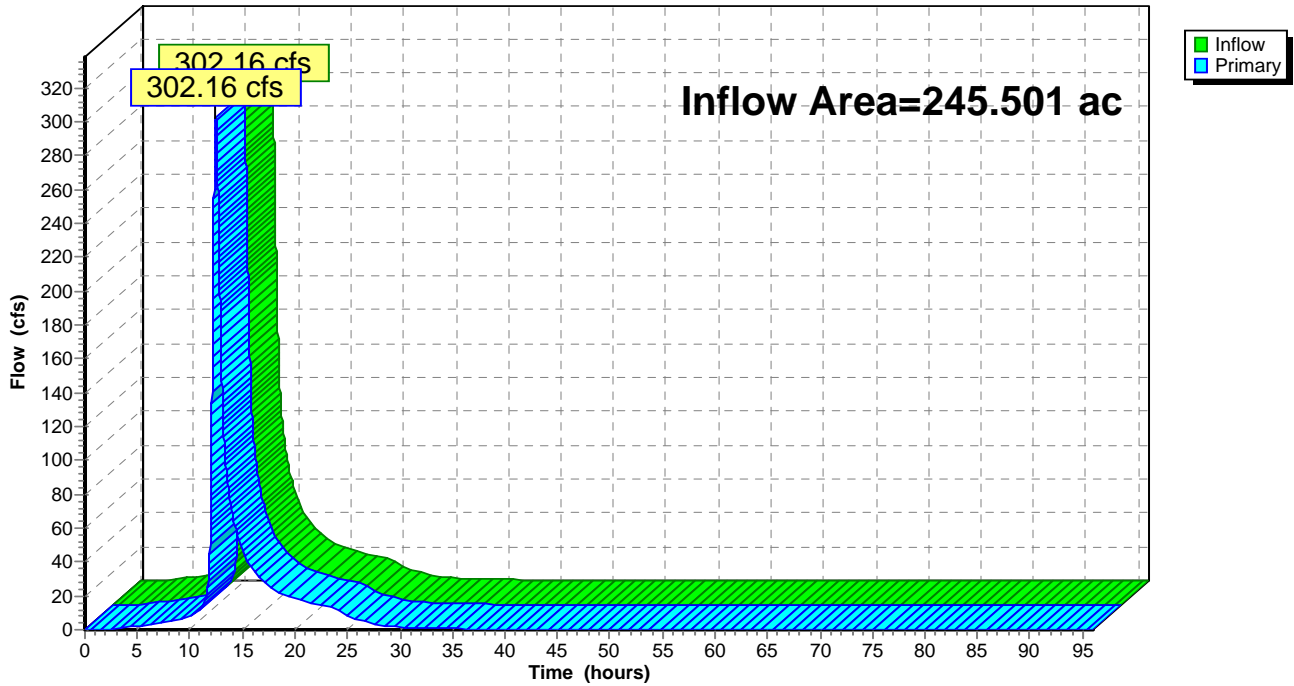
Summary for Link 49L: Sum of P-13 and W-5 Discharges to Rice Creek

Inflow Area = 245.501 ac, 51.49% Impervious, Inflow Depth = 2.75" for 10-Year event
Inflow = 302.16 cfs @ 12.46 hrs, Volume= 56.170 af
Primary = 302.16 cfs @ 12.46 hrs, Volume= 56.170 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 49L: Sum of P-13 and W-5 Discharges to Rice Creek

Hydrograph

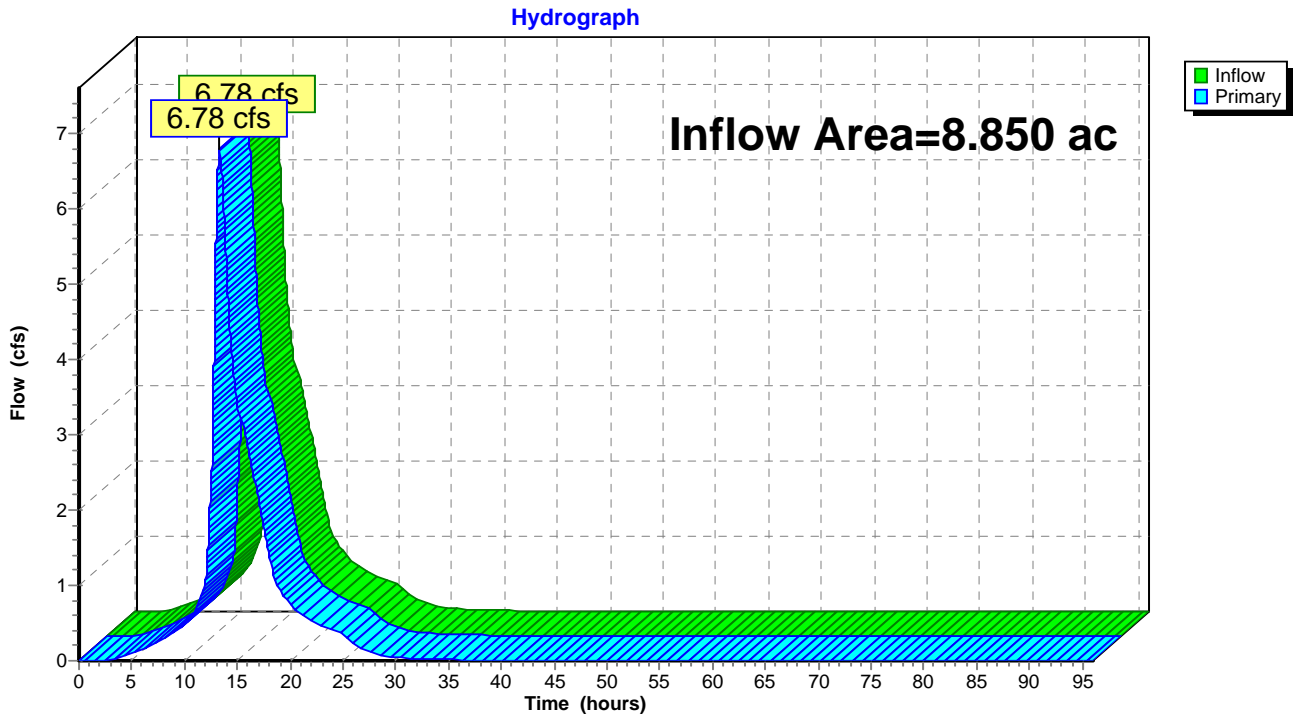


Summary for Link 50L: Outlet #1 Discharge to Round Lake

Inflow Area = 8.850 ac, 65.20% Impervious, Inflow Depth = 3.22" for 10-Year event
Inflow = 6.78 cfs @ 13.30 hrs, Volume= 2.372 af
Primary = 6.78 cfs @ 13.30 hrs, Volume= 2.372 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 50L: Outlet #1 Discharge to Round Lake

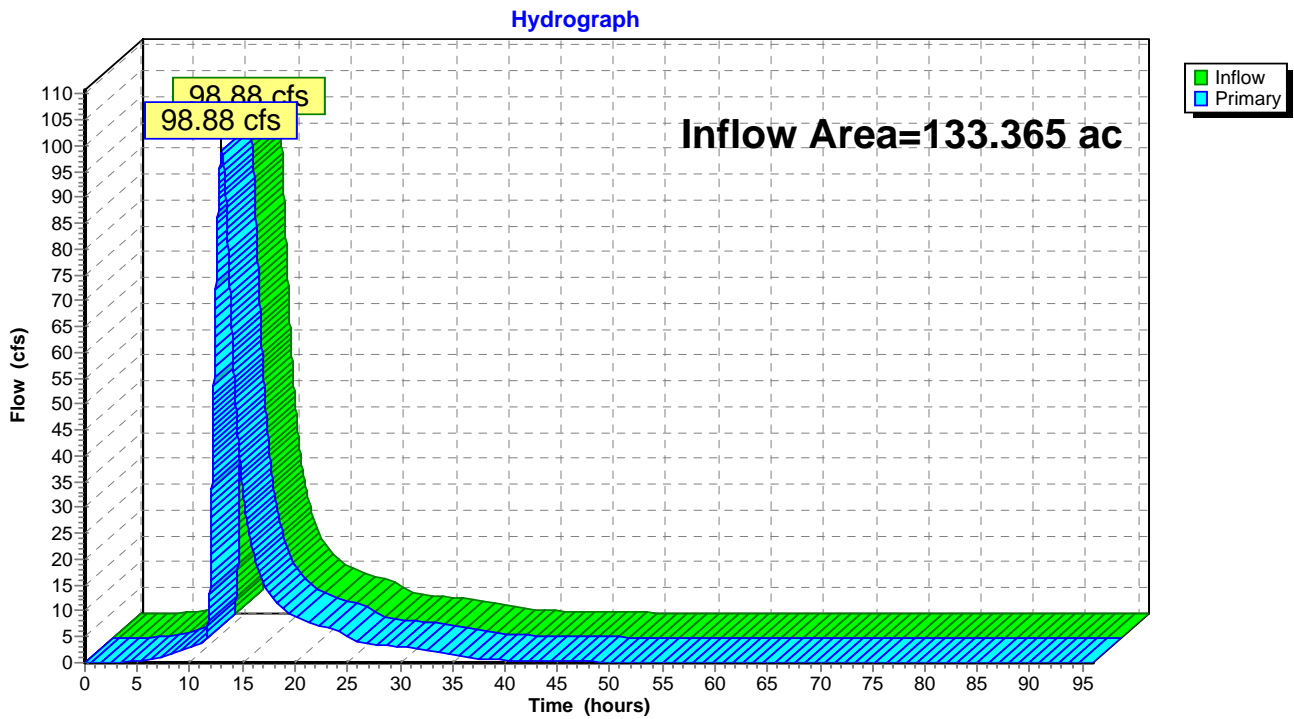


Summary for Link 51L: Outlet #2 Discharge to Round Lake

Inflow Area = 133.365 ac, 58.87% Impervious, Inflow Depth > 2.77" for 10-Year event
Inflow = 98.88 cfs @ 13.04 hrs, Volume= 30.840 af
Primary = 98.88 cfs @ 13.04 hrs, Volume= 30.840 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 51L: Outlet #2 Discharge to Round Lake



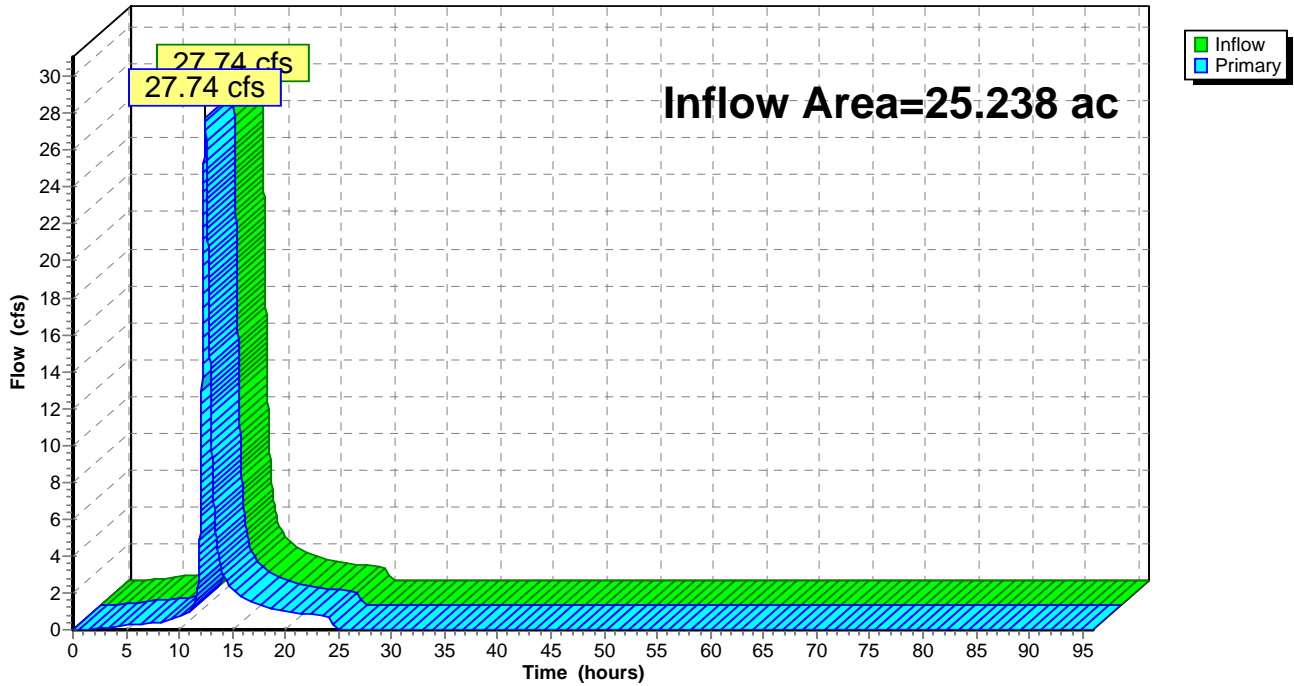
Summary for Link 52L: Outlet #3 Discharge to Round Lake

Inflow Area = 25.238 ac, 19.96% Impervious, Inflow Depth = 1.72" for 10-Year event
Inflow = 27.74 cfs @ 12.43 hrs, Volume= 3.622 af
Primary = 27.74 cfs @ 12.43 hrs, Volume= 3.622 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 52L: Outlet #3 Discharge to Round Lake

Hydrograph



Time span=0.00-96.00 hrs, dt=0.01 hrs, 9601 points
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: To Rice Creek	Runoff Area=1.601 ac 31.98% Impervious Runoff Depth=5.20" Tc=5.7 min CN=74/98 Runoff=11.87 cfs 0.693 af
Subcatchment SB 1: SB 1	Runoff Area=52.192 ac 48.35% Impervious Runoff Depth=5.65" Tc=53.1 min CN=74/98 Runoff=152.90 cfs 24.560 af
Subcatchment SB 10: SB 10	Runoff Area=6.389 ac 7.62% Impervious Runoff Depth=4.52" Tc=7.3 min CN=74/98 Runoff=39.72 cfs 2.409 af
Subcatchment SB 11: SB 11	Runoff Area=3.293 ac 32.16% Impervious Runoff Depth=5.28" Tc=11.7 min CN=74/100 Runoff=18.83 cfs 1.448 af
Subcatchment SB 12: SB 12	Runoff Area=1.382 ac 38.71% Impervious Runoff Depth=5.38" Tc=9.5 min CN=74/98 Runoff=8.83 cfs 0.620 af
Subcatchment SB 13: SB 13	Runoff Area=2.985 ac 30.99% Impervious Runoff Depth=5.24" Tc=9.4 min CN=74/100 Runoff=18.60 cfs 1.304 af
Subcatchment SB 14: SB 14	Runoff Area=10.225 ac 42.62% Impervious Runoff Depth=5.49" Tc=4.3 min CN=74/98 Runoff=84.34 cfs 4.677 af
Subcatchment SB 15: SB 15	Runoff Area=58.564 ac 48.22% Impervious Runoff Depth=5.64" Tc=31.3 min CN=74/98 Runoff=226.38 cfs 27.541 af
Subcatchment SB 16: SB 16	Runoff Area=32.428 ac 33.53% Impervious Runoff Depth=5.24" Tc=12.1 min CN=74/98 Runoff=183.61 cfs 14.156 af
Subcatchment SB 17: SB 17	Runoff Area=7.608 ac 48.41% Impervious Runoff Depth=5.76" Tc=4.3 min CN=74/100 Runoff=64.20 cfs 3.655 af
Subcatchment SB 18: SB 18	Runoff Area=52.908 ac 84.55% Impervious Runoff Depth=6.65" Tc=33.5 min CN=74/98 Runoff=224.72 cfs 29.299 af
Subcatchment SB 19: SB 19	Runoff Area=21.198 ac 39.93% Impervious Runoff Depth=5.41" Tc=24.7 min CN=74/98 Runoff=89.07 cfs 9.565 af
Subcatchment SB 2: SB 2	Runoff Area=11.400 ac 84.29% Impervious Runoff Depth=6.64" Tc=16.6 min CN=74/98 Runoff=67.49 cfs 6.306 af
Subcatchment SB 22: SB 22	Runoff Area=41.911 ac 82.19% Impervious Runoff Depth=6.12" Tc=41.0 min CN=49/98 Runoff=144.97 cfs 21.385 af
Subcatchment SB 24: SB 24	Runoff Area=4.939 ac 98.22% Impervious Runoff Depth=7.02" Tc=7.5 min CN=74/98 Runoff=42.62 cfs 2.890 af
Subcatchment SB 25: SB 25	Runoff Area=5.012 ac 95.71% Impervious Runoff Depth=6.95" Tc=10.7 min CN=74/98 Runoff=37.25 cfs 2.904 af

Full Buildout_HydroCAD_2015 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

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Subcatchment SB 26: SB 26	Runoff Area=14.335 ac 98.27% Impervious Runoff Depth=7.02" Tc=25.4 min CN=74/98 Runoff=73.07 cfs 8.390 af
Subcatchment SB 27: SB 27 (Thumb Road)	Runoff Area=6.629 ac 97.12% Impervious Runoff Depth=6.92" Tc=27.6 min CN=49/98 Runoff=31.89 cfs 3.821 af
Subcatchment SB 28: SB 28	Runoff Area=6.955 ac 46.76% Impervious Runoff Depth=5.60" Tc=14.6 min CN=74/98 Runoff=38.37 cfs 3.247 af
Subcatchment SB 29: SB 29	Runoff Area=10.214 ac 37.73% Impervious Runoff Depth=5.35" Tc=19.1 min CN=74/98 Runoff=48.13 cfs 4.557 af
Subcatchment SB 3: SB 3	Runoff Area=37.668 ac 41.46% Impervious Runoff Depth=5.46" Tc=15.3 min CN=74/98 Runoff=199.96 cfs 17.130 af
Subcatchment SB 4: SB 4	Runoff Area=0.599 ac 19.70% Impervious Runoff Depth=4.90" Tc=5.9 min CN=74/100 Runoff=4.20 cfs 0.245 af
Subcatchment SB 5: SB 5	Runoff Area=7.853 ac 70.37% Impervious Runoff Depth=6.25" Tc=59.3 min CN=74/98 Runoff=23.32 cfs 4.093 af
Subcatchment SB 51: Offsite Subbasin 51	Runoff Area=25.238 ac 19.96% Impervious Runoff Depth=4.09" Tc=32.8 min CN=65/98 Runoff=70.31 cfs 8.599 af
Subcatchment SB 6: SB 6	Runoff Area=0.997 ac 24.47% Impervious Runoff Depth=5.05" Tc=20.3 min CN=74/100 Runoff=4.33 cfs 0.419 af
Subcatchment SB 7: SB 7	Runoff Area=21.555 ac 84.83% Impervious Runoff Depth=6.65" Tc=5.7 min CN=74/98 Runoff=192.64 cfs 11.950 af
Subcatchment SB 8: SB 8	Runoff Area=29.595 ac 30.01% Impervious Runoff Depth=5.14" Tc=47.1 min CN=74/98 Runoff=86.02 cfs 12.680 af
Subcatchment SB 9: SB 9	Runoff Area=25.789 ac 33.17% Impervious Runoff Depth=5.23" Tc=30.0 min CN=74/98 Runoff=96.02 cfs 11.237 af
Pond 3P: P-3	Peak Elev=919.71' Storage=19.292 af Inflow=321.63 cfs 62.160 af Outflow=267.75 cfs 62.142 af
Pond 4P: P-4	Peak Elev=917.80' Storage=1.640 af Inflow=23.32 cfs 4.093 af Primary=12.66 cfs 2.270 af Secondary=3.35 cfs 1.823 af Outflow=16.01 cfs 4.093 af
Pond 7P: P-7	Peak Elev=915.93' Storage=1.523 af Inflow=86.02 cfs 12.680 af Outflow=87.44 cfs 12.680 af
Pond 9P: P-9	Peak Elev=915.84' Storage=0.601 af Inflow=166.30 cfs 23.916 af Outflow=166.11 cfs 23.916 af
Pond 10P: P-10	Peak Elev=898.47' Storage=1.545 af Inflow=164.99 cfs 24.555 af Primary=15.81 cfs 11.415 af Secondary=148.71 cfs 13.140 af Outflow=164.52 cfs 24.555 af
Pond 11P: P-11	Peak Elev=912.79' Storage=9.054 af Inflow=172.55 cfs 25.364 af Primary=155.34 cfs 21.533 af Secondary=4.92 cfs 3.830 af Outflow=160.26 cfs 25.363 af

Pond 12P: P-12 Peak Elev=895.80' Storage=10.192 af Inflow=175.25 cfs 34.346 af
Outflow=138.66 cfs 34.338 af

Pond 13P: P-13 Peak Elev=885.58' Storage=10.282 af Inflow=652.01 cfs 110.403 af
Primary=608.53 cfs 105.483 af Secondary=20.47 cfs 4.917 af Outflow=629.00 cfs 110.401 af

Pond 14P: P-14 Peak Elev=895.17' Storage=9.341 af Inflow=89.07 cfs 9.565 af
Outflow=16.07 cfs 9.563 af

Pond 23P: Thumb Infiltration (Thumb TP Peak Elev=903.88' Storage=3.880 af Inflow=171.27 cfs 25.207 af
Outflow=171.19 cfs 21.467 af

Pond 31P: SB 18 Infiltration Peak Elev=903.48' Storage=3.485 af Inflow=224.72 cfs 29.299 af
Outflow=224.59 cfs 25.979 af

Pond 36P: Culverts passing flow Peak Elev=890.33' Storage=0.013 af Inflow=224.59 cfs 25.979 af
Primary=125.00 cfs 22.689 af Secondary=99.53 cfs 3.290 af Outflow=224.53 cfs 25.979 af

Pond CRH-1: CRH-1 Peak Elev=878.81' Storage=0.760 af Inflow=38.37 cfs 3.247 af
Discarded=0.37 cfs 0.563 af Primary=25.16 cfs 2.685 af Outflow=25.53 cfs 3.247 af

Pond CRH-2: CRH-2 Peak Elev=883.78' Storage=1.468 af Inflow=48.13 cfs 4.557 af
Discarded=0.47 cfs 0.986 af Primary=27.38 cfs 3.571 af Outflow=27.85 cfs 4.557 af

Pond CRH-3: CRH-3 Peak Elev=879.83' Storage=0.769 af Inflow=30.15 cfs 4.265 af
Discarded=0.38 cfs 0.519 af Primary=25.52 cfs 3.745 af Outflow=25.90 cfs 4.265 af

Pond P1/P2: P-1/P-2 Peak Elev=925.63' Storage=5.762 af Inflow=181.28 cfs 33.757 af
Outflow=177.47 cfs 33.749 af

Pond P5/P6: P-5/P-6 Peak Elev=932.56' Storage=13.463 af Inflow=234.37 cfs 20.279 af
Primary=119.36 cfs 16.461 af Secondary=3.15 cfs 1.701 af Outflow=122.50 cfs 18.162 af

Pond P8: P-8 Peak Elev=899.15' Storage=1.415 af Inflow=39.72 cfs 2.409 af
24.0" Round Culvert n=0.013 L=380.0' S=0.0028 '/' Outflow=11.00 cfs 2.403 af

Pond W-1: W-1 Peak Elev=915.37' Storage=0.443 af Inflow=6.17 cfs 2.242 af
Outflow=2.99 cfs 2.242 af

Pond W-2: W-2 Peak Elev=929.60' Storage=0.681 af Inflow=3.15 cfs 1.701 af
12.0" Round Culvert n=0.013 L=300.0' S=0.0437 '/' Outflow=1.17 cfs 1.553 af

Pond W-3: W-3 Peak Elev=915.49' Storage=1.553 af Inflow=1.17 cfs 1.553 af
Outflow=0.00 cfs 0.000 af

Pond W-4: W-4 Peak Elev=909.35' Storage=1.383 af Inflow=22.11 cfs 5.134 af
12.0" Round Culvert n=0.013 L=170.0' S=0.0235 '/' Outflow=4.36 cfs 5.114 af

Pond W-5: W-5 Peak Elev=883.35' Storage=10.267 af Inflow=76.11 cfs 8.572 af
Outflow=17.76 cfs 8.565 af

Link 49L: Sum of P-13 and W-5 Discharges to Rice Creek

Inflow=623.69 cfs 114.048 af
Primary=623.69 cfs 114.048 af

Link 50L: Outlet #1 Discharge to Round Lake

Inflow=15.52 cfs 4.512 af
Primary=15.52 cfs 4.512 af

Link 51L: Outlet #2 Discharge to Round Lake

Inflow=267.75 cfs 62.142 af
Primary=267.75 cfs 62.142 af

Link 52L: Outlet #3 Discharge to Round Lake

Inflow=70.31 cfs 8.599 af
Primary=70.31 cfs 8.599 af

Total Runoff Area = 501.462 ac Runoff Volume = 239.780 af Average Runoff Depth = 5.74"
45.62% Pervious = 228.758 ac 54.38% Impervious = 272.704 ac

Summary for Subcatchment 1S: To Rice Creek

Runoff = 11.87 cfs @ 12.03 hrs, Volume= 0.693 af, Depth= 5.20"

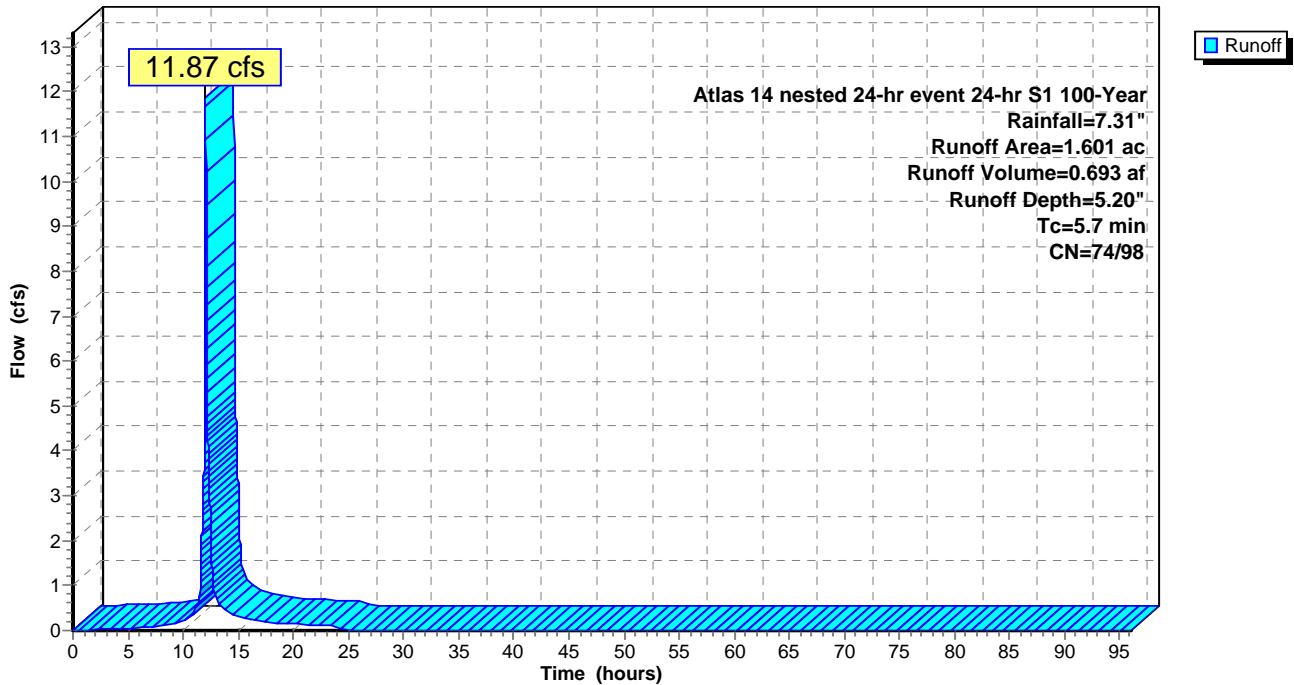
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 0.512	98	impervious
* 1.089	74	pervious
1.601	82	Weighted Average
1.089		68.02% Pervious Area
0.512		31.98% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.7					Direct Entry,

Subcatchment 1S: To Rice Creek

Hydrograph



Summary for Subcatchment SB 1: SB 1

Runoff = 152.90 cfs @ 12.68 hrs, Volume= 24.560 af, Depth= 5.65"

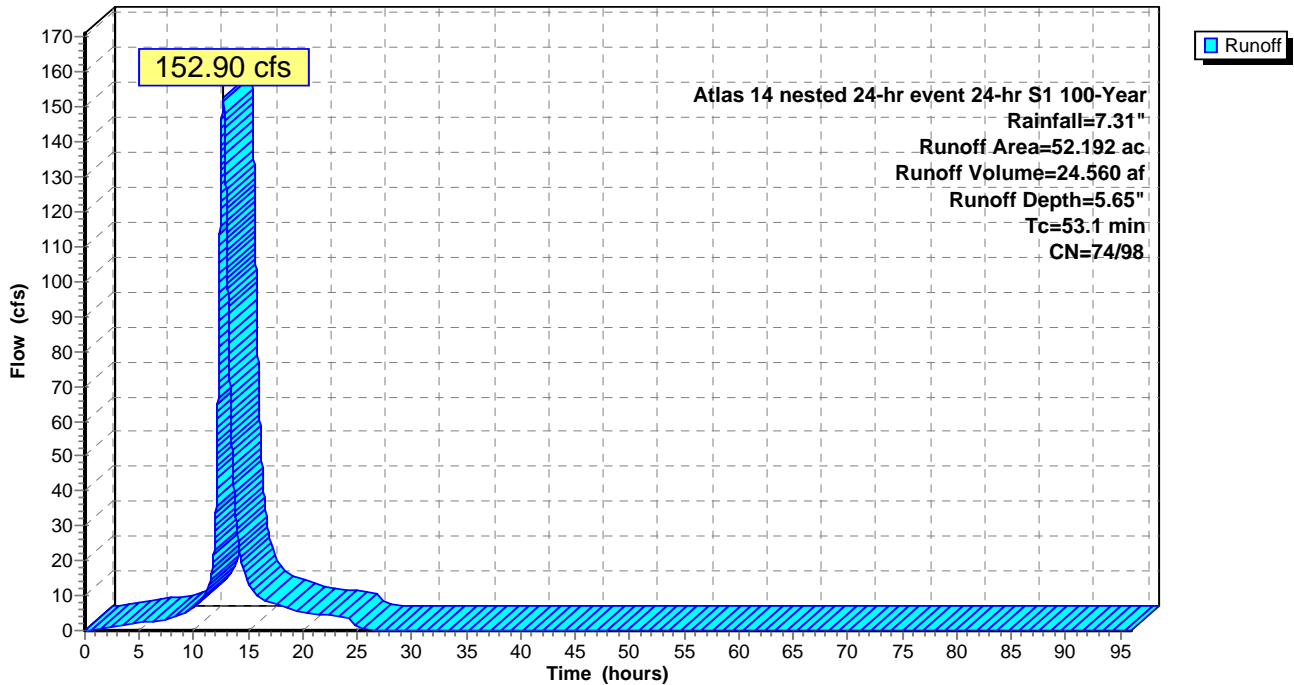
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 26.958	74	pervious
* 25.234	98	impervious
52.192	86	Weighted Average
26.958		51.65% Pervious Area
25.234		48.35% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
53.1					Direct Entry,

Subcatchment SB 1: SB 1

Hydrograph



Summary for Subcatchment SB 10: SB 10

Runoff = 39.72 cfs @ 12.05 hrs, Volume= 2.409 af, Depth= 4.52"

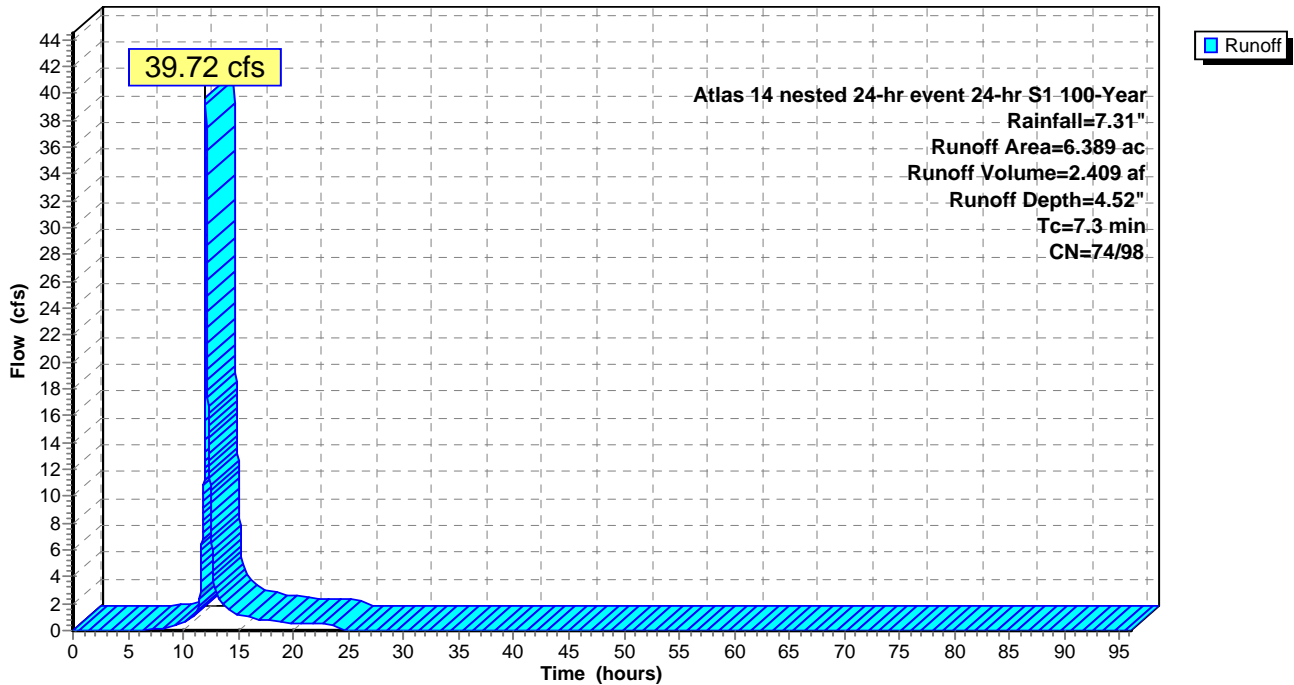
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 5.902	74	pervious
* 0.487	98	impervious
6.389	76	Weighted Average
5.902		92.38% Pervious Area
0.487		7.62% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.3					Direct Entry,

Subcatchment SB 10: SB 10

Hydrograph



Summary for Subcatchment SB 11: SB 11

Runoff = 18.83 cfs @ 12.11 hrs, Volume= 1.448 af, Depth= 5.28"

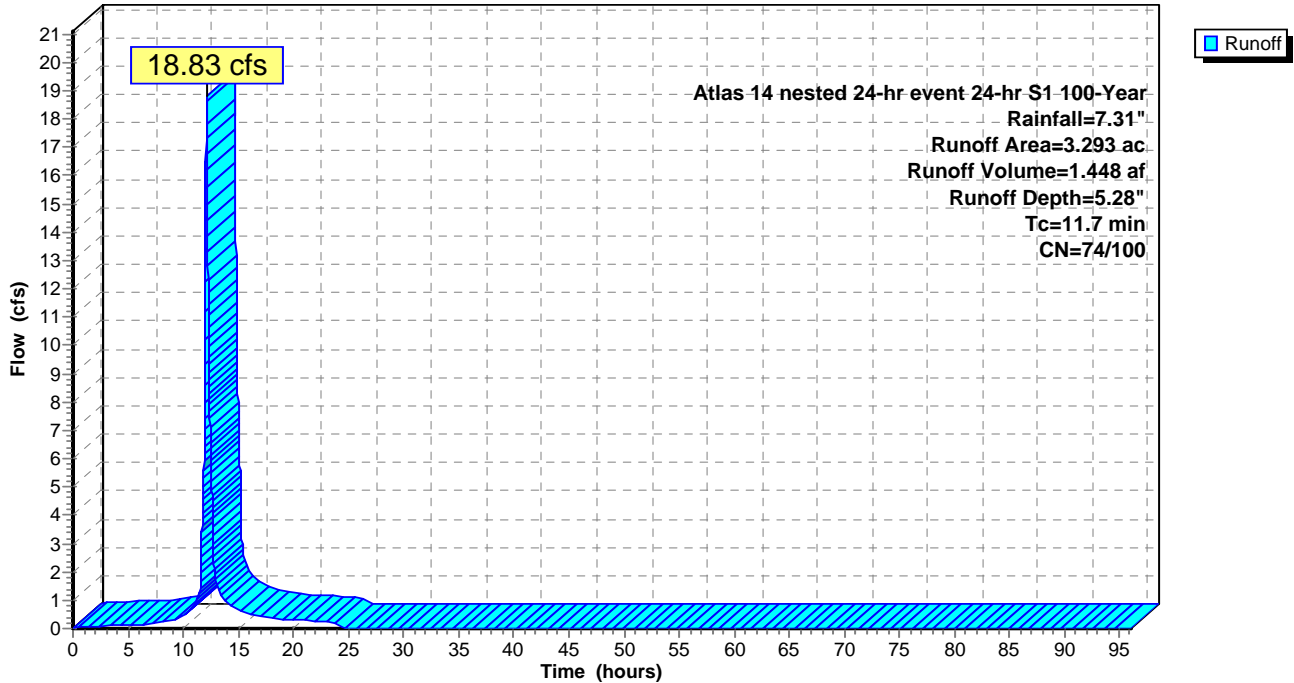
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 2.234	74	pervious
* 1.059	100	impervious
3.293	82	Weighted Average
2.234		67.84% Pervious Area
1.059		32.16% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.7					Direct Entry,

Subcatchment SB 11: SB 11

Hydrograph



Summary for Subcatchment SB 12: SB 12

Runoff = 8.83 cfs @ 12.08 hrs, Volume= 0.620 af, Depth= 5.38"

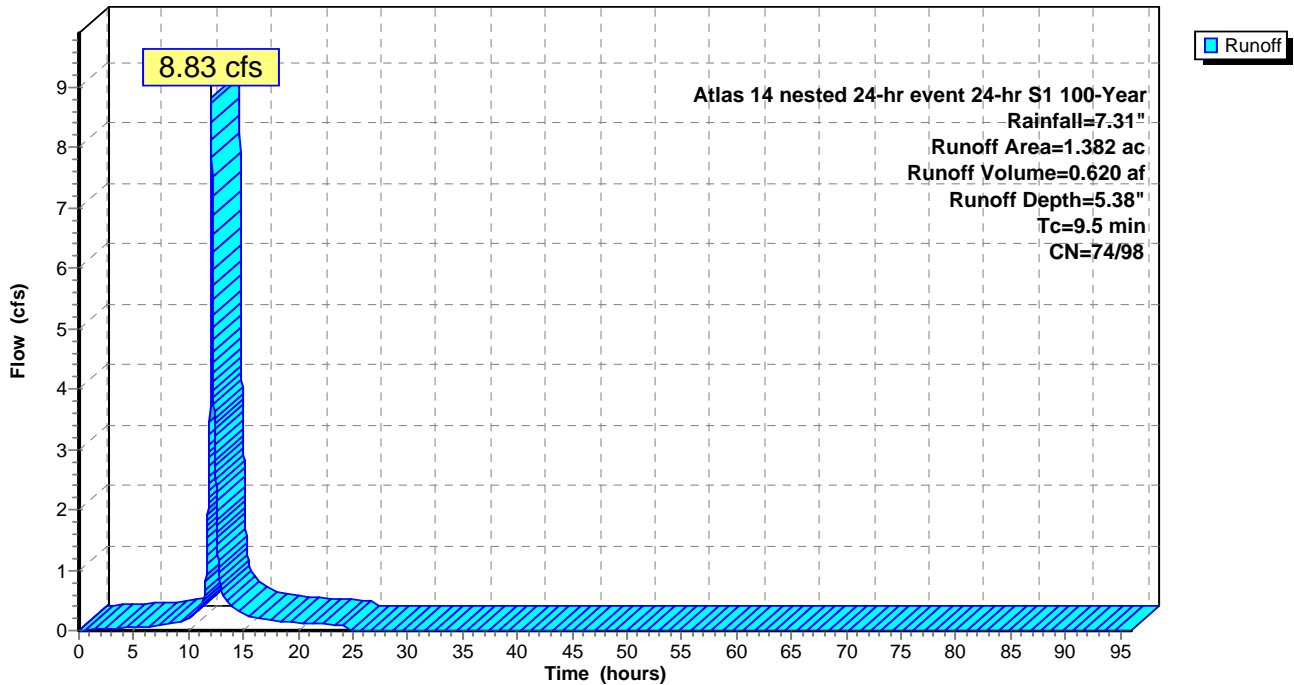
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 0.847	74	pervious
* 0.535	98	impervious
1.382	83	Weighted Average
0.847		61.29% Pervious Area
0.535		38.71% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.5					Direct Entry,

Subcatchment SB 12: SB 12

Hydrograph



Summary for Subcatchment SB 13: SB 13

Runoff = 18.60 cfs @ 12.08 hrs, Volume= 1.304 af, Depth= 5.24"

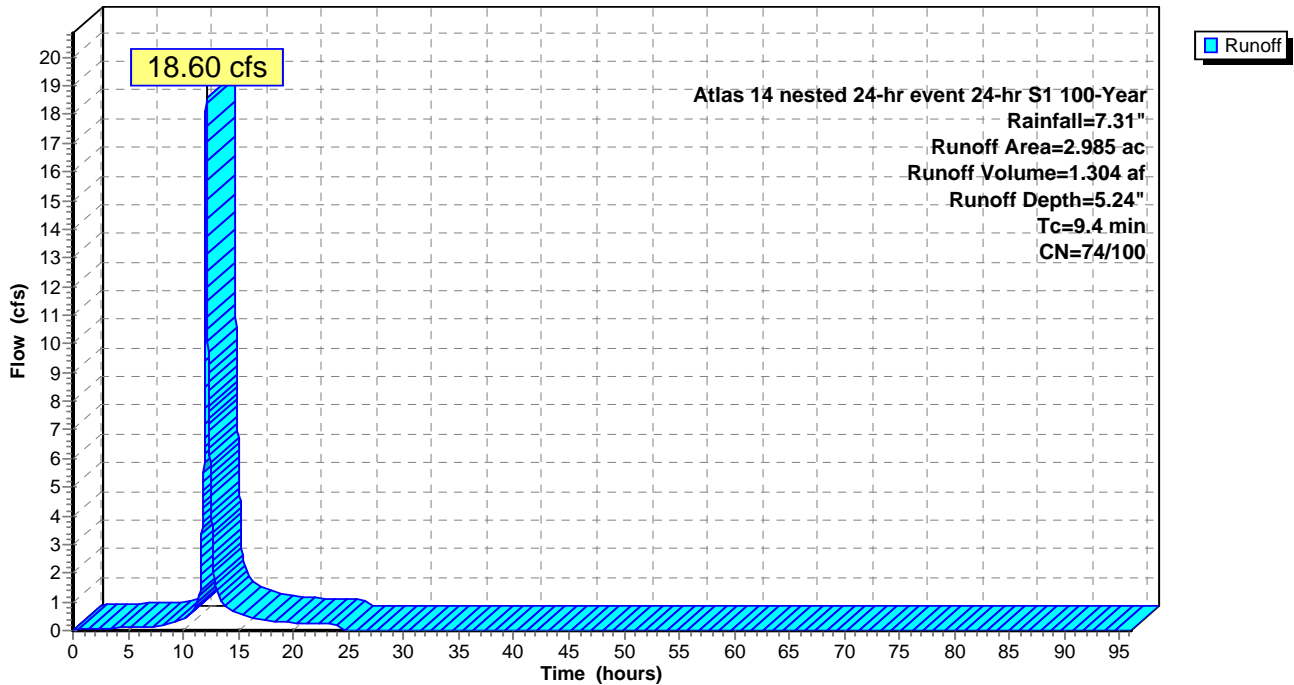
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 2.060	74	pervious
* 0.925	100	impervious
2.985	82	Weighted Average
2.060		69.01% Pervious Area
0.925		30.99% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.4					Direct Entry,

Subcatchment SB 13: SB 13

Hydrograph



Summary for Subcatchment SB 14: SB 14

Runoff = 84.34 cfs @ 12.02 hrs, Volume= 4.677 af, Depth= 5.49"

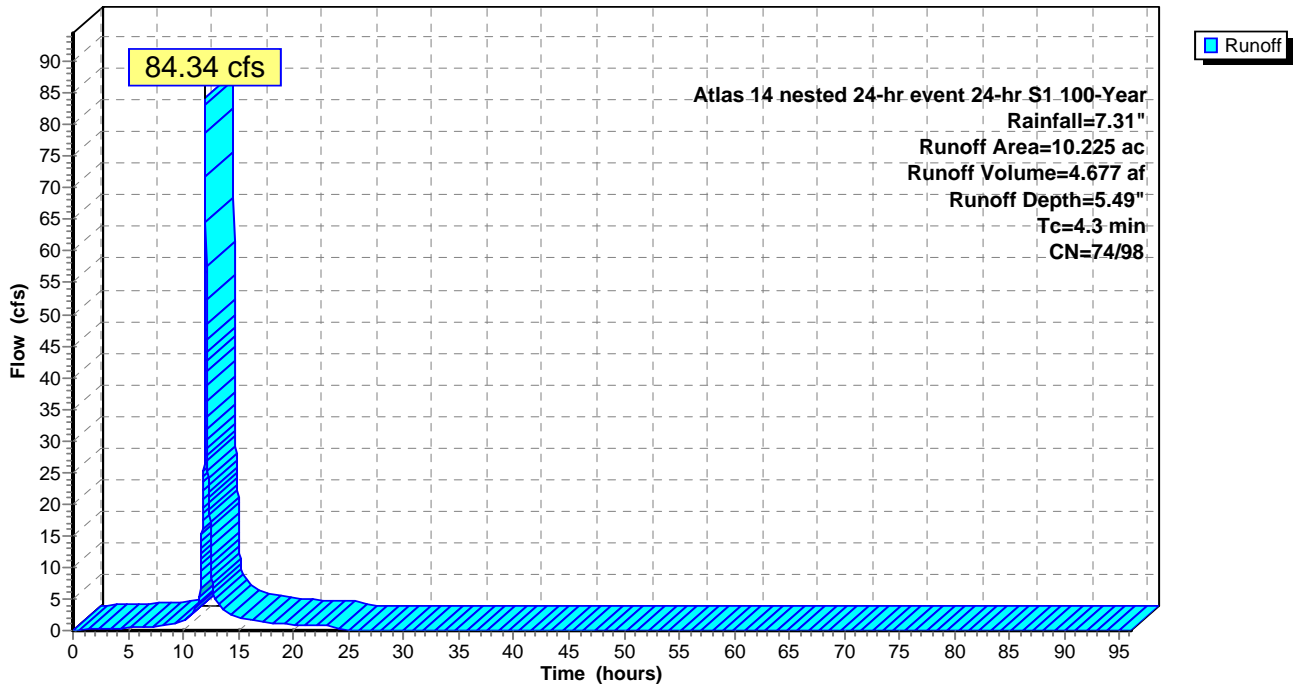
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 5.867	74	pervious
* 4.358	98	impervious
10.225	84	Weighted Average
5.867		57.38% Pervious Area
4.358		42.62% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.3					Direct Entry,

Subcatchment SB 14: SB 14

Hydrograph



Summary for Subcatchment SB 15: SB 15

Runoff = 226.38 cfs @ 12.38 hrs, Volume= 27.541 af, Depth= 5.64"

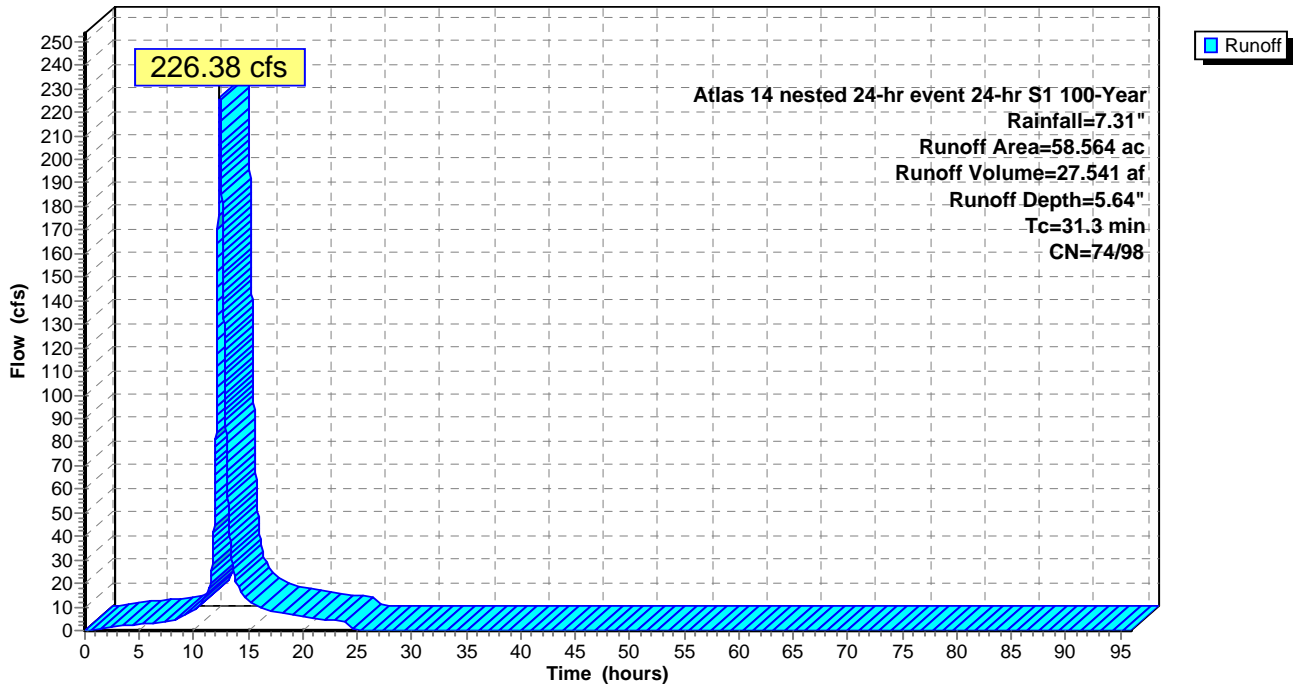
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 30.326	74	pervious
* 28.238	98	impervious
58.564	86	Weighted Average
30.326		51.78% Pervious Area
28.238		48.22% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
31.3					Direct Entry,

Subcatchment SB 15: SB 15

Hydrograph



Summary for Subcatchment SB 16: SB 16

Runoff = 183.61 cfs @ 12.12 hrs, Volume= 14.156 af, Depth= 5.24"

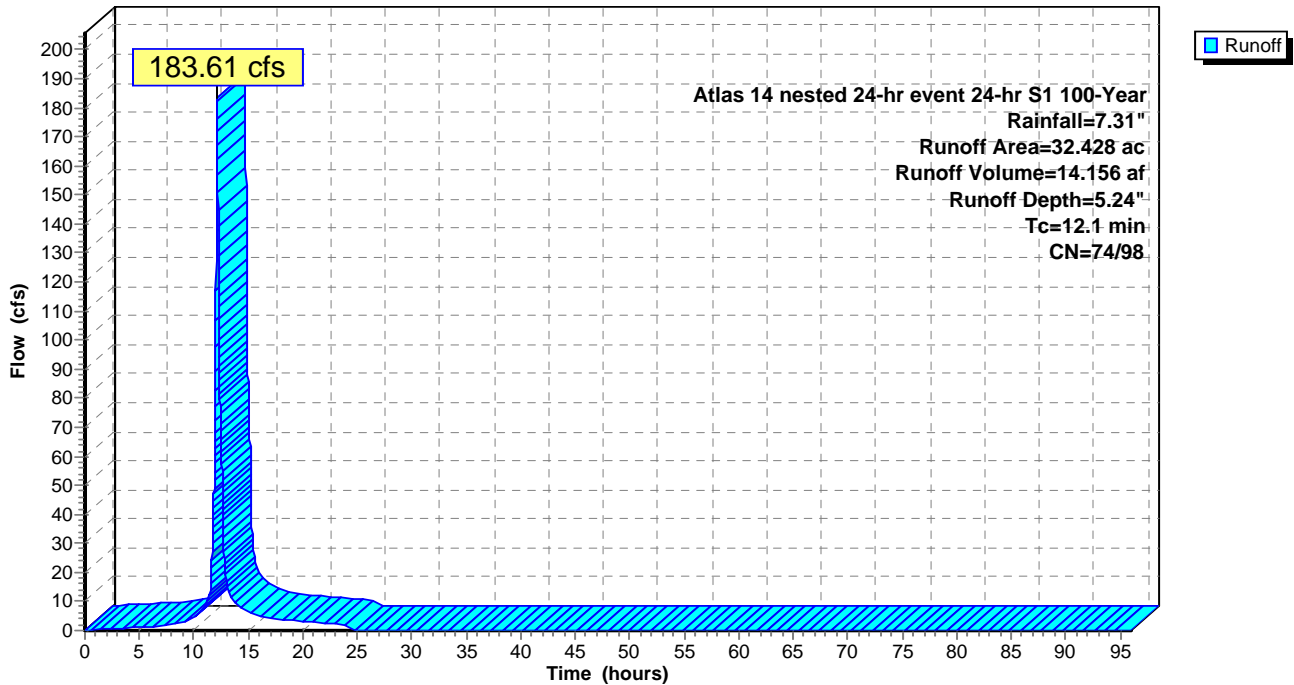
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 21.555	74	pervious
* 10.873	98	impervious
32.428	82	Weighted Average
21.555		66.47% Pervious Area
10.873		33.53% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.1					Direct Entry,

Subcatchment SB 16: SB 16

Hydrograph



Summary for Subcatchment SB 17: SB 17

Runoff = 64.20 cfs @ 12.02 hrs, Volume= 3.655 af, Depth= 5.76"

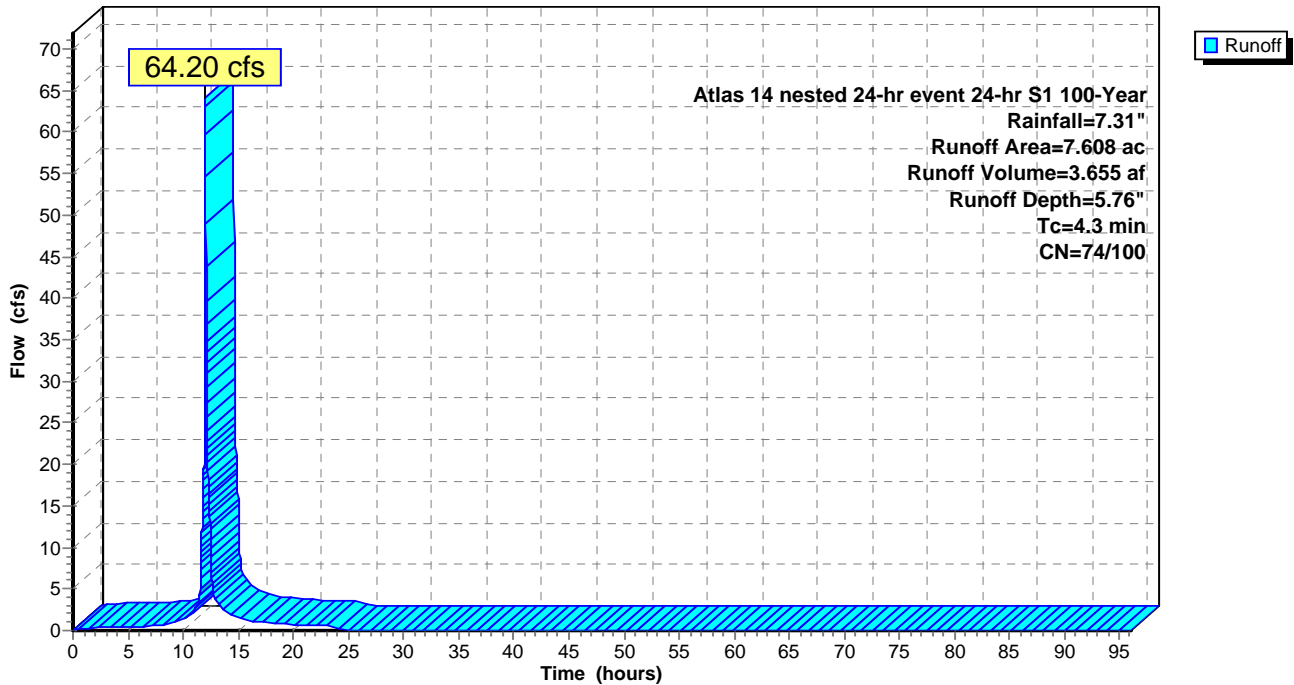
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 3.925	74	pervious
* 3.683	100	impervious
7.608	87	Weighted Average
3.925		51.59% Pervious Area
3.683		48.41% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.3					Direct Entry,

Subcatchment SB 17: SB 17

Hydrograph



Summary for Subcatchment SB 18: SB 18

Runoff = 224.72 cfs @ 12.40 hrs, Volume= 29.299 af, Depth= 6.65"

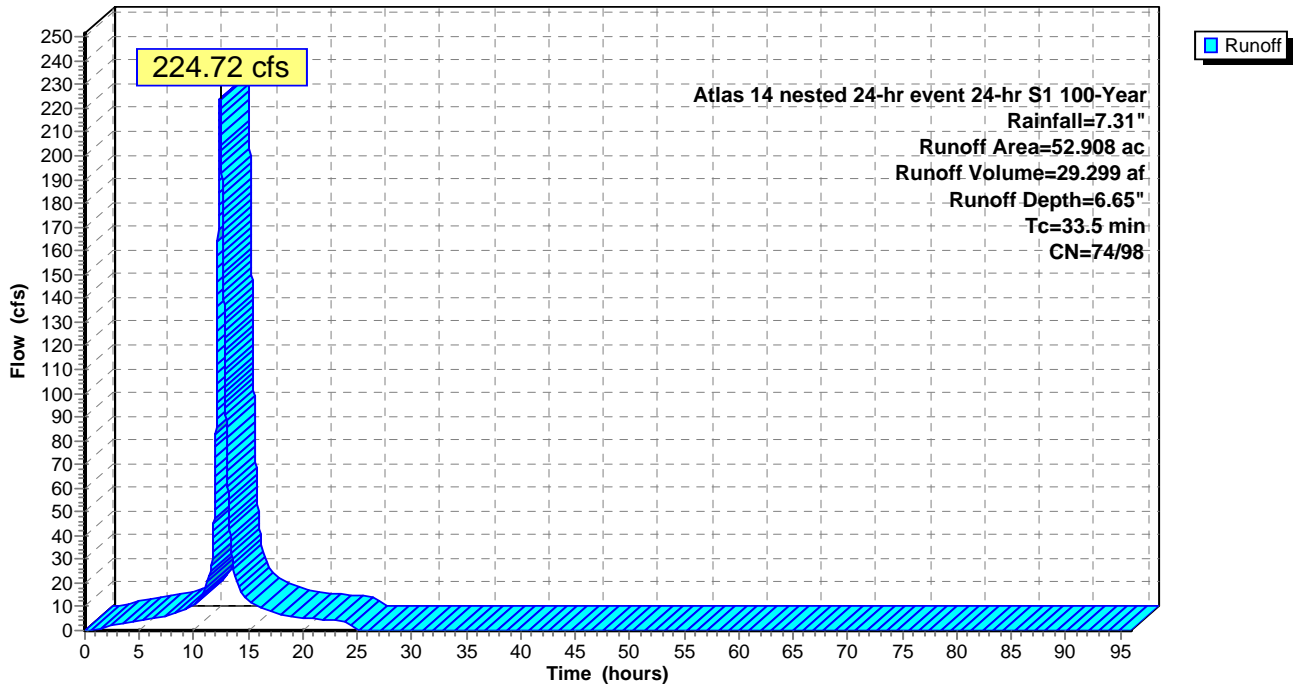
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 8.172	74	pervious
* 44.736	98	impervious
52.908	94	Weighted Average
8.172		15.45% Pervious Area
44.736		84.55% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
33.5					Direct Entry,

Subcatchment SB 18: SB 18

Hydrograph



Summary for Subcatchment SB 19: SB 19

Runoff = 89.07 cfs @ 12.29 hrs, Volume= 9.565 af, Depth= 5.41"

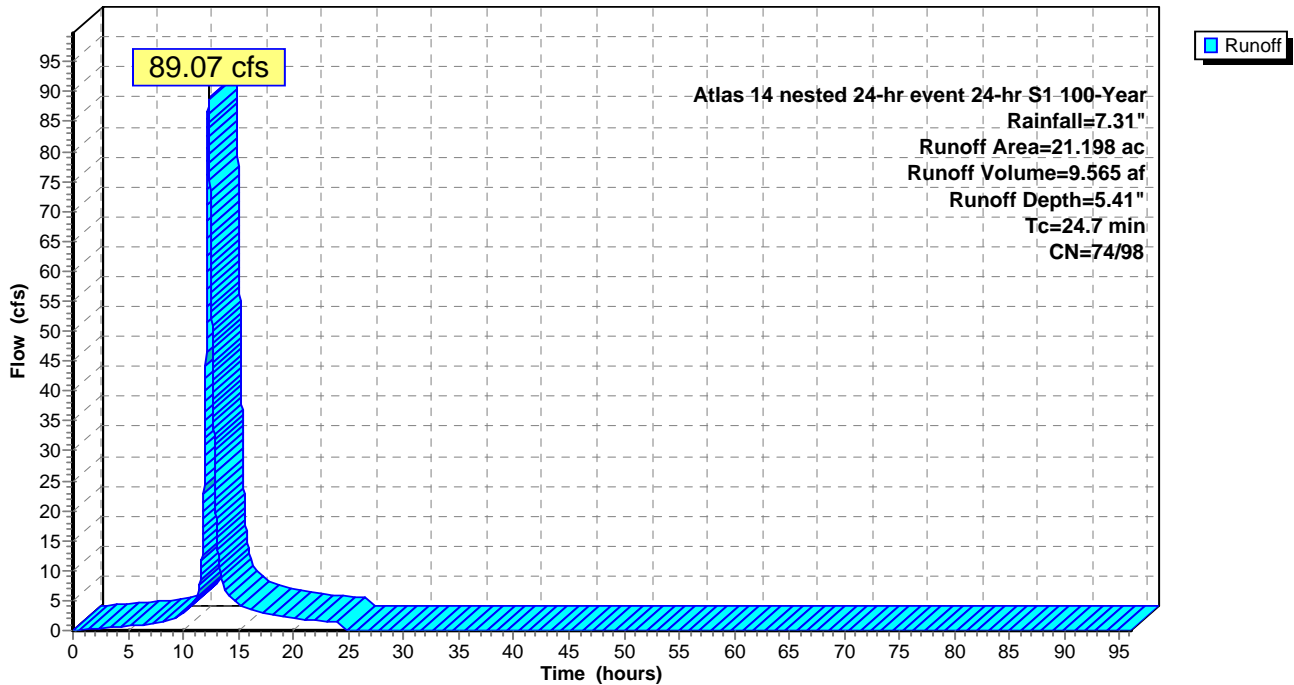
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 12.734	74	pervious
* 8.464	98	impervious
21.198	84	Weighted Average
12.734		60.07% Pervious Area
8.464		39.93% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.7					Direct Entry,

Subcatchment SB 19: SB 19

Hydrograph



Summary for Subcatchment SB 2: SB 2

Runoff = 67.49 cfs @ 12.17 hrs, Volume= 6.306 af, Depth= 6.64"

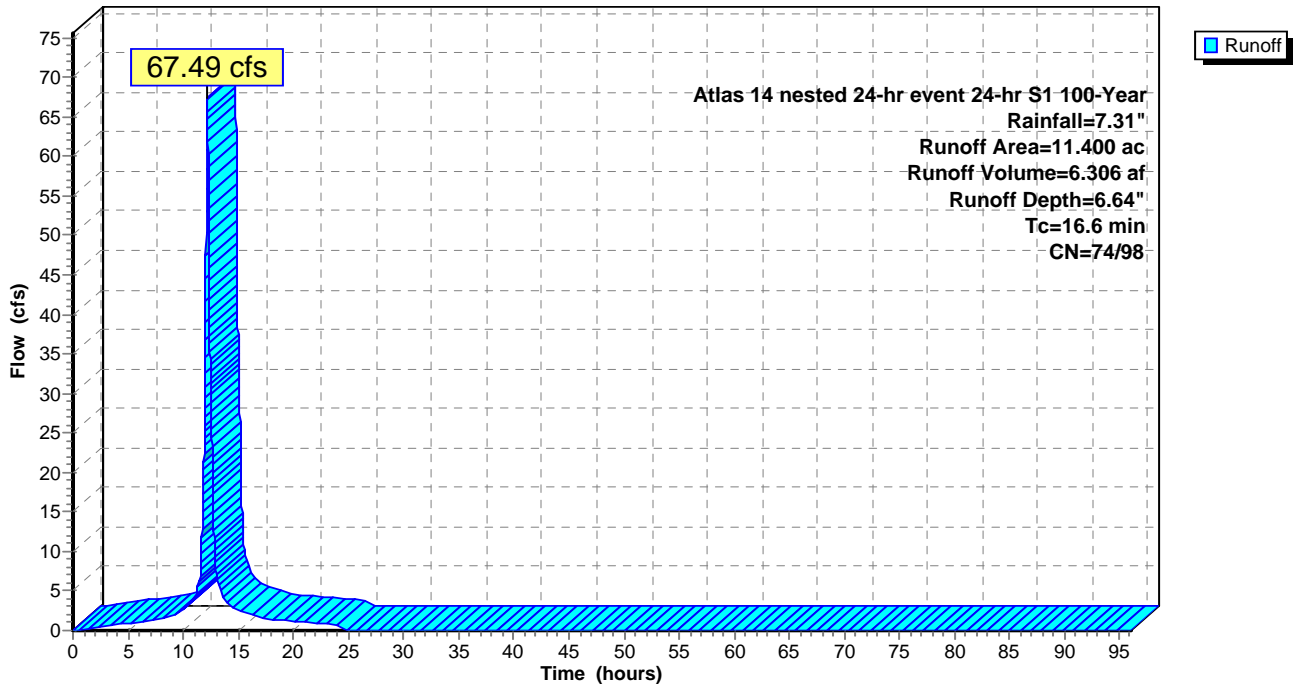
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 1.791	74	pervious
* 9.609	98	impervious
11.400	94	Weighted Average
1.791		15.71% Pervious Area
9.609		84.29% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.6					Direct Entry,

Subcatchment SB 2: SB 2

Hydrograph



Summary for Subcatchment SB 22: SB 22

Runoff = 144.97 cfs @ 12.52 hrs, Volume= 21.385 af, Depth= 6.12"

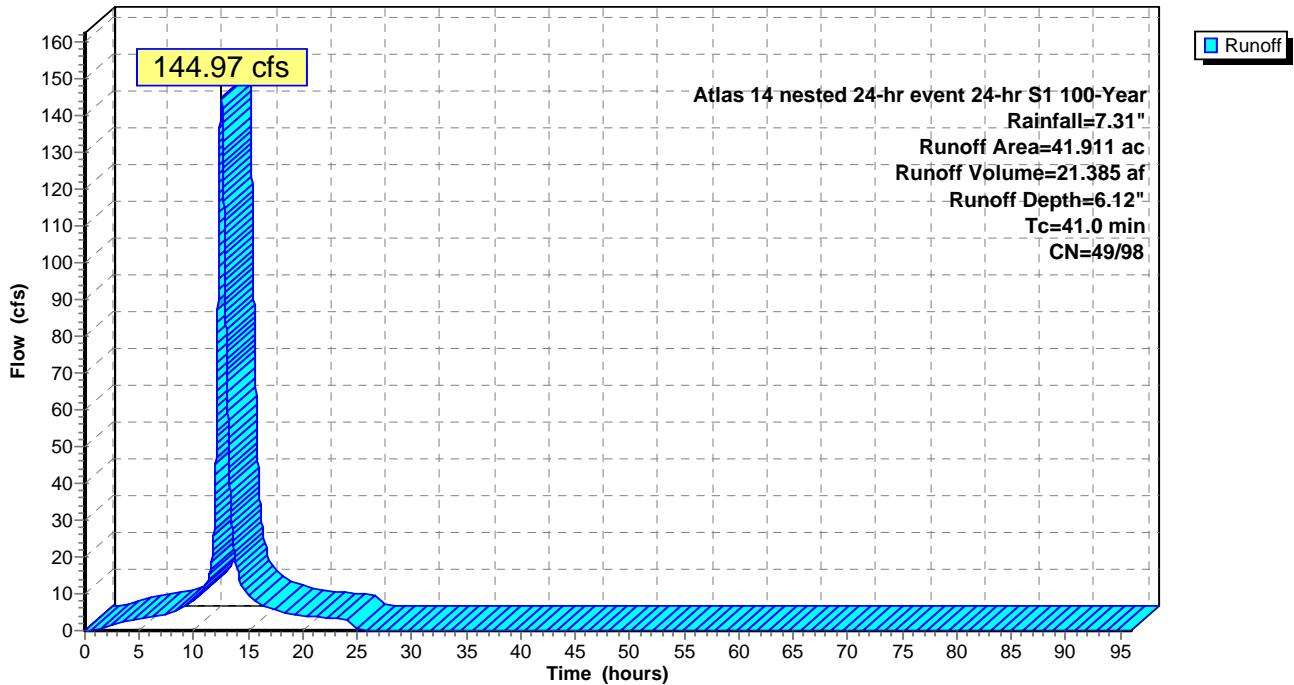
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 7.465	49	Pervious
* 34.446	98	Impervious
41.911	89	Weighted Average
7.465		17.81% Pervious Area
34.446		82.19% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
41.0					Direct Entry,

Subcatchment SB 22: SB 22

Hydrograph



Summary for Subcatchment SB 24: SB 24

Runoff = 42.62 cfs @ 12.05 hrs, Volume= 2.890 af, Depth= 7.02"

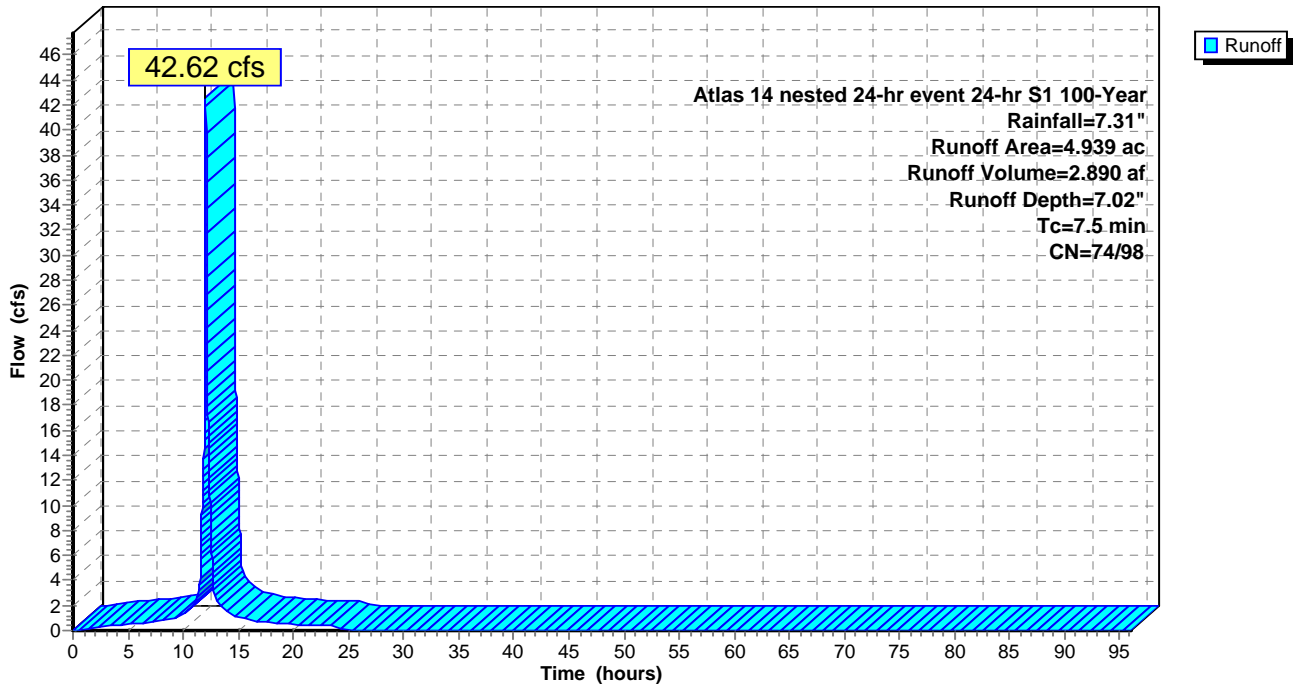
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 0.088	74	permiabile
* 4.851	98	impermiabile
4.939	98	Weighted Average
0.088		1.78% Pervious Area
4.851		98.22% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.5					Direct Entry,

Subcatchment SB 24: SB 24

Hydrograph



Summary for Subcatchment SB 25: SB 25

Runoff = 37.25 cfs @ 12.09 hrs, Volume= 2.904 af, Depth= 6.95"

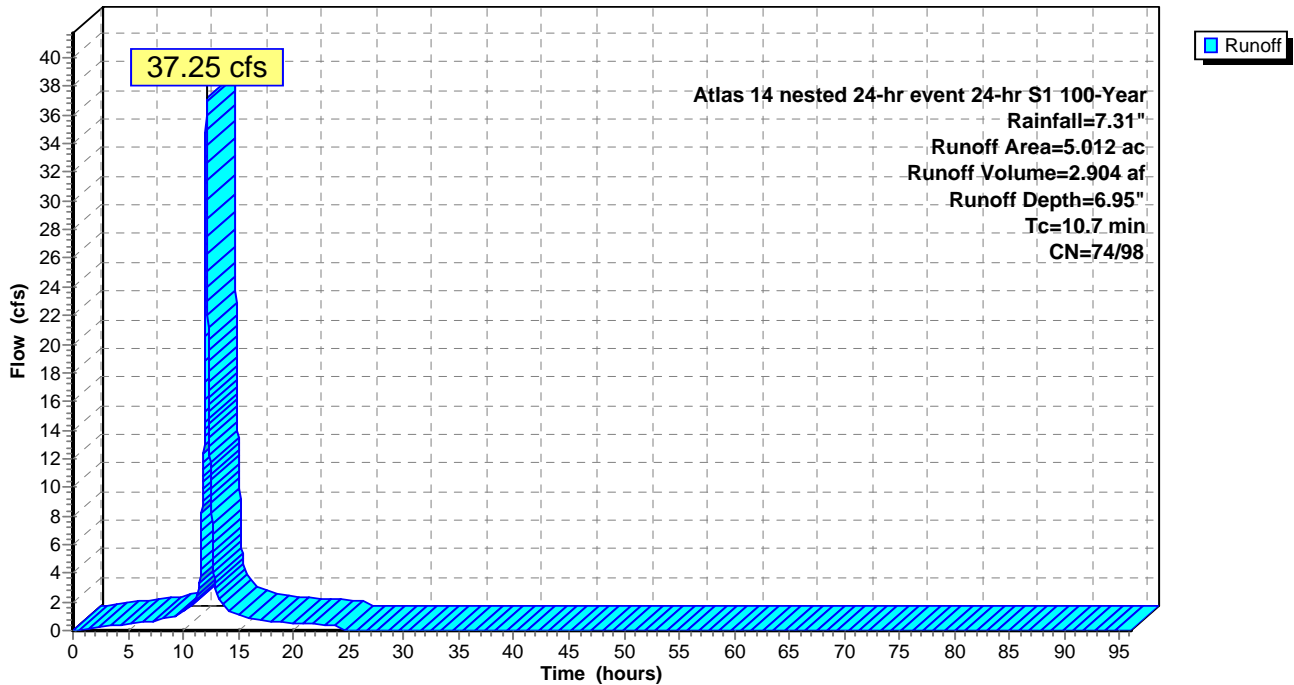
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 0.215	74	pervious
* 4.797	98	impervious
5.012	97	Weighted Average
0.215		4.29% Pervious Area
4.797		95.71% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7					Direct Entry,

Subcatchment SB 25: SB 25

Hydrograph



Summary for Subcatchment SB 26: SB 26

Runoff = 73.07 cfs @ 12.28 hrs, Volume= 8.390 af, Depth= 7.02"

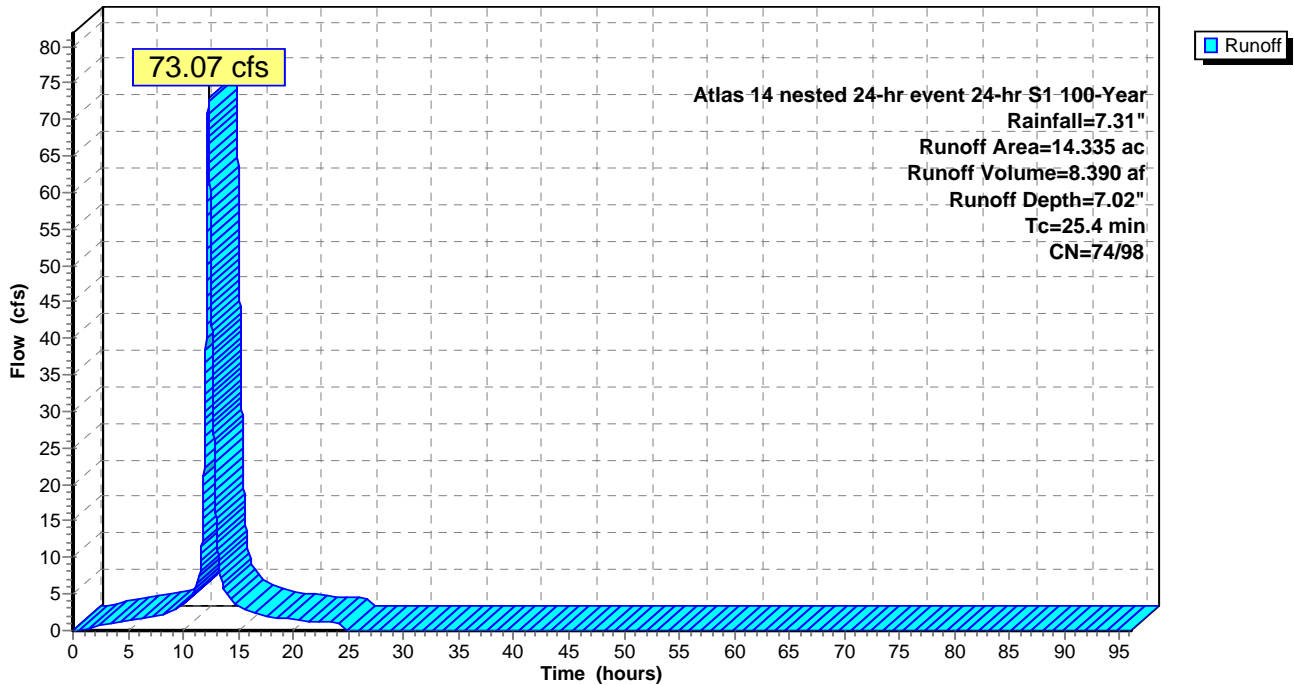
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 0.248	74	pervious
* 14.087	98	impervious
14.335	98	Weighted Average
0.248		1.73% Pervious Area
14.087		98.27% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
25.4					Direct Entry,

Subcatchment SB 26: SB 26

Hydrograph



Summary for Subcatchment SB 27: SB 27 (Thumb Road)

Runoff = 31.89 cfs @ 12.31 hrs, Volume= 3.821 af, Depth= 6.92"

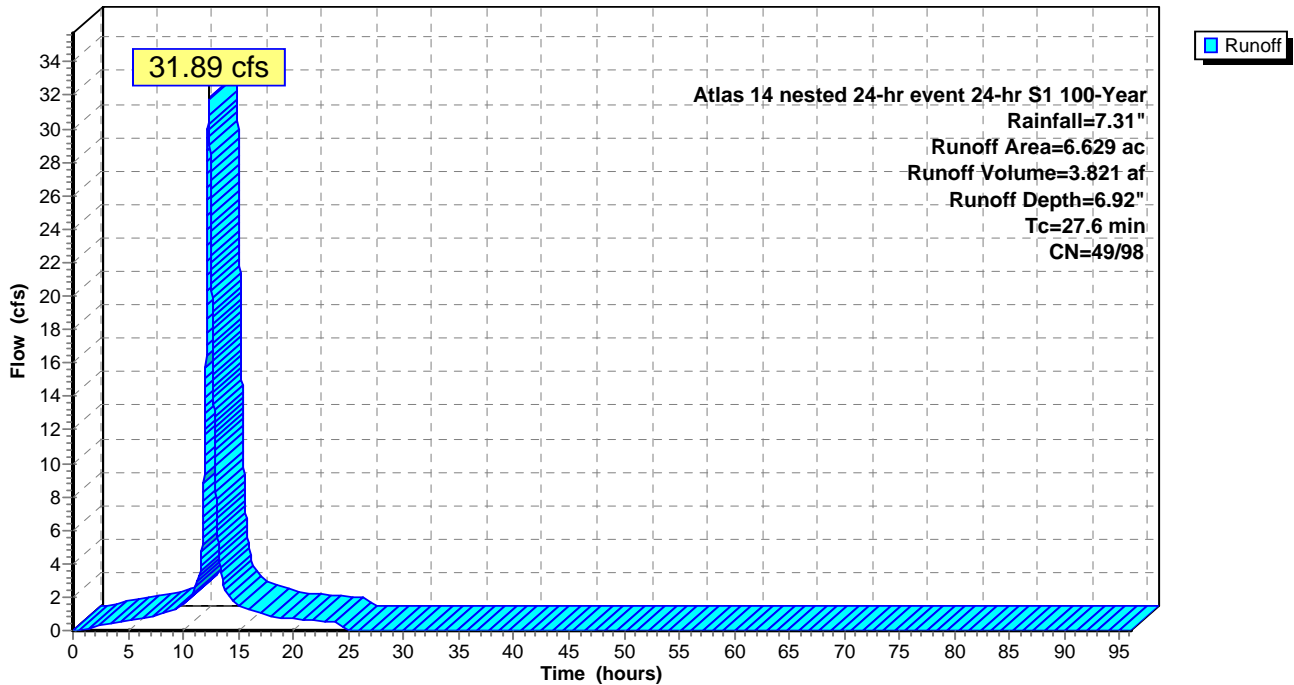
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 0.191	49	Pervious
* 6.438	98	Impervious
6.629	97	Weighted Average
0.191		2.88% Pervious Area
6.438		97.12% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.6					Direct Entry,

Subcatchment SB 27: SB 27 (Thumb Road)

Hydrograph



Summary for Subcatchment SB 28: SB 28

Runoff = 38.37 cfs @ 12.15 hrs, Volume= 3.247 af, Depth= 5.60"

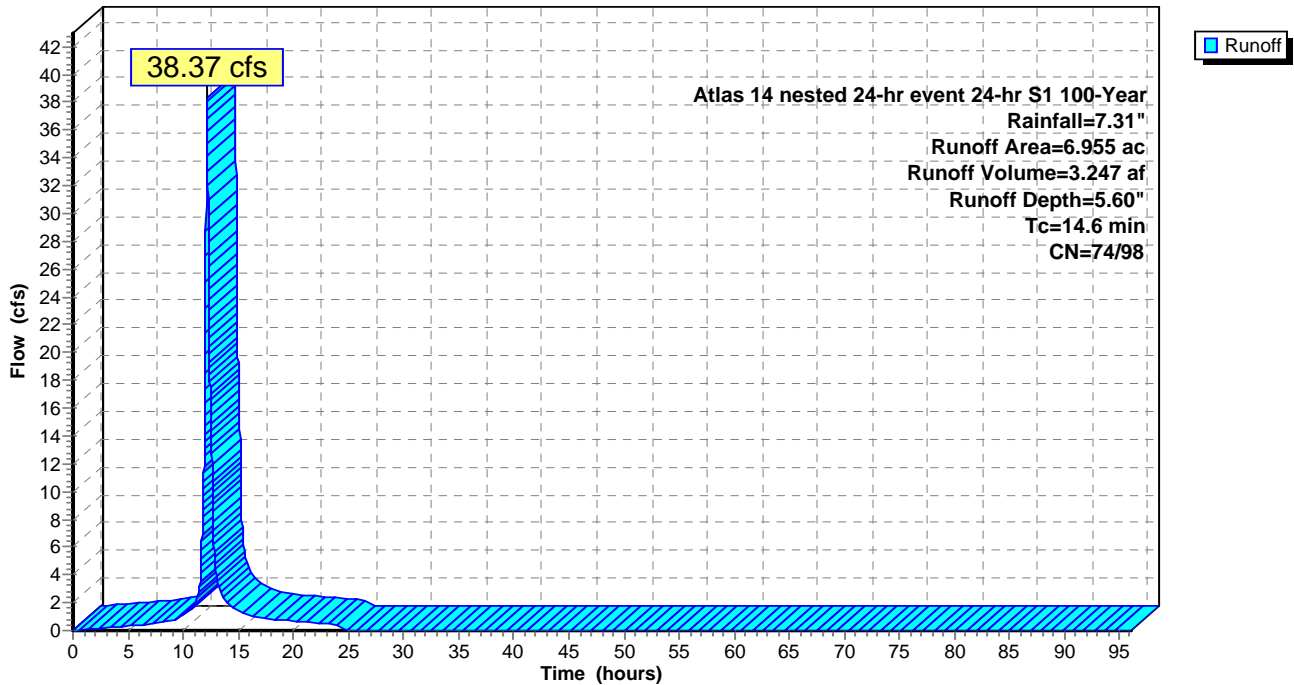
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 3.703	74	pervious
* 3.252	98	impervious
6.955	85	Weighted Average
3.703		53.24% Pervious Area
3.252		46.76% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.6					Direct Entry,

Subcatchment SB 28: SB 28

Hydrograph



Summary for Subcatchment SB 29: SB 29

Runoff = 48.13 cfs @ 12.21 hrs, Volume= 4.557 af, Depth= 5.35"

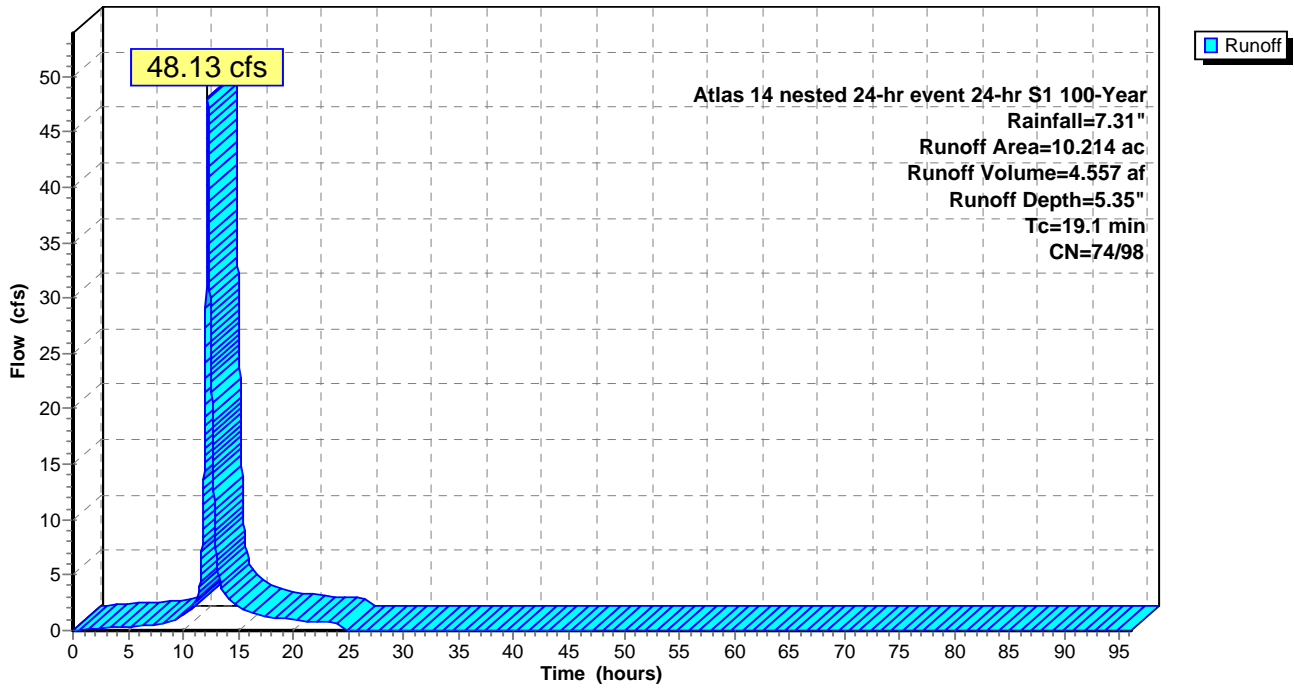
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 6.360	74	pervious
* 3.854	98	impervious
10.214	83	Weighted Average
6.360		62.27% Pervious Area
3.854		37.73% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.1					Direct Entry,

Subcatchment SB 29: SB 29

Hydrograph



Summary for Subcatchment SB 3: SB 3

Runoff = 199.96 cfs @ 12.16 hrs, Volume= 17.130 af, Depth= 5.46"

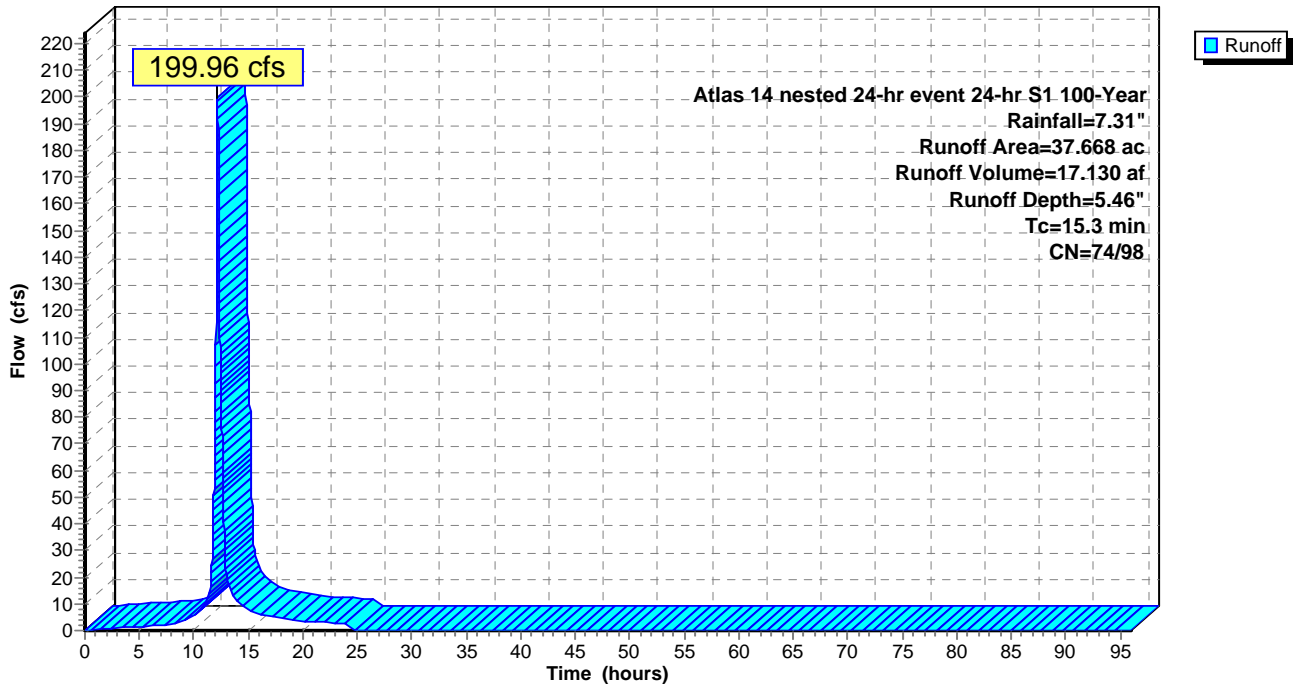
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 22.050	74	Pervious
* 15.618	98	Impervious
37.668	84	Weighted Average
22.050		58.54% Pervious Area
15.618		41.46% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.3					Direct Entry,

Subcatchment SB 3: SB 3

Hydrograph



Summary for Subcatchment SB 4: SB 4

Runoff = 4.20 cfs @ 12.04 hrs, Volume= 0.245 af, Depth= 4.90"

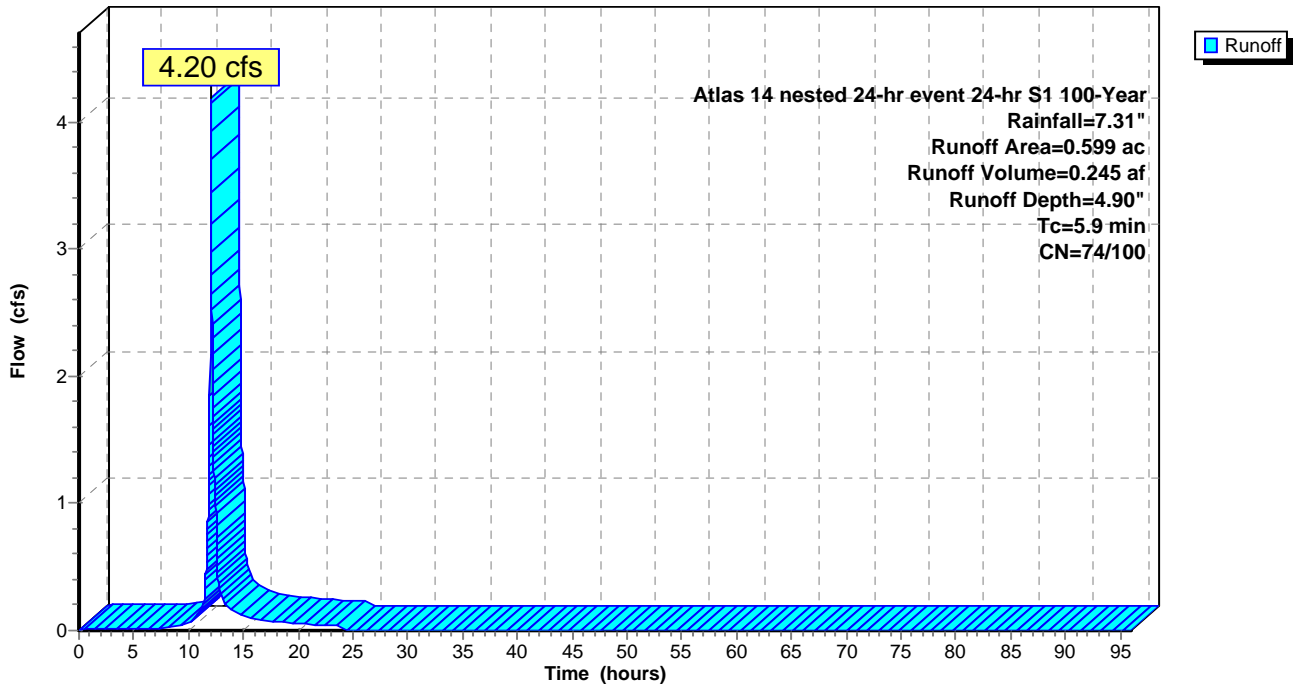
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 0.481	74	pervious
* 0.118	100	impervious
0.599	79	Weighted Average
0.481		80.30% Pervious Area
0.118		19.70% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.9					Direct Entry,

Subcatchment SB 4: SB 4

Hydrograph



Summary for Subcatchment SB 5: SB 5

Runoff = 23.32 cfs @ 12.72 hrs, Volume= 4.093 af, Depth= 6.25"

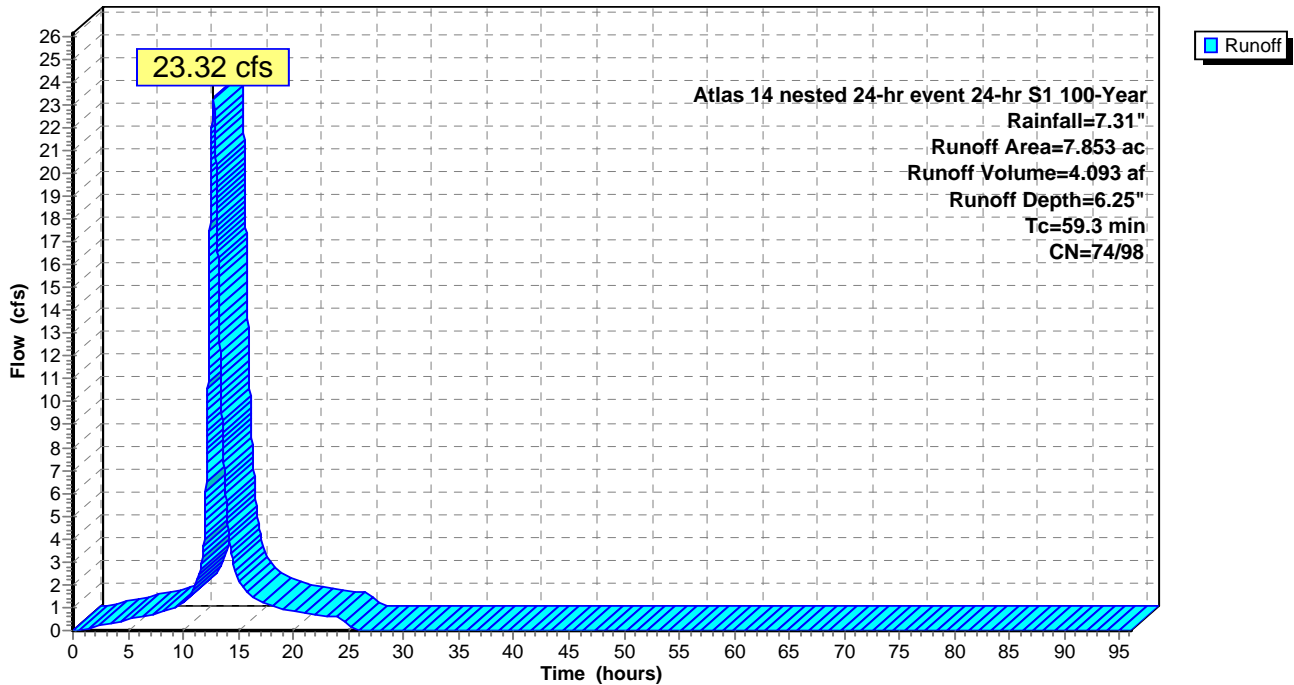
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 2.327	74	pervious
* 5.526	98	impervious
7.853	91	Weighted Average
2.327		29.63% Pervious Area
5.526		70.37% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
59.3					Direct Entry,

Subcatchment SB 5: SB 5

Hydrograph



Summary for Subcatchment SB 51: Offsite Subbasin 51

Runoff = 70.31 cfs @ 12.43 hrs, Volume= 8.599 af, Depth= 4.09"

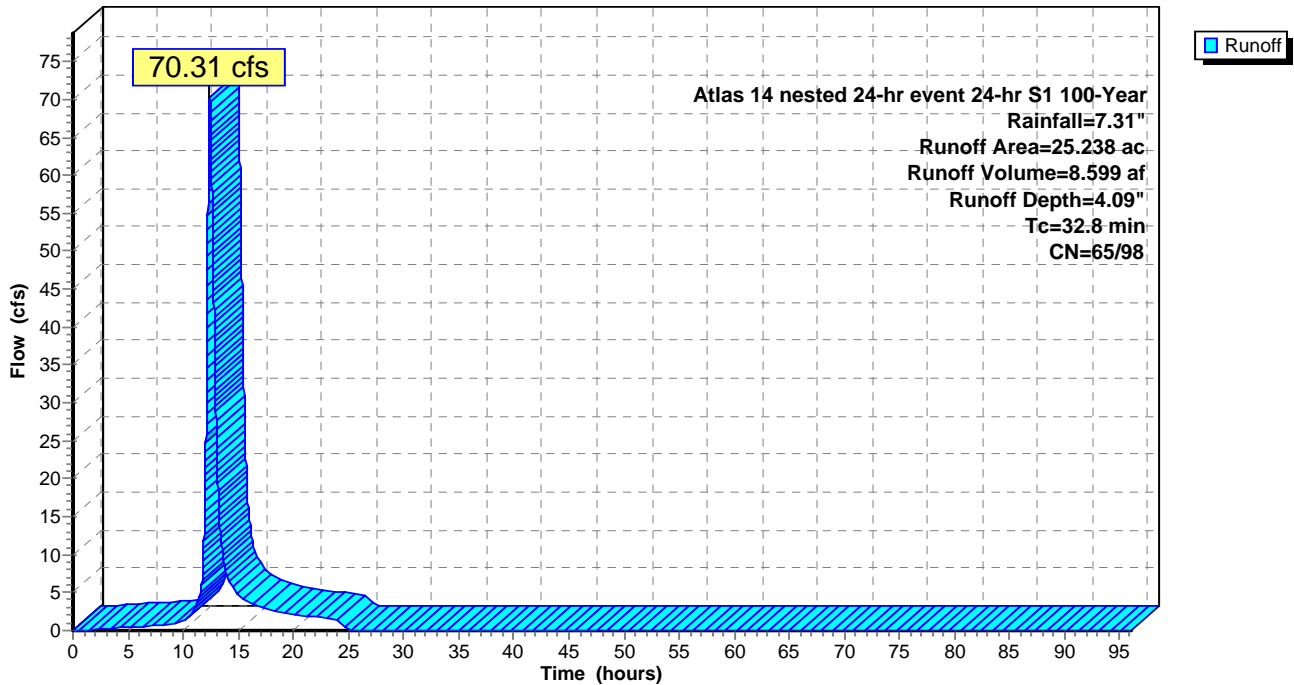
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 20.200	65	Offsite subbasin 51
* 5.038	98	
25.238	72	Weighted Average
20.200		80.04% Pervious Area
5.038		19.96% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.8					Direct Entry,

Subcatchment SB 51: Offsite Subbasin 51

Hydrograph



Summary for Subcatchment SB 6: SB 6

Runoff = 4.33 cfs @ 12.23 hrs, Volume= 0.419 af, Depth= 5.05"

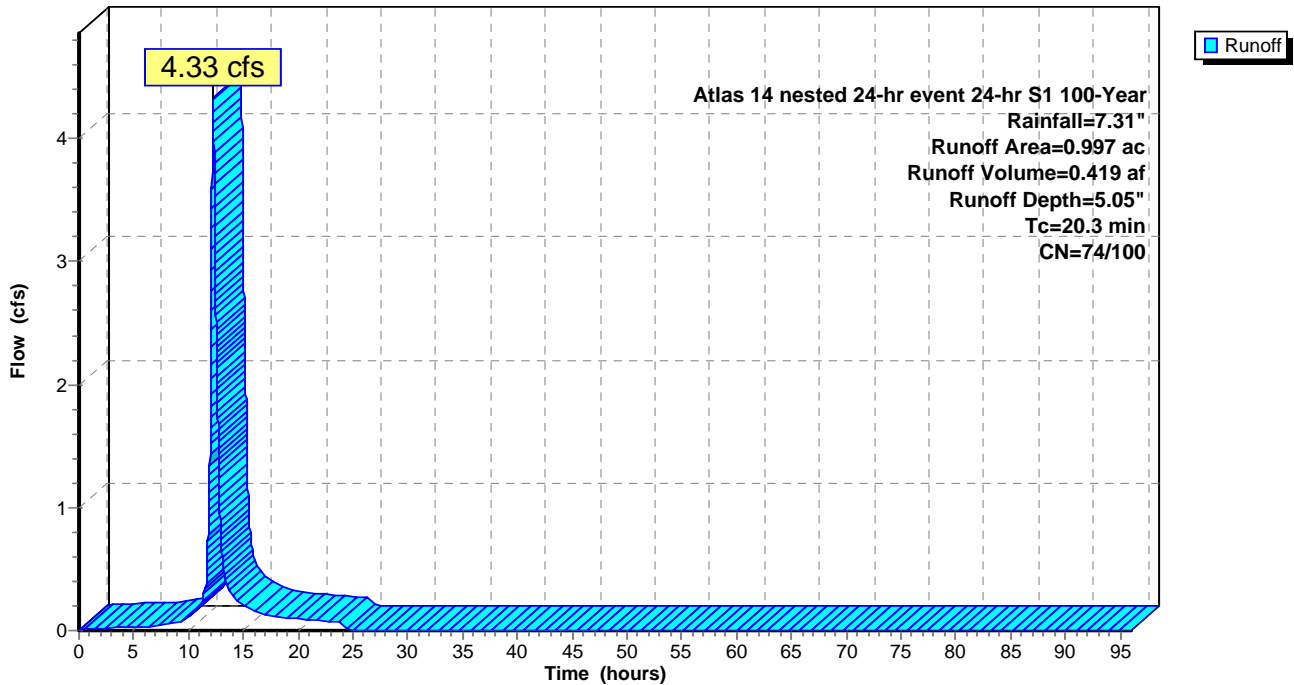
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 0.753	74	pervious
* 0.244	100	impervious
0.997	80	Weighted Average
0.753		75.53% Pervious Area
0.244		24.47% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.3					Direct Entry,

Subcatchment SB 6: SB 6

Hydrograph



Summary for Subcatchment SB 7: SB 7

Runoff = 192.64 cfs @ 12.03 hrs, Volume= 11.950 af, Depth= 6.65"

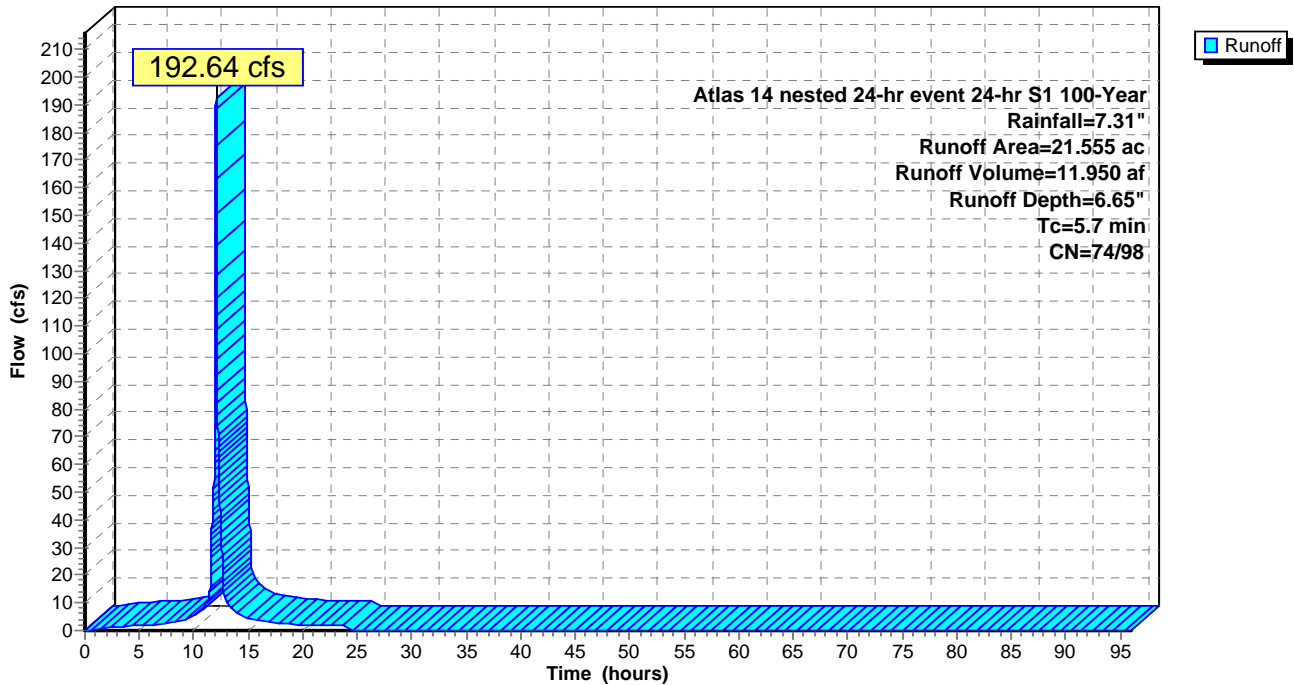
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 3.269	74	pervious
* 18.286	98	impervious
21.555	94	Weighted Average
3.269		15.17% Pervious Area
18.286		84.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.7					Direct Entry,

Subcatchment SB 7: SB 7

Hydrograph



Summary for Subcatchment SB 8: SB 8

Runoff = 86.02 cfs @ 12.61 hrs, Volume= 12.680 af, Depth= 5.14"

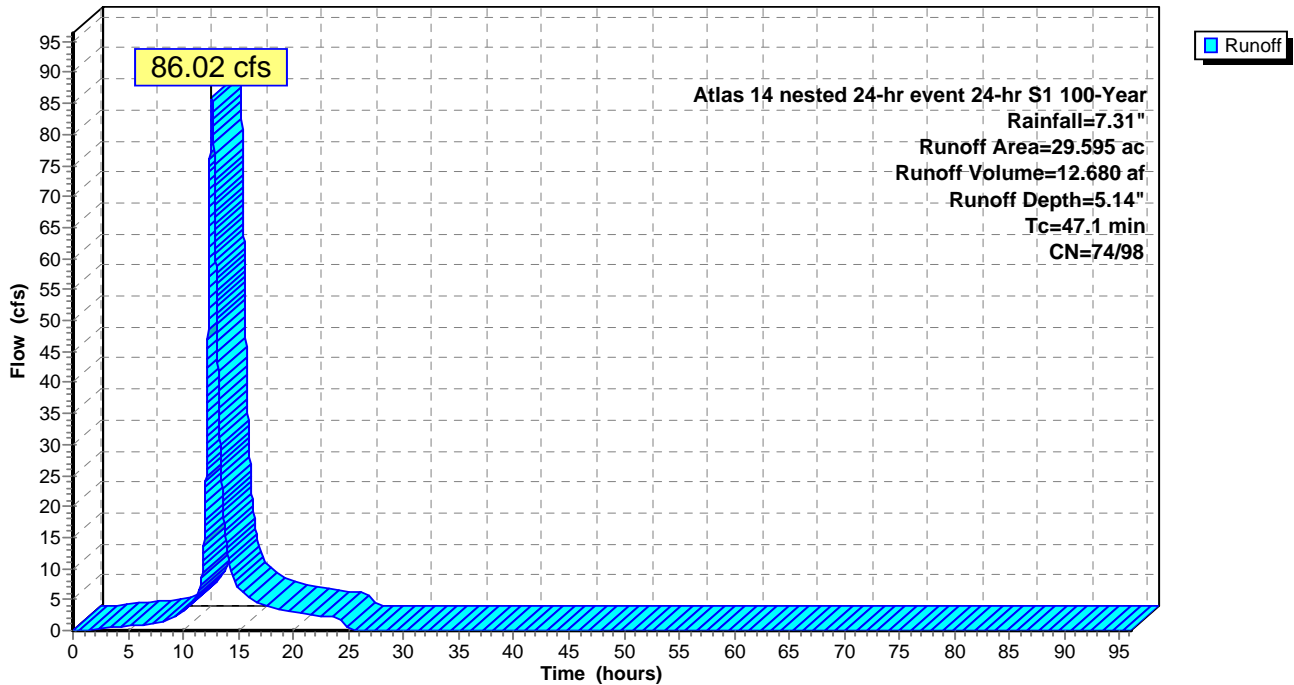
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 20.714	74	pervious
* 8.881	98	impervious
29.595	81	Weighted Average
20.714		69.99% Pervious Area
8.881		30.01% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
47.1					Direct Entry,

Subcatchment SB 8: SB 8

Hydrograph



Summary for Subcatchment SB 9: SB 9

Runoff = 96.02 cfs @ 12.37 hrs, Volume= 11.237 af, Depth= 5.23"

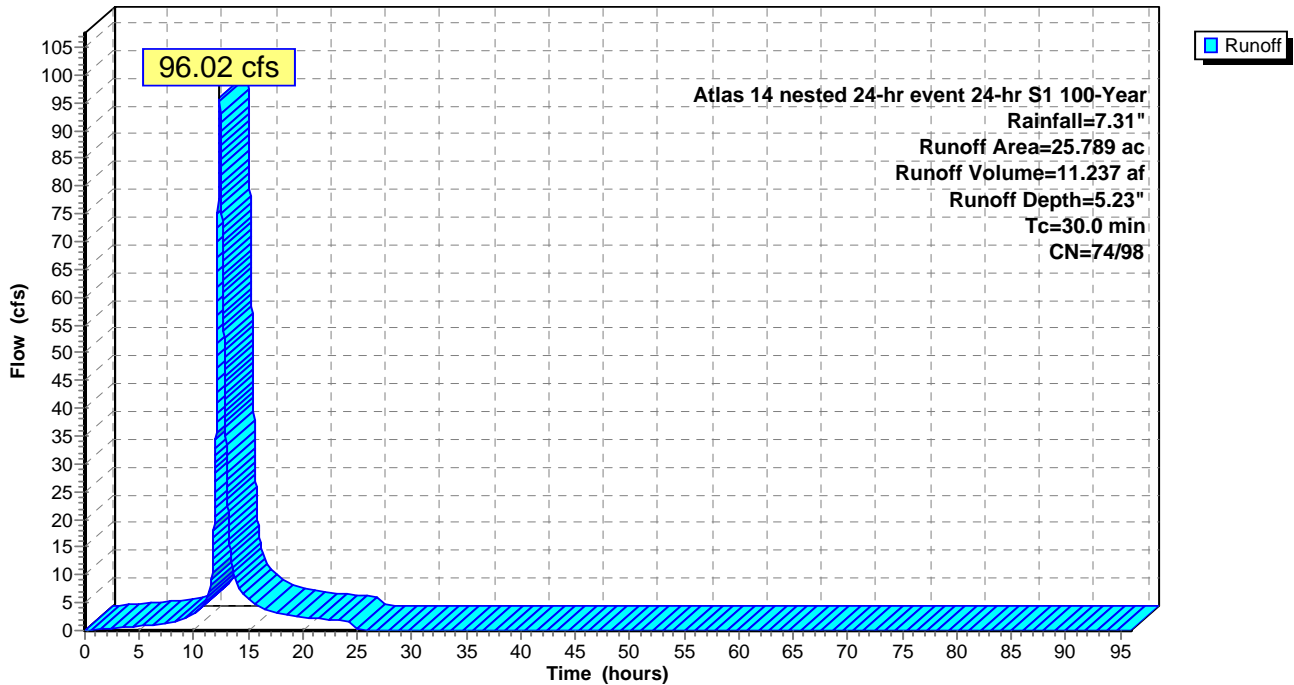
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. UI as Pervious, Time Span= 0.00-96.00 hrs, dt= 0.01
 Atlas 14 nested 24-hr event 24-hr S1 100-Year Rainfall=7.31"

Area (ac)	CN	Description
* 17.234	74	permiabile
* 8.555	98	impermiabile
25.789	82	Weighted Average
17.234		66.83% Pervious Area
8.555		33.17% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.0					Direct Entry,

Subcatchment SB 9: SB 9

Hydrograph



Summary for Pond 3P: P-3

Inflow Area = 133.365 ac, 58.87% Impervious, Inflow Depth = 5.59" for 100-Year event
 Inflow = 321.63 cfs @ 12.46 hrs, Volume= 62.160 af
 Outflow = 267.75 cfs @ 12.78 hrs, Volume= 62.142 af, Atten= 17%, Lag= 19.2 min
 Primary = 267.75 cfs @ 12.78 hrs, Volume= 62.142 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 914.00' Surf.Area= 1.790 ac Storage= 5.827 af
 Peak Elev= 919.71' @ 12.78 hrs Surf.Area= 2.880 ac Storage= 19.292 af (13.465 af above start)

Plug-Flow detention time= 190.2 min calculated for 56.315 af (91% of inflow)
 Center-of-Mass det. time= 93.0 min (936.3 - 843.3)

Volume	Invert	Avail.Storage	Storage Description
#1	909.85'	20.423 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
909.85	1.130	0.000	0.000
912.00	1.360	2.677	2.677
916.00	2.220	7.160	9.837
918.00	2.570	4.790	14.627
920.10	2.950	5.796	20.423

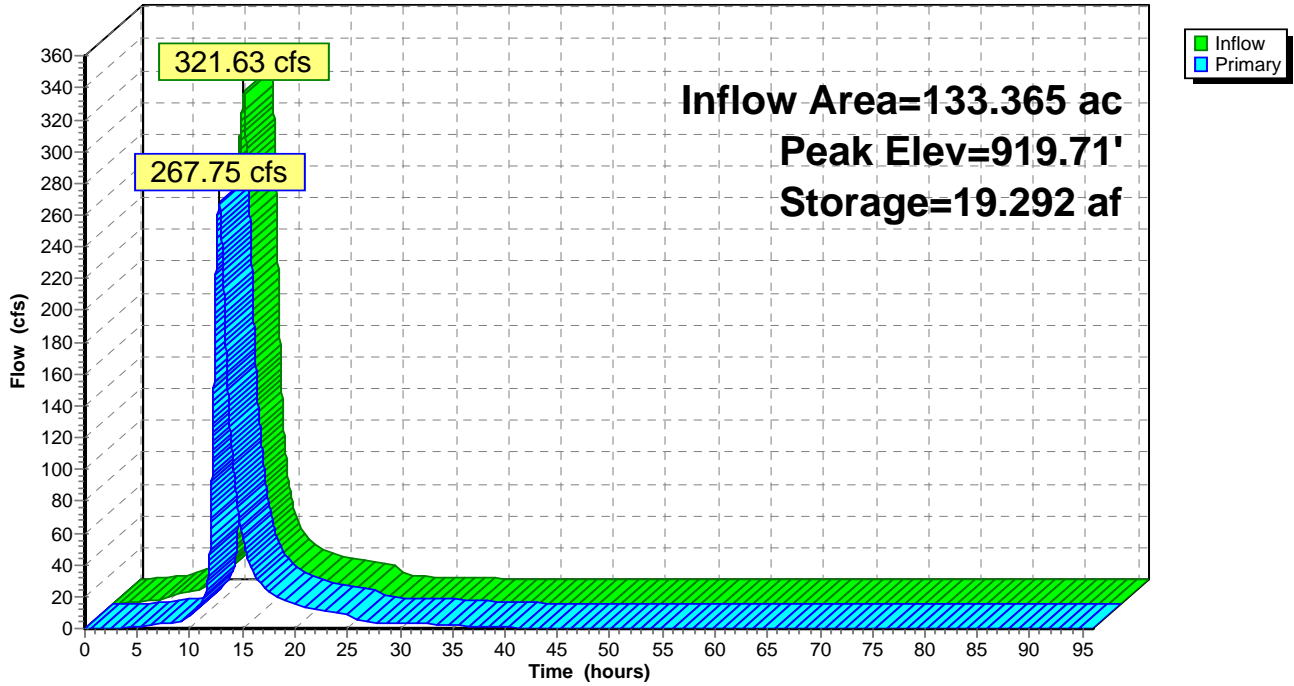
Device	Routing	Invert	Outlet Devices
#1	Primary	914.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	918.25'	10.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#3	Primary	915.00'	7.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)

Primary OutFlow Max=267.74 cfs @ 12.78 hrs HW=919.71' TW=0.00' (Dynamic Tailwater)

- 1=Orifice/Grate (Orifice Controls 9.04 cfs @ 11.51 fps)
- 2=Sharp-Crested Rectangular Weir (Weir Controls 56.11 cfs @ 3.95 fps)
- 3=Sharp-Crested Rectangular Weir (Weir Controls 202.60 cfs @ 7.10 fps)

Pond 3P: P-3

Hydrograph



Summary for Pond 4P: P-4

Inflow Area = 7.853 ac, 70.37% Impervious, Inflow Depth = 6.25" for 100-Year event
 Inflow = 23.32 cfs @ 12.72 hrs, Volume= 4.093 af
 Outflow = 16.01 cfs @ 13.17 hrs, Volume= 4.093 af, Atten= 31%, Lag= 26.9 min
 Primary = 12.66 cfs @ 13.17 hrs, Volume= 2.270 af
 Secondary = 3.35 cfs @ 13.15 hrs, Volume= 1.823 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 915.00' Surf.Area= 0.275 ac Storage= 0.646 af
 Peak Elev= 917.80' @ 13.17 hrs Surf.Area= 0.438 ac Storage= 1.640 af (0.995 af above start)

Plug-Flow detention time= 181.5 min calculated for 3.447 af (84% of inflow)
 Center-of-Mass det. time= 53.0 min (859.7 - 806.7)

Volume	Invert	Avail.Storage	Storage Description
#1	910.90'	1.728 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
910.90	0.070	0.000	0.000
912.00	0.090	0.088	0.088
914.00	0.220	0.310	0.398
916.00	0.330	0.550	0.948
918.00	0.450	0.780	1.728

Device	Routing	Invert	Outlet Devices
#1	Primary	915.00'	6.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Secondary	915.00'	9.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	915.95'	24.0" Round RCP_Round 24" L= 50.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 915.80' / 915.95' S= -0.0030 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Primary OutFlow Max=12.66 cfs @ 13.17 hrs HW=917.80' TW=0.00' (Dynamic Tailwater)

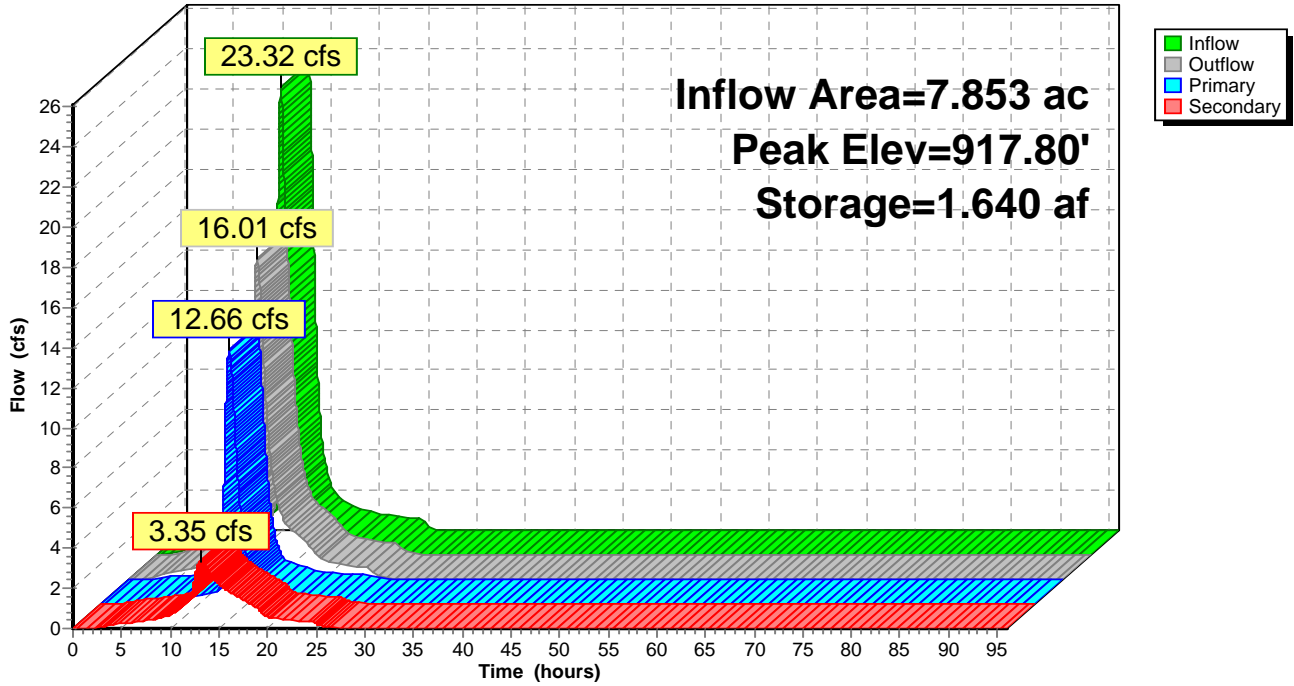
- ↑1=Orifice/Grate (Orifice Controls 1.58 cfs @ 8.06 fps)
- ↑3=RCP_Round 24" (Barrel Controls 11.07 cfs @ 4.37 fps)

Secondary OutFlow Max=3.35 cfs @ 13.15 hrs HW=917.80' TW=915.32' (Dynamic Tailwater)

- ↑2=Orifice/Grate (Orifice Controls 3.35 cfs @ 7.58 fps)

Pond 4P: P-4

Hydrograph



Summary for Pond 7P: P-7

[90] Warning: Qout>Qin may require smaller dt or Finer Routing

Inflow Area = 29.595 ac, 30.01% Impervious, Inflow Depth = 5.14" for 100-Year event
 Inflow = 86.02 cfs @ 12.61 hrs, Volume= 12.680 af
 Outflow = 87.44 cfs @ 12.62 hrs, Volume= 12.680 af, Atten= 0%, Lag= 0.9 min
 Primary = 87.44 cfs @ 12.62 hrs, Volume= 12.680 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 915.00' Surf.Area= 0.440 ac Storage= 1.062 af
 Peak Elev= 915.93' @ 12.54 hrs Surf.Area= 0.551 ac Storage= 1.523 af (0.461 af above start)

Plug-Flow detention time= 81.8 min calculated for 11.617 af (92% of inflow)
 Center-of-Mass det. time= 7.2 min (832.2 - 825.0)

Volume	Invert	Avail.Storage	Storage Description
#1	910.95'	2.122 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
910.95	0.110	0.000	0.000
912.00	0.180	0.152	0.152
914.00	0.340	0.520	0.672
915.00	0.440	0.390	1.062
916.00	0.560	0.500	1.562
917.00	0.560	0.560	2.122

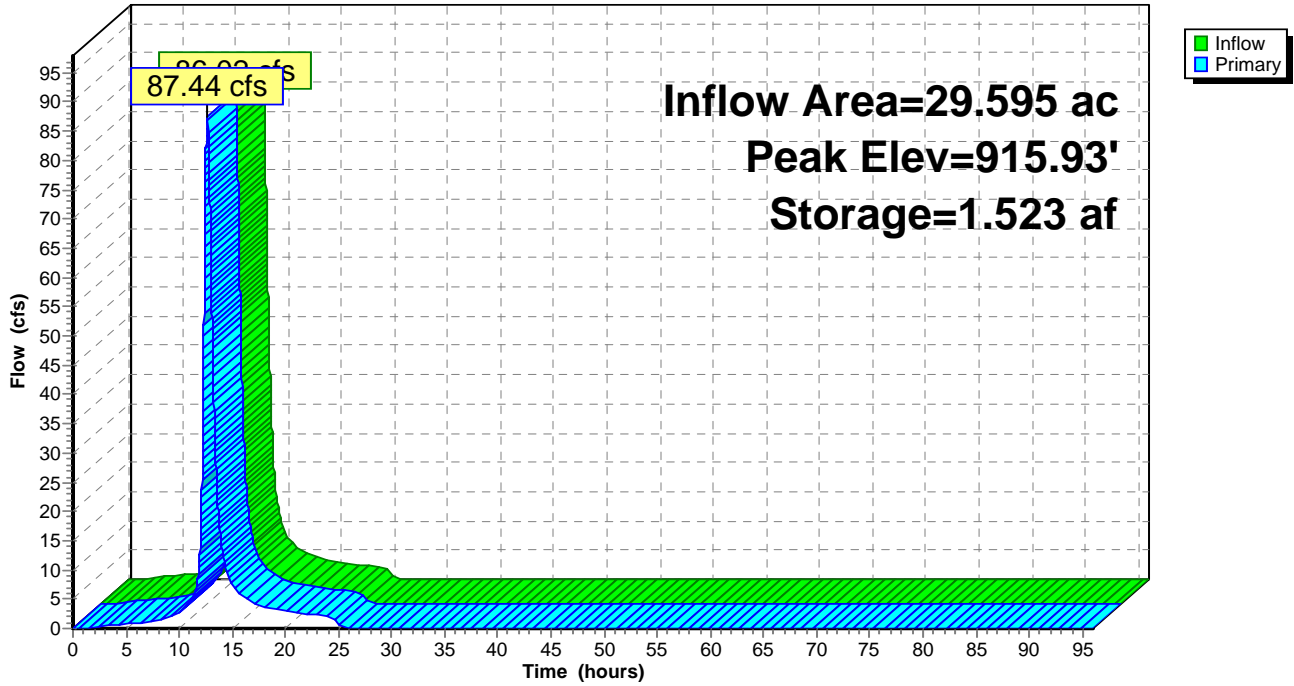
Device	Routing	Invert	Outlet Devices
#1	Primary	915.00'	75.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=88.53 cfs @ 12.62 hrs HW=915.92' TW=915.81' (Dynamic Tailwater)

↑1=Broad-Crested Rectangular Weir (Weir Controls 88.53 cfs @ 1.29 fps)

Pond 7P: P-7

Hydrograph



Summary for Pond 9P: P-9

Inflow Area = 55.384 ac, 31.48% Impervious, Inflow Depth = 5.18" for 100-Year event
 Inflow = 166.30 cfs @ 12.47 hrs, Volume= 23.916 af
 Outflow = 166.11 cfs @ 12.49 hrs, Volume= 23.916 af, Atten= 0%, Lag= 1.3 min
 Primary = 166.11 cfs @ 12.49 hrs, Volume= 23.916 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 915.00' Surf.Area= 0.210 ac Storage= 0.353 af
 Peak Elev= 915.84' @ 12.49 hrs Surf.Area= 0.379 ac Storage= 0.601 af (0.248 af above start)

Plug-Flow detention time= 20.1 min calculated for 23.564 af (99% of inflow)
 Center-of-Mass det. time= 1.8 min (821.8 - 820.1)

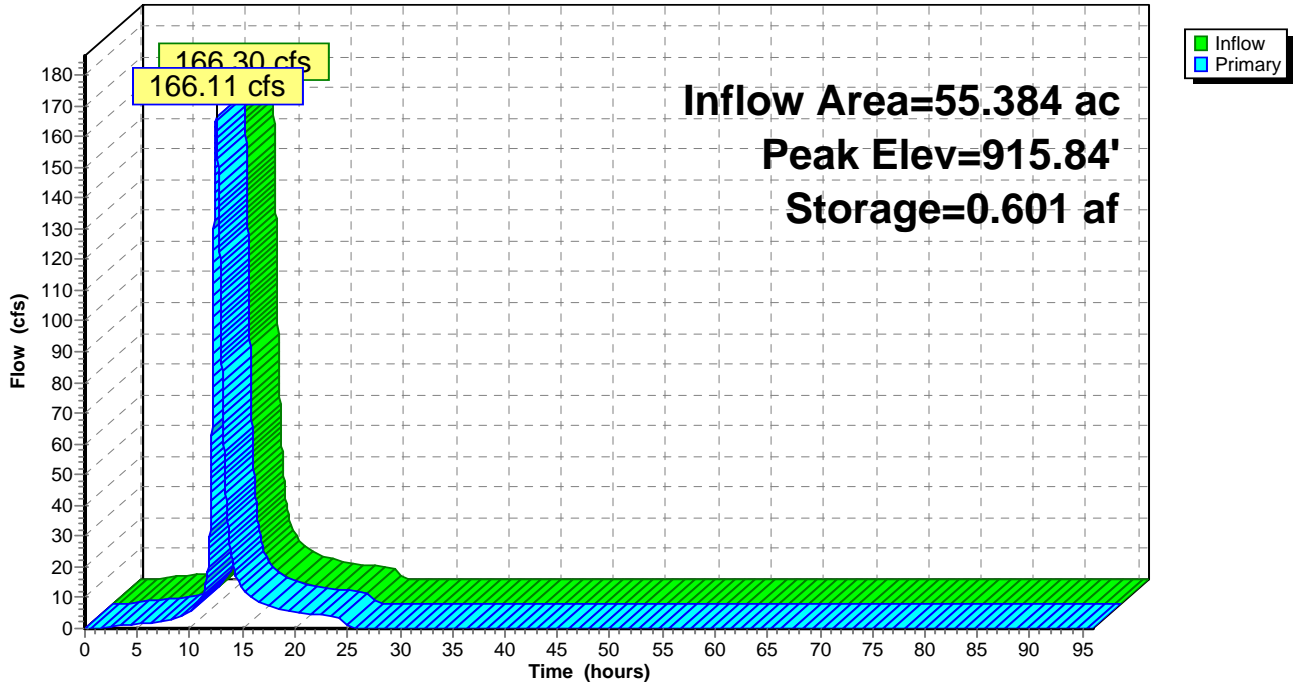
Volume	Invert	Avail.Storage	Storage Description
#1	910.50'	1.673 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
910.50	0.020	0.000	0.000
912.00	0.050	0.052	0.052
913.00	0.070	0.060	0.112
914.00	0.100	0.085	0.198
915.00	0.210	0.155	0.353
916.00	0.410	0.310	0.662
918.00	0.600	1.010	1.673

Device	Routing	Invert	Outlet Devices
#1	Primary	915.00'	80.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=166.10 cfs @ 12.49 hrs HW=915.84' TW=912.69' (Dynamic Tailwater)
 ↑1=Broad-Crested Rectangular Weir (Weir Controls 166.10 cfs @ 2.46 fps)

Pond 9P: P-9

Hydrograph



Summary for Pond 10P: P-10

[95] Warning: Outlet Device #1 rise exceeded

Inflow Area = 66.448 ac, 29.37% Impervious, Inflow Depth = 4.43" for 100-Year event
 Inflow = 164.99 cfs @ 12.63 hrs, Volume= 24.555 af
 Outflow = 164.52 cfs @ 12.65 hrs, Volume= 24.555 af, Atten= 0%, Lag= 1.5 min
 Primary = 15.81 cfs @ 12.65 hrs, Volume= 11.415 af
 Secondary = 148.71 cfs @ 12.65 hrs, Volume= 13.140 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 896.00' Surf.Area= 0.290 ac Storage= 0.700 af
 Peak Elev= 898.47' @ 12.65 hrs Surf.Area= 0.396 ac Storage= 1.545 af (0.845 af above start)

Plug-Flow detention time= 55.6 min calculated for 23.855 af (97% of inflow)
 Center-of-Mass det. time= 16.6 min (903.8 - 887.2)

Volume	Invert	Avail.Storage	Storage Description
#1	892.00'	1.760 af	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
892.00	0.120	0.000	0.000
893.00	0.140	0.130	0.130
895.00	0.190	0.330	0.460
896.00	0.290	0.240	0.700
897.00	0.330	0.310	1.010
899.00	0.420	0.750	1.760

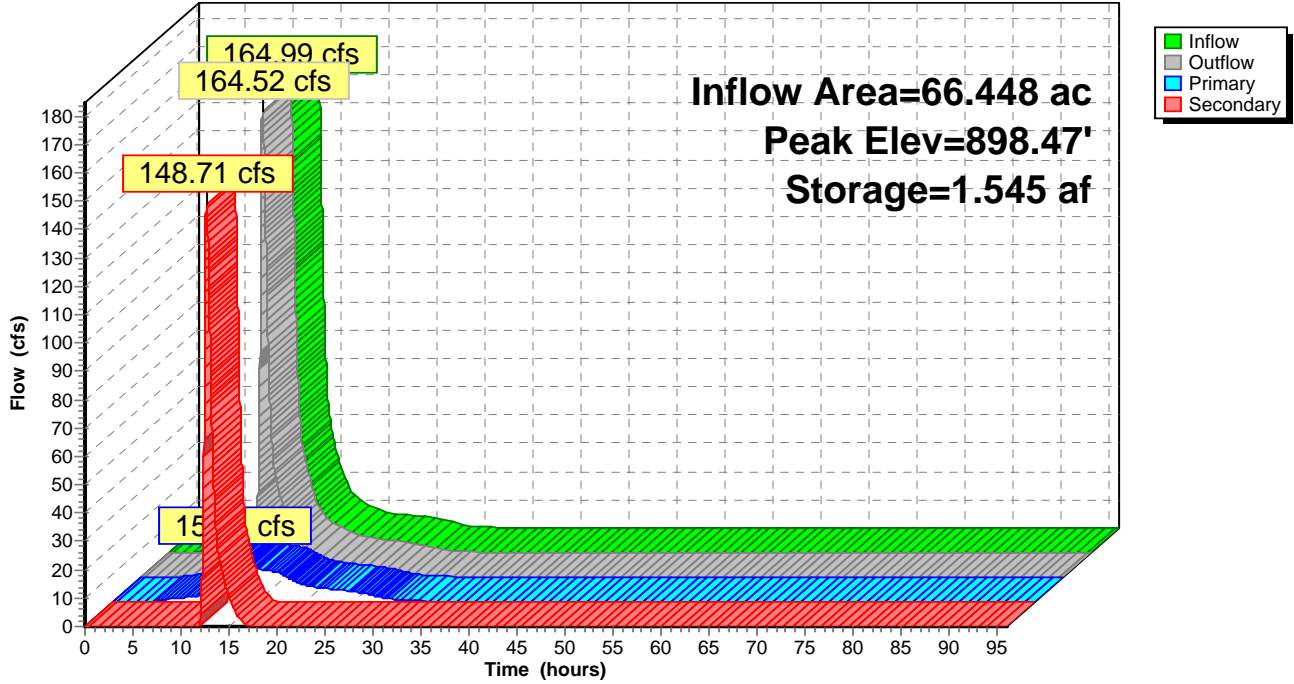
Device	Routing	Invert	Outlet Devices
#1	Primary	896.00'	2.5' long x 1.00' rise Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Secondary	897.40'	50.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=15.81 cfs @ 12.65 hrs HW=898.47' TW=895.43' (Dynamic Tailwater)
 ↖1=Sharp-Crested Rectangular Weir (Orifice Controls 15.81 cfs @ 6.87 fps)

Secondary OutFlow Max=148.69 cfs @ 12.65 hrs HW=898.47' TW=895.43' (Dynamic Tailwater)
 ↖2=Broad-Crested Rectangular Weir (Weir Controls 148.69 cfs @ 2.77 fps)

Pond 10P: P-10

Hydrograph



Summary for Pond 11P: P-11

Inflow Area = 58.677 ac, 31.52% Impervious, Inflow Depth = 5.19" for 100-Year event
 Inflow = 172.55 cfs @ 12.48 hrs, Volume= 25.364 af
 Outflow = 160.26 cfs @ 12.64 hrs, Volume= 25.363 af, Atten= 7%, Lag= 9.3 min
 Primary = 155.34 cfs @ 12.64 hrs, Volume= 21.533 af
 Secondary = 4.92 cfs @ 12.59 hrs, Volume= 3.830 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 909.00' Surf.Area= 1.210 ac Storage= 3.640 af
 Peak Elev= 912.79' @ 12.64 hrs Surf.Area= 1.655 ac Storage= 9.054 af (5.414 af above start)

Plug-Flow detention time= 194.6 min calculated for 21.723 af (86% of inflow)
 Center-of-Mass det. time= 85.1 min (904.7 - 819.6)

Volume	Invert	Avail.Storage	Storage Description
#1	905.00'	9.405 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
905.00	0.760	0.000	0.000
906.00	0.820	0.790	0.790
908.00	0.950	1.770	2.560
909.00	1.210	1.080	3.640
910.00	1.320	1.265	4.905
912.00	1.560	2.880	7.785
913.00	1.680	1.620	9.405

Device	Routing	Invert	Outlet Devices
#1	Primary	909.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	910.00'	24.0" Round RCP_Round 24" L= 200.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 910.00' / 909.00' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Primary	910.00'	24.0" Round RCP_Round 24" L= 200.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 910.00' / 909.00' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#4	Primary	912.00'	60.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88
#5	Secondary	909.00'	12.0" Round RCP_Round 12" L= 150.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 909.00' / 908.00' S= 0.0067 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=155.33 cfs @ 12.64 hrs HW=912.79' TW=898.47' (Dynamic Tailwater)

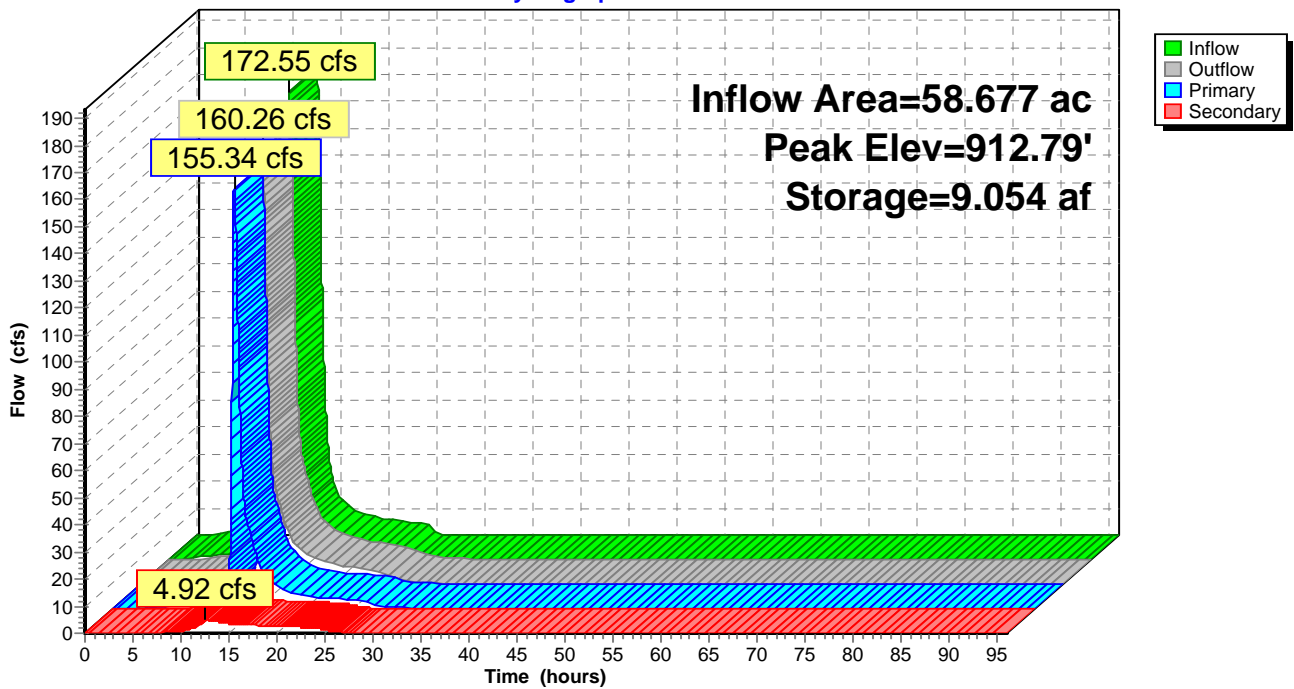
- 1=Orifice/Grate (Orifice Controls 7.36 cfs @ 9.37 fps)
- 2=RCP_Round 24" (Barrel Controls 17.55 cfs @ 5.59 fps)
- 3=RCP_Round 24" (Barrel Controls 17.55 cfs @ 5.59 fps)
- 4=Broad-Crested Rectangular Weir (Weir Controls 112.86 cfs @ 2.38 fps)

Secondary OutFlow Max=4.92 cfs @ 12.59 hrs HW=912.78' TW=909.18' (Dynamic Tailwater)

- 5=RCP_Round 12" (Outlet Controls 4.92 cfs @ 6.27 fps)

Pond 11P: P-11

Hydrograph



Summary for Pond 12P: P-12

Inflow Area = 79.658 ac, 31.13% Impervious, Inflow Depth = 5.17" for 100-Year event
 Inflow = 175.25 cfs @ 12.64 hrs, Volume= 34.346 af
 Outflow = 138.66 cfs @ 12.95 hrs, Volume= 34.338 af, Atten= 21%, Lag= 18.7 min
 Primary = 138.66 cfs @ 12.95 hrs, Volume= 34.338 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 893.00' Surf.Area= 1.640 ac Storage= 5.075 af
 Peak Elev= 895.80' @ 12.95 hrs Surf.Area= 2.014 ac Storage= 10.192 af (5.117 af above start)

Plug-Flow detention time= 213.2 min calculated for 29.263 af (85% of inflow)
 Center-of-Mass det. time= 60.1 min (991.0 - 930.9)

Volume	Invert	Avail.Storage	Storage Description
#1	889.00'	10.590 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
889.00	1.070	0.000	0.000
890.00	1.150	1.110	1.110
892.00	1.330	2.480	3.590
893.00	1.640	1.485	5.075
894.00	1.770	1.705	6.780
896.00	2.040	3.810	10.590

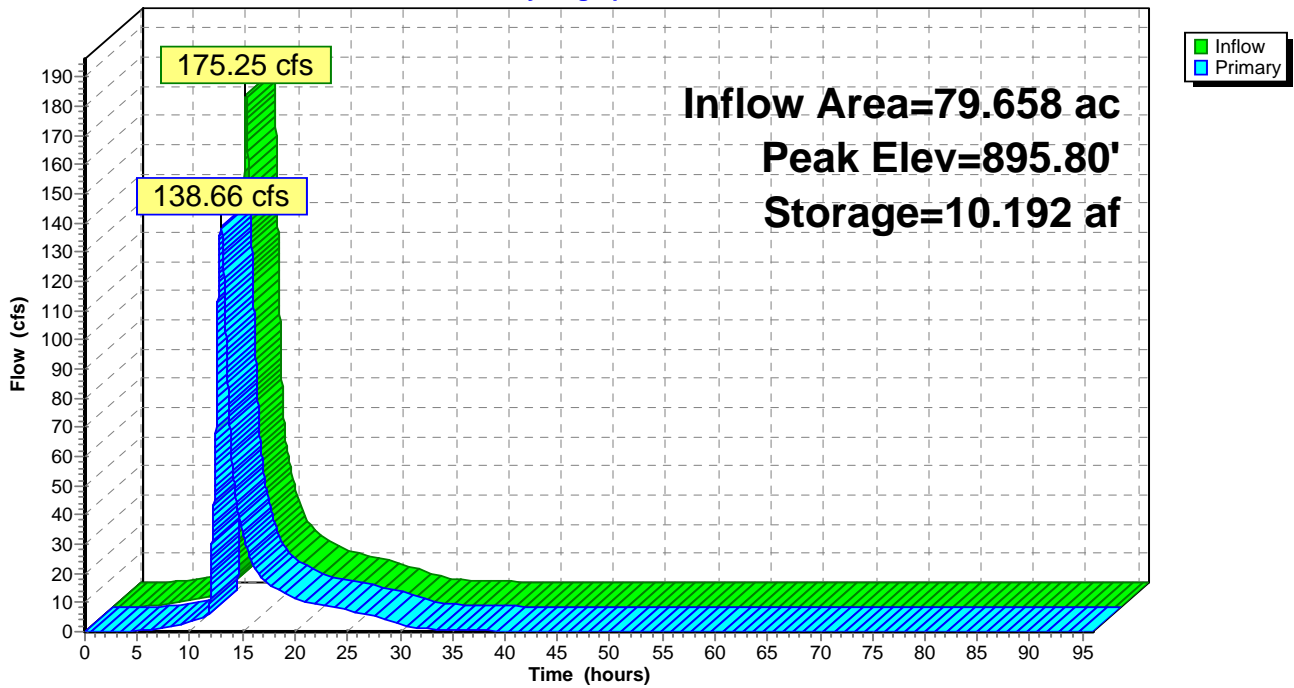
Device	Routing	Invert	Outlet Devices
#1	Primary	893.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	893.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf
#4	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf
#5	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf
#6	Primary	893.50'	43.8" W x 26.6" H, R=22.5"/62.0" Pipe Arch RCP_Arch 44x27 L= 30.0' Box, 30-75° wingwalls, rounded crown, Ke= 0.200 Inlet / Outlet Invert= 893.50' / 893.35' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 6.29 sf

Primary OutFlow Max=138.65 cfs @ 12.95 hrs HW=895.80' TW=884.90' (Dynamic Tailwater)

- 1=Orifice/Grate (Orifice Controls 6.33 cfs @ 8.06 fps)
- 2=Orifice/Grate (Orifice Controls 6.33 cfs @ 8.06 fps)
- 3=RCP_Arch 44x27 (Barrel Controls 31.50 cfs @ 5.79 fps)
- 4=RCP_Arch 44x27 (Barrel Controls 31.50 cfs @ 5.79 fps)
- 5=RCP_Arch 44x27 (Barrel Controls 31.50 cfs @ 5.79 fps)
- 6=RCP_Arch 44x27 (Barrel Controls 31.50 cfs @ 5.79 fps)

Pond 12P: P-12

Hydrograph



Summary for Pond 13P: P-13

Inflow Area = 237.893 ac, 51.59% Impervious, Inflow Depth = 5.57" for 100-Year event
 Inflow = 652.01 cfs @ 12.35 hrs, Volume= 110.403 af
 Outflow = 629.00 cfs @ 12.46 hrs, Volume= 110.401 af, Atten= 4%, Lag= 6.6 min
 Primary = 608.53 cfs @ 12.46 hrs, Volume= 105.483 af
 Secondary = 20.47 cfs @ 12.46 hrs, Volume= 4.917 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 883.00' Surf.Area= 1.870 ac Storage= 4.265 af
 Peak Elev= 885.58' @ 12.46 hrs Surf.Area= 2.805 ac Storage= 10.282 af (6.017 af above start)

Plug-Flow detention time= 64.3 min calculated for 106.125 af (96% of inflow)
 Center-of-Mass det. time= 12.0 min (871.5 - 859.5)

Volume	Invert	Avail.Storage	Storage Description
#1	878.00'	11.490 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
878.00	0.000	0.000	0.000
879.00	0.630	0.315	0.315
880.00	0.730	0.680	0.995
882.00	1.070	1.800	2.795
883.00	1.870	1.470	4.265
884.00	2.220	2.045	6.310
886.00	2.960	5.180	11.490

Device	Routing	Invert	Outlet Devices
#1	Primary	883.00'	55.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88
#2	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#3	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#4	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#5	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#6	Secondary	883.00'	12.0" Round RCP_Round 12" L= 100.0' RCP, groove end projecting, Ke= 0.200

Inlet / Outlet Invert= 883.00' / 882.75' S= 0.0025 1' Cc= 0.900
n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=608.52 cfs @ 12.46 hrs HW=885.58' TW=0.00' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Weir Controls 608.52 cfs @ 4.29 fps)

Secondary OutFlow Max=20.47 cfs @ 12.46 hrs HW=885.58' TW=883.29' (Dynamic Tailwater)

2=RCP_Round 12" (Barrel Controls 4.09 cfs @ 5.21 fps)

3=RCP_Round 12" (Barrel Controls 4.09 cfs @ 5.21 fps)

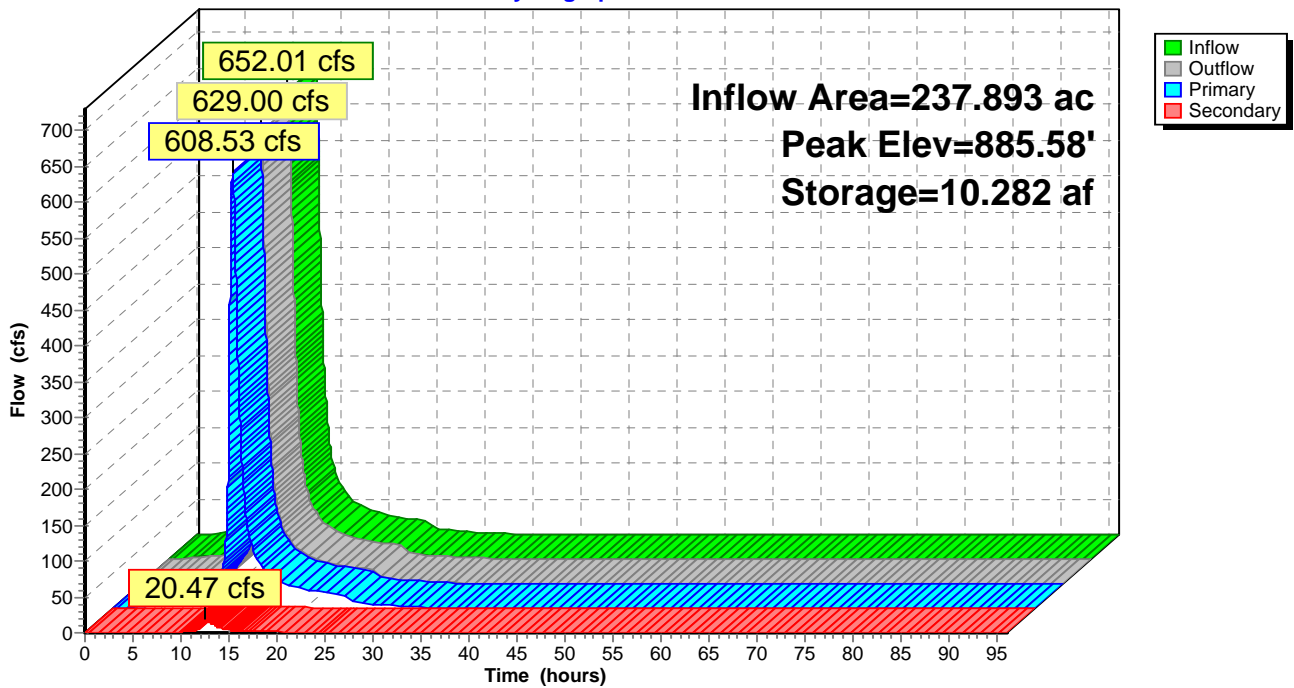
4=RCP_Round 12" (Barrel Controls 4.09 cfs @ 5.21 fps)

5=RCP_Round 12" (Barrel Controls 4.09 cfs @ 5.21 fps)

6=RCP_Round 12" (Barrel Controls 4.09 cfs @ 5.21 fps)

Pond 13P: P-13

Hydrograph



Summary for Pond 14P: P-14

Inflow Area = 21.198 ac, 39.93% Impervious, Inflow Depth = 5.41" for 100-Year event
 Inflow = 89.07 cfs @ 12.29 hrs, Volume= 9.565 af
 Outflow = 16.07 cfs @ 13.06 hrs, Volume= 9.563 af, Atten= 82%, Lag= 46.4 min
 Primary = 16.07 cfs @ 13.06 hrs, Volume= 9.563 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 892.00' Surf.Area= 1.380 ac Storage= 4.490 af
 Peak Elev= 895.17' @ 13.06 hrs Surf.Area= 1.695 ac Storage= 9.341 af (4.851 af above start)

Plug-Flow detention time= 580.1 min calculated for 5.073 af (53% of inflow)
 Center-of-Mass det. time= 239.8 min (1,035.6 - 795.8)

Volume	Invert	Avail.Storage	Storage Description
#1	888.00'	9.910 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
888.00	0.950	0.000	0.000
890.00	1.080	2.030	2.030
892.00	1.380	2.460	4.490
893.00	1.470	1.425	5.915
894.00	1.570	1.520	7.435
895.50	1.730	2.475	9.910

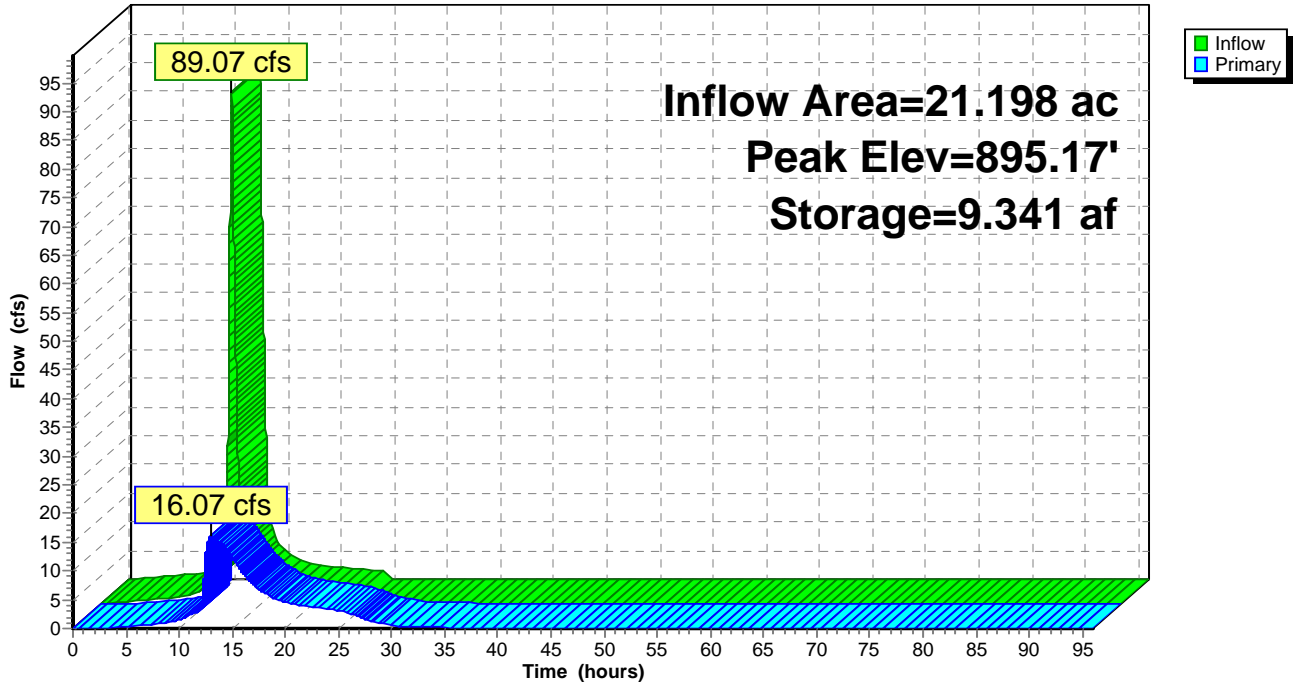
Device	Routing	Invert	Outlet Devices
#1	Primary	892.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	893.00'	18.0" Round RCP_Round 18" L= 50.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 893.00' / 892.75' S= 0.0050 1' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=16.07 cfs @ 13.06 hrs HW=895.17' (Free Discharge)

- ↑ 1=Orifice/Grate (Orifice Controls 6.73 cfs @ 8.57 fps)
- └ 2=RCP_Round 18" (Barrel Controls 9.34 cfs @ 5.29 fps)

Pond 14P: P-14

Hydrograph



Summary for Pond 23P: Thumb Infiltration (Thumb TP load only)

Inflow Area = 48.540 ac, 84.23% Impervious, Inflow Depth = 6.23" for 100-Year event
 Inflow = 171.27 cfs @ 12.44 hrs, Volume= 25.207 af
 Outflow = 171.19 cfs @ 12.45 hrs, Volume= 21.467 af, Atten= 0%, Lag= 0.6 min
 Primary = 171.19 cfs @ 12.45 hrs, Volume= 21.467 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 903.88' @ 12.45 hrs Surf.Area= 1.000 ac Storage= 3.880 af

Plug-Flow detention time= 126.7 min calculated for 21.467 af (85% of inflow)
 Center-of-Mass det. time= 58.7 min (836.9 - 778.1)

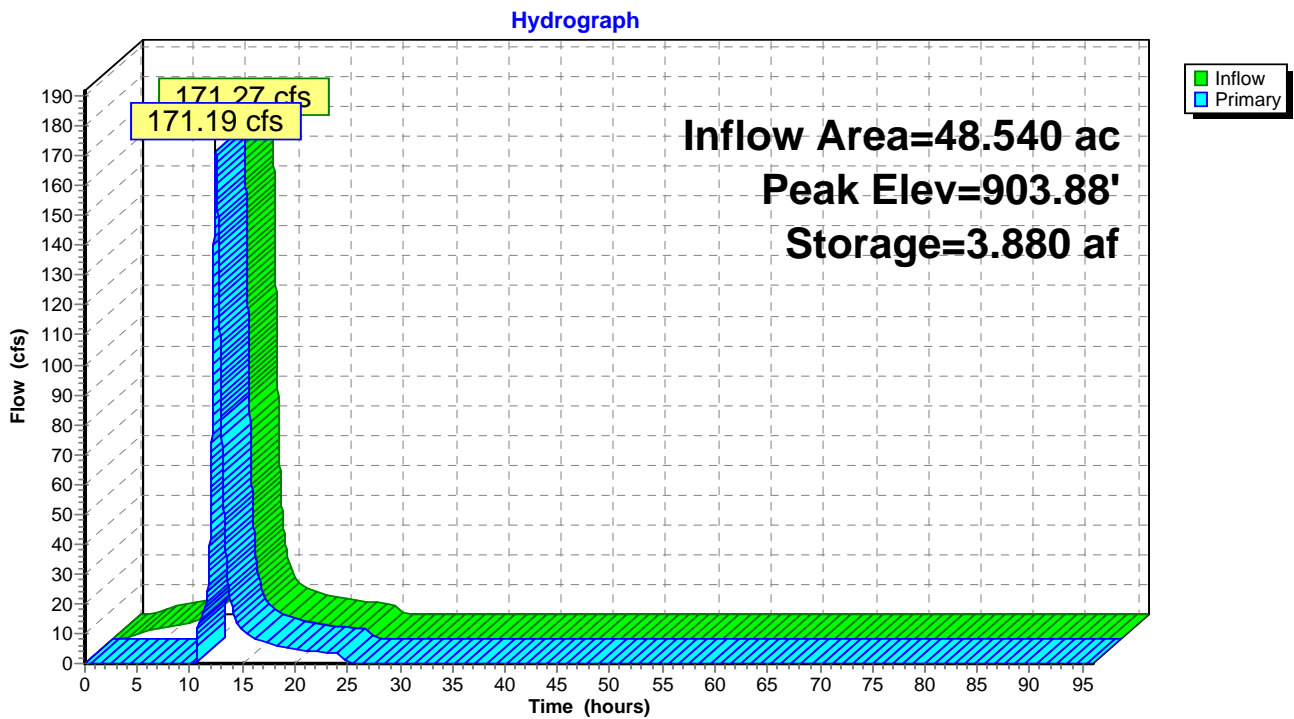
Volume	Invert	Avail.Storage	Storage Description
#1	900.00'	5.000 af	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
900.00	1.000	0.000	0.000
901.00	1.000	1.000	1.000
902.00	1.000	1.000	2.000
903.00	1.000	1.000	3.000
904.00	1.000	1.000	4.000
905.00	1.000	1.000	5.000

Device	Routing	Invert	Outlet Devices
#1	Primary	903.74'	1,000.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) 5.0' Crest Height

Primary OutFlow Max=171.15 cfs @ 12.45 hrs HW=903.88' (Free Discharge)
 ↑1=Sharp-Crested Rectangular Weir (Weir Controls 171.15 cfs @ 1.23 fps)

Pond 23P: Thumb Infiltration (Thumb TP load only)



Summary for Pond 31P: SB 18 Infiltration

Inflow Area = 52.908 ac, 84.55% Impervious, Inflow Depth = 6.65" for 100-Year event
 Inflow = 224.72 cfs @ 12.40 hrs, Volume= 29.299 af
 Outflow = 224.59 cfs @ 12.40 hrs, Volume= 25.979 af, Atten= 0%, Lag= 0.3 min
 Primary = 224.59 cfs @ 12.40 hrs, Volume= 25.979 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 903.48' @ 12.40 hrs Surf.Area= 1.000 ac Storage= 3.485 af

Plug-Flow detention time= 105.3 min calculated for 25.979 af (89% of inflow)
 Center-of-Mass det. time= 48.6 min (823.5 - 774.8)

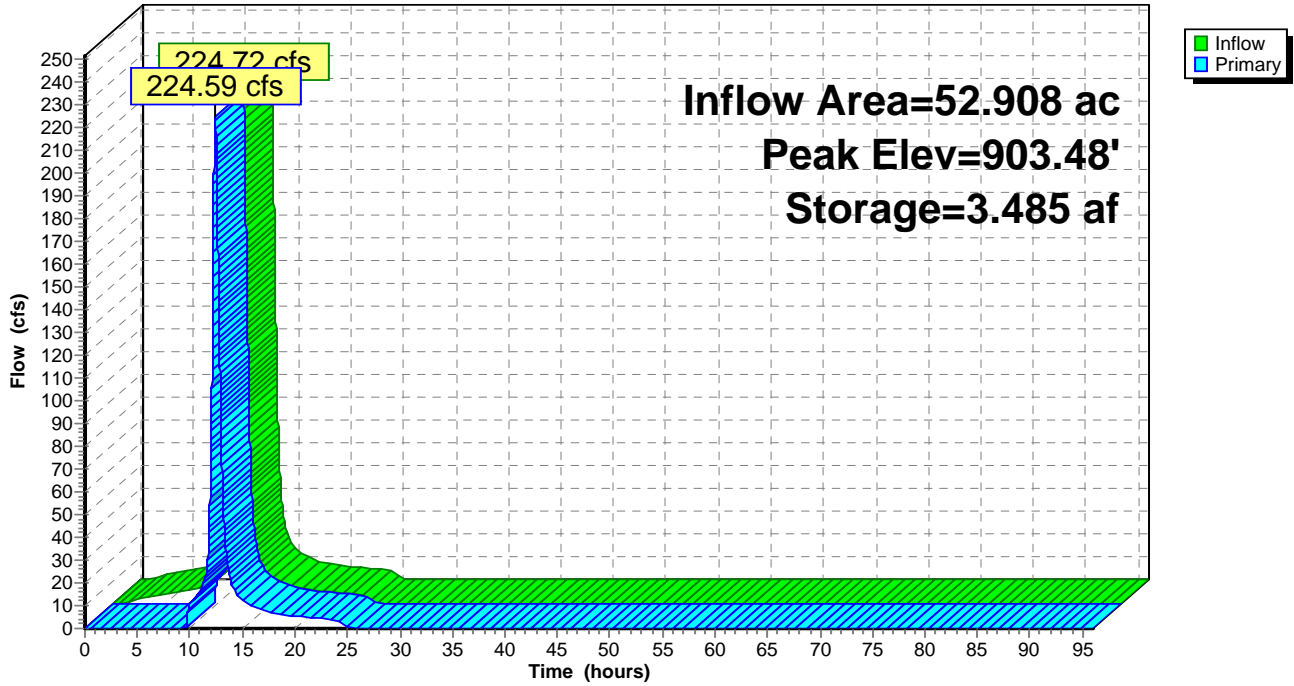
Volume	Invert	Avail.Storage	Storage Description
#1	900.00'	5.000 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
900.00	1.000	0.000	0.000
901.00	1.000	1.000	1.000
902.00	1.000	1.000	2.000
903.00	1.000	1.000	3.000
904.00	1.000	1.000	4.000
905.00	1.000	1.000	5.000

Device	Routing	Invert	Outlet Devices
#1	Primary	903.32'	1,000.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) 0.8' Crest Height

Primary OutFlow Max=224.52 cfs @ 12.40 hrs HW=903.48' TW=890.33' (Dynamic Tailwater)
 ↑1=Sharp-Crested Rectangular Weir (Weir Controls 224.52 cfs @ 1.36 fps)

Pond 31P: SB 18 Infiltration

Hydrograph



Summary for Pond 36P: Culverts passing flow beneath Spine Road

Inflow Area = 52.908 ac, 84.55% Impervious, Inflow Depth = 5.89" for 100-Year event
 Inflow = 224.59 cfs @ 12.40 hrs, Volume= 25.979 af
 Outflow = 224.53 cfs @ 12.41 hrs, Volume= 25.979 af, Atten= 0%, Lag= 0.2 min
 Primary = 125.00 cfs @ 12.14 hrs, Volume= 22.689 af
 Secondary = 99.53 cfs @ 12.41 hrs, Volume= 3.290 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 890.33' @ 12.41 hrs Surf.Area= 0.006 ac Storage= 0.013 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 0.0 min (823.5 - 823.5)

Volume	Invert	Avail.Storage	Storage Description
#1	887.00'	0.025 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
887.00	0.000	0.000	0.000
887.50	0.003	0.001	0.001
890.50	0.006	0.014	0.014
892.00	0.009	0.011	0.025

Device	Routing	Invert	Outlet Devices
#1	Primary	887.00'	Special & User-Defined Head (feet) 0.00 0.10 0.20 0.30 0.40 0.50 5.00 Disch. (cfs) 0.000 25.000 50.000 75.000 100.000 125.000 125.000
#2	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#3	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#4	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#5	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#6	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#7	Secondary	887.50'	18.0" Round RCP_Round 18" L= 100.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/1 Cc= 0.900

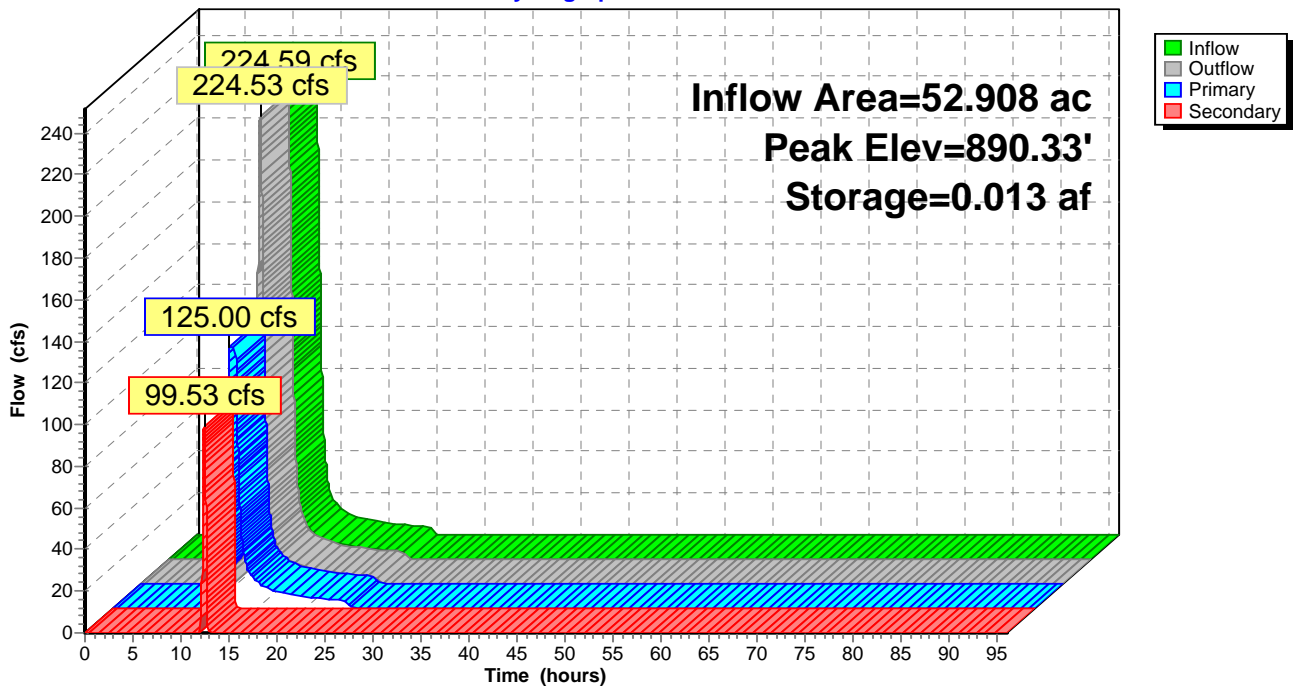
- #8 Secondary 887.50' n= 0.013, Flow Area= 1.77 sf **18.0" Round RCP_Round 18"**
 L= 100.0' RCP, groove end w/headwall, Ke= 0.200
 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/ Cc= 0.900
- #9 Secondary 887.50' n= 0.013, Flow Area= 1.77 sf **18.0" Round RCP_Round 18"**
 L= 100.0' RCP, groove end w/headwall, Ke= 0.200
 Inlet / Outlet Invert= 887.50' / 886.50' S= 0.0100 1/ Cc= 0.900
 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=125.00 cfs @ 12.14 hrs HW=887.69' TW=884.86' (Dynamic Tailwater)
 ↳1=Special & User-Defined (Custom Controls 125.00 cfs)

Secondary OutFlow Max=99.47 cfs @ 12.41 hrs HW=890.33' TW=885.56' (Dynamic Tailwater)
 ↳2=RCP_Round 18" (Barrel Controls 12.43 cfs @ 7.04 fps)
 ↳3=RCP_Round 18" (Barrel Controls 12.43 cfs @ 7.04 fps)
 ↳4=RCP_Round 18" (Barrel Controls 12.43 cfs @ 7.04 fps)
 ↳5=RCP_Round 18" (Barrel Controls 12.43 cfs @ 7.04 fps)
 ↳6=RCP_Round 18" (Barrel Controls 12.43 cfs @ 7.04 fps)
 ↳7=RCP_Round 18" (Barrel Controls 12.43 cfs @ 7.04 fps)
 ↳8=RCP_Round 18" (Barrel Controls 12.43 cfs @ 7.04 fps)
 ↳9=RCP_Round 18" (Barrel Controls 12.43 cfs @ 7.04 fps)

Pond 36P: Culverts passing flow beneath Spine Road

Hydrograph



Summary for Pond CRH-1: CRH-1

Inflow Area = 6.955 ac, 46.76% Impervious, Inflow Depth = 5.60" for 100-Year event
 Inflow = 38.37 cfs @ 12.15 hrs, Volume= 3.247 af
 Outflow = 25.53 cfs @ 12.31 hrs, Volume= 3.247 af, Atten= 33%, Lag= 9.5 min
 Discarded = 0.37 cfs @ 12.31 hrs, Volume= 0.563 af
 Primary = 25.16 cfs @ 12.31 hrs, Volume= 2.685 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 878.81' @ 12.31 hrs Surf.Area= 0.463 ac Storage= 0.760 af

Plug-Flow detention time= 114.2 min calculated for 3.247 af (100% of inflow)
 Center-of-Mass det. time= 114.3 min (895.5 - 781.2)

Volume	Invert	Avail.Storage	Storage Description
#1	876.00'	0.850 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
876.00	0.150	0.000	0.000
878.00	0.300	0.450	0.450
879.00	0.500	0.400	0.850

Device	Routing	Invert	Outlet Devices
#1	Discarded	876.00'	0.800 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 0.00'
#2	Primary	877.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 877.00' / 876.00' S= 0.0065 ' /' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Primary	877.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 877.00' / 876.00' S= 0.0065 ' /' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Discarded OutFlow Max=0.37 cfs @ 12.31 hrs HW=878.81' (Free Discharge)

↳ **1=Exfiltration** (Controls 0.37 cfs)

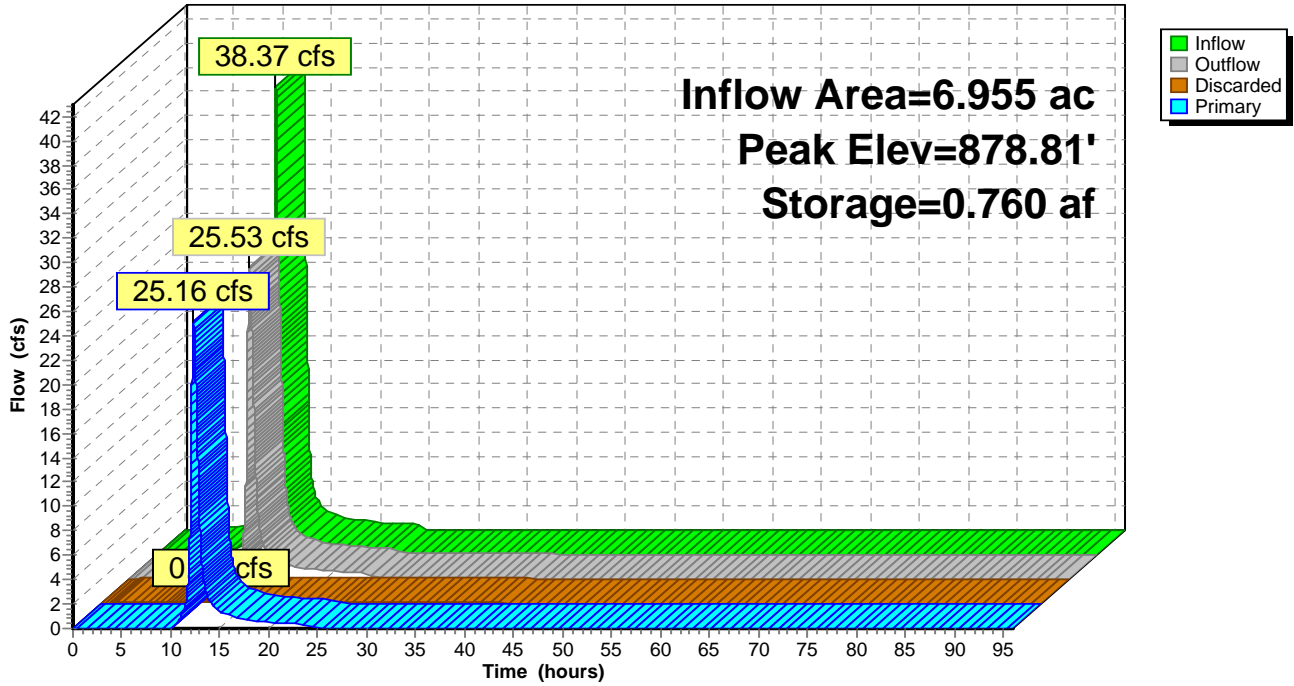
Primary OutFlow Max=25.15 cfs @ 12.31 hrs HW=878.81' (Free Discharge)

↳ **2=Culvert** (Barrel Controls 12.58 cfs @ 5.53 fps)

↳ **3=Culvert** (Barrel Controls 12.58 cfs @ 5.53 fps)

Pond CRH-1: CRH-1

Hydrograph



Summary for Pond CRH-2: CRH-2

Inflow Area = 10.214 ac, 37.73% Impervious, Inflow Depth = 5.35" for 100-Year event
 Inflow = 48.13 cfs @ 12.21 hrs, Volume= 4.557 af
 Outflow = 27.85 cfs @ 12.47 hrs, Volume= 4.557 af, Atten= 42%, Lag= 15.7 min
 Discarded = 0.47 cfs @ 12.47 hrs, Volume= 0.986 af
 Primary = 27.38 cfs @ 12.47 hrs, Volume= 3.571 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 883.78' @ 12.47 hrs Surf.Area= 0.578 ac Storage= 1.468 af

Plug-Flow detention time= 190.2 min calculated for 4.557 af (100% of inflow)
 Center-of-Mass det. time= 190.4 min (982.8 - 792.4)

Volume	Invert	Avail.Storage	Storage Description
#1	880.00'	1.600 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
880.00	0.200	0.000	0.000
882.00	0.400	0.600	0.600
884.00	0.600	1.000	1.600

Device	Routing	Invert	Outlet Devices
#1	Discarded	880.00'	0.800 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 0.00'
#2	Primary	881.50'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 881.50' / 881.00' S= 0.0032 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Primary	881.50'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 881.50' / 881.00' S= 0.0032 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Discarded OutFlow Max=0.47 cfs @ 12.47 hrs HW=883.78' (Free Discharge)

↑ **1=Exfiltration** (Controls 0.47 cfs)

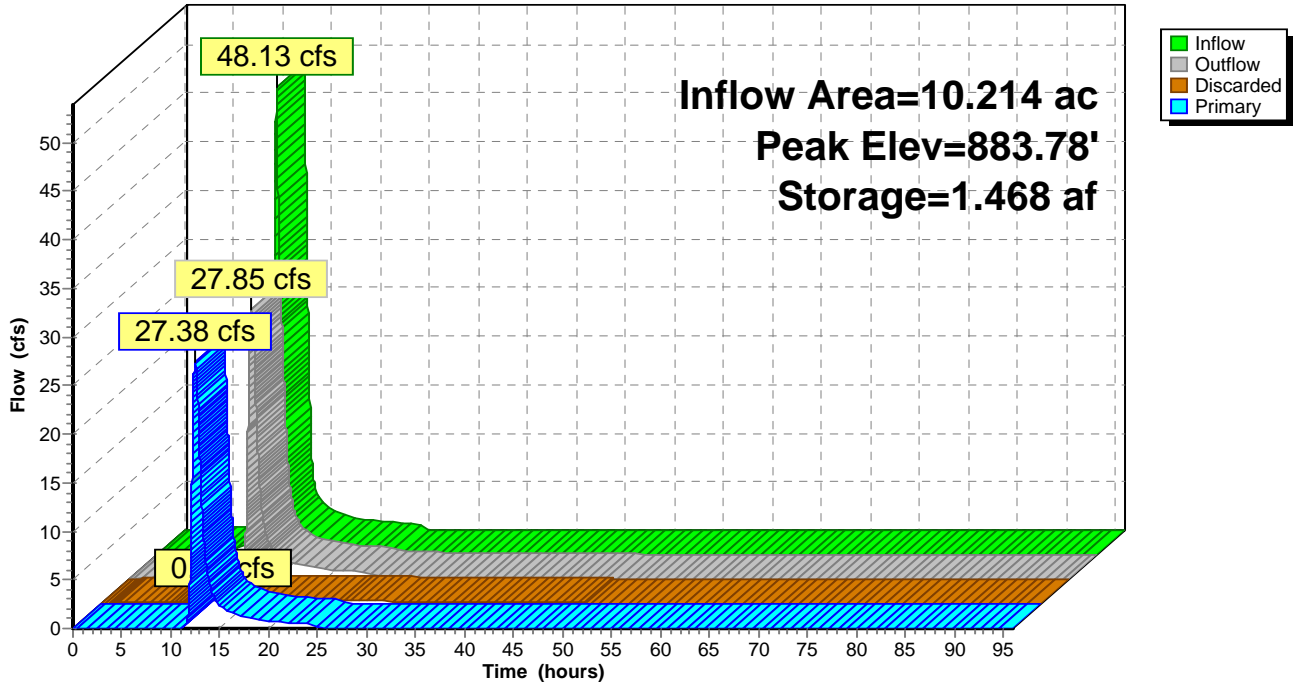
Primary OutFlow Max=27.38 cfs @ 12.47 hrs HW=883.78' TW=879.69' (Dynamic Tailwater)

↑ **2=Culvert** (Barrel Controls 13.69 cfs @ 4.79 fps)

↑ **3=Culvert** (Barrel Controls 13.69 cfs @ 4.79 fps)

Pond CRH-2: CRH-2

Hydrograph



Summary for Pond CRH-3: CRH-3

Inflow Area = 11.815 ac, 36.95% Impervious, Inflow Depth = 4.33" for 100-Year event
 Inflow = 30.15 cfs @ 12.44 hrs, Volume= 4.265 af
 Outflow = 25.90 cfs @ 12.71 hrs, Volume= 4.265 af, Atten= 14%, Lag= 15.7 min
 Discarded = 0.38 cfs @ 12.71 hrs, Volume= 0.519 af
 Primary = 25.52 cfs @ 12.71 hrs, Volume= 3.745 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 879.83' @ 12.71 hrs Surf.Area= 0.466 ac Storage= 0.769 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 82.2 min (919.3 - 837.1)

Volume	Invert	Avail.Storage	Storage Description
#1	877.00'	0.850 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
877.00	0.150	0.000	0.000
879.00	0.300	0.450	0.450
880.00	0.500	0.400	0.850

Device	Routing	Invert	Outlet Devices
#1	Discarded	877.00'	0.800 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 0.00'
#2	Primary	878.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 878.00' / 877.00' S= 0.0065 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#3	Primary	878.00'	24.0" Round Culvert L= 155.0' Ke= 0.500 Inlet / Outlet Invert= 878.00' / 877.00' S= 0.0065 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Discarded OutFlow Max=0.38 cfs @ 12.71 hrs HW=879.83' (Free Discharge)

↳ **1=Exfiltration** (Controls 0.38 cfs)

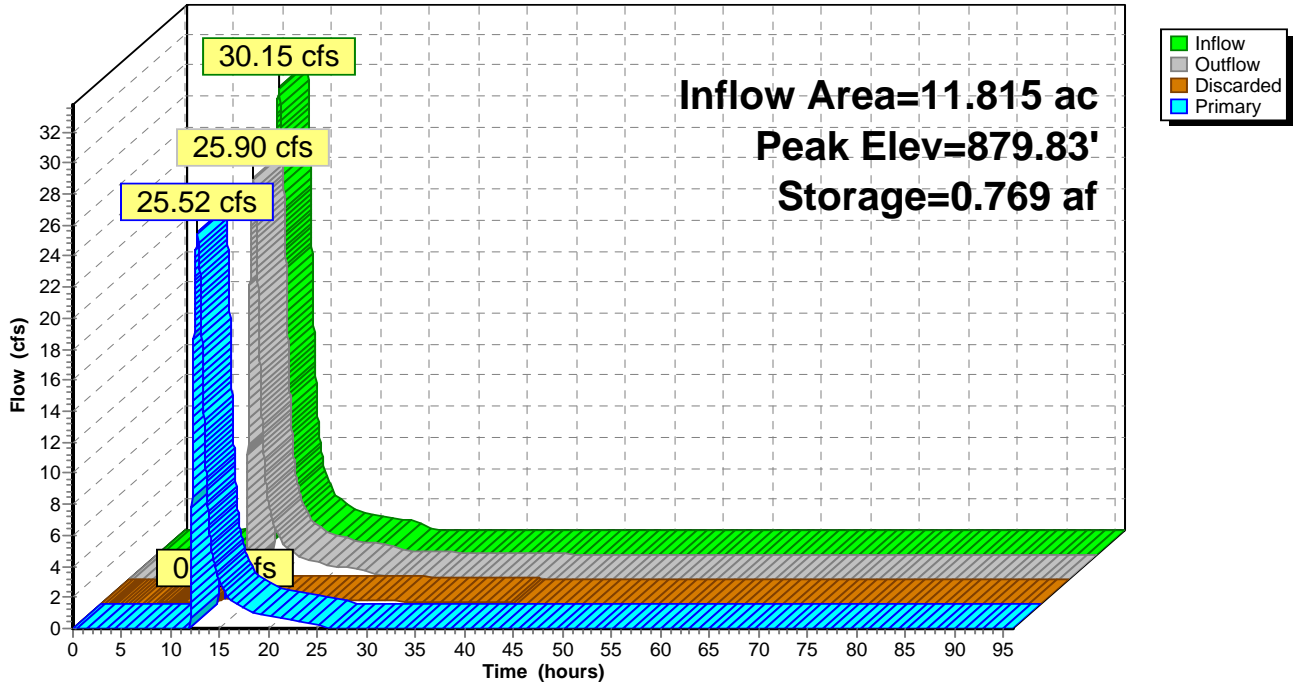
Primary OutFlow Max=25.52 cfs @ 12.71 hrs HW=879.83' (Free Discharge)

↳ **2=Culvert** (Barrel Controls 12.76 cfs @ 5.55 fps)

↳ **3=Culvert** (Barrel Controls 12.76 cfs @ 5.55 fps)

Pond CRH-3: CRH-3

Hydrograph



Summary for Pond P1/P2: P-1/P-2

Inflow Area = 68.531 ac, 57.92% Impervious, Inflow Depth = 5.91" for 100-Year event
 Inflow = 181.28 cfs @ 12.57 hrs, Volume= 33.757 af
 Outflow = 177.47 cfs @ 12.65 hrs, Volume= 33.749 af, Atten= 2%, Lag= 5.2 min
 Primary = 177.47 cfs @ 12.65 hrs, Volume= 33.749 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 924.00' Surf.Area= 1.270 ac Storage= 3.500 af
 Peak Elev= 925.63' @ 12.65 hrs Surf.Area= 1.514 ac Storage= 5.762 af (2.262 af above start)

Plug-Flow detention time= 130.6 min calculated for 30.246 af (90% of inflow)
 Center-of-Mass det. time= 40.6 min (839.6 - 799.0)

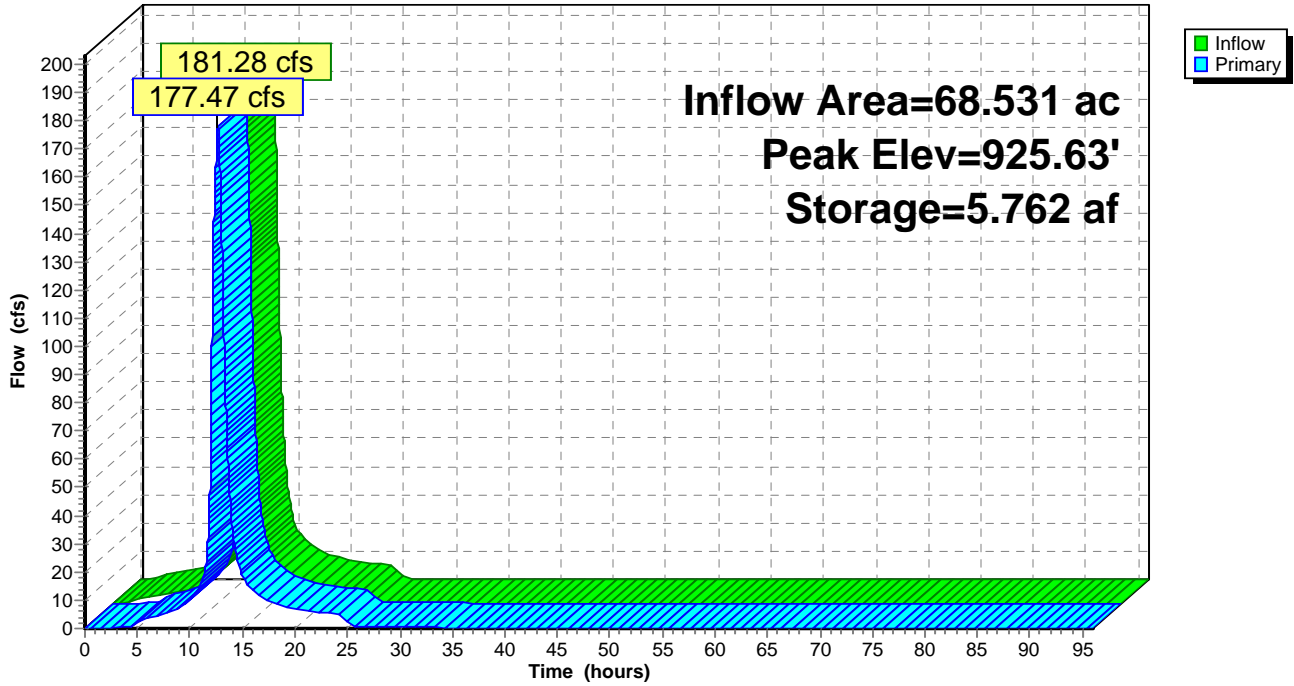
Volume	Invert	Avail.Storage	Storage Description
#1	920.00'	6.340 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
920.00	0.650	0.000	0.000
922.00	0.790	1.440	1.440
924.00	1.270	2.060	3.500
926.00	1.570	2.840	6.340

Device	Routing	Invert	Outlet Devices
#1	Primary	924.40'	40.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Primary	924.00'	6.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=177.46 cfs @ 12.65 hrs HW=925.62' TW=919.66' (Dynamic Tailwater)
 1=Sharp-Crested Rectangular Weir (Weir Controls 176.25 cfs @ 3.62 fps)
 2=Orifice/Grate (Orifice Controls 1.21 cfs @ 6.14 fps)

Pond P1/P2: P-1/P-2

Hydrograph



Summary for Pond P5/P6: P-5/P-6

Inflow Area = 43.279 ac, 47.44% Impervious, Inflow Depth = 5.62" for 100-Year event
 Inflow = 234.37 cfs @ 12.15 hrs, Volume= 20.279 af
 Outflow = 122.50 cfs @ 12.39 hrs, Volume= 18.162 af, Atten= 48%, Lag= 14.7 min
 Primary = 119.36 cfs @ 12.39 hrs, Volume= 16.461 af
 Secondary = 3.15 cfs @ 12.39 hrs, Volume= 1.701 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 929.00' Surf.Area= 1.975 ac Storage= 5.062 af
 Peak Elev= 932.56' @ 12.39 hrs Surf.Area= 2.681 ac Storage= 13.463 af (8.400 af above start)

Plug-Flow detention time= 329.8 min calculated for 13.098 af (65% of inflow)
 Center-of-Mass det. time= 152.1 min (932.6 - 780.5)

Volume	Invert	Avail.Storage	Storage Description
#1	926.00'	14.650 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
926.00	1.510	0.000	0.000
928.00	1.710	3.220	3.220
930.00	2.240	3.950	7.170
931.00	2.400	2.320	9.490
933.00	2.760	5.160	14.650

Device	Routing	Invert	Outlet Devices
#1	Primary	930.00'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	930.50'	7.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#3	Primary	931.50'	14.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#4	Secondary	930.00'	9.0" Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=119.34 cfs @ 12.39 hrs HW=932.56' TW=919.05' (Dynamic Tailwater)

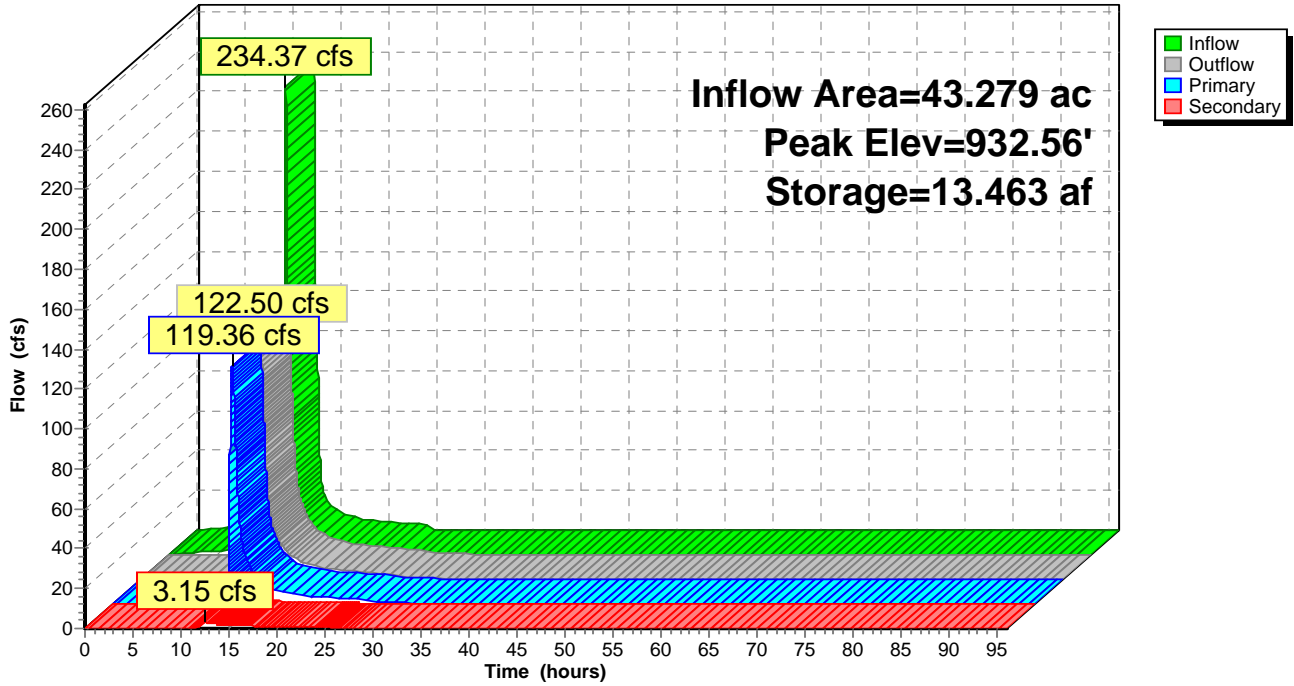
- ↑1=Orifice/Grate (Orifice Controls 6.05 cfs @ 7.71 fps)
- └2=Sharp-Crested Rectangular Weir (Weir Controls 63.85 cfs @ 4.70 fps)
- └3=Sharp-Crested Rectangular Weir (Weir Controls 49.44 cfs @ 3.37 fps)

Secondary OutFlow Max=3.15 cfs @ 12.39 hrs HW=932.56' TW=929.14' (Dynamic Tailwater)

- ↑4=Orifice/Grate (Orifice Controls 3.15 cfs @ 7.12 fps)

Pond P5/P6: P-5/P-6

Hydrograph



Summary for Pond P8: P-8

Inflow Area = 6.389 ac, 7.62% Impervious, Inflow Depth = 4.52" for 100-Year event
 Inflow = 39.72 cfs @ 12.05 hrs, Volume= 2.409 af
 Outflow = 11.00 cfs @ 12.33 hrs, Volume= 2.403 af, Atten= 72%, Lag= 16.4 min
 Primary = 11.00 cfs @ 12.33 hrs, Volume= 2.403 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 897.00' Surf.Area= 0.300 ac Storage= 0.495 af
 Peak Elev= 899.15' @ 12.57 hrs Surf.Area= 0.496 ac Storage= 1.415 af (0.920 af above start)

Plug-Flow detention time= 268.2 min calculated for 1.908 af (79% of inflow)
 Center-of-Mass det. time= 129.7 min (940.4 - 810.7)

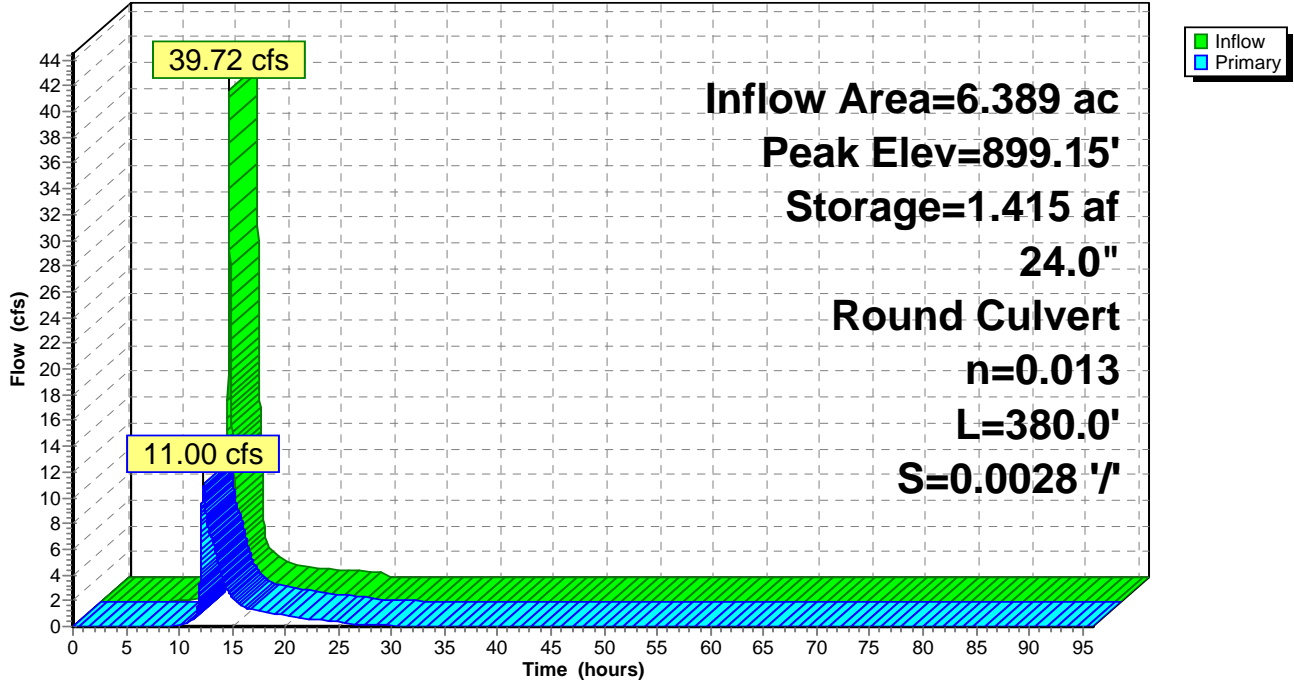
Volume	Invert	Avail.Storage	Storage Description
#1	893.00'	1.850 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
893.00	0.030	0.000	0.000
894.00	0.070	0.050	0.050
896.00	0.150	0.220	0.270
897.00	0.300	0.225	0.495
898.00	0.450	0.375	0.870
900.00	0.530	0.980	1.850

Device	Routing	Invert	Outlet Devices
#1	Primary	897.00'	24.0" Round RCP_Round 24" L= 380.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 897.00' / 895.94' S= 0.0028 1/'' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

Primary OutFlow Max=10.95 cfs @ 12.33 hrs HW=899.10' TW=897.82' (Dynamic Tailwater)
 ↳1=RCP_Round 24" (Outlet Controls 10.95 cfs @ 4.12 fps)

Pond P8: P-8

Hydrograph



Summary for Pond W-1: W-1

Inflow Area = 0.997 ac, 24.47% Impervious, Inflow Depth = 26.99" for 100-Year event
 Inflow = 6.17 cfs @ 12.25 hrs, Volume= 2.242 af
 Outflow = 2.99 cfs @ 14.14 hrs, Volume= 2.242 af, Atten= 52%, Lag= 113.3 min
 Primary = 2.99 cfs @ 14.14 hrs, Volume= 2.242 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 915.37' @ 14.14 hrs Surf.Area= 0.760 ac Storage= 0.443 af

Plug-Flow detention time= 109.6 min calculated for 2.242 af (100% of inflow)
 Center-of-Mass det. time= 109.3 min (974.6 - 865.3)

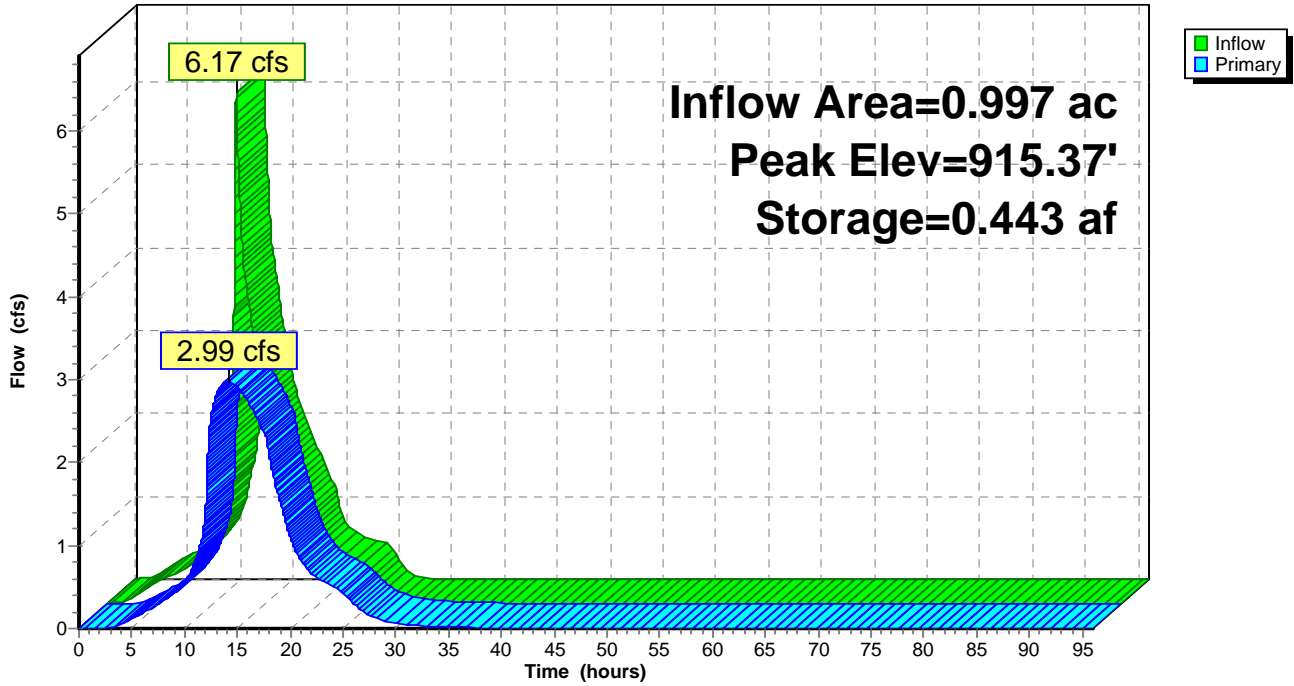
Volume	Invert	Avail.Storage	Storage Description
#1	914.75'	0.950 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
914.75	0.660	0.000	0.000
916.00	0.860	0.950	0.950

Device	Routing	Invert	Outlet Devices
#1	Primary	914.75'	12.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=2.99 cfs @ 14.14 hrs HW=915.37' TW=0.00' (Dynamic Tailwater)
 ↑**1=Orifice/Grate** (Orifice Controls 2.99 cfs @ 3.80 fps)

Pond W-1: W-1

Hydrograph



Summary for Pond W-2: W-2

Inflow = 3.15 cfs @ 12.39 hrs, Volume= 1.701 af
 Outflow = 1.17 cfs @ 18.92 hrs, Volume= 1.553 af, Atten= 63%, Lag= 391.6 min
 Primary = 1.17 cfs @ 18.92 hrs, Volume= 1.553 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 929.60' @ 18.92 hrs Surf.Area= 1.191 ac Storage= 0.681 af

Plug-Flow detention time= 533.4 min calculated for 1.553 af (91% of inflow)
 Center-of-Mass det. time= 473.2 min (1,532.2 - 1,059.1)

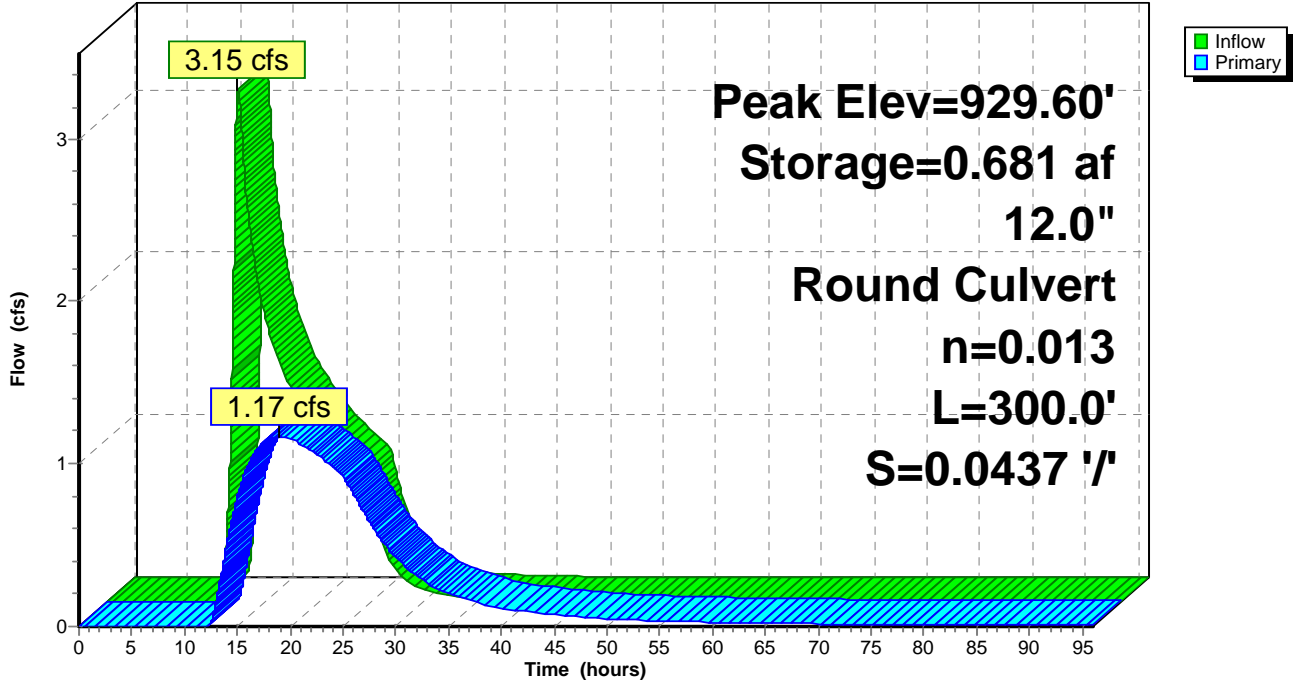
Volume	Invert	Avail.Storage	Storage Description
#1	929.00'	1.175 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
929.00	1.090	0.000	0.000
930.00	1.260	1.175	1.175

Device	Routing	Invert	Outlet Devices
#1	Primary	929.10'	12.0" Round RCP_Round 12" L= 300.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 929.10' / 916.00' S= 0.0437 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=1.17 cfs @ 18.92 hrs HW=929.60' TW=914.96' (Dynamic Tailwater)
 ↳1=RCP_Round 12" (Inlet Controls 1.17 cfs @ 3.00 fps)

Pond W-2: W-2

Hydrograph



Summary for Pond W-3: W-3

Inflow = 1.17 cfs @ 18.92 hrs, Volume= 1.553 af
 Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

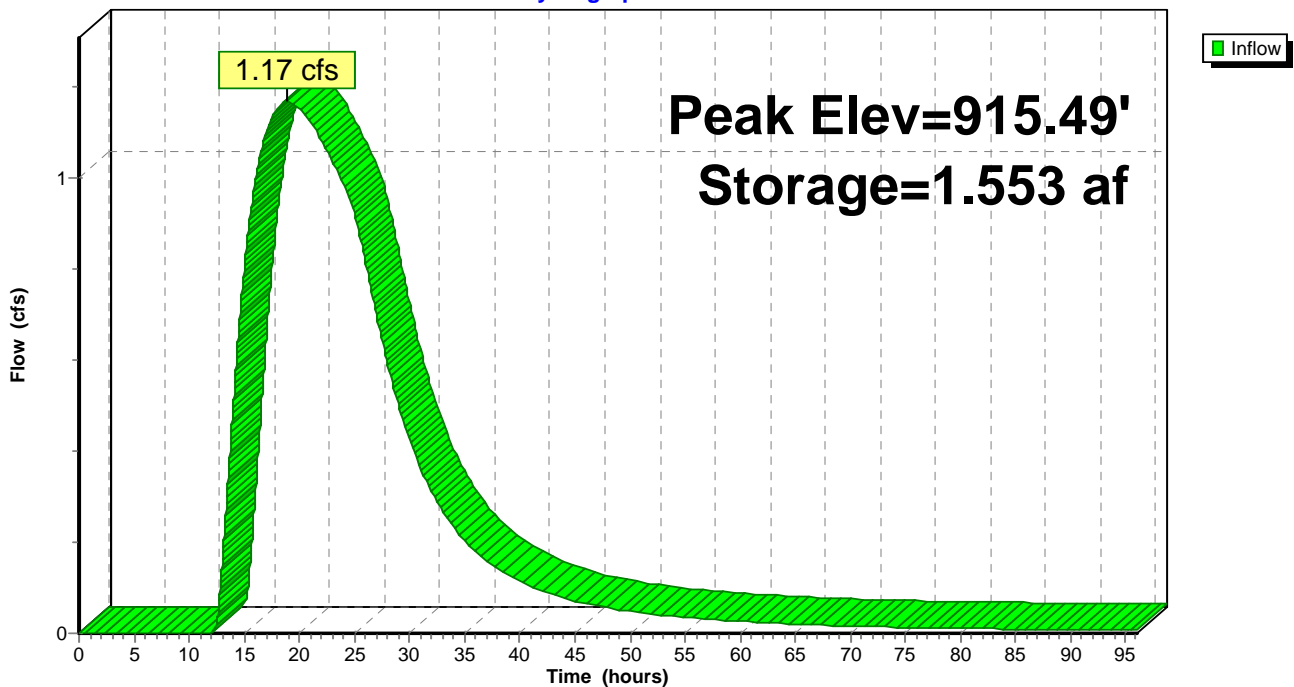
Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 915.49' @ 96.00 hrs Surf.Area= 2.163 ac Storage= 1.553 af

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description
#1	914.75'	2.680 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
914.75	2.040	0.000	0.000
915.00	2.080	0.515	0.515
916.00	2.250	2.165	2.680

Pond W-3: W-3

Hydrograph



Summary for Pond W-4: W-4

Inflow Area = 2.985 ac, 30.99% Impervious, Inflow Depth = 20.64" for 100-Year event
 Inflow = 22.11 cfs @ 12.08 hrs, Volume= 5.134 af
 Outflow = 4.36 cfs @ 14.67 hrs, Volume= 5.114 af, Atten= 80%, Lag= 155.4 min
 Primary = 4.36 cfs @ 14.67 hrs, Volume= 5.114 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Peak Elev= 909.35' @ 14.67 hrs Surf.Area= 1.266 ac Storage= 1.383 af

Plug-Flow detention time= 248.3 min calculated for 5.114 af (100% of inflow)
 Center-of-Mass det. time= 244.9 min (1,203.6 - 958.7)

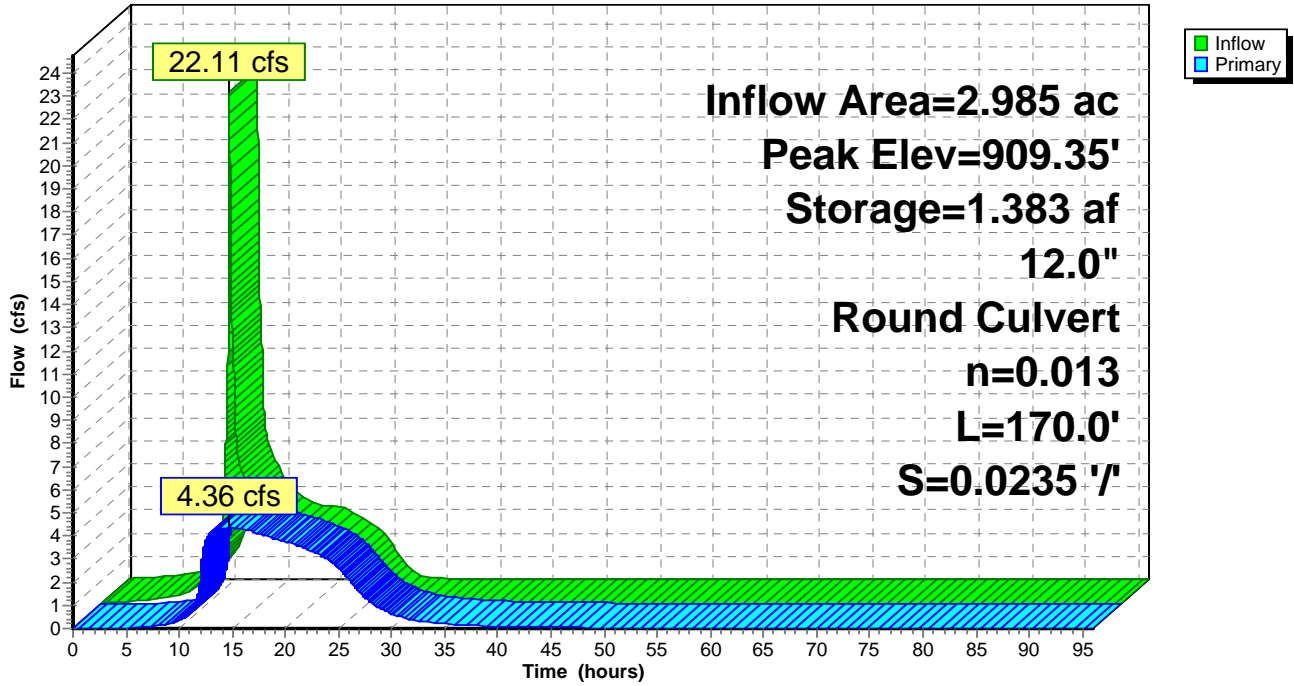
Volume	Invert	Avail.Storage	Storage Description
#1	908.00'	2.280 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
908.00	0.780	0.000	0.000
910.00	1.500	2.280	2.280

Device	Routing	Invert	Outlet Devices
#1	Primary	908.00'	12.0" Round RCP_Round 12" L= 170.0' RCP, groove end w/headwall, Ke= 0.200 Inlet / Outlet Invert= 908.00' / 904.00' S= 0.0235 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=4.36 cfs @ 14.67 hrs HW=909.35' TW=894.39' (Dynamic Tailwater)
 ↳1=RCP_Round 12" (Inlet Controls 4.36 cfs @ 5.55 fps)

Pond W-4: W-4

Hydrograph



Summary for Pond W-5: W-5

Inflow Area = 7.608 ac, 48.41% Impervious, Inflow Depth = 13.52" for 100-Year event
 Inflow = 76.11 cfs @ 12.02 hrs, Volume= 8.572 af
 Outflow = 17.76 cfs @ 13.09 hrs, Volume= 8.565 af, Atten= 77%, Lag= 64.3 min
 Primary = 17.76 cfs @ 13.09 hrs, Volume= 8.565 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Starting Elev= 882.75' Surf.Area= 4.887 ac Storage= 7.134 af
 Peak Elev= 883.35' @ 13.09 hrs Surf.Area= 5.709 ac Storage= 10.267 af (3.132 af above start)

Plug-Flow detention time= 1,065.0 min calculated for 1.430 af (17% of inflow)
 Center-of-Mass det. time= 188.1 min (1,009.6 - 821.6)

Volume	Invert	Avail.Storage	Storage Description
#1	881.00'	11.097 af	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
881.00	3.270	0.000	0.000
882.00	4.190	3.730	3.730
883.00	5.120	4.655	8.385
883.49	5.950	2.712	11.097

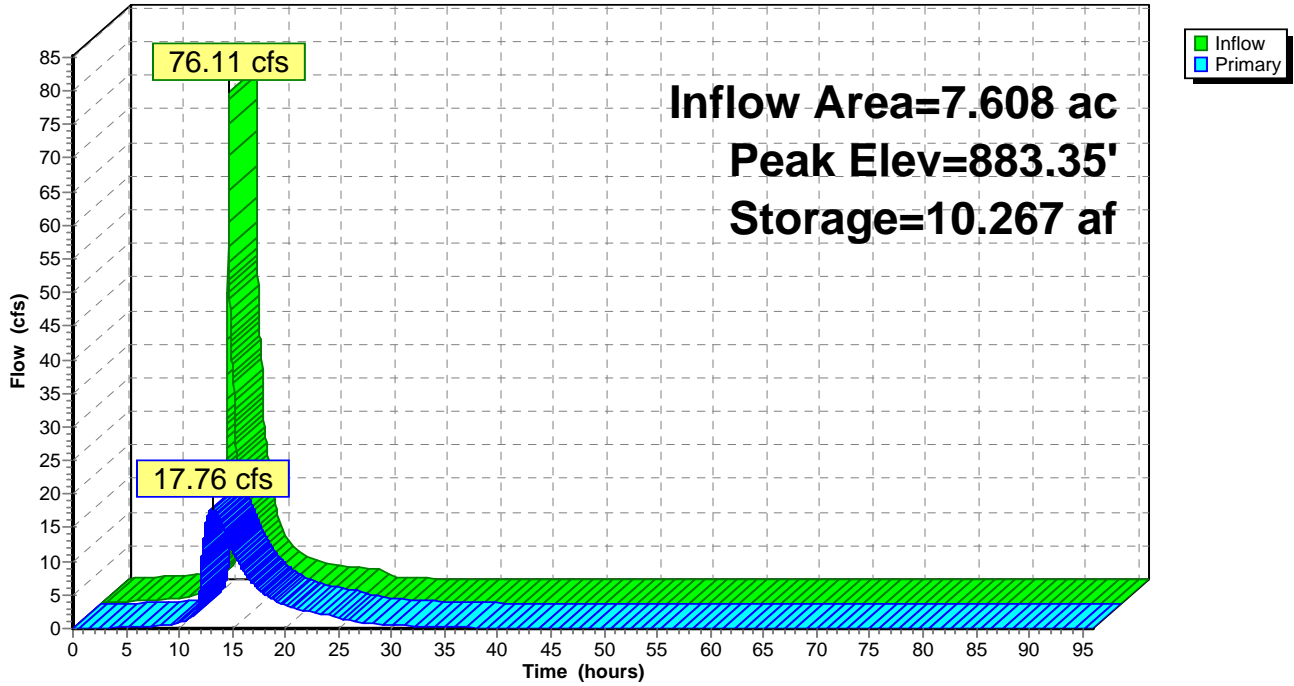
Device	Routing	Invert	Outlet Devices
#1	Primary	882.75'	6.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Primary	882.75'	6.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)

Primary OutFlow Max=17.76 cfs @ 13.09 hrs HW=883.35' TW=0.00' (Dynamic Tailwater)

- 1=Sharp-Crested Rectangular Weir (Weir Controls 8.88 cfs @ 2.53 fps)
- 2=Sharp-Crested Rectangular Weir (Weir Controls 8.88 cfs @ 2.53 fps)

Pond W-5: W-5

Hydrograph

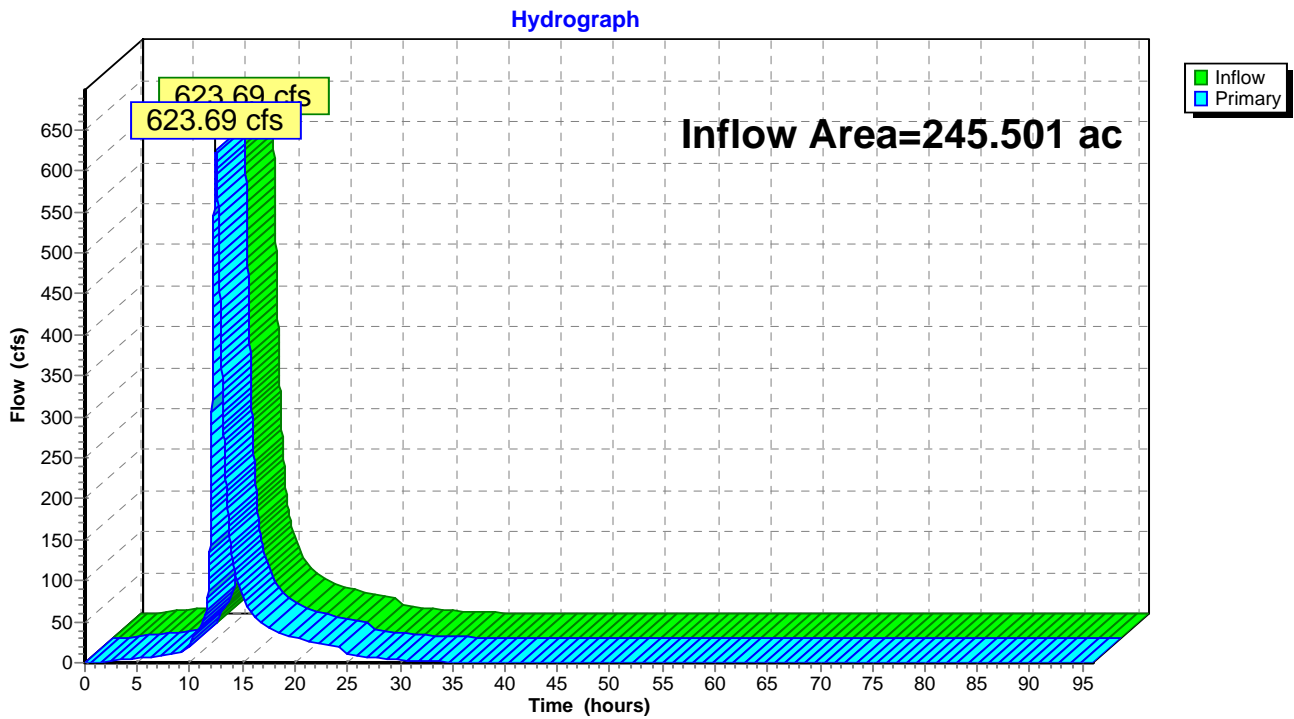


Summary for Link 49L: Sum of P-13 and W-5 Discharges to Rice Creek

Inflow Area = 245.501 ac, 51.49% Impervious, Inflow Depth = 5.57" for 100-Year event
Inflow = 623.69 cfs @ 12.46 hrs, Volume= 114.048 af
Primary = 623.69 cfs @ 12.46 hrs, Volume= 114.048 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 49L: Sum of P-13 and W-5 Discharges to Rice Creek

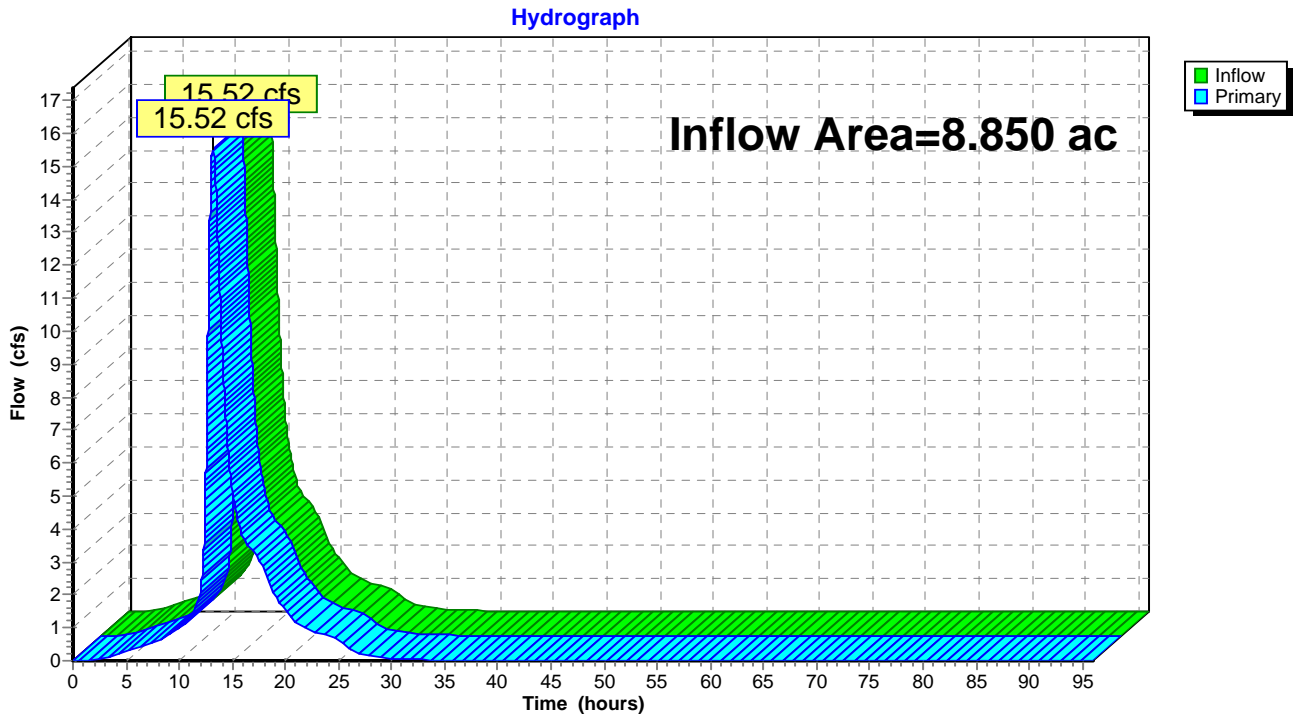


Summary for Link 50L: Outlet #1 Discharge to Round Lake

Inflow Area = 8.850 ac, 65.20% Impervious, Inflow Depth = 6.12" for 100-Year event
Inflow = 15.52 cfs @ 13.18 hrs, Volume= 4.512 af
Primary = 15.52 cfs @ 13.18 hrs, Volume= 4.512 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 50L: Outlet #1 Discharge to Round Lake

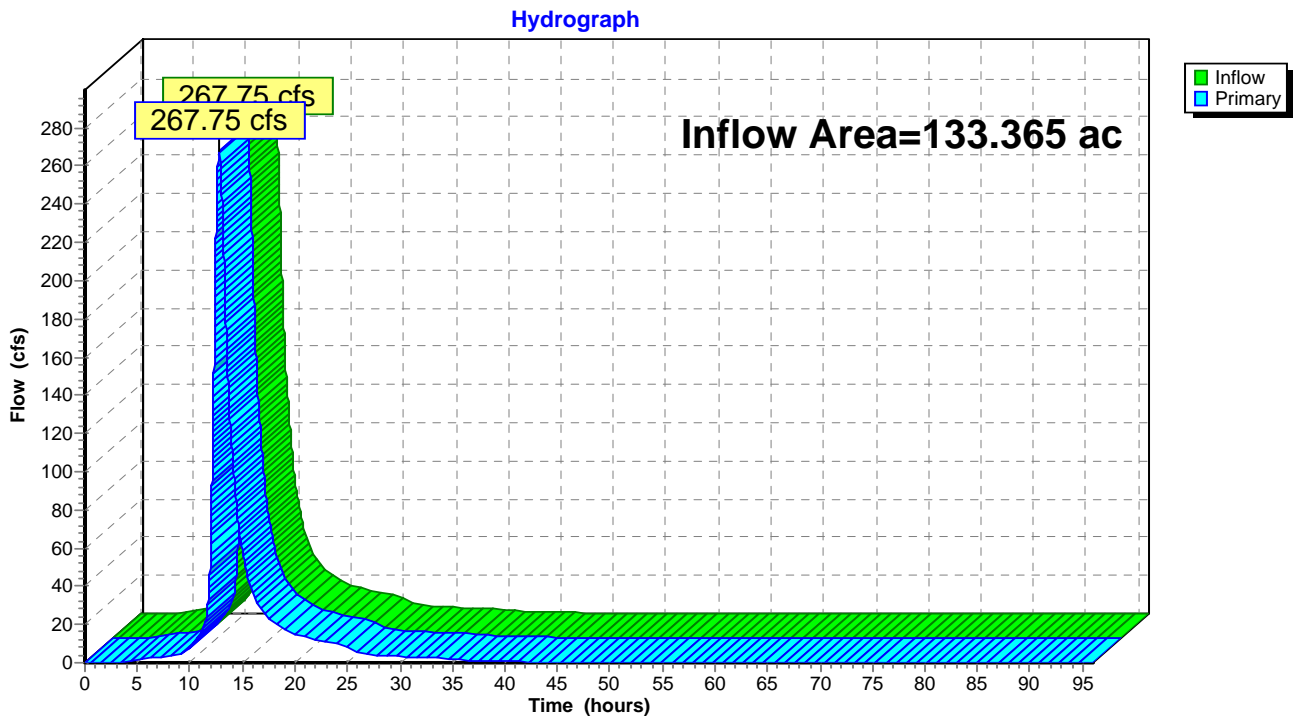


Summary for Link 51L: Outlet #2 Discharge to Round Lake

Inflow Area = 133.365 ac, 58.87% Impervious, Inflow Depth = 5.59" for 100-Year event
Inflow = 267.75 cfs @ 12.78 hrs, Volume= 62.142 af
Primary = 267.75 cfs @ 12.78 hrs, Volume= 62.142 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 51L: Outlet #2 Discharge to Round Lake



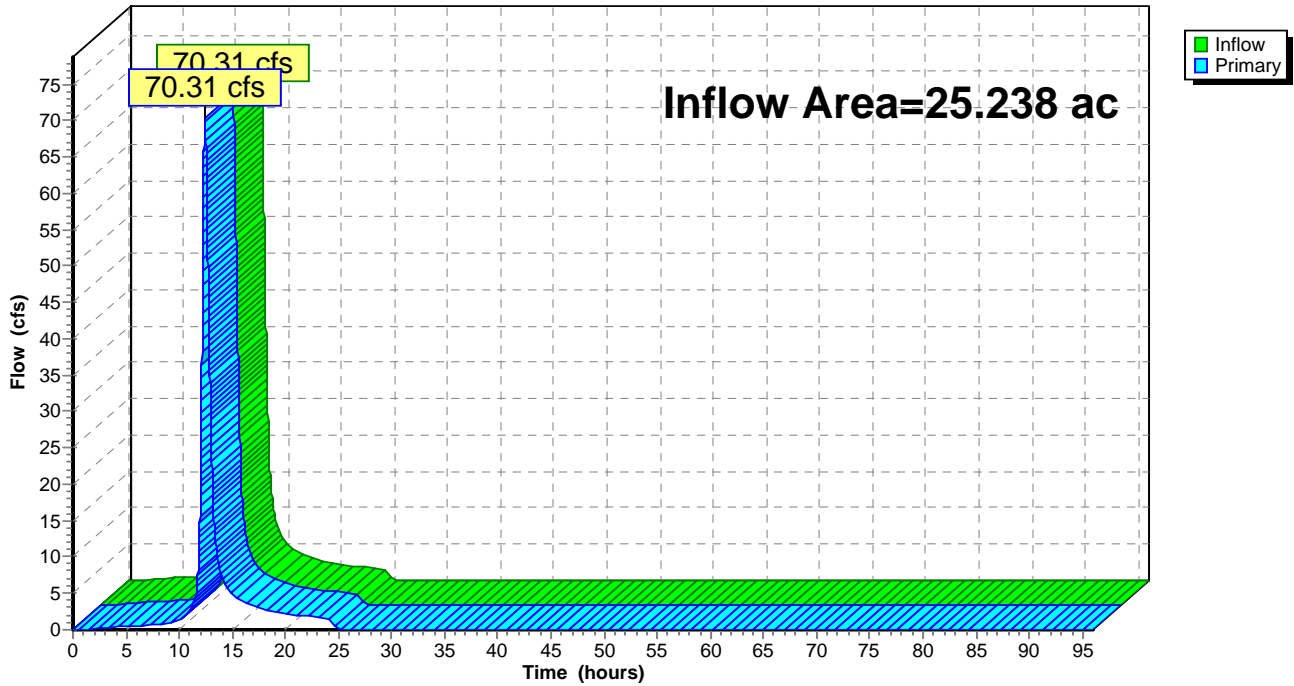
Summary for Link 52L: Outlet #3 Discharge to Round Lake

Inflow Area = 25.238 ac, 19.96% Impervious, Inflow Depth = 4.09" for 100-Year event
Inflow = 70.31 cfs @ 12.43 hrs, Volume= 8.599 af
Primary = 70.31 cfs @ 12.43 hrs, Volume= 8.599 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Link 52L: Outlet #3 Discharge to Round Lake

Hydrograph



Technical Memo



Responsive partner.
Exceptional outcomes.

To: Beth Kunkel, Kimley-Horn

From: Pamela Massaro, PE, Wenck Associates, Inc.
Mary Pate-Holt, EIT, Wenck Associates, Inc.
Adam Marsh, EIT, Wenck Associates, Inc.

Date: July 30, 2015

Subject: P8 Water Quality Modeling

1.1 Introduction

The purpose of this technical memorandum is to summarize the stormwater water quality modeling completed using the P8 model as presented in the Comprehensive Stormwater Management Plan (CSMP) submitted to the Rice Creek Watershed District (RCWD) for the portion of the former Twin Cities Army Ammunition Plant (TCAAP) Site being redeveloped by Ramsey County (County), the City of Arden Hills (City), the County's consultants (Kimley Horn, Wenck Associates Inc.), and future Developers. The Site, under 2012 existing conditions, provides few stormwater control structures to reduce discharge rate and just grassy swales as best management practices (BMPs) to improve water quality treatment before stormwater leaves the Site.

1.2 Project Location

The 427-acre site is located in the Ramsey County, Minnesota in the cities of Arden Hills and New Brighton, the Twin Cities Army Ammunitions Plant (TCAAP) is bounded by U.S. Interstate Highway 35 on the west, Minnesota Highway 96 to the south and US Highway 10 to the Southwest (**CSMP Figure 2-1**).

Rice Creek divides the Site into two sections: the portion south of Rice Creek, and the portion north of Rice Creek. The Site drains to both Rice Creek and Round Lake (**CSMP Figure 2-2**). The portion north of Rice Creek is north of County Road H and drains to Rice Creek, the northern two-thirds of the Site south of Rice Creek drains to Rice Creek, and the southern third of the Site south of Rice Creek drains south to Round Lake.

1.3 Water Quality Modeling Scenarios

The Site, under proposed fully developed conditions, will meet RCWD's water quality treatment requirements through a combination of onsite infiltration and wet detention ponds. Hydrology and Hydraulics results are summarized in a separate HydroCAD Modeling technical memorandum. This technical memorandum summarizes the evaluation of phosphorous removal using the P8 water quality model for interim and fully developed conditions at the Site. Wet detention ponds (stand alone and connected in series) hydraulically connected to wetlands (both mitigation and existing), a vegetated swale, and

“to be designed” infiltration practices are planned to provide the water quality treatment to reduce the total phosphorous (TP) load from stormwater before discharging to the Resource of Concern (ROC). Dead storage volume, of the wet detention pond, is the most important design parameter influencing the pollutant removal efficiency as it provides runoff storage and water quality treatment between storms. Phosphorous removals were evaluated for several stages of development and various treatment trains by discharge point; Table 1 describes the different model scenarios evaluated by ROC.

Table 1: P8 Modeling Scenarios

Resource of Concern	Development Stage
Round Lake	
Interim Scenario 1	Construction of Spine Road, mitigation wetlands (W-1, W-2), stormwater ponds (P-2, P-4, P-5, P-6), 60” RCP and 36” RCP to Outfall #2 (60” under Hwy 10), and connections to Outfall #1 (24” under Hwy 10) (as shown in CSMP Figure 4-5) The Site’s open space is undeveloped and vegetated (CN 74).
Scenario 2	Fully Developed Conditions (CSMP Figure 4-5 with landuse per CSMP Figure 3-1)
Rice Creek	
Interim Scenario 1	Construction of Spine Road, County Road H/I-35W Interchange, remainder of Rice Creek, Old Highway 8 extension Road, mitigation wetlands (W-3, W-4, W-5), stormwater ponds (P-7, P-8, P-9, P-10, P-11, P-12, and P-13), MnDOT ponds (CRH-1, CRH-2, and CRH-3), and vegetated swale (SB-18) connected to P-13 under Spine Road. The Site’s open space is undeveloped and vegetated (CN 74).
Interim Scenario 2	Interim Scenario 1 plus stormwater pond (P-14), development east of Spine Road (landuse per CSMP Figure 3-1). Assume there is no development or infiltration device in SB-18 (CSMP Figure 4-7).
Scenario 3	Fully Developed Conditions (CSMP Figure 4-5 with landuse per CSMP Figure 3-1)

Many figures referenced in this technical memorandum are from the CSMP document and referenced as “CSMP Figure X-X”.

2.1 P8 Computer Model

P8 (Program for Predicting Polluting Particle Passage through Pits, Puddles and Ponds, IEP, Inc., 1990) is a computer model used for predicting the generation and transport of stormwater runoff pollutants in urban watersheds. P8 is a useful diagnostic tool for evaluating and designing watershed improvements like green infrastructure. The model requires user input on watershed characteristics, green infrastructure dimensions, local precipitation and temperature, and water quality parameters.

P8 calculates runoff separately from pervious and impervious areas. Calculations for pervious areas use the Soil Conservation Service (SCS) Curve Number (CN) method. The

CN for pervious areas was 74 and the impervious area CNs was assigned 98 for all scenarios tested. To represent changes in development, the percent impervious was adjusted for the modeling scenarios presented in Table 1.

The percent impervious of scenario 1 (for both resources of concern) was calculated using Table 2 of the HydroCAD Technical Memorandum. In the fully developed conditions (for both resources of concern), the percent impervious was calculated using Table 3 of the HydroCAD Technical Memorandum. The Rice Creek interim scenario 2 model uses the interim percent impervious for subwatershed 18 (Table 2 of the HydroCAD Technical Memorandum) and the full development percent impervious for the rest of the site (Table 3 of the HydroCAD Technical Memorandum). The surface areas of the ponds and wetlands were included in the tributary area directly contributing runoff into each pond (see the Stormwater Pond Data Summary Tables included in the Appendix of the HydroCAD Technical Memorandum). This provides a conservative estimate for the amount of TP removed. Runoff from impervious areas begins once the cumulative storm rainfall exceeds the specified depression storage, with the runoff rate equal to the rainfall intensity.

The ponds in P8 were built using area, volume, and outlet information from HydroCAD. Surface areas were inputted for each pond's bottom, permanent pool, and flood pool. The permanent pool was calculated as the surface area at the primary outlet. The flood pool surface area was calculated at the top contour of the pond. Permanent and flood pool storage for each pond are shown in the Stormwater Pond Data Summary Tables included in the Appendix of the HydroCAD Technical Memorandum. Outlet devices are shown in the Stormwater Pond Data Summary Tables included in the Appendix of the HydroCAD Technical Memorandum

The P8 model uses an hourly precipitation record (rain and snowfall) and daily temperature record. Precipitation and temperature data were obtained from the Minneapolis-St. Paul International Airport. Records from 2001 to 2010 were used for this study. Model results summarized herein are annual averages reported by weight (pounds of TP per year).

Wenck selected the NURP₅₀ particle file for the TCAAP study. The component concentrations in the NURP₅₀ file represent the 50th percentile (median) values compiled in the US EPA's Nationwide Urban Runoff Program (NURP).

2.2 Proposed Stormwater Treatment

Wenck evaluated the proposed TP loads, by weight, for runoff into Round Lake and Rice Creek/Long Lake at each discharge point were evaluated individually (**CSMP Figures 4-6 and 4-7**). P8, under fully developed conditions, predicts runoff loading from subwatersheds that are directed to stormwater BMPs (e.g., ponds) for treatment. The proposed ponds P-1, P-2, P-3, P-4, P-5, P-6, and wetlands W-1 and W-2 discharge to Round Lake. While proposed ponds P-7, P-8, P-9, P-10, P-11, P-12, P-13, P-14, CRH-1, CHR-2, CHR-3, and wetlands Wi, W-3, W-4, and W-5 discharge to Rice Creek/Long Lake.

2.2.1 Infiltration BMPs

Runoff from the west side of the site (subwatershed 18) and the thumb will be treated by infiltration once the site is fully developed (**CSMP Figure 5-1, Areas 3 and 4**). This was modeled in P8, under fully developed conditions, by creating an infiltration basin sized to infiltrate the runoff from the proposed impervious surfaces. For Area 3 on **CSMP Figure 5-1**, the infiltration basin was sized to infiltrate 3.32 acre-ft (1.1 inches) of runoff. For Area 4 on **CSMP Figure 5-1**, the infiltration basin was sized to infiltrate 3.74 acre-ft (1.1 inches) of runoff. The modeled removal efficiency for the proposed ponds reflect the least effective BMP that will be installed. Under interim scenarios 1 and 2, a grassy swale was used as an infiltration treatment device in subwatershed 18. The grassy swale provides 6.15 acre-feet of infiltration volume. The P8 input details for the swale are shown in Table 2.

Table 2: Swale Details

Parameter	Measurement
Flow Path Length	3,400 feet
Flow Path Slope	1%
Bottom Width	4 feet
Side Slope	0.33 ft/ft
Maximum Depth	4.5 feet
Infiltration Rate	0.45 in/hour
Manning's n	0.15
Infiltration Volume	6.15 acre-feet

2.3 Pollutant Reduction

The Rice Creek Watershed District (RCWD) recommended TP loads (lbs/year) are reduced by at least 50%.

2.3.1 ROC: Rice Creek TP Reductions

The proposed ponding systems will reduce the annual TP load to Rice Creek by 65% under the public infrastructure improvement conditions, aka interim scenario 1 (**Table 3**, total), 55% in worst case, aka interim scenario 2 (**Table 4**, total), and 64% in fully developed conditions, aka scenario 3 (**Table 5**, total). TP load reductions at each discharge point are shown in **Tables 3-5**. The outfall locations and tributary areas are shown in **CSMP Figure 6-5**. Ponds CRH-1 and CRH-3 are not meeting the 50% reduction in annual TP load because they are only 30% designed; these ponds are required to have infiltration if the soil borings indicate infiltration is possible. Results from P8 are provided in **Appendix A**.

Table 3: TP Load Reductions to Rice Creek (Interim Scenario 1: Public Infrastructure Improvements)

Discharge Point	Total TP Inflow Load* (lbs./year)	Total Outflow Load (lbs./year)	% Reduction
Outfall #5 (Pond P-13)	75.3	28.9	62%
Outfall #4 (CRH-1)**	7.1	3.8	46%
Outfall #8 (CRH-3)**	5.4	4.2	22%
Outfall #9 (Thumb with Infiltration BMP)	19.0	1.0	95%
Total	106.8	37.9	65%

*The total TP inflow load for each discharge point was calculated by summing the TP loads from each contributing watershed (i.e. total TP inflow load for CRH-3 = TP load from CRH-2 + TP load from CRH-3).

** Ponds CRH-1 and CHR-3 are 30% designed.

Table 4: TP Load Reductions to Rice Creek (Interim Scenario 2: Worst Case)

Discharge Point	Total TP Inflow Load* (lbs./year)	Total Outflow Load (lbs./year)	% Reduction
Outfall #5 (Pond P-13)	189.6	91	52%
Outfall #10 (Pond P-14)	18.8	7.1	62%
Outfall #4 (CRH-1)**	8.6	4.9	43%
Outfall #8 (CRH-3)**	4.5	3.4	24%
Outfall #9 (Thumb with Infiltration BMP)	16.8	0.8	95%
Total	238.3	107.2	55%

*The total TP inflow load for each discharge point was calculated by summing the TP loads from each contributing watershed (i.e. total TP inflow load for CRH-3 = TP load from CRH-2 + TP load from CRH-3).

** Ponds CRH-1 and CHR-3 are 30% designed.

Table 5: TP Load Reductions to Rice Creek (Scenario 3: Fully Developed Conditions)

Discharge Point	Watershed Inflow (lbs./year)	Total Outflow (lbs./year)	% Reduction
Outfall #5 (Pond P-13)	279	99.9	64%
Outfall #10 (Pond P-14)	18.8	7.1	62%
Outfall #4 (CRH-1)**	7.1	3.8	46%
Outfall #8 (CRH-3)**	5.4	4.2	22%
Outfall #9 (Thumb with Infiltration BMP)	13.6	0.4	97%
Total	323.9	115.4	64%

*The total TP inflow load for each discharge point was calculated by summing the TP loads from each contributing watershed (i.e. total TP inflow load for CRH-3 = TP load from CRH-2 + TP load from CRH-3).

** Ponds CRH-1 and CHR-3 are 30% designed, see text above.

2.3.1 ROC: Round Lake TP Reductions

The proposed ponding systems will reduce the annual TP load to Round Lake by 53% in under the public infrastructure improvement conditions, aka interim scenario 1 (**Table 6**, total) and 60% in fully developed conditions, aka scenario 2 (**Tables 7**, total). TP load reductions at each discharge point are shown in **Tables 6-7**. The outfall locations and tributary areas are shown in **CSMP Figure 6-5**. Results from P8 are provided in **Appendix A**.

Table 6: TP Load Reductions to Round Lake (Interim Scenario 1: Public Infrastructure Improvements)

Discharge Point	Total TP Inflow Load* (lbs./year)	Total Outflow Load (lbs./year)	% Reduction
Outfall #2 (Pond P-3)	36.2	17	53%
Outfall #1 (Pond P-4)	1.7	0.8	53%
Total	37.9	17.8	53%

*The total TP inflow load for each discharge point was calculated by summing the TP loads from each contributing watershed (i.e. total TP inflow load for Outfall #1 = TP load from Pond 4 + TP load from Wetland 1).

Table 7: TP Load Reductions to Round Lake (Scenario 2: Fully Developed Conditions)

Discharge Point	Total TP Inflow Load* (lbs./year)	Total Outflow Load (lbs./year)	% Reduction
Outfall #2 (Pond P-3)	168.8	66.3	61%
Outfall #1 (Pond P-4)	12.4	6.1	51%
Total	181.2	72.4	60%

*The total TP inflow load for each discharge point was calculated by summing the TP loads from each contributing watershed (i.e. total TP inflow load for Outfall #1 = TP load from Pond 4 + TP load from Wetland 1)

3.1 Conclusion

The proposed ponding systems will reduce TP loads from the proposed site conditions by more than 50%, meeting RCWD standards. For runoff into Rice Creek, the TP load is 65% in interim scenario 1, 55% in interim scenario 2, and 64% in scenario 3. The proposed ponding systems will reduce the annual TP load to Round Lake by 53% in interim scenario 1 and 60% in scenario 2.

Appendix A

Round Lake P8 Results
Rice Creek/Long Lake P8 Results

Round Lake P8 Results
Interim Scenario 1

				Info.prn	
P8 Urban Catchment Model, Version 3.5					Run Date
06/08/15					
Case	Round Lake - Spine Road Only.p	FirstDate	01/01/01		Precip(in)
296.7					
Title	Round Lake	LastDate	12/31/10		Rain(in)
273.57					
PrecFile	precip1970-2010.pcp	Events	598		Snow(in)
23.12					
PartFile	nurp50.p8p	TotalHrs	87576		TotalYrs
9.99					

File Directory	T:\1382 KimleyHorn\01 TCAAP\TASK 03 Stormwater Prelim
Design\Models\P8\	
Case Title	Round Lake
Case File	Round Lake - Spine Road Only.p8c
Particle File	nurp50.p8p
Temperature File	temp1970-2011.tmp
Storm File	precip1970-2010.pcp
Precip Scale Factor	1
Watersheds	5
Devices	6
Particles	5
WQ Components	7
Start Date	06/01/00
Keep Date	01/01/01
Stop Date	12/31/10
Storm Count	598
Total Hours	87576
Wet Hours	9265
Precip (in)	297
Rain (in)	274
Snowfall (in)	23
Snowmelt (in)	22
EvapoTran(in)	303
Overall TSS Removal(%)	1
Water Balance Error(%)	0
TSS Mass Balance Error	0

Inputs.prn

P8 Urban Catchment Model, Version 3.5			Run Date
06/08/15			
Case	Round Lake - Spine Road Only.p	FirstDate	01/01/01
296.7			Precip(in)
Title	Round Lake	LastDate	12/31/10
273.57			Rain(in)
PrecFile	precip1970-2010.pcp	Events	598
23.12			Snow(in)
PartFile	nurp50.p8p	TotalHrs	87576
9.99			TotalYrs

Case Title	Round Lake
Case Data File	Round Lake - Spine Road Only.p8c
Path	T:\1382 KimleyHorn\01 TCAAP\TASK 03 Stormwater
Prelim Design\Models\P8\ Case Notes:	Round Lake
Ponds 1/2, 5/6, and 3	
Storm Data File	precip1970-2010.pcp
Particle File	nurp50.p8p
Air Temp File File	temp1970-2011.tmp

Time Steps Per Hour	8
Minimum Inter-Event Time (hrs)	10
Maximum Continuity Error %	2
Rainfall Breakpoint (inches)	0.8
Precipitation Scale Factor	1
Air Temp Offset (deg-F)	0
Loops Thru Storm File	1
Simulation Dates	
Start	6/1/2000
Keep	1/1/2001
Stop	12/31/2010

Max Snowfall Temperature (deg-f)	32.0
SnowMelt Temperature (deg-f)	32.0
Snowmelt Coef (in/degF-Day)	0.06
Soil Freeze Temp (deg-F)	32.0
Snowmelt Abstraction Factor	1.00
Evapo-Trans. Calibration Factor	1.00
Growing Season Start Month	5
Growing Season End Month	10

5-Day Antecedent Rainfall + Runoff (inches)		
CN Antecedent Moisture Condition	AMC-II	AMC-III
Growing Season	1.40	2.10
NonGrowing Season	0.50	1.10

Watershed Data						
Watershed Name	Subbasins	Subbasins	Subbasins	3Subbasin	6Subbasin	5
Runoff to Device	Pond 2	Outfall #2	Pond 5/6	Wetland 1	Pond 4	
Infiltration to Device						
Watershed Area	68.5	21.55	43.27	0.1	7.86	
SCS Curve Number (Pervious)	74	74	74	74	74	
Scale Factor for Pervious Runoff	1	1	1	1	1	
Indirectly Connected Imperv Frac	0	0	0	0	0	
UnSwept Impervious Fraction	0.07	0	0.19	0.1	0.06	
UnSwept Depression Storage (inch	0.02	0.02	0.02	0.02	0.02	
UnSwept Imperv. Runoff Coefficie	1	1	1	1	1	
UnSwept Scale Factor for Particl	1	1	1	1	1	
Swept Impervious Fraction	0	0	0	0	0	
Swept Depression Storage (inches	0.02	0	0.02	0	0	
Swept Imperv. Runoff Coefficient	1	1	1	1	1	

	Inputs.prn				
Swept Scale Factor for Particle	1	1	1	1	1
Sweeping Frequency	0	0	0	0.5	0.5
Sweeping Efficiency	1	1	1	1	1
Sweeping Start Date (MMDD)	101	101	101	101	101
Sweeping Stop Date (MMDD)	1231	1231	1231	1231	1231

Device Data	Pond 2	Pond 5/6	Pond 4	Wetland 1	Outfall
Device Name	#1Outfall #2				
Device Type	POND	POND	POND	POND	PIPE
PIPE					
Infiltration Outlet					
Normal Outlet	Outfall #2	Outfall #2	Outfall #1	Outfall #1	
Spillway Outlet	Outfall #2	Outfall #2	Outfall #1	Outfall #1	
Particle Removal Scale Factor	1	1	1	1	
Bottom Elevation (ft)	0	0	0	0	
Bottom Area (acres)	0.095	2.151	0	0.129	
Permanent Pool Area (acres)	0.368	2.729	0.266	0.241	
Permanent Pool Volume (ac-ft)	0.897	6.8	0.5	0	
Perm Pool Infiltr Rate (in/hr)	0	0	0	0	
Flood Pool Area (acres)	0.637	3.737	0.593	0.38	
Flood Pool Volume (ac-ft)	1.005	6.1	0.9	0.2	
Flood Pool Infiltr Rate (in/hr)	0	0	0	0	
Infiltr Basin Void Fraction (%)					
Detention Pond Outlet Parameters					
Outlet Type	ORIFICE	ORIFICE	ORIFICE	ORIFICE	
Outlet Orifice Diameter (in)	6	12	12	6	
Orifice Discharge Coef	1	1	0.6	0.6	
Outlet Weir Length (ft)					
Weir Discharge Coef					
Perforated Riser Height (ft)					
Number of Holes in Riser					
Holes Diameter					
Flood Pool Drain Time (hrs)					
Swale Parameters					
Length of Flow Path (ft)					
Slope of Flow Path %					
Bottom width (ft)					
Side Slope (ft-v/ft-h)					
Maximum Depth of Flow (ft)					
Mannings n Constant					
Hydraulic Model					
Pipe, Splitter, Aquifer Parameter					
Hydraulic Res. Time (hrs)					0
0					

Particle Data	nurp50.p8p	P10%	P30%	P50%	P80%
Particle File	nurp50.p8p				
Particle Class	P0%				
Filtration Efficiency	90	100	100	100	100
Settling Velocity (ft/	0	0.03	0.3	1.5	15
First Order Decay Rate	0	0	0	0	0
2nd Order Decay (1/day	0	0	0	0	0
Impervious Runoff Conc	1	0	0	0	0
Pervious Runoff Conc (1	100	100	100	200
Pervious Conc Exponent	0	1	1	1	1
Accum. Rate (lbs-ac-da	0	1.75	1.75	1.75	3.5
Particle Removal Rate	0	0.25	0.25	0.25	0.25
Washoff Coefficient	0	20	20	20	20
Washoff Exponent	0	2	2	2	2
Sweeper Efficiency	0	0	0	5	15

Water Quality Component Data

Component Name	TSS	Inputs.prn		CU	PB	ZN	
		TP	TKN				
HC							
Water Quality Criteria (ppm)							
0.1	Level 1	5	0.025	2	2	0.02	5
0.5	Level 2	10	0.05	1	0.0048	0.014	0.0362
1	Level 3	20	0.1	0.5	0.02	0.15	0.38
Content Scale Factor							
1		1	1	1	1	1	1
Particle Composition (mg/kg)							
P0%		0	99000	600000	13600	2000	640000
250000							
P10%		1000000	3850	15000	340	180	1600
22500							
P30%		1000000	3850	15000	340	180	1600
22500							
P50%		1000000	3850	15000	340	180	1600
22500							
P80%		1000000	0	0	340	180	0
22500							

Inputs.prn

Inputs.prn

				Network.prn	
P8 Urban Catchment Model, Version 3.5					Run Date
06/08/15					
Case	Round Lake - Spine Road Only.p	FirstDate	01/01/01		Precip(in)
296.7					
Title	Round Lake	LastDate	12/31/10		Rain(in)
273.57					
PrecFile	precip1970-2010.pcp	Events	598		Snow(in)
23.12					
PartFile	nurp50.p8p	TotalHrs	87576		TotalYrs
9.99					

Devices Listed in Downstream Order

Device:	Pond 2	Type:	POND
	Discharges normal outlet to		Outfall #2
	Discharges spillway to		Outfall #2
	Runoff from watershed		Subbasins 1, 2, 24
Device:	Pond 5/6	Type:	POND
	Discharges normal outlet to		Outfall #2
	Discharges spillway to		Outfall #2
	Runoff from watershed		Subbasins 3, 4, 25
Device:	Pond 4	Type:	POND
	Discharges normal outlet to		Outfall #1
	Discharges spillway to		Outfall #1
	Runoff from watershed		Subbasin 5
Device:	Wetland 1	Type:	POND
	Discharges normal outlet to		Outfall #1
	Discharges spillway to		Outfall #1
	Runoff from watershed		Subbasin 6
Device:	Outfall #1	Type:	PIPE
Device:	Outfall #2	Type:	PIPE
	Runoff from watershed		Subbasins 7

Connected UnSwept Areas		Directly Connected Swept Areas		Street Sweeping Parameters		Directly	
Depress Sweep	Total Imperv	Depress	Percol	Imperv	Start	Indirect	Pervious
Watershed	Area	Outflow	Runoff	Curve	Imperv	Load	Imperv
Storage	Runoff	Load	Storage	Load	Date	Date	Sweep
Freq	acres	Device	Device	Number	Fraction	Factor	Fraction
Label	Factor	Fraction	Coef	Factor	MMDD	MMDD	Effic
inches	Coef	inches	Coef	Factor	MMDD	MMDD	Effic
1/week							
Subbasins 1,	68.5	Pond 2		74	0.000	1	0.07
0.02	1	0.02	1	101	1231	1	0
Subbasins 7	21.55	Outfall #2		74	0.000	1	0
0.02	1	0	1	101	1231	1	0
Subbasins 3,	43.27	Pond 5/6		74	0.000	1	0.19
0.02	1	0.02	1	101	1231	1	0
Subbasin 6	0.1	wetland 1		74	0.000	1	0.1
0.02	1	0	1	101	1231	1	0.5
Subbasin 5	7.86	Pond 4		74	0.000	1	0.06
0.02	1	0	1	101	1231	1	0.5

MassBalances.prn

P8 Urban Catchment Model, Version 3.5

Run Date 06/08/15
 Case Round Lake - Spine Road Only.p8c FirstDate 01/01/01
 Precip(in) 296.7
 Title Round Lake LastDate 12/31/10
 Rain(in) 273.57
 PrecFile precip1970-2010.pcp Events 598
 Snow(in) 23.12
 PartFile nurp50.p8p TotalHrs 87576
 TotalYrs 9.99

Mass Balances by Device and Variable

Device: OVERALL Type: NONE Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	524.83	0.07	111293.3	11140.0	78.02
06 normal outlet	524.83	0.07	17335.7	1735.2	12.15
08 sedimen + decay	0.00	0.00	93957.1	9404.7	
09 total inflow	524.83	0.07	111293.3	11140.0	78.02
10 surface outflow	524.83	0.07	17335.7	1735.2	12.15
12 total outflow	524.83	0.07	17335.7	1735.2	12.15
13 total trapped	0.00	0.00	93957.1	9404.7	
14 storage increase	0.00	0.00	0.4	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	84.4	84.4	

Device: OVERALL Type: NONE Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	524.83	0.07	398.3	39.9	0.28
06 normal outlet	524.83	0.07	197.6	19.8	0.14
08 sedimen + decay	0.00	0.00	200.4	20.1	
09 total inflow	524.83	0.07	398.3	39.9	0.28
10 surface outflow	524.83	0.07	197.6	19.8	0.14
12 total outflow	524.83	0.07	197.6	19.8	0.14
13 total trapped	0.00	0.00	200.4	20.1	
14 storage increase	0.00	0.00	0.3	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	50.3	50.3	

Device: Pond 2 Type: POND Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	215.76	0.03	43357.2	4339.9	73.93
06 normal outlet	192.85	0.03	5629.3	563.5	10.74
07 spillway outlet	22.92	0.00	4146.3	415.0	66.56
08 sedimen + decay	0.00	0.00	33581.5	3361.4	
09 total inflow	215.76	0.03	43357.2	4339.9	73.93
10 surface outflow	215.76	0.03	9775.5	978.5	16.67
12 total outflow	215.76	0.03	9775.5	978.5	16.67
13 total trapped	0.00	0.00	33581.5	3361.4	
14 storage increase	0.00	0.00	0.2	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	77.5	77.5	

Device: Pond 2 Type: POND Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	215.76	0.03	158.2	15.8	0.27
06 normal outlet	192.85	0.03	73.1	7.3	0.14
07 spillway outlet	22.92	0.00	19.6	2.0	0.32

MassBalances.prn

Reduction (%) 0.00 0.00 52.3 52.3

Device: Wetland 1 Type: POND Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	0.38	0.00	81.5	8.2	78.61
06 normal outlet	0.38	0.00	1.9	0.2	1.87
08 sedimen + decay	0.00	0.00	79.5	8.0	
09 total inflow	0.38	0.00	81.5	8.2	78.61
10 surface outflow	0.38	0.00	1.9	0.2	1.87
12 total outflow	0.38	0.00	1.9	0.2	1.87
13 total trapped	0.00	0.00	79.5	8.0	
14 storage increase	0.00	0.00	0.0	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	97.6	97.6	

Device: Wetland 1 Type: POND Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	0.38	0.00	0.3	0.0	0.28
06 normal outlet	0.38	0.00	0.1	0.0	0.11
08 sedimen + decay	0.00	0.00	0.2	0.0	
09 total inflow	0.38	0.00	0.3	0.0	0.28
10 surface outflow	0.38	0.00	0.1	0.0	0.11
12 total outflow	0.38	0.00	0.1	0.0	0.11
13 total trapped	0.00	0.00	0.2	0.0	
14 storage increase	0.00	0.00	0.0	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	62.1	62.1	

Device: Outfall #1 Type: PIPE Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
02 upstream device	23.40	0.00	459.2	46.0	7.22
06 normal outlet	23.40	0.00	459.2	46.0	7.22
09 total inflow	23.40	0.00	459.2	46.0	7.22
10 surface outflow	23.40	0.00	459.2	46.0	7.22
12 total outflow	23.40	0.00	459.2	46.0	7.22
Reduction (%)	0.00	0.00	0.0	0.0	

Device: Outfall #1 Type: PIPE Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
02 upstream device	23.40	0.00	8.0	0.8	0.13
06 normal outlet	23.40	0.00	8.0	0.8	0.13
09 total inflow	23.40	0.00	8.0	0.8	0.13
10 surface outflow	23.40	0.00	8.0	0.8	0.13
12 total outflow	23.40	0.00	8.0	0.8	0.13
Reduction (%)	0.00	0.00	0.0	0.0	

Device: Outfall #2 Type: PIPE Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	34.51	0.00	4497.2	450.1	47.95
02 upstream device	466.92	0.06	12379.3	1239.1	9.75
06 normal outlet	501.43	0.07	16876.5	1689.3	12.38
09 total inflow	501.43	0.07	16876.5	1689.3	12.38
10 surface outflow	501.43	0.07	16876.5	1689.3	12.38
12 total outflow	501.43	0.07	16876.5	1689.3	12.38
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	0.0	0.0	

Device: Outfall #2 Type: PIPE Variable: TP

MassBalances.prn

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	34.51	0.00	19.7	2.0	0.21
02 upstream device	466.92	0.06	169.9	17.0	0.13
06 normal outlet	501.43	0.07	189.6	19.0	0.14
09 total inflow	501.43	0.07	189.6	19.0	0.14
10 surface outflow	501.43	0.07	189.6	19.0	0.14
12 total outflow	501.43	0.07	189.6	19.0	0.14
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	0.0	0.0	

Round Lake P8 Results
Scenario 2: Fully Developed Conditions

P8 Urban Catchment Model, Version 3.5		Info.prn		Run Date
06/08/15				
Case	Round Lake.p8c	FirstDate	01/01/01	Precip(in)
296.7				
Title	Round Lake	LastDate	12/31/10	Rain(in)
273.57				
PrecFile	precip1970-2010.pcp	Events	598	Snow(in)
23.12				
PartFile	nurp50.p8p	TotalHrs	87576	TotalYrs
9.99				

File Directory	T:\1382 KimleyHorn\01 TCAAP\TASK 03 Stormwater Prelim
Design\Models\P8\	
Case Title	Round Lake
Case File	Round Lake.p8c
Particle File	nurp50.p8p
Temperature File	temp1970-2011.tmp
Storm File	precip1970-2010.pcp
Precip Scale Factor	1
Watersheds	5
Devices	7
Particles	5
WQ Components	7
Start Date	06/01/00
Keep Date	01/01/01
Stop Date	12/31/10
Storm Count	598
Total Hours	87576
Wet Hours	9265
Precip (in)	297
Rain (in)	274
Snowfall (in)	23
Snowmelt (in)	22
EvapoTran(in)	303
Overall TSS Removal(%)	1
Water Balance Error(%)	0
TSS Mass Balance Error	0

Inputs.prn

P8 Urban Catchment Model, Version 3.5				Run Date
06/08/15				
Case	Round Lake.p8c	FirstDate	01/01/01	Precip(in)
296.7				
Title	Round Lake	LastDate	12/31/10	Rain(in)
273.57				
PrecFile	precip1970-2010.pcp	Events	598	Snow(in)
23.12				
PartFile	nurp50.p8p	TotalHrs	87576	TotalYrs
9.99				

Case Title	Round Lake
Case Data File	Round Lake.p8c
Path	T:\1382 KimleyHorn\01 TCAAP\TASK 03 Stormwater
Prelim Design\Models\P8\ Case Notes:	Round Lake
Ponds 1/2, 5/6, and 3	
Storm Data File	precip1970-2010.pcp
Particle File	nurp50.p8p
Air Temp File File	temp1970-2011.tmp

Time Steps Per Hour	8
Minimum Inter-Event Time (hrs)	10
Maximum Continuity Error %	2
Rainfall Breakpoint (inches)	0.8
Precipitation Scale Factor	1
Air Temp Offset (deg-F)	0
Loops Thru Storm File	1
Simulation Dates	
Start	6/1/2000
Keep	1/1/2001
Stop	12/31/2010

Max Snowfall Temperature (deg-f)	32.0
SnowMelt Temperature (deg-f)	32.0
Snowmelt Coef (in/degF-Day)	0.06
Soil Freeze Temp (deg-F)	32.0
Snowmelt Abstraction Factor	1.00
Evapo-Trans. Calibration Factor	1.00
Growing Season Start Month	5
Growing Season End Month	10

5-Day Antecedent Rainfall + Runoff (inches)		
CN Antecedent Moisture Condition	AMC-II	AMC-III
Growing Season	1.40	2.10
NonGrowing Season	0.50	1.10

Watershed Data						
Watershed Name	Watershed	Watershed	Watershed	5Subbasin	6Subbasin	5
Runoff to Device	Pond 1/2	Pond 3	Pond 5/6	wetland 1	Pond 4	
Infiltration to Device						
Watershed Area	68.5	21.55	43.27	0.997	7.853	
SCS Curve Number (Pervious)	74	74	74	74	74	
Scale Factor for Pervious Runoff	1	1	1	1	1	
Indirectly Connected Imperv Frac	0	0	0	0	0	
UnSwept Impervious Fraction	0.58	0.85	0.47	0.2447	0.7037	
UnSwept Depression Storage (inch	0.02	0.02	0.02	0.02	0.02	
UnSwept Imperv. Runoff Coefficie	1	1	1	1	1	
UnSwept Scale Factor for Particl	1	1	1	1	1	
Swept Impervious Fraction	0	0	0	0	0	
Swept Depression Storage (inches	0.02	0.02	0.02	0	0	
Swept Imperv. Runoff Coefficient	1	1	1	1	1	

	Inputs.prn				
Swept Scale Factor for Particle	1	1	1	1	1
Sweeping Frequency	0	0	0	0.5	0.5
Sweeping Efficiency	1	1	1	1	1
Sweeping Start Date (MMDD)	101	101	101	101	101
Sweeping Stop Date (MMDD)	1231	1231	1231	1231	1231

Device Data	Pond 1/2	Pond 3	Pond 5/6	Pond 4	Wetland 1
Device Name					
Outfall #1					
Outfall #2					
Device Type	POND	POND	POND	POND	POND
PIPE	PIPE				
Infiltration Outlet					
Normal Outlet	Pond 3	Outfall #2	Pond 3	Outfall #1	Outfall #1
Spillway Outlet	Pond 3	Outfall #2	Pond 3	Outfall #1	Outfall #1
Particle Removal Scale Factor	1	1	1	1	1
Bottom Elevation (ft)	0	0	0	0	0
Bottom Area (acres)	0.865	1.401	2.151	0	0.129
Permanent Pool Area (acres)	1.295	2.27	2.729	0.266	0.241
Permanent Pool Volume (ac-ft)	4.3	7.4	6.8	0.5	0
Perm Pool Infiltr Rate (in/hr)	0	0	0	0	0
Flood Pool Area (acres)	1.539	3.678	3.737	0.593	0.38
Flood Pool Volume (ac-ft)	2.4	16.3	6.1	0.9	0.3
Flood Pool Infiltr Rate (in/hr)	0	0	0	0	0
Infiltr Basin Void Fraction (%)					
Detention Pond Outlet Parameters					
Outlet Type	ORIFICE	ORIFICE	ORIFICE	ORIFICE	ORIFICE
Outlet Orifice Diameter (in)	6	12	12	12	6
Orifice Discharge Coef	1	1	1	0.6	0.6
Outlet Weir Length (ft)					
Weir Discharge Coef					
Perforated Riser Height (ft)					
Number of Holes in Riser					
Holes Diameter					
Flood Pool Drain Time (hrs)					
Swale Parameters					
Length of Flow Path (ft)					
Slope of Flow Path %					
Bottom width (ft)					
Side Slope (ft-v/ft-h)					
Maximum Depth of Flow (ft)					
Mannings n Constant					
Hydraulic Model					
Pipe, Splitter, Aquifer Parameter					
Hydraulic Res. Time (hrs)	0	0			

Particle Data	nurp50.p8p	P10%	P30%	P50%	P80%
Particle File					
Particle Class	P0%				
Filtration Efficiency	90	100	100	100	100
Settling Velocity (ft/	0	0.03	0.3	1.5	15
First Order Decay Rate	0	0	0	0	0
2nd Order Decay (1/day	0	0	0	0	0
Impervious Runoff Conc	1	0	0	0	0
Pervious Runoff Conc (1	100	100	100	200
Pervious Conc Exponent	0	1	1	1	1
Accum. Rate (lbs-ac-da	0	1.75	1.75	1.75	3.5
Particle Removal Rate	0	0.25	0.25	0.25	0.25
Washoff Coefficient	0	20	20	20	20
Washoff Exponent	0	2	2	2	2
Sweeper Efficiency	0	0	0	5	15

Water Quality Component Data

Component Name	TSS	Inputs.prn		CU	PB	ZN	
		TP	TKN				
HC							
Water Quality Criteria (ppm)							
0.1	Level 1	5	0.025	2	2	0.02	5
0.5	Level 2	10	0.05	1	0.0048	0.014	0.0362
1	Level 3	20	0.1	0.5	0.02	0.15	0.38
Content Scale Factor							
1	1	1	1	1	1	1	
Particle Composition (mg/kg)							
P0%	0	99000	600000	13600	2000	640000	
250000							
P10%	1000000	3850	15000	340	180	1600	
22500							
P30%	1000000	3850	15000	340	180	1600	
22500							
P50%	1000000	3850	15000	340	180	1600	
22500							
P80%	1000000	0	0	340	180	0	
22500							

Inputs.prn

Inputs.prn

		Network.prn		
P8 Urban Catchment Model, Version 3.5				Run Date
06/08/15		FirstDate	01/01/01	Precip(in)
Case 296.7	Round Lake.p8c	LastDate	12/31/10	Rain(in)
Title 273.57	Round Lake	Events	598	Snow(in)
PrecFile 23.12	precip1970-2010.pcp	TotalHrs	87576	TotalYrs
PartFile 9.99	nurp50.p8p			

Devices Listed in Downstream Order

Device:	Pond 1/2	Type:	POND
	Discharges normal outlet to		Pond 3
	Discharges spillway to		Pond 3
	Runoff from watershed		Watershed 1/2
Device:	Pond 5/6	Type:	POND
	Discharges normal outlet to		Pond 3
	Discharges spillway to		Pond 3
	Runoff from watershed		Watershed 5/6
Device:	Pond 3	Type:	POND
	Discharges normal outlet to		Outfall #2
	Discharges spillway to		Outfall #2
	Runoff from watershed		Watershed 3
Device:	Pond 4	Type:	POND
	Discharges normal outlet to		Outfall #1
	Discharges spillway to		Outfall #1
	Runoff from watershed		Subbasin 5
Device:	Wetland 1	Type:	POND
	Discharges normal outlet to		Outfall #1
	Discharges spillway to		Outfall #1
	Runoff from watershed		Subbasin 6
Device:	Outfall #1	Type:	PIPE
Device:	Outfall #2	Type:	PIPE

watersheds.prn

P8-V3.X

Round Lake.p8c

Connected UnSwept Areas		Directly Connected	Swept Areas		Street Sweeping Parameters		Directly Connected		
Depress Sweep	Storage Runoff	Total Imperv Area	Depress	Percol	Imperv	Start	Indirect	Pervious	
Watershed	Freq	Load	Outflow Imperv Storage	Runoff	Curve	Imperv	Load	Imperv	
Label	Coef	acres	Device	Device	Number	Fraction	Factor	Fraction	
inches	1/week	Factor	Fraction	Coef	Factor	MMDD	MMDD	Effic	
Watershed	1/2	68.5	Pond 1/2		74	0.000		1	0.58
0.02	1	1	0	0.02	1	1	101	1231	1
Watershed	3	21.55	Pond 3		74	0.000		1	0.85
0.02	1	1	0	0.02	1	1	101	1231	1
Watershed	5/6	43.27	Pond 5/6		74	0.000		1	0.47
0.02	1	1	0	0.02	1	1	101	1231	1
Subbasin	6	0.997	wetland 1		74	0.000		1	0.2447
0.02	1	1	0	0	1	1	101	1231	1
Subbasin	5	7.853	Pond 4		74	0.000		1	0.7037
0.02	1	1	0	0	1	1	101	1231	1

MassBalances.prn

P8 Urban Catchment Model, Version 3.5

Run Date 06/08/15

Case	Round Lake.p8c	FirstDate	01/01/01
Precip(in)	296.7	LastDate	12/31/10
Title	Round Lake	Events	598
Rain(in)	273.57	PartFile	nurp50.p8p
PrecFile	precip1970-2010.pcp	TotalHrs	87576
Snow(in)	23.12		
TotalYrs	9.99		

Mass Balances by Device and Variable

Device: OVERALL Type: NONE Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	2089.33	0.29	539726.0	54024.4	95.04
06 normal outlet	2089.33	0.29	42179.9	4222.0	7.43
08 sedimen + decay	0.00	0.00	497542.0	49801.9	
09 total inflow	2089.33	0.29	539726.0	54024.4	95.04
10 surface outflow	2089.33	0.29	42179.9	4222.0	7.43
12 total outflow	2089.33	0.29	42179.9	4222.0	7.43
13 total trapped	0.00	0.00	497542.0	49801.9	
14 storage increase	0.00	0.00	3.7	0.4	
15 mass balance chec	0.00	0.00	0.3	0.0	
Reduction (%)	0.00	0.00	92.2	92.2	

Device: OVERALL Type: NONE Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	2089.33	0.29	1809.0	181.1	0.32
06 normal outlet	2089.33	0.29	723.4	72.4	0.13
08 sedimen + decay	0.00	0.00	1085.5	108.7	
09 total inflow	2089.33	0.29	1809.0	181.1	0.32
10 surface outflow	2089.33	0.29	723.4	72.4	0.13
12 total outflow	2089.33	0.29	723.4	72.4	0.13
13 total trapped	0.00	0.00	1085.5	108.7	
14 storage increase	0.00	0.00	0.1	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	60.0	60.0	

Device: Pond 1/2 Type: POND Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	988.60	0.14	255096.9	25534.2	94.94
06 normal outlet	815.77	0.11	28710.8	2873.8	12.95
07 spillway outlet	172.83	0.02	11334.5	1134.5	24.13
08 sedimen + decay	0.00	0.00	215049.7	21525.6	
09 total inflow	988.60	0.14	255096.9	25534.2	94.94
10 surface outflow	988.60	0.14	40045.3	4008.4	14.90
12 total outflow	988.60	0.14	40045.3	4008.4	14.90
13 total trapped	0.00	0.00	215049.7	21525.6	
14 storage increase	0.00	0.00	1.9	0.2	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	84.3	84.3	

Device: Pond 1/2 Type: POND Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	988.60	0.14	855.3	85.6	0.32
06 normal outlet	815.77	0.11	328.6	32.9	0.15
07 spillway outlet	172.83	0.02	87.6	8.8	0.19

Rice Creek P8 Results
Interim Scenario 1

				Info.prn	
P8 Urban Catchment Model, Version 3.5					Run Date
07/30/15					
Case	Rice Creek Spine Road.p8c	FirstDate	01/01/01		Precip(in)
296.7					
Title	Rice Creek	LastDate	12/31/10		Rain(in)
273.57					
PrecFile	precip1970-2010.pcp	Events	598		Snow(in)
23.12					
PartFile	nurp50.p8p	TotalHrs	87576		TotalYrs
9.99					

File Directory	T:\1382 KimleyHorn\01 TCAAP\TASK 03 Stormwater Prelim
Design\CSMP_Final\Appendix A P8\P8\	
Case Title	Rice Creek
Case File	Rice Creek Spine Road.p8c
Particle File	nurp50.p8p
Temperature File	temp1970-2011.tmp
Storm File	precip1970-2010.pcp
Precip Scale Factor	1

Watersheds	16
Devices	19
Particles	5
WQ Components	7

Start Date	06/01/00
Keep Date	01/01/01
Stop Date	12/31/10
Storm Count	598
Total Hours	87576
Wet Hours	9265
Precip (in)	297
Rain (in)	274
Snowfall (in)	23
Snowmelt (in)	22
EvapoTran(in)	303

Overall TSS Removal(%)	1
Water Balance Error(%)	0
TSS Mass Balance Error	0

Inputs.prn

P8 Urban Catchment Model, Version 3.5				Run Date
07/30/15				
Case	Rice Creek Spine Road.p8c	FirstDate	01/01/01	Precip(in)
296.7				
Title	Rice Creek	LastDate	12/31/10	Rain(in)
273.57				
PrecFile	precip1970-2010.pcp	Events	598	Snow(in)
23.12				
PartFile	nurp50.p8p	TotalHrs	87576	TotalYrs
9.99				

Case Title	Rice Creek
Case Data File	Rice Creek Spine Road.p8c
Path	T:\1382 KimleyHorn\01 TCAAP\TASK 03 Stormwater
Prelim Design\CSMP_Final\Appendix A P8\P8\	
Case Notes:	Spine Road Only
Storm Data File	precip1970-2010.pcp
Particle File	nurp50.p8p
Air Temp File File	temp1970-2011.tmp

Time Steps Per Hour	4
Minimum Inter-Event Time (hrs)	10
Maximum Continuity Error %	2
Rainfall Breakpoint (inches)	0.8
Precipitation Scale Factor	1
Air Temp Offset (deg-F)	0
Loops Thru Storm File	1
Simulation Dates	
Start	6/1/2000
Keep	1/1/2001
Stop	12/31/2010

Max Snowfall Temperature (deg-f)	32.0
SnowMelt Temperature (deg-f)	32.0
Snowmelt Coef (in/degF-Day)	0.06
Soil Freeze Temp (deg-F)	32.0
Snowmelt Abstraction Factor	1.00
Evapo-Trans. Calibration Factor	1.00
Growing Season Start Month	5
Growing Season End Month	10

5-Day Antecedent Rainfall + Runoff (inches)		
CN Antecedent Moisture Condition	AMC-II	AMC-III
Growing Season	1.40	2.10
NonGrowing Season	0.50	1.10

Watershed Data											
Watershed Name			SB 8	SB 10	SB 9	SB 12	SB 11				
SB 14	SB 13	SB 18	SB 15, 16,	SB 17	Inflow fro	SB 19	SB 28	SB			
29	15	SB 22 and 27									
Runoff to Device			Pond 7	Pond 8	Pond 9	Pond 10	Pond 11				
Pond 12	Wetland 4	SB 18	Swal	Pond 13	Wetland 5	Wetland 3	Outfall #4	CHR-1			
CRH-2	CRH-3	Thumb	Infiltration								
Infiltration to Device											
Watershed Area			29.69	6.39	25.78	1.39	3.29				
10.23	2.98	52.79	105.345	7.608	4.311	21.19	6.955				
10.214	1.601	48.539									
SCS Curve Number (Pervious)			74	74	74	74	74	74	74	74	74
74	74	74	74	74	74	74	74	74	74	74	74
74	74	80									
Scale Factor for Pervious Runoff			1	1	1	1	1	1	1	1	1
1	1	1	1	1	1	1	1	1	1	1	1

Inputs.prn

Indirectly Connected	1	1	Imperv Frac	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0	0	0
Unswept Impervious Fraction	0.16	0.2617	0	0.0551	0.05	0.0012	0.2098	0.3678	0.1517	0.4841	0.1861	0	0.4676
0.3773	0.3198	0.1138	0	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02
Unswept Depression Storage (inch	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02
0.02	0.02	0.02	0.02	1	1	1	1	1	1	1	1	1	1
Unswept Imperv. Runoff Coefficie	1	1	1	1	1	1	1	1	1	1	1	1	1
1	1	1	1	1	1	1	1	1	1	1	1	1	1
Unswept Scale Factor for Particl	1	1	1	1	1	1	1	1	1	1	1	1	1
1	1	1	1	1	1	1	1	1	1	1	1	1	1
Swept Impervious Fraction	0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0	0.02
Swept Depression Storage (inches	0.02	0.02	0	0.02	0.02	0	0.02	0.02	0.02	0.02	0.02	0	0.02
0.02	0	0.02	0	1	1	1	1	1	1	1	1	1	0
Swept Imperv. Runoff Coefficient	1	1	1	1	1	1	1	1	1	1	1	1	0
1	1	1	1	1	1	1	1	1	1	1	1	1	1
Swept Scale Factor for Particle	1	1	1	1	1	1	1	1	1	1	1	1	1
1	1	1	1	1	1	1	1	1	1	1	1	1	1
Sweeping Frequency	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
0.5	0.5	0.5	0.5	1	1	1	1	1	1	1	1	1	1
Sweeping Efficiency	1	1	1	1	1	1	1	1	1	1	1	1	1
1	1	1	1	101	101	101	101	101	101	101	101	101	101
Sweeping Start Date (MMDD)	101	101	101	101	101	101	101	101	101	101	101	101	101
101	101	101	101	1231	1231	1231	1231	1231	1231	1231	1231	1231	1231
Sweeping Stop Date (MMDD)	1231	1231	1231	1231	1231	1231	1231	1231	1231	1231	1231	1231	1231
1231	1231	1231	1231										

Device Data

Device Name	Pond 12	Pond 13	Outfall #5	To Rice Cr	Pond 7	Pond 8	Pond 9	Pond 10	Pond 11
CHR-1	CRH-2	CRH-3	CRH-3	CRH-3	SB 18	Swalthumb	Infiltration	Wetland 3	Wetland 2
Device Type	POND	POND	PIPE	POND	POND	PIPE	POND	POND	POND
POND	POND	PIPE	POND	POND	INF_BASIN	INF_BASIN	POND	POND	POND
Infiltration Outlet									
Normal Outlet	Pond 13	Outfall #5	To Rice Cr	Pond 9	Pond 10	Pond 11	Pond 12	Pond 10	Pond 10
Rice Cr	CRH-3	CRH-3	To Rice Cr	To Rice Cr	To Rice Cr	To Rice Cr	To Rice Cr	Wetland 3	To
Spillway Outlet									
Pond 13	Outfall #5	CRH-3	To Rice Cr	Pond 9	Pond 10	Pond 11	Pond 12	Pond 10	Pond 10
Rice Cr	CRH-3	CRH-3	To Rice Cr	To Rice Cr	To Rice Cr	To Rice Cr	To Rice Cr	Wetland 3	To
Particle Removal Scale Factor	1	1	1	1	1	1	1	1	1
1	1	1	1	1	1	1	1	1	1
Bottom Elevation (ft)	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0
Bottom Area (acres)	1.2	0.02	0	0.143	0	0.057	0.024	0.13	0.97
1.2	0.02	0	0	0	0	0	2.157	2.157	

Inputs.prn

0.15	0.2	0.15	0	0	0	0.41	0.288	1.32	
Permanent Pool Area (acres)		0.548	0.26			2.355	2.18		
1.64	1.988	0	0						
0.225	0.35	0.225	1	0.6		0.5	0.8	4.5	
Permanent Pool Volume (ac-ft)		0	0			0	0		
5.7	6.2	0	0						
0.187	0.412	0.187	0	0		0	0	0	0
Perm Pool Infiltration Rate (in/hr)		0	0			0	0	0	0
0	0	0	0						
Flood Pool Area (acres)		0.723	0.669			0.75	0.61	2.11	
2.38	4.03	1.891	1.891			2.653	2.591	0.5	
0.6	0.5	0.31	3.74						
Flood Pool Volume (ac-ft)		0.5	0.9			0.4	1.1	6.6	
5.3	8.1	2.5	5.8			2.6	1.2		
0.663	1.188	0.663	6.15	3.74		0	0	0	0
Flood Pool Infiltration Rate (in/hr)		0	0			0	0	0	0
0	0	0	0			0	0	0	0
Infiltration Basin Void Fraction (%)		0.45	0.45						

		100	100						
Detention Pond Outlet Parameters									
Outlet Type		WEIR	ORIFICE			WEIR	WEIR	ORIFICE	
ORIFICE WEIR		ORIFICE	ORIFICE			ORIFICE	ORIFICE		
ORIFICE ORIFICE		ORIFICE							
Outlet Orifice Diameter (in)			24					12	24
12		12	6			12	12		
24	24								
Orifice Discharge Coef			1					0.6	0.6
0.6		0.6	0.6			0.6	0.6		0.6
0.6	0.6								
Outlet Weir Length (ft)		75				80	50		
55									
Weir Discharge Coef		3.3				3.3	3.3		
3.3									
Perforated Riser Height (ft)									
Number of Holes in Riser									
Holes Diameter									
Flood Pool Drain Time (hrs)									
Swale Parameters									
Length of Flow Path (ft)									
Slope of Flow Path %									
Bottom Width (ft)									
Side Slope (ft-v/ft-h)									
Maximum Depth of Flow (ft)									
Mannings n Constant									
Hydraulic Model									
Pipe, Splitter, Aquifer Parameter									
Hydraulic Res. Time (hrs)									0
0									

Particle Data						
Particle File	nurp50.p8p					
Particle Class	P0%	P10%	P30%	P50%	P80%	
Filtration Efficiency	90	100	100	100	100	
Settling Velocity (ft/	0	0.03	0.3	1.5	15	
First Order Decay Rate	0	0	0	0	0	
2nd Order Decay (1/day	0	0	0	0	0	
Impervious Runoff Conc	1	0	0	0	0	
Pervious Runoff Conc (1	100	100	100	200	
Pervious Conc Exponent	0	1	1	1	1	

		Inputs.prn					
Accum. Rate (lbs-ac-da	0	1.75	1.75	1.75	3.5		
Particle Removal Rate	0	0.25	0.25	0.25	0.25		
Washoff Coefficient	0	20	20	20	20		
Washoff Exponent	0	2	2	2	2		
Sweeper Efficiency	0	0	0	5	15		
Water Quality Component Data							
Component Name	TSS	TP	TKN	CU	PB	ZN	
HC							
Water Quality Criteria (ppm)							
0.1	Level 1	5	0.025	2	2	0.02	5
0.5	Level 2	10	0.05	1	0.0048	0.014	0.0362
1	Level 3	20	0.1	0.5	0.02	0.15	0.38
Content Scale Factor							
1	1	1	1	1	1	1	
Particle Composition (mg/kg)							
P0%	0	99000	600000	13600	2000	640000	
250000							
P10%	1000000	3850	15000	340	180	1600	
22500							
P30%	1000000	3850	15000	340	180	1600	
22500							
P50%	1000000	3850	15000	340	180	1600	
22500							
P80%	1000000	0	0	340	180	0	
22500							

Inputs.prn

Inputs.prn

P8 Urban Catchment Model, Version 3.5				Run Date
07/30/15				
Case	Rice Creek Spine Road.p8c	FirstDate	01/01/01	Precip(in)
296.7				
Title	Rice Creek	LastDate	12/31/10	Rain(in)
273.57				
PrecFile	precip1970-2010.pcp	Events	598	Snow(in)
23.12				
PartFile	nurp50.p8p	TotalHrs	87576	TotalYrs
9.99				

Devices Listed in Downstream Order

Device:	Pond 7	Type:	POND
	Discharges normal outlet to		Pond 9
	Discharges spillway to		Pond 9
	Runoff from watershed		SB 8
Device:	Pond 8	Type:	POND
	Discharges normal outlet to		Pond 10
	Discharges spillway to		Pond 10
	Runoff from watershed		SB 10
Device:	Outfall #4	Type:	PIPE
	Discharges normal outlet to		To Rice Creek
	Runoff from watershed		SB 19
Device:	Wetland 4	Type:	POND
	Discharges normal outlet to		Pond 12
	Discharges spillway to		Pond 12
	Runoff from watershed		SB 13
Device:	Wetland 5	Type:	POND
	Runoff from watershed		SB 17
Device:	Wetland 2	Type:	POND
	Discharges normal outlet to		Wetland 3
	Discharges spillway to		Wetland 3
Device:	Wetland 3	Type:	POND
	Discharges normal outlet to		Pond 9
	Discharges spillway to		Pond 9
	Runoff from watershed		Inflow from Ponds 4/5
Device:	Pond 9	Type:	POND
	Discharges normal outlet to		Pond 11
	Discharges spillway to		Pond 11
	Runoff from watershed		SB 9
Device:	Pond 11	Type:	POND
	Discharges normal outlet to		Pond 10
	Discharges spillway to		Pond 10
	Runoff from watershed		SB 11
Device:	Pond 10	Type:	POND
	Discharges normal outlet to		Pond 12
	Discharges spillway to		Pond 12
	Runoff from watershed		SB 12
Device:	Pond 12	Type:	POND
	Discharges normal outlet to		Pond 13
	Discharges spillway to		Pond 13

	Runoff from watershed	Network.prn	SB 14
Device:	CHR-1	Type:	POND
	Discharges normal outlet to		To Rice Creek
	Discharges spillway to		To Rice Creek
	Runoff from watershed		SB 28
Device:	CRH-2	Type:	POND
	Discharges normal outlet to		CRH-3
	Discharges spillway to		CRH-3
	Runoff from watershed		SB 29
Device:	CRH-3	Type:	POND
	Discharges normal outlet to		To Rice Creek
	Discharges spillway to		To Rice Creek
	Runoff from watershed		1S
Device:	SB 18 Swale	Type:	INF_BASIN
	Discharges spillway to		Pond 13
	Runoff from watershed		SB 18
Device:	Pond 13	Type:	POND
	Discharges normal outlet to		Outfall #5
	Discharges spillway to		Outfall #5
	Runoff from watershed		SB 15, 16, 26
Device:	Outfall #5	Type:	PIPE
	Discharges normal outlet to		To Rice Creek
Device:	Thumb Infiltration	Type:	INF_BASIN
	Discharges spillway to		To Rice Creek
	Runoff from watershed		SB 22 and 27
Device:	To Rice Creek	Type:	PIPE

Depress Sweep Watershed Storage Freq Label inches 1/week	UnSwept Runoff Coef	Areas Directly Connected Total Imperv Area Load acres Factor	Outflow Imperv Storage Device Fraction inches	Percol Runoff Coef	Swept Areas--Street Pervious Imperv Start Load Date	Street Sweeping Parameters Indirect Pervious Load Sweep	Parameters Pervious Fraction Effc	
SB 8	1	29.69	Pond 7	1	74	0.000	1	0.0551
0.02	1	1	0.02	1	101	1231	1	0.5
SB 10	1	6.39	Pond 8	1	74	0.000	1	0.05
0.02	1	1	0.02	1	101	1231	1	0.5
SB 9	1	25.78	Pond 9	1	74	0.000	1	0.0012
0.02	1	1	0.02	1	101	1231	1	0.5
SB 12	1	1.39	Pond 10	1	74	0.000	1	0.2098
0.02	1	1	0.02	1	101	1231	1	0.5
SB 11	1	3.29	Pond 11	1	74	0.000	1	0.3678
0.02	1	1	0.02	1	101	1231	1	0.5
SB 14	1	10.23	Pond 12	1	74	0.000	1	0.16
0.02	1	1	0.02	1	101	1231	1	0.5
SB 13	1	2.98	wetland 4	1	74	0.000	1	0.2617
0.02	1	1	0.02	1	101	1231	1	0.5
SB 18	1	52.79	SB 18 Swale	1	74	0.000	1	0
0.02	1	1	0	1	101	1231	1	0.5
SB 15, 16, 26	1	105.345	Pond 13	1	74	0.000	1	0.1517
0.02	1	1	0.02	1	101	1231	1	0.5
SB 17	1	7.608	wetland 5	1	74	0.000	1	0.4841
0.02	1	1	0.02	1	101	1231	1	0.5
Inflow from P	1	4.311	wetland 3	1	74	0.000	1	0.1861
0.02	1	1	0	1	101	1231	1	0.5
SB 19	1	21.19	outfall #4	1	74	0.000	1	0
0.02	1	1	0.02	1	101	1231	1	0.5
SB 28	1	6.955	CHR-1	1	74	0.000	1	0.4676
0.02	1	1	0	1	101	1231	1	0.5
SB 29	1	10.214	CRH-2	0	74	0.000	1	0.3773
0.02	1	1	0	1	101	1231	1	0.5
1S	1	1.601	CRH-3	1	74	0.000	1	0.3198
0.02	1	1	0	1	101	1231	1	0.5
SB 22 and 27	1	48.539	Thumb Infiltration	1	80	0.000	1	0.1138
0.02	1	1	0.02	1	101	1231	1	0.5

MassBalances.prn

P8 Urban Catchment Model, Version 3.5

Run Date	07/30/15			
Case	Rice Creek Spine Road.p8c	FirstDate	01/01/01	
Precip(in)	296.7			
Title	Rice Creek	LastDate	12/31/10	
Rain(in)	273.57			
PrecFile	precip1970-2010.pcp	Events	598	
Snow(in)	23.12			
PartFile	nurp50.p8p	TotalHrs	87576	
TotalYrs	9.99			

Mass Balances by Device and Variable

Device: OVERALL Type: NONE Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	1462.01	0.20	319623.0	31993.0	80.43
03 infiltrate	321.31	0.04	8515.7	852.4	9.75
04 exfiltrate	321.31	0.04	0.0	0.0	0.00
05 filtered	0.00	0.00	8515.7	852.4	
06 normal outlet	1140.94	0.16	39272.9	3931.1	12.66
08 sedimen + decay	0.00	0.00	271832.0	27209.3	
09 total inflow	1462.01	0.20	319623.0	31993.0	80.43
10 surface outflow	1140.94	0.16	39272.9	3931.1	12.66
11 groundw outflow	321.31	0.04	0.0	0.0	0.00
12 total outflow	1462.25	0.20	39272.9	3931.1	9.88
13 total trapped	0.00	0.00	280347.7	28061.7	
14 storage increase	0.00	0.00	1.0	0.1	
15 mass balance chec	-0.24	0.00	1.3	0.1	
Reduction (%)	0.00	0.00	87.7	87.7	

Device: OVERALL Type: NONE Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	1462.01	0.20	1131.7	113.3	0.28
03 infiltrate	321.31	0.04	119.0	11.9	0.14
04 exfiltrate	321.31	0.04	8.6	0.9	0.01
05 filtered	0.00	0.00	110.4	11.0	
06 normal outlet	1140.94	0.16	444.5	44.5	0.14
08 sedimen + decay	0.00	0.00	566.9	56.7	
09 total inflow	1462.01	0.20	1131.7	113.3	0.28
10 surface outflow	1140.94	0.16	444.5	44.5	0.14
11 groundw outflow	321.31	0.04	8.6	0.9	0.01
12 total outflow	1462.25	0.20	453.2	45.4	0.11
13 total trapped	0.00	0.00	677.2	67.8	
14 storage increase	0.00	0.00	1.3	0.1	
15 mass balance chec	-0.24	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	59.8	59.8	

Device: Pond 7 Type: POND Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	83.73	0.01	16079.2	1609.5	70.65
06 normal outlet	83.76	0.01	2768.9	277.2	12.16
08 sedimen + decay	0.00	0.00	13310.3	1332.3	
09 total inflow	83.73	0.01	16079.2	1609.5	70.65
10 surface outflow	83.76	0.01	2768.9	277.2	12.16
12 total outflow	83.76	0.01	2768.9	277.2	12.16
13 total trapped	0.00	0.00	13310.3	1332.3	
14 storage increase	0.00	0.00	0.0	0.0	
15 mass balance chec	-0.03	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	82.8	82.8	

MassBalances.prn

15 mass balance chec 0.00 0.00 0.0 0.0
 Reduction (%) 0.00 0.00 90.5 90.5

Device: SB 18 Swale Type: INF_BASIN Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	84.53	0.01	48.1	4.8	0.21
03 infiltrate	79.77	0.01	26.4	2.6	0.12
04 exfiltrate	79.77	0.01	2.1	0.2	0.01
05 filtered	0.00	0.00	24.2	2.4	
07 spillway outlet	4.76	0.00	4.6	0.5	0.35
08 sedimen + decay	0.00	0.00	17.1	1.7	
09 total inflow	84.53	0.01	48.1	4.8	0.21
10 surface outflow	4.76	0.00	4.6	0.5	0.35
11 groundw outflow	79.77	0.01	2.1	0.2	0.01
12 total outflow	84.53	0.01	6.7	0.7	0.03
13 total trapped	0.00	0.00	41.3	4.1	
14 storage increase	0.00	0.00	0.0	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	86.0	86.0	

Device: Pond 13 Type: POND Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	522.22	0.07	118722.9	11883.7	83.64
02 upstream device	287.93	0.04	2836.2	283.9	3.62
06 normal outlet	810.20	0.11	19922.0	1994.1	9.05
08 sedimen + decay	0.00	0.00	101636.4	10173.4	
09 total inflow	810.15	0.11	121559.1	12167.6	55.20
10 surface outflow	810.20	0.11	19922.0	1994.1	9.05
12 total outflow	810.20	0.11	19922.0	1994.1	9.05
13 total trapped	0.00	0.00	101636.4	10173.4	
14 storage increase	0.00	0.00	0.7	0.1	
15 mass balance chec	-0.05	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	83.6	83.6	

Device: Pond 13 Type: POND Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	522.22	0.07	414.8	41.5	0.29
02 upstream device	287.93	0.04	86.7	8.7	0.11
06 normal outlet	810.20	0.11	289.1	28.9	0.13
08 sedimen + decay	0.00	0.00	212.0	21.2	
09 total inflow	810.15	0.11	501.4	50.2	0.23
10 surface outflow	810.20	0.11	289.1	28.9	0.13
12 total outflow	810.20	0.11	289.1	28.9	0.13
13 total trapped	0.00	0.00	212.0	21.2	
14 storage increase	0.00	0.00	0.4	0.0	
15 mass balance chec	-0.05	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	42.3	42.3	

Device: Outfall #5 Type: PIPE Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
02 upstream device	810.20	0.11	19922.0	1994.1	9.05
06 normal outlet	810.20	0.11	19922.0	1994.1	9.05
09 total inflow	810.20	0.11	19922.0	1994.1	9.05
10 surface outflow	810.20	0.11	19922.0	1994.1	9.05
12 total outflow	810.20	0.11	19922.0	1994.1	9.05
Reduction (%)	0.00	0.00	0.0	0.0	

Device: Outfall #5 Type: PIPE Variable: TP

MassBalances.prn

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
02 upstream device	810.20	0.11	289.1	28.9	0.13
06 normal outlet	810.20	0.11	289.1	28.9	0.13
09 total inflow	810.20	0.11	289.1	28.9	0.13
10 surface outflow	810.20	0.11	289.1	28.9	0.13
12 total outflow	810.20	0.11	289.1	28.9	0.13
Reduction (%)	0.00	0.00	0.0	0.0	

Device: Thumb Infiltration Type: INF_BASIN Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	246.11	0.03	53473.0	5352.4	79.94
03 infiltrate	241.54	0.03	7228.7	723.6	11.01
04 exfiltrate	241.54	0.03	0.0	0.0	0.00
05 filtered	0.00	0.00	7228.7	723.6	
07 spillway outlet	4.57	0.00	696.4	69.7	56.03
08 sedimen + decay	0.00	0.00	45547.9	4559.2	
09 total inflow	246.11	0.03	53473.0	5352.4	79.94
10 surface outflow	4.57	0.00	696.4	69.7	56.03
11 groundw outflow	241.54	0.03	0.0	0.0	0.00
12 total outflow	246.11	0.03	696.4	69.7	1.04
13 total trapped	0.00	0.00	52776.5	5282.7	
14 storage increase	0.00	0.00	0.0	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	98.7	98.7	

Device: Thumb Infiltration Type: INF_BASIN Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	246.11	0.03	189.7	19.0	0.28
03 infiltrate	241.54	0.03	92.6	9.3	0.14
04 exfiltrate	241.54	0.03	6.5	0.7	0.01
05 filtered	0.00	0.00	86.1	8.6	
07 spillway outlet	4.57	0.00	3.8	0.4	0.30
08 sedimen + decay	0.00	0.00	93.4	9.3	
09 total inflow	246.11	0.03	189.7	19.0	0.28
10 surface outflow	4.57	0.00	3.8	0.4	0.30
11 groundw outflow	241.54	0.03	6.5	0.7	0.01
12 total outflow	246.11	0.03	10.3	1.0	0.02
13 total trapped	0.00	0.00	179.5	18.0	
14 storage increase	0.00	0.00	0.0	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	94.6	94.6	

Device: To Rice Creek Type: PIPE Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
02 upstream device	1047.28	0.14	32078.7	3210.9	11.27
06 normal outlet	1047.28	0.14	32078.7	3210.9	11.27
09 total inflow	1047.28	0.14	32078.7	3210.9	11.27
10 surface outflow	1047.28	0.14	32078.7	3210.9	11.27
12 total outflow	1047.28	0.14	32078.7	3210.9	11.27
Reduction (%)	0.00	0.00	0.0	0.0	

Device: To Rice Creek Type: PIPE Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
02 upstream device	1047.28	0.14	392.2	39.3	0.14
06 normal outlet	1047.28	0.14	392.2	39.3	0.14
09 total inflow	1047.28	0.14	392.2	39.3	0.14
10 surface outflow	1047.28	0.14	392.2	39.3	0.14
12 total outflow	1047.28	0.14	392.2	39.3	0.14
Reduction (%)	0.00	0.00	0.0	0.0	

Rice Creek P8 Results
Interim Scenario 2

				Info.prn	
P8 Urban Catchment Model, Version 3.5					Run Date
07/30/15					
Case	Rice Creek Full Build without	FirstDate	01/01/01		Precip(in)
296.7					
Title	Rice Creek	LastDate	12/31/10		Rain(in)
273.57					
PrecFile	precip1970-2010.pcp	Events	598		Snow(in)
23.12					
PartFile	nurp50.p8p	TotalHrs	87576		TotalYrs
9.99					

File Directory	C:\Users\patmf0648\Desktop\P8\
Case Title	Rice Creek
Case File	Rice Creek Full Build without Infiltration Dev.p8c
Particle File	nurp50.p8p
Temperature File	temp1970-2011.tmp
Storm File	precip1970-2010.pcp
Precip Scale Factor	1

Watersheds	16
Devices	19
Particles	5
WQ Components	7

Start Date	06/01/00
Keep Date	01/01/01
Stop Date	12/31/10
Storm Count	598
Total Hours	87576
Wet Hours	9265
Precip (in)	297
Rain (in)	274
Snowfall (in)	23
Snowmelt (in)	22
EvapoTran(in)	303

Overall TSS Removal(%)	1
Water Balance Error(%)	0
TSS Mass Balance Error	0

Inputs.prn

P8 Urban Catchment Model, Version 3.5
 07/30/15
 Case Rice Creek Full Build without FirstDate 01/01/01 Run Date
 296.7 Precip(in)
 Title Rice Creek LastDate 12/31/10 Rain(in)
 273.57
 PrecFile precip1970-2010.pcp Events 598 Snow(in)
 23.12
 PartFile nurp50.p8p TotalHrs 87576 TotalYrs
 9.99

Case Title Rice Creek
 Case Data File Rice Creek Full Build without Infiltration Dev.p8c
 Path C:\Users\patmf0648\Desktop\P8\
 Case Notes: No Development in SB 18 (no infiltration)

Storm Data File precip1970-2010.pcp
 Particle File nurp50.p8p
 Air Temp File File temp1970-2011.tmp

Time Steps Per Hour 4
 Minimum Inter-Event Time (hrs) 10
 Maximum Continuity Error % 2
 Rainfall Breakpoint (inches) 0.8
 Precipitation Scale Factor 1
 Air Temp Offset (deg-F) 0
 Loops Thru Storm File 1
 Simulation Dates
 Start 6/1/2000
 Keep 1/1/2001
 Stop 12/31/2010

Max Snowfall Temperature (deg-f) 32.0
 SnowMelt Temperature (deg-f) 32.0
 Snowmelt Coef (in/degF-Day) 0.06
 Soil Freeze Temp (deg-F) 32.0
 Snowmelt Abstraction Factor 1.00
 Evapo-Trans. Calibration Factor 1.00
 Growing Season Start Month 5
 Growing Season End Month 10

5-Day Antecedent Rainfall + Runoff (inches)
 CN Antecedent Moisture Condition AMC-II AMC-III
 Growing Season 1.40 2.10
 NonGrowing Season 0.50 1.10

Watershed Data
 Watershed Name SB 8 SB 10 SB 9 SB 12 SB 11
 SB 14 SB 13 SB 18 SB 15, 16, SB 17 SB 19 Inflow fro SB 28 SB
 29 SB 1 SB 22 and 27
 Runoff to Device Pond 7 Pond 8 Pond 9 Pond 10 Pond 11
 Pond 12 Wetland 4 SB 18 Swal Pond 13 Wetland 5 Pond 14 (OWetland 3 CRH-2
 CRH-1 CRH-3 Thumb Infiltration
 Infiltration to Device
 Watershed Area 29.6 6.39 25.79 1.38 3.29
 10.23 2.985 52.79 105.327 7.608 21.198 4.16 6.955
 10.214 1.601 43.013
 SCS Curve Number (Pervious) 74 74 74 74 74 74 74 74
 74 74 74 74 74 74 74 74
 74 74 80
 Scale Factor for Pervious Runoff 1 1 1 1 1 1 1 1
 1 1 1 1 1 1 1 1

Inputs.prn

Indirectly Connected	1	1	0	0	0	0	0	0	0	0	0	0	0
Imperv Frac	0	0	0	0	0	0	0	0	0	0	0	0	0
Unswept Impervious Fraction	0.4262	0.3099	0	0.505	0.3001	0.076	0.3317	0.3871	0.3216	0.47	0.4676		
Unswept Depression Storage (inch)	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02
Unswept Imperv. Runoff Coefficient	1	1	1	1	1	1	1	1	1	1	1	1	1
Unswept Scale Factor for Particle	1	1	1	1	1	1	1	1	1	1	1	1	1
Swept Impervious Fraction	0	0	0	0	0	0	0	0	0	0	0	0	0
Swept Depression Storage (inches)	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0	0.02	0	0.02
Swept Imperv. Runoff Coefficient	1	1	1	1	1	1	1	1	1	1	1	1	1
Swept Scale Factor for Particle	1	1	1	1	1	1	1	1	1	1	1	1	1
Sweeping Frequency	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
Sweeping Efficiency	1	1	1	1	1	1	1	1	1	1	1	1	1
Sweeping Start Date (MMDD)	101	101	101	101	101	101	101	101	101	101	101	101	101
Sweeping Stop Date (MMDD)	1231	1231	1231	1231	1231	1231	1231	1231	1231	1231	1231	1231	1231

Device Data

Device Name	Pond 12	Pond 13	Pond 14	Pond 7	Pond 8	Pond 9	Pond 10	Pond 11
	CRH-1	CRH-2	To Rice Cr	CRH-3	SB 18	Swalthumb Infiltration	Wetland 3	Wetland 2
Device Type	POND	POND	PIPE	POND	POND	PIPE	POND	POND
Infiltration Outlet								
Normal Outlet	Pond 13	Outfall #5	To Rice Cr	Pond 9	Pond 10	Pond 11	Pond 12	Pond 10
Spillway Outlet	Rice Cr	CRH-3	To Rice Creek	To Rice Cr	Pond 13	To Rice Creek	Pond 9	Wetland 3
Particle Removal Scale Factor	1	1	1	1	1	1	1	1
Bottom Elevation (ft)	0	0	0	0	0	0	0	0
Bottom Area (acres)	1.2	0	0.95	0	0.11	0.03	0.03	0.12
							2.157	2.157

Inputs.prn

0.15	0.2	0.15	0	0	0	0.21	0.29	1.21
Permanent Pool Area (acres)		0.44	0.3			2.355	2.18	
1.64	1.87	1.38						
0.225	0.35	0.225	1.1	0.5	0.4	0.7	3.6	
Permanent Pool Volume (ac-ft)								
5.1	4.3	4.5	0	0	0	0	0	
0.187	0.412	0.187	0	0	0	0	0	0
Perm Pool Infiltr Rate (in/hr)								
0	0	0	0	0	0	0	0	0
Flood Pool Area (acres)		0.5	0.53	0.6	0.42	1.68		
2.04	2.96	1.73	1.891	1.891	2.653	2.591	0.5	
0.65	0.5	0.31	3.74					
Flood Pool Volume (ac-ft)								
5.1	6.3	4.9	2.5	5.8	0.3	0.9	5.4	
0.663	1.18	0.663	6.15	3.74	2.6	1.2		
Flood Pool Infiltr Rate (in/hr)								
0	0	0	0	0	0	0	0	0
Infiltr Basin Void Fraction (%)		0.45	0.45					

	100	100						
Detention Pond Outlet Parameters								
Outlet Type	WEIR	ORIFICE	WEIR	ORIFICE	WEIR	WEIR	ORIFICE	ORIFICE
ORIFICE WEIR ORIFICE	ORIFICE	ORIFICE	ORIFICE	ORIFICE	ORIFICE	ORIFICE	ORIFICE	ORIFICE
Outlet Orifice Diameter (in)				24			12	24
12	24	12	12	6	12	12	12	24
Orifice Discharge Coef				1			0.6	0.6
0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6
Outlet Weir Length (ft)			75		80	50		
55								
Weir Discharge Coef			3.3		3.3	3.3		
3.3								
Perforated Riser Height (ft)								
Number of Holes in Riser								
Holes Diameter								
Flood Pool Drain Time (hrs)								
Swale Parameters								
Length of Flow Path (ft)								
Slope of Flow Path %								
Bottom Width (ft)								
Side Slope (ft-v/ft-h)								
Maximum Depth of Flow (ft)								
Mannings n Constant								
Hydraulic Model								
Pipe, Splitter, Aquifer Parameter								
Hydraulic Res. Time (hrs)								0

Particle Data						
Particle File	nurp50.p8p					
Particle Class	P0%	P10%	P30%	P50%	P80%	
Filtration Efficiency	90	100	100	100	100	
Settling Velocity (ft/	0	0.03	0.3	1.5	15	
First Order Decay Rate	0	0	0	0	0	
2nd Order Decay (1/day	0	0	0	0	0	
Impervious Runoff Conc	1	0	0	0	0	
Pervious Runoff Conc (1	100	100	100	200	
Pervious Conc Exponent	0	1	1	1	1	

		Inputs.prn					
Accum. Rate (lbs-ac-da	0	1.75	1.75	1.75	3.5		
Particle Removal Rate	0	0.25	0.25	0.25	0.25		
Washoff Coefficient	0	20	20	20	20		
Washoff Exponent	0	2	2	2	2		
Sweeper Efficiency	0	0	0	5	15		
Water Quality Component Data							
Component Name	TSS	TP	TKN	CU	PB	ZN	
HC							
Water Quality Criteria (ppm)							
0.1	Level 1	5	0.025	2	2	0.02	5
0.5	Level 2	10	0.05	1	0.0048	0.014	0.0362
1	Level 3	20	0.1	0.5	0.02	0.15	0.38
Content Scale Factor							
1	1	1	1	1	1	1	
Particle Composition (mg/kg)							
P0%	0	99000	600000	13600	2000	640000	
250000							
P10%	1000000	3850	15000	340	180	1600	
22500							
P30%	1000000	3850	15000	340	180	1600	
22500							
P50%	1000000	3850	15000	340	180	1600	
22500							
P80%	1000000	0	0	340	180	0	
22500							

Inputs.prn

Inputs.prn

P8 Urban Catchment Model, Version 3.5

Run Date

07/30/15

Case 296.7 Rice Creek Full Build without FirstDate 01/01/01

Precip(in)

Title 273.57 Rice Creek LastDate 12/31/10

Rain(in)

PrecFile 23.12 precip1970-2010.pcp Events 598

Snow(in)

PartFile 9.99 nurp50.p8p TotalHrs 87576

TotalYrs

Devices Listed in Downstream Order

Device:	Pond 7	Type:	POND
	Discharges normal outlet to		Pond 9
	Discharges spillway to		Pond 9
	Runoff from watershed		SB 8
Device:	Pond 8	Type:	POND
	Discharges normal outlet to		Pond 10
	Discharges spillway to		Pond 10
	Runoff from watershed		SB 10
Device:	Pond 14 (Outfall #10	Type:	POND
	Runoff from watershed		SB 19
Device:	Wetland 4	Type:	POND
	Discharges normal outlet to		Pond 12
	Discharges spillway to		Pond 12
	Runoff from watershed		SB 13
Device:	Wetland 5	Type:	POND
	Discharges spillway to		Outfall #5
	Runoff from watershed		SB 17
Device:	Wetland 2	Type:	POND
	Discharges normal outlet to		Wetland 3
	Discharges spillway to		Wetland 3
Device:	Wetland 3	Type:	POND
	Discharges normal outlet to		Pond 9
	Discharges spillway to		Pond 9
	Runoff from watershed		Inflow from Ponds 4/5
Device:	Pond 9	Type:	POND
	Discharges normal outlet to		Pond 11
	Discharges spillway to		Pond 11
	Runoff from watershed		SB 9
Device:	Pond 11	Type:	POND
	Discharges normal outlet to		Pond 10
	Discharges spillway to		Pond 10
	Runoff from watershed		SB 11
Device:	Pond 10	Type:	POND
	Discharges normal outlet to		Pond 12
	Discharges spillway to		Pond 12
	Runoff from watershed		SB 12
Device:	Pond 12	Type:	POND
	Discharges normal outlet to		Pond 13
	Discharges spillway to		Pond 13

	Runoff from watershed	Network.prn	SB 14
Device:	CRH-1	Type:	POND
	Discharges normal outlet to		To Rice Creek
	Discharges spillway to		To Rice Creek
	Runoff from watershed		SB 29
Device:	CRH-2	Type:	POND
	Discharges normal outlet to		CRH-3
	Discharges spillway to		CRH-3
	Runoff from watershed		SB 28
Device:	CRH-3	Type:	POND
	Discharges normal outlet to		To Rice Creek
	Discharges spillway to		To Rice Creek
	Runoff from watershed		SB 1
Device:	SB 18 Swale	Type:	INF_BASIN
	Discharges spillway to		Pond 13
	Runoff from watershed		SB 18
Device:	Pond 13	Type:	POND
	Discharges normal outlet to		Outfall #5
	Discharges spillway to		Outfall #5
	Runoff from watershed		SB 15, 16, 26
Device:	Outfall #5	Type:	PIPE
	Discharges normal outlet to		To Rice Creek
Device:	Thumb Infiltration	Type:	INF_BASIN
	Discharges spillway to		To Rice Creek
	Runoff from watershed		SB 22 and 27
Device:	To Rice Creek	Type:	PIPE

Connected Unswept Areas		Directly Connected Swept Areas		Street Sweeping Parameters		Pervious		Indirect Pervious	
Depress Sweep Watershed Storage Freq Label inches 1/week	Runoff Coef	Total Imperv Area Load acres Factor	Depress Outflow Imperv Storage Device Fraction inches	Percol Runoff Coef	Curve Load Factor	Start Date MMDD	Stop Date MMDD	Load Sweep Factor Effic	Imperv Fraction
SB 8	1	29.6	Pond 7	1	1	74	0.000	1	0.3001
0.02	1	1	0.02	1	1	101	1231	1	0.5
SB 10	1	6.39	Pond 8	1	1	74	0.000	1	0.076
0.02	1	1	0.02	1	1	101	1231	1	0.5
SB 9	1	25.79	Pond 9	1	1	74	0.000	1	0.3317
0.02	1	1	0.02	1	1	101	1231	1	0.5
SB 12	1	1.38	Pond 10	1	1	74	0.000	1	0.3871
0.02	1	1	0.02	1	1	101	1231	1	0.5
SB 11	1	3.29	Pond 11	1	1	74	0.000	1	0.3216
0.02	1	1	0.02	1	1	101	1231	1	0.5
SB 14	1	10.23	Pond 12	1	1	74	0.000	1	0.4262
0.02	1	1	0.02	1	1	101	1231	1	0.5
SB 13	1	2.985	wetland 4	1	1	74	0.000	1	0.3099
0.02	1	1	0.02	1	1	101	1231	1	0.5
SB 18	1	52.79	SB 18 Swale	1	1	74	0.000	1	0
0.02	1	1	0.02	1	1	101	1231	1	0.5
SB 15, 16, 17	2	105.327	Pond 13	1	1	74	0.000	1	0.505
0.02	1	1	0.02	1	1	101	1231	1	0.5
SB 17	1	7.608	wetland 5	1	1	74	0.000	1	0.4841
0.02	1	1	0.02	1	1	101	1231	1	0.5
SB 19	1	21.198	Pond 14 (Outfall #10)	1	1	74	0.000	1	0.3993
0.02	1	1	0.02	1	1	101	1231	1	0.5
Inflow from P	1	4.16	wetland 3	1	1	74	0.000	1	0.47
0.02	1	1	0	1	1	101	1231	1	0.5
SB 28	1	6.955	CRH-2	1	1	74	0.000	1	0.4676
0.02	1	1	0	1	1	101	1231	1	0.5
SB 29	1	10.214	CRH-1	1	1	74	0.000	1	0.3773
0.02	1	1	0	1	1	101	1231	1	0.5
SB 1	1	1.601	CRH-3	1	1	74	0.000	1	0.3198
0.02	1	1	0	1	1	101	1231	1	0.5
SB 22 and 27	1	43.013	Thumb Infiltration	1	1	80	0.000	1	0.1138
0.02	1	1	0.02	1	1	101	1231	1	0.5

MassBalances.prn

P8 Urban Catchment Model, Version 3.5

Run Date 07/30/15
 Case Rice Creek Full Build without InfFirstDate 01/01/01
 Precip(in) 296.7
 Title Rice Creek LastDate 12/31/10
 Rain(in) 273.57
 PrecFile precip1970-2010.pcp Events 598
 Snow(in) 23.12
 PartFile nurp50.p8p TotalHrs 87576
 TotalYrs 9.99

Mass Balances by Device and Variable

Device: OVERALL Type: NONE Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	2887.51	0.40	711779.7	71246.2	90.69
03 infiltrate	295.35	0.04	7763.6	777.1	9.67
04 exfiltrate	295.35	0.04	0.0	0.0	0.00
05 filtered	0.00	0.00	7763.6	777.1	
06 normal outlet	2593.69	0.36	114410.0	11452.0	16.23
08 sedimen + decay	0.00	0.00	589602.3	59016.8	
09 total inflow	2887.51	0.40	711779.7	71246.2	90.69
10 surface outflow	2593.69	0.36	114410.0	11452.0	16.23
11 groundw outflow	295.35	0.04	0.0	0.0	0.00
12 total outflow	2889.04	0.40	114410.0	11452.0	14.57
13 total trapped	0.00	0.00	597365.9	59793.9	
14 storage increase	0.00	0.00	2.7	0.3	
15 mass balance chec	-1.53	0.00	1.0	0.1	
Reduction (%)	0.00	0.00	83.9	83.9	

Device: OVERALL Type: NONE Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	2887.51	0.40	2421.2	242.4	0.31
03 infiltrate	295.35	0.04	109.1	10.9	0.14
04 exfiltrate	295.35	0.04	7.9	0.8	0.01
05 filtered	0.00	0.00	101.2	10.1	
06 normal outlet	2593.69	0.36	1117.2	111.8	0.16
08 sedimen + decay	0.00	0.00	1194.6	119.6	
09 total inflow	2887.51	0.40	2421.2	242.4	0.31
10 surface outflow	2593.69	0.36	1117.2	111.8	0.16
11 groundw outflow	295.35	0.04	7.9	0.8	0.01
12 total outflow	2889.04	0.40	1125.2	112.6	0.14
13 total trapped	0.00	0.00	1295.8	129.7	
14 storage increase	0.00	0.00	0.2	0.0	
15 mass balance chec	-1.53	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	53.5	53.5	

Device: Pond 7 Type: POND Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	243.91	0.03	59979.5	6003.7	90.47
06 normal outlet	244.52	0.03	12052.6	1206.4	18.13
08 sedimen + decay	0.00	0.00	47926.6	4797.3	
09 total inflow	243.91	0.03	59979.5	6003.7	90.47
10 surface outflow	244.52	0.03	12052.6	1206.4	18.13
12 total outflow	244.52	0.03	12052.6	1206.4	18.13
13 total trapped	0.00	0.00	47926.6	4797.3	
14 storage increase	0.00	0.00	0.3	0.0	
15 mass balance chec	-0.62	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	79.9	79.9	

MassBalances.prn

Device: Pond 7 Type: POND Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	243.91	0.03	204.2	20.4	0.31
06 normal outlet	244.52	0.03	108.8	10.9	0.16
08 sedimen + decay	0.00	0.00	95.4	9.5	
09 total inflow	243.91	0.03	204.2	20.4	0.31
10 surface outflow	244.52	0.03	108.8	10.9	0.16
12 total outflow	244.52	0.03	108.8	10.9	0.16
13 total trapped	0.00	0.00	95.4	9.5	
14 storage increase	0.00	0.00	0.0	0.0	
15 mass balance chec	-0.62	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	46.7	46.7	

Device: Pond 8 Type: POND Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	20.98	0.00	4270.0	427.4	74.90
06 normal outlet	20.98	0.00	379.7	38.0	6.66
08 sedimen + decay	0.00	0.00	3890.3	389.4	
09 total inflow	20.98	0.00	4270.0	427.4	74.90
10 surface outflow	20.98	0.00	379.7	38.0	6.66
12 total outflow	20.98	0.00	379.7	38.0	6.66
13 total trapped	0.00	0.00	3890.3	389.4	
14 storage increase	0.00	0.00	0.0	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	91.1	91.1	

Device: Pond 8 Type: POND Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	20.98	0.00	15.5	1.6	0.27
06 normal outlet	20.98	0.00	7.0	0.7	0.12
08 sedimen + decay	0.00	0.00	8.5	0.8	
09 total inflow	20.98	0.00	15.5	1.6	0.27
10 surface outflow	20.98	0.00	7.0	0.7	0.12
12 total outflow	20.98	0.00	7.0	0.7	0.12
13 total trapped	0.00	0.00	8.5	0.8	
14 storage increase	0.00	0.00	0.0	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	54.6	54.6	

Device: Pond 14 (Outfall #10) Type: POND Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	221.19	0.03	55698.1	5575.1	92.64
06 normal outlet	221.19	0.03	3087.4	309.0	5.14
08 sedimen + decay	0.00	0.00	52610.2	5266.1	
09 total inflow	221.19	0.03	55698.1	5575.1	92.64
10 surface outflow	221.19	0.03	3087.4	309.0	5.14
12 total outflow	221.19	0.03	3087.4	309.0	5.14
13 total trapped	0.00	0.00	52610.2	5266.1	
14 storage increase	0.00	0.00	0.4	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	94.5	94.5	

Device: Pond 14 (Outfall #10) Type: POND Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	221.19	0.03	188.2	18.8	0.31
06 normal outlet	221.19	0.03	71.1	7.1	0.12
08 sedimen + decay	0.00	0.00	116.9	11.7	

MassBalances.prn

15 mass balance chec -0.14 0.00 0.0 0.0
 Reduction (%) 0.00 0.00 7.5 7.5

Device: Pond 12 Type: POND Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	112.83	0.02	28547.2	2857.5	93.08
02 upstream device	614.83	0.08	10049.3	1005.9	6.01
06 normal outlet	727.67	0.10	6421.4	642.8	3.25
08 sedimen + decay	0.00	0.00	32174.9	3220.6	
09 total inflow	727.67	0.10	38596.5	3863.4	19.51
10 surface outflow	727.67	0.10	6421.4	642.8	3.25
12 total outflow	727.67	0.10	6421.4	642.8	3.25
13 total trapped	0.00	0.00	32174.9	3220.6	
14 storage increase	0.00	0.00	0.2	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	83.4	83.4	

Device: Pond 12 Type: POND Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	112.83	0.02	96.3	9.6	0.31
02 upstream device	614.83	0.08	203.4	20.4	0.12
06 normal outlet	727.67	0.10	220.0	22.0	0.11
08 sedimen + decay	0.00	0.00	79.7	8.0	
09 total inflow	727.67	0.10	299.7	30.0	0.15
10 surface outflow	727.67	0.10	220.0	22.0	0.11
12 total outflow	727.67	0.10	220.0	22.0	0.11
13 total trapped	0.00	0.00	79.7	8.0	
14 storage increase	0.00	0.00	0.0	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	26.6	26.6	

Device: CRH-1 Type: POND Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	101.61	0.01	25475.6	2550.0	92.25
06 normal outlet	101.61	0.01	5758.7	576.4	20.85
08 sedimen + decay	0.00	0.00	19716.9	1973.6	
09 total inflow	101.61	0.01	25475.6	2550.0	92.25
10 surface outflow	101.61	0.01	5758.7	576.4	20.85
12 total outflow	101.61	0.01	5758.7	576.4	20.85
13 total trapped	0.00	0.00	19716.9	1973.6	
14 storage increase	0.00	0.00	0.0	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	77.4	77.4	

Device: CRH-1 Type: POND Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	101.61	0.01	86.2	8.6	0.31
06 normal outlet	101.61	0.01	48.9	4.9	0.18
08 sedimen + decay	0.00	0.00	37.3	3.7	
09 total inflow	101.61	0.01	86.2	8.6	0.31
10 surface outflow	101.61	0.01	48.9	4.9	0.18
12 total outflow	101.61	0.01	48.9	4.9	0.18
13 total trapped	0.00	0.00	37.3	3.7	
14 storage increase	0.00	0.00	0.0	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	43.3	43.3	

Device: CRH-2 Type: POND Variable: TSS

MassBalances.prn

07 spillway outlet	4.76	0.00	1041.3	104.2	80.49
08 sedimen + decay	0.00	0.00	8633.4	864.2	
09 total inflow	84.53	0.01	10961.7	1097.2	47.71
10 surface outflow	4.76	0.00	1041.3	104.2	80.49
11 groundw outflow	79.77	0.01	0.0	0.0	0.00
12 total outflow	84.53	0.01	1041.3	104.2	4.53
13 total trapped	0.00	0.00	9920.4	993.0	
14 storage increase	0.00	0.00	0.0	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	90.5	90.5	

Device: SB 18 Swale Type: INF_BASIN Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	84.53	0.01	48.1	4.8	0.21
03 infiltrate	79.77	0.01	26.4	2.6	0.12
04 exfiltrate	79.77	0.01	2.1	0.2	0.01
05 filtered	0.00	0.00	24.2	2.4	
07 spillway outlet	4.76	0.00	4.6	0.5	0.35
08 sedimen + decay	0.00	0.00	17.1	1.7	
09 total inflow	84.53	0.01	48.1	4.8	0.21
10 surface outflow	4.76	0.00	4.6	0.5	0.35
11 groundw outflow	79.77	0.01	2.1	0.2	0.01
12 total outflow	84.53	0.01	6.7	0.7	0.03
13 total trapped	0.00	0.00	41.3	4.1	
14 storage increase	0.00	0.00	0.0	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	86.0	86.0	

Device: Pond 13 Type: POND Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	1345.33	0.19	344217.7	34454.8	94.14
02 upstream device	732.43	0.10	7462.6	747.0	3.75
06 normal outlet	2077.74	0.29	95897.0	9598.9	16.98
08 sedimen + decay	0.00	0.00	255781.9	25602.7	
09 total inflow	2077.76	0.29	351680.3	35201.8	62.27
10 surface outflow	2077.74	0.29	95897.0	9598.9	16.98
12 total outflow	2077.74	0.29	95897.0	9598.9	16.98
13 total trapped	0.00	0.00	255781.9	25602.7	
14 storage increase	0.00	0.00	1.4	0.1	
15 mass balance chec	0.02	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	72.7	72.7	

Device: Pond 13 Type: POND Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	1345.33	0.19	1157.1	115.8	0.32
02 upstream device	732.43	0.10	224.5	22.5	0.11
06 normal outlet	2077.74	0.29	908.8	91.0	0.16
08 sedimen + decay	0.00	0.00	472.9	47.3	
09 total inflow	2077.76	0.29	1381.7	138.3	0.24
10 surface outflow	2077.74	0.29	908.8	91.0	0.16
12 total outflow	2077.74	0.29	908.8	91.0	0.16
13 total trapped	0.00	0.00	472.9	47.3	
14 storage increase	0.00	0.00	0.0	0.0	
15 mass balance chec	0.02	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	34.2	34.2	

Device: Outfall #5 Type: PIPE Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
02 upstream device	2077.74	0.29	95897.0	9598.9	16.98

MassBalances.prn

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
02 upstream device	2278.84	0.31	993.8	99.5	0.16
06 normal outlet	2278.84	0.31	993.8	99.5	0.16
09 total inflow	2278.84	0.31	993.8	99.5	0.16
10 surface outflow	2278.84	0.31	993.8	99.5	0.16
12 total outflow	2278.84	0.31	993.8	99.5	0.16
Reduction (%)	0.00	0.00	0.0	0.0	

Rice Creek P8 Results
Scenario 3: Fully Developed Conditions

P8 Urban Catchment Model, Version 3.5	Info.prn	Run Date
06/10/15		
Case	Rice Creek Full Build.p8c	FirstDate 01/01/01
296.7		Precip(in)
Title	Rice Creek	LastDate 12/31/10
273.57		Rain(in)
PrecFile	precip1970-2010.pcp	Events 598
23.12		Snow(in)
PartFile	nurp50.p8p	TotalHrs 87576
9.99		TotalYrs

File Directory	T:\1382 KimleyHorn\01 TCAAP\TASK 03 Stormwater Prelim
Design\Models\P8\	
Case Title	Rice Creek
Case File	Rice Creek Full Build.p8c
Particle File	nurp50.p8p
Temperature File	temp1970-2011.tmp
Storm File	precip1970-2010.pcp
Precip Scale Factor	1

Watersheds	16
Devices	19
Particles	5
WQ Components	7

Start Date	06/01/00
Keep Date	01/01/01
Stop Date	12/31/10
Storm Count	598
Total Hours	87576
Wet Hours	9265
Precip (in)	297
Rain (in)	274
Snowfall (in)	23
Snowmelt (in)	22
EvapoTran(in)	303

Overall TSS Removal(%)	1
Water Balance Error(%)	0
TSS Mass Balance Error	0

Inputs.prn

P8 Urban Catchment Model, Version 3.5				Run Date
06/10/15				
Case	Rice Creek Full Build.p8c	FirstDate	01/01/01	Precip(in)
296.7				
Title	Rice Creek	LastDate	12/31/10	Rain(in)
273.57				
PrecFile	precip1970-2010.pcp	Events	598	Snow(in)
23.12				
PartFile	nurp50.p8p	TotalHrs	87576	TotalYrs
9.99				

Case Title	Rice Creek
Case Data File	Rice Creek Full Build.p8c
Path	T:\1382 KimleyHorn\01 TCAAP\TASK 03 Stormwater
Prelim Design\Models\P8\	
Case Notes:	Full Build
Storm Data File	precip1970-2010.pcp
Particle File	nurp50.p8p
Air Temp File File	temp1970-2011.tmp

Time Steps Per Hour	4
Minimum Inter-Event Time (hrs)	10
Maximum Continuity Error %	2
Rainfall Breakpoint (inches)	0.8
Precipitation Scale Factor	1
Air Temp Offset (deg-F)	0
Loops Thru Storm File	1
Simulation Dates	
Start	6/1/2000
Keep	1/1/2001
Stop	12/31/2010

Max Snowfall Temperature (deg-f)	32.0
SnowMelt Temperature (deg-f)	32.0
Snowmelt Coef (in/degF-Day)	0.06
Soil Freeze Temp (deg-F)	32.0
Snowmelt Abstraction Factor	1.00
Evapo-Trans. Calibration Factor	1.00
Growing Season Start Month	5
Growing Season End Month	10

5-Day Antecedent Rainfall + Runoff (inches)		
CN Antecedent Moisture Condition	AMC-II	AMC-III
Growing Season	1.40	2.10
NonGrowing Season	0.50	1.10

Watershed Data										
Watershed Name			SB 8	SB 10	SB 9	SB 12	SB 11			
SB 14	SB 13	SB 18	SB 15, 16,	SB 17	SB 19	Inflow fro	SB 28	SB		
29	SB 1	SB 22 and 27								
Runoff to Device			Pond 7	Pond 8	Pond 9	Pond 10	Pond 11			
Pond 12	Wetland 4	Infiltrati	Pond 13	Wetland 5	Pond 14	(OWetland 3	CRH-1			
CRH-2	CRH-3	Thumb Infiltration								
Infiltration to Device										
Watershed Area			29.6	6.39	25.79	1.38	3.29			
10.23	2.985	52.908	105.327	7.608	21.198	4.16	6.955			
10.214	1.601	7.656								
SCS Curve Number (Pervious)			74	74	74	74	74	74	74	74
74	74	74	74	74	74	74	74	74	74	74
74	74	74								
Scale Factor for Pervious Runoff			1	1	1	1	1	1	1	1
1	1	1	1	1	1	1	1	1	1	1

Inputs.prn

Indirectly Connected	1	1	Imperv Frac	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0	0	0
Unswept Impervious Fraction	0.4262	0.3099	0.8455	0.505	0.3	0.076	0.3317	0.3871	0.3216	0.47	0.4676	0.3216	0.3216
0.3773	0.3198	0.8423	0.8423	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02
Unswept Depression Storage (inch)	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02
0.02	0.02	0.02	0.02	1	1	1	1	1	1	1	1	1	1
Unswept Imperv. Runoff Coefficient	1	1	1	1	1	1	1	1	1	1	1	1	1
1	1	1	1	1	1	1	1	1	1	1	1	1	1
Unswept Scale Factor for Particl	1	1	1	1	1	1	1	1	1	1	1	1	1
1	1	1	1	1	1	1	1	1	1	1	1	1	1
Swept Impervious Fraction	0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02
Swept Depression Storage (inches)	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02
0.02	0.02	0.02	0.02	1	1	1	1	1	1	1	1	1	1
Swept Imperv. Runoff Coefficient	1	1	1	1	1	1	1	1	1	1	1	1	1
1	1	1	1	1	1	1	1	1	1	1	1	1	1
Swept Scale Factor for Particle	1	1	1	1	1	1	1	1	1	1	1	1	1
1	1	1	1	1	1	1	1	1	1	1	1	1	1
Sweeping Frequency	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
0.5	0.5	0.5	0.5	1	1	1	1	1	1	1	1	1	1
Sweeping Efficiency	1	1	1	1	1	1	1	1	1	1	1	1	1
1	1	1	1	101	101	101	101	101	101	101	101	101	101
Sweeping Start Date (MMDD)	101	101	101	101	101	101	101	101	101	101	101	101	101
101	101	101	101	1231	1231	1231	1231	1231	1231	1231	1231	1231	1231
Sweeping Stop Date (MMDD)	1231	1231	1231	1231	1231	1231	1231	1231	1231	1231	1231	1231	1231
1231	1231	1231	1231										

Device Data

Device Name	Pond 12	Pond 13	Infiltrati	Pond 14	Pond 7	Pond 8	Pond 9	Pond 10	Pond 11
	Wetland 2	CRH-1	CRH-2	To Rice Cr	Wetland 4	CRH-3	Thumb Infiltration	Wetland 5	Wetland 3
Device Type	POND	POND	INF_BASIN	POND	POND	POND	POND	PIPE	POND
	POND	POND	POND	PIPE	POND	INF_BASIN			
Infiltration Outlet									
Normal Outlet									
	Pond 13	Outfall #5		Pond 9	Pond 10	Pond 11	Pond 12	Pond 10	Pond 10
	Wetland 3	To Rice Cr	CRH-3	To Rice Cr	Outfall #5	To Rice Cr	To Rice Cr	To Rice Cr	Pond 9
Spillway Outlet									
	Pond 13	Outfall #5	Pond 13	To Rice Cr	Pond 12	Outfall #5			Pond 9
	Wetland 3	To Rice Cr	CRH-3	To Rice Cr	To Rice Cr	To Rice Creek			
Particle Removal Scale Factor	1	1	1	1	1	1	1	1	1
1	1	1	1	1	1	1	1	1	1
Bottom Elevation (ft)	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0
Bottom Area (acres)	1.2	0.02	0	0.143	0.057	0	0.024	0.13	0.97
1.2	0.02	0	1.22	0	0	0	0.024	0.13	0.97

2.157	0.15	0.2	Inputs.prn			0				
Permanent Pool Area (acres)	1.64	1.988	0.548	0	0.26	0	0.41	0.288	1.32	
2.18	0.225	0.35	1.557	0	0.225	0		2.355		
Permanent Pool Volume (ac-ft)	5.7	6.2	1	0	0.6	0	0.5	0.8	4.5	0
Perm Pool Infiltration Rate (in/hr)	0	0	0.187	0	0	0	0	0	0	0
Flood Pool Area (acres)	2.38	4.03	0.723	0	0.669	0.75		0.61	2.11	
Flood Pool Volume (ac-ft)	2.591	0.5	2.179	1.891	1.891	3.74		2.653		
Flood Pool Infiltration Rate (in/hr)	5.3	8.1	0.5	2.5	0.9	5.8	0.4	1.1	2.6	6.6
Infiltration Basin Void Fraction (%)	0	0	0.663	0	3.74	0	0	0	0	0
	0	0	0	0	0.45	0	0	0	0	0
	0	0	0	0	0.45					

Detention Pond Outlet Parameters			100			100		
Outlet Type	WEIR	ORIFICE	WEIR	ORIFICE	WEIR	WEIR	ORIFICE	ORIFICE
ORIFICE	WEIR	ORIFICE	ORIFICE	ORIFICE	ORIFICE	ORIFICE	ORIFICE	ORIFICE
Outlet Orifice Diameter (in)	12	24	12	24	6	12	12	12
Orifice Discharge Coef	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6
Outlet Weir Length (ft)	55	80	50					
Weir Discharge Coef	3.3	3.3	3.3					
Perforated Riser Height (ft)								
Number of Holes in Riser								
Holes Diameter								
Flood Pool Drain Time (hrs)								
Swale Parameters								
Length of Flow Path (ft)								
Slope of Flow Path %								
Bottom Width (ft)								
Side Slope (ft-v/ft-h)								
Maximum Depth of Flow (ft)								
Mannings n Constant								
Hydraulic Model								
Pipe, Splitter, Aquifer Parameter								
Hydraulic Res. Time (hrs)	0							0

Particle Data		nurp50.p8p				
Particle File						
Particle Class	P0%	P10%	P30%	P50%	P80%	
Filtration Efficiency	90	100	100	100	100	
Settling Velocity (ft/	0	0.03	0.3	1.5	15	
First Order Decay Rate	0	0	0	0	0	
2nd Order Decay (1/day	0	0	0	0	0	
Impervious Runoff Conc	1	0	0	0	0	
Pervious Runoff Conc (1	100	100	100	200	
Pervious Conc Exponent	0	1	1	1	1	

		Inputs.prn					
Accum. Rate (lbs-ac-da	0	1.75	1.75	1.75	3.5		
Particle Removal Rate	0	0.25	0.25	0.25	0.25		
Washoff Coefficient	0	20	20	20	20		
Washoff Exponent	0	2	2	2	2		
Sweeper Efficiency	0	0	0	5	15		
Water Quality Component Data							
Component Name	TSS	TP	TKN	CU	PB	ZN	
HC							
Water Quality Criteria (ppm)							
0.1	Level 1	5	0.025	2	2	0.02	5
0.5	Level 2	10	0.05	1	0.0048	0.014	0.0362
1	Level 3	20	0.1	0.5	0.02	0.15	0.38
Content Scale Factor							
1	1	1	1	1	1	1	
Particle Composition (mg/kg)							
P0%	0	99000	600000	13600	2000	640000	
250000							
P10%	1000000	3850	15000	340	180	1600	
22500							
P30%	1000000	3850	15000	340	180	1600	
22500							
P50%	1000000	3850	15000	340	180	1600	
22500							
P80%	1000000	0	0	340	180	0	
22500							

Inputs.prn

Inputs.prn

P8 Urban Catchment Model, Version 3.5				Run Date
06/10/15				
Case	Rice Creek Full Build.p8c	FirstDate	01/01/01	Precip(in)
296.7				
Title	Rice Creek	LastDate	12/31/10	Rain(in)
273.57				
PrecFile	precip1970-2010.pcp	Events	598	Snow(in)
23.12				
PartFile	nurp50.p8p	TotalHrs	87576	TotalYrs
9.99				

Devices Listed in Downstream Order

Device:	Pond 7	Type:	POND
	Discharges normal outlet to		Pond 9
	Discharges spillway to		Pond 9
	Runoff from watershed		SB 8
Device:	Pond 8	Type:	POND
	Discharges normal outlet to		Pond 10
	Discharges spillway to		Pond 10
	Runoff from watershed		SB 10
Device:	Infiltration BMP	Type:	INF_BASIN
	Discharges spillway to		Pond 13
	Runoff from watershed		SB 18
Device:	Pond 14 (Outfall #10)	Type:	POND
	Discharges normal outlet to		To Rice Creek
	Discharges spillway to		To Rice Creek
	Runoff from watershed		SB 19
Device:	Wetland 4	Type:	POND
	Discharges normal outlet to		Pond 12
	Discharges spillway to		Pond 12
	Runoff from watershed		SB 13
Device:	Wetland 5	Type:	POND
	Discharges normal outlet to		Outfall #5
	Discharges spillway to		Outfall #5
	Runoff from watershed		SB 17
Device:	Wetland 2	Type:	POND
	Discharges normal outlet to		Wetland 3
	Discharges spillway to		Wetland 3
Device:	Wetland 3	Type:	POND
	Discharges normal outlet to		Pond 9
	Discharges spillway to		Pond 9
	Runoff from watershed		Inflow from Ponds 4/5
Device:	Pond 9	Type:	POND
	Discharges normal outlet to		Pond 11
	Discharges spillway to		Pond 11
	Runoff from watershed		SB 9
Device:	Pond 11	Type:	POND
	Discharges normal outlet to		Pond 10
	Discharges spillway to		Pond 10
	Runoff from watershed		SB 11
Device:	Pond 10	Type:	POND

		Network.prn	
	Discharges normal outlet to	Pond 12	
	Discharges spillway to	Pond 12	
	Runoff from watershed	SB 12	
Device:	Pond 12	Type:	POND
	Discharges normal outlet to	Pond 13	
	Discharges spillway to	Pond 13	
	Runoff from watershed	SB 14	
Device:	Pond 13	Type:	POND
	Discharges normal outlet to	Outfall #5	
	Discharges spillway to	Outfall #5	
	Runoff from watershed	SB 15, 16, 26	
Device:	Outfall #5	Type:	PIPE
	Discharges normal outlet to	To Rice Creek	
Device:	CRH-1	Type:	POND
	Discharges normal outlet to	To Rice Creek	
	Discharges spillway to	To Rice Creek	
	Runoff from watershed	SB 28	
Device:	CRH-2	Type:	POND
	Discharges normal outlet to	CRH-3	
	Discharges spillway to	CRH-3	
	Runoff from watershed	SB 29	
Device:	CRH-3	Type:	POND
	Discharges normal outlet to	To Rice Creek	
	Discharges spillway to	To Rice Creek	
	Runoff from watershed	SB 1	
Device:	Thumb Infiltration	Type:	INF_BASIN
	Discharges spillway to	To Rice Creek	
	Runoff from watershed	SB 22 and 27	
Device:	To Rice Creek	Type:	PIPE

Connected UnSwept Areas		Directly Connected Swept Areas		Street Sweeping Parameters		Directly Connected Swept Areas		Street Sweeping Parameters	
Depress Sweep Watershed Storage Freq Label inches 1/week	Runoff Coef	Total Imperv Area Load acres Factor	Depress Outflow Imperv Storage Device Fraction inches	Percol Runoff Coef	Curve Load Date	Imperv Fraction MMDD	Indirect Pervious Factor	Start Stop Date	Load Sweep Effc
SB 8	1	29.6	Pond 7	1	74	0.000	1		0.3
0.02	1	1	0.02	1	101	1231	1		0.5
SB 10	1	6.39	Pond 8	1	74	0.000	1		0.076
0.02	1	1	0.02	1	101	1231	1		0.5
SB 9	1	25.79	Pond 9	1	74	0.000	1		0.3317
0.02	1	1	0.02	1	101	1231	1		0.5
SB 12	1	1.38	Pond 10	1	74	0.000	1		0.3871
0.02	1	1	0.02	1	101	1231	1		0.5
SB 11	1	3.29	Pond 11	1	74	0.000	1		0.3216
0.02	1	1	0.02	1	101	1231	1		0.5
SB 14	1	10.23	Pond 12	1	74	0.000	1		0.4262
0.02	1	1	0.02	1	101	1231	1		0.5
SB 13	1	2.985	Wetland 4	1	74	0.000	1		0.3099
0.02	1	1	0.02	1	101	1231	1		0.5
SB 18	1	52.908	Infiltration BMP	1	74	0.000	1		0.8455
0.02	1	1	0.02	1	101	1231	1		0.5
SB 15, 16, 26	1	105.327	Pond 13	1	74	0.000	1		0.505
0.02	1	1	0.02	1	101	1231	1		0.5
SB 17	1	7.608	Wetland 5	1	74	0.000	1		0.4841
0.02	1	1	0.02	1	101	1231	1		0.5
SB 19	1	21.198	Pond 14 (Outfall #10)	1	74	0.000	1		0.3993
0.02	1	1	0.02	1	101	1231	1		0.5
Inflow from P	1	4.16	Wetland 3	1	74	0.000	1		0.47
0.02	1	1	0	1	101	1231	1		0.5
SB 28	1	6.955	CRH-1	1	74	0.000	1		0.4676
0.02	1	1	0	1	101	1231	1		0.5
SB 29	1	10.214	CRH-2	1	74	0.000	1		0.3773
0.02	1	1	0	1	101	1231	1		0.5
SB 1	1	1.601	CRH-3	1	74	0.000	1		0.3198
0.02	1	1	0	1	101	1231	1		0.5
SB 22 and 27	1	7.656	Thumb Infiltration	1	74	0.000	1		0.8423
0.02	1	1	0.02	1	101	1231	1		0.5

MassBalances.prn

P8 Urban Catchment Model, Version 3.5

Run Date 06/10/15

Case	Rice Creek Full Build.p8c	FirstDate	01/01/01
Precip(in)	296.7	LastDate	12/31/10
Title	Rice Creek	Events	598
Rain(in)	273.57	TotalHrs	87576
PrecFile	precip1970-2010.pcp		
Snow(in)	23.12		
PartFile	nurp50.p8p		
TotalYrs	9.99		

Mass Balances by Device and Variable

Device: OVERALL Type: NONE Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	3814.06	0.53	976169.8	97710.6	94.16
03 infiltrate	1065.79	0.15	41546.7	4158.7	14.34
04 exfiltrate	1065.79	0.15	0.0	0.0	0.00
05 filtered	0.00	0.00	41546.7	4158.7	
06 normal outlet	2749.56	0.38	112085.8	11219.3	15.00
08 sedimen + decay	0.00	0.00	822533.1	82332.2	
09 total inflow	3814.06	0.53	976169.8	97710.6	94.16
10 surface outflow	2749.56	0.38	112085.8	11219.3	15.00
11 groundw outflow	1065.79	0.15	0.0	0.0	0.00
12 total outflow	3815.34	0.53	112085.8	11219.3	10.81
13 total trapped	0.00	0.00	864079.9	86490.9	
14 storage increase	0.00	0.00	3.1	0.3	
15 mass balance chec	-1.29	0.00	1.0	0.1	
Reduction (%)	0.00	0.00	88.5	88.5	

Device: OVERALL Type: NONE Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	3814.06	0.53	3281.2	328.4	0.32
03 infiltrate	1065.79	0.15	445.5	44.6	0.15
04 exfiltrate	1065.79	0.15	28.7	2.9	0.01
05 filtered	0.00	0.00	416.8	41.7	
06 normal outlet	2749.56	0.38	1151.2	115.2	0.15
08 sedimen + decay	0.00	0.00	1684.4	168.6	
09 total inflow	3814.06	0.53	3281.2	328.4	0.32
10 surface outflow	2749.56	0.38	1151.2	115.2	0.15
11 groundw outflow	1065.79	0.15	28.7	2.9	0.01
12 total outflow	3815.34	0.53	1179.9	118.1	0.11
13 total trapped	0.00	0.00	2101.2	210.3	
14 storage increase	0.00	0.00	0.1	0.0	
15 mass balance chec	-1.29	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	64.0	64.0	

Device: Pond 7 Type: POND Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	243.84	0.03	59961.6	6001.9	90.47
06 normal outlet	244.26	0.03	11590.2	1160.1	17.46
08 sedimen + decay	0.00	0.00	48371.2	4841.8	
09 total inflow	243.84	0.03	59961.6	6001.9	90.47
10 surface outflow	244.26	0.03	11590.2	1160.1	17.46
12 total outflow	244.26	0.03	11590.2	1160.1	17.46
13 total trapped	0.00	0.00	48371.2	4841.8	
14 storage increase	0.00	0.00	0.2	0.0	
15 mass balance chec	-0.42	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	80.7	80.7	

MassBalances.prn

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	1074.32	0.15	940.7	94.2	0.32
03 infiltrate	910.87	0.13	382.2	38.3	0.15
04 exfiltrate	910.87	0.13	24.5	2.5	0.01
05 filtered	0.00	0.00	357.7	35.8	
07 spillway outlet	163.45	0.02	80.7	8.1	0.18
08 sedimen + decay	0.00	0.00	477.8	47.8	
09 total inflow	1074.32	0.15	940.7	94.2	0.32
10 surface outflow	163.45	0.02	80.7	8.1	0.18
11 groundw outflow	910.87	0.13	24.5	2.5	0.01
12 total outflow	1074.32	0.15	105.2	10.5	0.04
13 total trapped	0.00	0.00	835.5	83.6	
14 storage increase	0.00	0.00	0.0	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	88.8	88.8	

Device: Pond 14 (Outfall #10) Type: POND Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	221.19	0.03	55698.1	5575.1	92.64
06 normal outlet	221.19	0.03	3588.4	359.2	5.97
08 sedimen + decay	0.00	0.00	52109.6	5216.0	
09 total inflow	221.19	0.03	55698.1	5575.1	92.64
10 surface outflow	221.19	0.03	3588.4	359.2	5.97
12 total outflow	221.19	0.03	3588.4	359.2	5.97
13 total trapped	0.00	0.00	52109.6	5216.0	
14 storage increase	0.00	0.00	0.1	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	93.6	93.6	

Device: Pond 14 (Outfall #10) Type: POND Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	221.19	0.03	188.2	18.8	0.31
06 normal outlet	221.19	0.03	73.2	7.3	0.12
08 sedimen + decay	0.00	0.00	115.0	11.5	
09 total inflow	221.19	0.03	188.2	18.8	0.31
10 surface outflow	221.19	0.03	73.2	7.3	0.12
12 total outflow	221.19	0.03	73.2	7.3	0.12
13 total trapped	0.00	0.00	115.0	11.5	
14 storage increase	0.00	0.00	0.0	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	61.1	61.1	

Device: Wetland 4 Type: POND Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	25.24	0.00	6225.9	623.2	90.74
06 normal outlet	25.26	0.00	2001.2	200.3	29.15
08 sedimen + decay	0.00	0.00	4224.3	422.8	
09 total inflow	25.24	0.00	6225.9	623.2	90.74
10 surface outflow	25.26	0.00	2001.2	200.3	29.15
12 total outflow	25.26	0.00	2001.2	200.3	29.15
13 total trapped	0.00	0.00	4224.3	422.8	
14 storage increase	0.00	0.00	0.0	0.0	
15 mass balance chec	-0.01	0.00	0.4	0.0	
Reduction (%)	0.00	0.00	67.9	67.9	

Device: Wetland 4 Type: POND Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	25.24	0.00	21.2	2.1	0.31
06 normal outlet	25.26	0.00	14.3	1.4	0.21

MassBalances.prn

Device: CRH-3 Type: POND Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	13.89	0.00	11.7	1.2	0.31
02 upstream device	101.61	0.01	42.4	4.2	0.15
06 normal outlet	115.50	0.02	41.7	4.2	0.13
08 sedimen + decay	0.00	0.00	12.4	1.2	
09 total inflow	115.50	0.02	54.1	5.4	0.17
10 surface outflow	115.50	0.02	41.7	4.2	0.13
12 total outflow	115.50	0.02	41.7	4.2	0.13
13 total trapped	0.00	0.00	12.4	1.2	
14 storage increase	0.00	0.00	0.0	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	22.9	22.9	

Device: Thumb Infiltration Type: INF_BASIN Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	154.92	0.02	40670.3	4070.9	96.59
03 infiltrate	154.92	0.02	5661.6	566.7	13.45
04 exfiltrate	154.92	0.02	0.0	0.0	0.00
05 filtered	0.00	0.00	5661.6	566.7	
08 sedimen + decay	0.00	0.00	35008.7	3504.2	
09 total inflow	154.92	0.02	40670.3	4070.9	96.59
11 groundw outflow	154.92	0.02	0.0	0.0	0.00
12 total outflow	154.92	0.02	0.0	0.0	0.00
13 total trapped	0.00	0.00	40670.3	4070.9	
14 storage increase	0.00	0.00	0.0	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	100.0	100.0	

Device: Thumb Infiltration Type: INF_BASIN Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
01 watershed inflows	154.92	0.02	135.6	13.6	0.32
03 infiltrate	154.92	0.02	63.3	6.3	0.15
04 exfiltrate	154.92	0.02	4.2	0.4	0.01
05 filtered	0.00	0.00	59.2	5.9	
08 sedimen + decay	0.00	0.00	72.3	7.2	
09 total inflow	154.92	0.02	135.6	13.6	0.32
11 groundw outflow	154.92	0.02	4.2	0.4	0.01
12 total outflow	154.92	0.02	4.2	0.4	0.01
13 total trapped	0.00	0.00	131.5	13.2	
14 storage increase	0.00	0.00	0.0	0.0	
15 mass balance chec	0.00	0.00	0.0	0.0	
Reduction (%)	0.00	0.00	96.9	96.9	

Device: To Rice Creek Type: PIPE Variable: TSS

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
02 upstream device	2749.56	0.38	112085.8	11219.3	15.00
06 normal outlet	2749.56	0.38	112085.8	11219.3	15.00
09 total inflow	2749.56	0.38	112085.8	11219.3	15.00
10 surface outflow	2749.56	0.38	112085.8	11219.3	15.00
12 total outflow	2749.56	0.38	112085.8	11219.3	15.00
Reduction (%)	0.00	0.00	0.0	0.0	

Device: To Rice Creek Type: PIPE Variable: TP

Mass Balance Term	Flow_acft	Flow_cfs	Load_lbs	Load_lbs/yr	Conc_ppm
02 upstream device	2749.56	0.38	1151.2	115.2	0.15
06 normal outlet	2749.56	0.38	1151.2	115.2	0.15

		MassBalances.prn			
09 total inflow	2749.56	0.38	1151.2	115.2	0.15
10 surface outflow	2749.56	0.38	1151.2	115.2	0.15
12 total outflow	2749.56	0.38	1151.2	115.2	0.15
Reduction (%)	0.00	0.00	0.0	0.0	

Soil Borings (2007)

Interim Report—Preliminary Geotechnical Evaluation

Twin Cities Army Ammunition Plant Redevelopment
Northeast of US Highway 10 and Highway 96
Arden Hills, Minnesota

Prepared for

Ryan Companies US, Inc.

October 3, 2007

American Engineering and Testing, Inc.
Braun Intertec Corporation

October 3, 2007

Ms. Genevieve McJilton
Ryan Companies US, Inc.
50 South 10th Street, Suite 300
Minneapolis, MN 55403

Re: Interim Report—Preliminary Geotechnical Evaluation
Twin Cities Army Ammunition Plant Redevelopment
Northeast of US Highway 10 and Highway 96
Arden Hills, Minnesota

Dear Ms. McJilton:

The following is a summary of the geotechnical evaluation being conducted by our firms, American Engineering and Testing, Inc. (AET), and Braun Intertec Corporation (Braun). The purpose of this summary is to inform the design team of the subsurface conditions that we found in our borings on the Twin Cities Army Ammunition Plant (TCAAP) site and to provide preliminary geotechnical analyses and general opinions on the suitability of the subsurface conditions for the proposed development.

Project Background

TCAAP covers approximately 2,370 acres. The site is bounded by Lexington Avenue on the east, Interstate Highway 35W and US Highway 10 on the west, Highway 96 on the south, and County Road I on the north.

Prior to 1941, this site was used mainly for agricultural purposes. Construction of the TCAAP facility started in August of 1941 by Federal Cartridge Corporation. The facility operated between 1942 and the mid-1970s. Many of the buildings on this site are currently vacant and several of the original buildings have been completely or partially demolished.

In December of 2006, AET and Braun submitted a joint work plan to conduct a preliminary geotechnical evaluation for the proposed TCAAP redevelopment. The geotechnical evaluation was to be performed together with an environmental investigation lead by Tetra Tech EM, Inc. (Tetra Tech), for whom AET and Braun would also be providing drilling services. As such, three types of investigation sites were defined for this project: (a) environmental investigation sites, (b) geotechnical investigation sites, and (c) combined environmental and geotechnical investigation sites. This summary discusses data obtained from the geotechnical sites and combined environmental and geotechnical investigation sites. Tetra Tech will provide a summary of their environmental investigation.

In preparation for our field exploration, we reviewed available geotechnical publications/reports pertinent to the site and the general area around the site, well logs obtained through the Minnesota Department of Health and historical aerial photographs, and noted geotechnical-related site characteristics during site visits. The information gathered from those preparatory efforts helped determine an appropriate soil boring grid spacing for this preliminary geotechnical evaluation, as well as targeting locations believed to be of significance from a geotechnical perspective; for example, where review of historical aerial photographs indicated previous wetland or swampy areas prior to the construction of TCAAP.

Proposed Development

Of the 2,370 acres encompassed by TCAAP, approximately 580 acres is slated for future redevelopment. Actual development will be concentrated in a subset of about 385 acres of the available 580 acres. The proposed development will include retail structures, generally one- to two-stories in height, a corporate campus adjacent to Interstate Highway 35W in the northwestern portion of the site, which may include several multi-story structures, residential structures, recreation areas, such as ball fields and parks, and associated infrastructure and roadways. There will be a designated wildlife corridor in the northeastern portion of the redevelopment area and a designated wetland conservation area within the Rice Creek Watershed (north central portion of redevelopment area).

The development will be completed in several phases over the next 10 to 15 years. However, it is our understanding that you plan to "rough" grade the entire site prior to the first phase of development.

General Overview of Surficial Geology

Based on our review of the available geotechnical publications/reports that were made available to us prior to beginning our geotechnical investigation, we understood that the surface soils on this site generally consist of outwash and lacustrine sands, surface fill, with some areas of organic soils and wetlands. The upper soil layers, typically referred to in previous studies as "Unit 1," vary in thicknesses from about 10 to 20 feet. Below the upper soil layers in most areas of the site, there is a cohesive and relatively impervious clay till (Unit 2) that varies in thickness from about 20 to 70 feet. Older glacial outwash and valley fill materials (Unit 3) underlie the till, and extend to depths on the order of 100 to 400 feet, or to the top of bedrock. The depth to bedrock varies considerably across the site. The bedrock in this area generally consists of weathered and fractured dolomite of the Prairie du Chien Group overlying Jordan Sandstone.

Perched groundwater is present above the Unit 2 "aquitard," with hydrostatic groundwater present in the Unit 3 glacial outwash above the bedrock. The bedrock also acts as a separate aquifer.

Review of historic aerial photographs indicates that depressions and wetlands existed throughout the site before TCAAP was first constructed. Many of those depressions and wetlands had been filled during the original development of the TCAAP site. Some of the borings drilled for previous environmental studies indicated buried organic soils, which probably represent buried wetland areas.

Summary of Results

Summary of Borings

We drilled a total of 219 Standard Penetration test borings at an approximate nominal grid spacing of 500 lineal feet across the site; the grid spacing varied along the perimeter of the site and in areas believed to be of significance from a geotechnical perspective (e.g., previous wetland areas). The approximate boring locations are shown on Figure 1; Westwood Professional Services surveyed the as-drilled boring locations and shot the ground surface elevations at the borings. The planned depth of most borings was 25 feet. However, several of the borings in the southeastern portion of the site (near Building 502) were extended to depths of 40 feet or more based on planned cuts of more than 15 feet shown on the preliminary grading plan. In addition, two borings in the northwestern portion of the site were drilled to a depth of about 100 feet; this is an area where heavily-loaded buildings are anticipated (i.e., multi-story corporate campus office buildings). We backfilled all borings with bentonite grout.

Summary of Soils Encountered

The native mineral soils that we found in our borings were generally consistent with the soils identified by previous investigations on this site. As indicated on the attached Subsurface Boring Log/Log of Boring sheets, we generally found varying thicknesses of topsoil, organic deposits and existing fill, overlying glacial and alluvial deposits consisting predominately of poorly graded sand, silty sand, clayey sand, lean clay, and, to a lesser extent, local deposits of sandy silt and silt. As anticipated, we did not encounter bedrock in any of our borings.

Table 1 provides a boring-by-boring tabulation of the depths and corresponding bottom elevations of topsoil, organic soils (swamp deposits), existing fill, and soft soils encountered in our borings. The topsoil, organic soils and existing fill have been grouped into a general category called "unsuitable soils" on Table 1, representing the greatest depth of topsoil, organic soil, existing fill, and/or soft soils. The estimated depths to unsuitable soils and corresponding bottom elevations are also presented in plan view on Figure 2, including contours of the estimated bottom elevations.

Groundwater Summary

Our drillers checked for groundwater in the boreholes as the borings were advanced. Based on our observations during and after drilling, the measured moisture content and apparent moisture condition of samples we collected, and soil properties such as color, we developed the groundwater elevation summary presented in Table 1 and on Figure 3.

Summary of Preliminary Geotechnical Recommendations

Building Areas—Spread Footings

The topsoil, swamp deposits, and soft clay are compressible and, in our opinion, are unsuitable for support of the proposed building structures. The existing fill overlying buried organic soils or existing fill containing organic material and debris are also considered to be unsuitable for support of the proposed buildings. For purposes of this preliminary evaluation, we assume that the unsuitable soils summarized in Table 1 will have to be removed from building areas and replaced with suitable, compacted backfill. More detailed supplemental investigations and evaluations will be required to further assess the horizontal and vertical extent of unsuitable soils on this site. As mentioned previously, Table 1 and Figure 2 present approximate excavation depths to remove unsuitable soils, along with corresponding bottom elevations of the unsuitable soils.

Please note that pending the results of further supplemental evaluations performed after building locations have been determined, it is possible that some of the existing fill might be able to be left in place below buildings, depending on the condition of the existing fill and the type of structure.

After necessary earthwork corrections, we estimate buildings on this site can generally be supported on typical concrete spread footings sized for the net allowable bearing capacities presented in Table 2 below.

Table 2. Range of Net Allowable Bearing Capacities

Soil Type	Soil Classification	Typical Range of Net Allowable Bearing Pressures (psf)	Typical Ground Improvement to Achieve Higher Limit of Bearing Capacities
Granular Glacial Deposits	SM, SP, SP-SM	2,000 to 4,000	Surface compaction of footing subgrades
Cohesive Glacial Deposits	SC, CL	2,000 to 3,000	Local subexcavation and replacement with aggregate
Granular Lacustrine Deposits	SM, SP, SP-SM	2,000 to 4,000	Surface compaction of footing subgrades
Cohesive Lacustrine Deposits	CL, ML	2,000 to 2,500	Local subexcavation and replacement with aggregate

Supplemental field explorations with pressuremeter testing and/or CPT soundings could be used to evaluate the feasibility of allowable bearing capacities higher than those summarized above.

Building Areas—Pile Supported

In the northwestern portion of the site, where heavily loaded office buildings are planned, it might be economical to support those buildings on deep foundations, depending on the structural loads and settlement tolerances of those buildings. Assuming that deep foundations would consist of commonly available 9 5/8-inch or 12 3/4-inch driven steel pipe pile extended to depths on the order of about 90 to 100 feet, we estimate that working capacities ranging from about 50 to 100 tons could be achieved in the northwestern portion of the site, based on data collected from our soil borings.

Parking and Roadway Areas

Swamp Deposits and Topsoil

It is our opinion that the swamp deposits encountered on this site are generally unsuitable for direct support of pavements. We recommend removing the organic soils from the upper 5 to 10 feet of pavement subgrades, and replacing them with suitable, compacted backfill. Provided that the surface vegetation and heavy root zone are removed, it is likely that low-organic topsoil materials could be left in place at depths of 3 feet or more below pavement subgrade elevations.

Existing Fill and Native Mineral Soils

Based on the results of our Standard Penetration tests, the existing fill and native mineral soils are, in our opinion, generally suitable for support of parking lots and roadways provided that pavement subgrades are adequately improved prior to placing pavement materials. We estimate that subgrade improvement methods would range from surface compaction of loose granular soils to subexcavation-and-replacement of soft clayey/silty soils to depths of 2 to 3 feet below proposed pavement subgrade elevations.

Preliminary Recommended Pavement Sections

For preliminary planning purposes, we estimate that regular-duty pavement sections, those supporting typical automobile traffic, would typically consist of 3 1/2 to 4 1/2 inches of bituminous over about 6 to 8 inches of aggregate base course. For heavy-duty pavement areas, we estimate that pavement sections will typically consist of 4 to 5 inches of bituminous over 8 to 12 inches of aggregate base course.

For concrete pavements placed over at least 6 inches of aggregate base course, concrete thicknesses will likely range from about 5 inches for regular-duty pavements to 7 inches for heavy-duty pavements.

Utility Support

Swamp Deposits

To the extent possible, we recommend routing utilities around swamp deposits. In the event that utilities will run through highly organic swamp deposits, utilities might have to be supported on deep foundations or utility subgrades will have to be corrected (i.e., swamp deposits removed and replaced with new compacted fill), especially if the grade is raised over the utility alignment. In the areas of buried organic soils where the grade will not be raised along the utility alignment, it might be possible to "float" the utilities across the swamp deposits; this would require additional further evaluation. Please note that the organic soils should be considered corrosive to metallic pipe, and all utility pipe material subject to corrosion should be cathodically protected or wrapped with polyethylene sleeves.

Existing Fill and Native Mineral Soils

In our opinion, the existing fill and native mineral soils are generally suitable for support of utilities (sanitary sewer, watermain and storm water pipes and associated manholes). Where the utility subgrade consists of clay or silt, granular pipe bedding could be required depending on the size and type of pipe. For larger utility structures, such as lift stations and pump houses, we recommend further geotechnical evaluation on a structure-by-structure basis.

Reuse of On-site Materials and Compaction

Please note that comments on the reuse of on-site materials as fill are based on geotechnical considerations only. Environmental conditions could also dictate the reuse of on-site materials, as on-site material planned for reuse as fill will be evaluated with respect to plans approved by the United States Environmental Protection Agency (EPA) and the Minnesota Pollution Control Agency (MPCA).

Building Areas

Materials having an organic content of no more than 3 percent are generally considered suitable for reuse as structural fill and backfill in building areas. You should anticipate that buildings exerting higher bearing pressures will require the use of relatively clean sand as fill below footings.

You should anticipate that most on-site borrow material will have to be moisture conditioned to near-optimum levels prior to compaction. Please note that soils classified as clayey sand, lean clay, silty sand and silt, and some fine-grained clean sand, will be difficult to compact if wet.

Pavement Areas

It is our opinion that you could consider placing soils with organic contents between 3 and 7 percent at depths greater than 3 feet below pavement subgrades as long as the overall thickness of this slightly organic material is limited to less than about 4 to 8 feet.

Within the upper 3 feet of pavement subgrades, the soils should consist of mineral soils with an organic content less than 3 percent.

Recycled Material

If you consider using demolition spoils as recycled aggregate base course or as haul road materials, we recommend that the recycled materials consist of crushed concrete and bituminous. We recommend that the use of recycled aggregate containing bituminous be limited to pavement areas only.

Dewatering

Based on the conditions we found in our borings, construction dewatering will be required during the excavation of unsuitable soils in building pad areas, and during excavation of deeper underground utilities such as sanitary sewer. It is our opinion that pumping from open sump pits would be effective as the dewatering method where the excavation terminates in clayey soils. Where the excavation terminates in granular soils, the contractor will likely have to use well points and/or wells, plus pumping from screened sumps at the excavation base ("mop-up sumps").

Environmental conditions could dictate the discharge and/or disposal of pumped groundwater, and all dewatering plans should be evaluated with respect to plans approved by the EPA and the MPCA.

Remarks

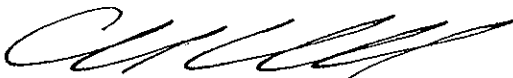
This report is for the exclusive use of the parties to which it has been addressed. Without written approval, we assume no responsibility to other parties regarding this report. Our evaluation, analyses and recommendations may not be appropriate for other parties or projects.

The conclusions contained in this report represent our professional opinions. American Engineering and Testing, Inc., and Braun Intertec Corporation endeavored to perform the engineering services for this project in a manner consistent with that degree of care and skill ordinarily exercised under similar circumstances by reputable members of its profession currently practicing in the same locality under similar budgetary and time constraints. No warranty, express or implied, is made.

If you have any questions regarding this report, please contact Tom Venema at 952.361.3781 or Bob Janssen at 651.487.7017.


Sincerely,

Prepared By:



Chad A. Underwood, PE, PG
American Engineering and Testing, Inc.
Senior Geotechnical Engineer

Prepared By:



Joel C. Kurpius, PE
Braun Intertec Corporation
Project Engineer

Reviewed By:



Thomas P. Venema, PE
American Engineering and Testing, Inc.
Principal Engineer

Reviewed By:



Robert J. Janssen, PE
Braun Intertec Corporation
Principal Engineer

Attachments:

- Table 1
- Figure 1
- Figure 2
- Figure 3
- Subsurface Boring Log/Log of Boring Sheets

Twin Cities Army Ammunition Plant (TCAAP) Redevelopment
Northeast of Highway 96 and US Highway 10
Arden Hills, Minnesota

Table 1

Boring Number	Surface Elevation	Topsoil / Organic Soils		Existing Fill		Soft Clays and Silts		Unsuitable Soil		Groundwater			
		Approximate Depth (ft)	Estimated Bottom Elevation	Approximate Depth (ft)	Estimated Bottom Elevation	Approximate Depth (ft)	Estimated Bottom Elevation	Approximate Depth (ft)	Estimated Bottom Elevation	Measured Depth (ft)	Measured Groundwater Elevation	Estimated Depth (ft)	Estimated Elevation
ST-1	908.1	0.5	907					0.5	907	23.0	879	23.0	879
ST-2	901.3	0.5	900					0.5	900				
ST-3	902.4	0.5	901					0.5	901				
ST-11	901.0	0.5	900					0.5	900	24.4	877	24.4	877
ST-19	898.1	1	897					1.0	897	18.0	881	18.0	881
ST-20	900.8	0.5	900					0.5	900	22.0	879	22.0	879
ST-21	900.6	0.5	900					0.5	900	24.0	877	24.0	877
ST-25	895.9	1	894					1.0	894	17.0	879	17.0	879
ST-26	896.2	0.5	895					0.5	895	19.0	878	19.0	878
ST-27	893.3	1	892					1.0	892	12.0	882	12.0	882
ST-31	892.9	0.5	892					0.5	892	11.0	882	11.0	882
ST-32	893.5	1	892					1.0	892	13.0	881	13.0	881
ST-33	895.7	1	894					1.0	894	14.0	882	14.0	882
ST-34	894.3	1	893					1.0	893	14.0	881	14.0	881
ST-35	896.0	0.5	895					0.5	895	16.0	881	16.0	881
ST-40	893.7	0.5	893					0.5	893	13.0	881	13.0	881
ST-41	892.5	0.7	891					0.7	891	12.0	881	12.0	881

Twin Cities Army Ammunition Plant (TCAAP) Redevelopment
Northeast of Highway 96 and US Highway 10
Arden Hills, Minnesota

Table 1

Boring Number	Surface Elevation	Topsoil / Organic Soils		Existing Fill		Soft Clays and Silts		Unsuitable Soil		Groundwater			
		Approximate Depth (ft)	Estimated Bottom Elevation	Approximate Depth (ft)	Estimated Bottom Elevation	Approximate Depth (ft)	Estimated Bottom Elevation	Approximate Depth (ft)	Estimated Bottom Elevation	Measured Depth (ft)	Measured Groundwater Elevation	Estimated Depth (ft)	Estimated Elevation
ST-42	885.3	11.5	873	9	876			11.5	873	10.1	876	10.1	876
ST-43	890.2			4	886			4.0	886	6.8	884	7.0	884
ST-44	888.6			2	886			2.0	886			7.0	882
ST-45	892.4			8.5	883			8.5	883	6.5	886	7.0	886
ST-46	895.4	22	873	12	883			22.0	873	11.0	885	11.0	885
ST-47	893.3			4	889			4.0	889	14.4	879	11.5	882
ST-48	890.9			1.5	889			1.5	889	11.0	880	11.0	880
ST-50	882.8	12	870	7	875			12.0	870	11.0	872	11.0	872
ST-51	881.2	12	869	7	874			12.0	869	9.0	873	9.0	873
ST-52	885.9			12	873			12.0	873	10.9	876	10.9	876
ST-53	885.5			7	878			7.0	878	10.3	876	10.3	876
ST-54	888.9			4.5	884	6	882	6.0	882	0.0	889	4.5	885
ST-55	892.4			2	890			2.0	890	0.0	893	9.5	883
ST-57	893.6	23.5	870	22	871			23.5	870	22.5	872	22.0	872
ST-59	884.4	12	872	11	873			12.0	872	12.1	873	12.1	873
ST-60	883.9	16.5	867	14	869			16.5	867	8.0	876	8.0	876
ST-61	885.9	8	877	7	878			8.0	877	9.4	877	9.4	877
ST-62	884.3	14	870	12	872			14.0	870	12.0	873	12.0	873
ST-63	886.1			9	877			9.0	877	14.0	873	14.0	873
ST-64	883.5	10.5	872	9	874			10.5	872	10.2	874	10.2	874
ST-65	886.2			2.5	883			2.5	883	10.0	877	10.0	877
ST-66	888.9			2	886			2.0	886	10.0	879	10.0	879
ST-67	888.7			3	885			3.0	885	6.0	883	6.0	883
ST-68	895.0	9	885	7	887			9.0	885	5.2	890	5.2	890
ST-69	894.8			1	893			1.0	893	4.0	891	4.0	891
ST-70	885.0	1.5	883					1.5	883	7.0	878	7.0	878
ST-71	887.3			1.5	885			1.5	885	7.0	881	7.0	881
ST-72	887.4			7	880			7.0	880	4.8	883	4.8	883
ST-73	891.3	6.5	884	4	887			6.5	884	9.0	883	9.0	883
ST-74	891.8			5	886			5.0	886	6.4	886	6.4	886
ST-75	898.8			4	894			4.0	894	4.3	895	4.3	895
ST-76	907.0			7	899			7.0	899			14.0	893
ST-77	892.4			7	885			7.0	885	11.0	882	11.0	882
ST-78	886.1	14	872	7	879			14.0	872	14.0	873	7.0	880
ST-79	888.9	0.5	888					0.5	888	7.0	882	7.0	882
ST-80	891.1			2	889			2.0	889			12.0	880
ST-81	895.1	1	894					1.0	894			9.5	886
ST-82	898.6	8.5	890	7	891			8.5	890			14.5	885
ST-83	903.0			4.5	898			4.5	898			9.5	894
ST-84	911.0			4.5	906			4.5	906			12.0	900
ST-85	892.1			2	890	7	885	7.0	885			9.5	883

Twin Cities Army Ammunition Plant (TCAAP) Redevelopment
Northeast of Highway 96 and US Highway 10
Arden Hills, Minnesota

Table 1

Boring Number	Surface Elevation	Topsoil / Organic Soils		Existing Fill		Soft Clays and Silts		Unsuitable Soil		Groundwater			
		Approximate Depth (ft)	Estimated Bottom Elevation	Approximate Depth (ft)	Estimated Bottom Elevation	Approximate Depth (ft)	Estimated Bottom Elevation	Approximate Depth (ft)	Estimated Bottom Elevation	Measured Depth (ft)	Measured Groundwater Elevation	Estimated Depth (ft)	Estimated Elevation
ST-86	897.0	0.5	896					0.5	896	8.1	889	8.1	889
ST-87	898.3			2	896			2.0	896			12.0	887
ST-88	889.7			6	883			6.0	883	8.0	882	8.0	882
ST-89	890.4			7	883			7.0	883	7.0	884	7.0	884
ST-90	891.0	2	888					2.0	888	6.0	885	6.0	885
ST-91	893.1	13.5	879	8.5	884			13.5	879	12.0	882	12.0	882
ST-92	898.4	7	891	2	896			7.0	891			18.0	881
ST-93	901.2			3.5	897			3.5	897			18.0	884
ST-94	922.9	3	919					3.0	919			18.0	905
ST-95	924.0	9	914	7	916			9.0	914	13.0	911	13.0	911
ST-96	892.3	9	883	4	888			9.0	883	9.0	884	9.0	884
ST-97	891.0			2.5	888			2.5	888	6.1	885	6.1	885
ST-98	892.3	9	883	7	885			9.0	883	8.0	885	8.0	885
ST-99	892.8	18	874	12	880	22	870	22.0	870	12.0	881	12.0	881
ST-100	895.7	9	886	6	889			9.0	886	9.0	887	9.0	887
ST-101	897.9	7	890	4.5	893	11	886	11.0	886	10.4	888	7.0	891
ST-102	902.2			7	895			7.0	895			7.0	896
ST-103	924.9			4	920			4.0	920			18.0	907
ST-104	934.9	12	922	7	927			12.0	922			23.0	912
ST-105	899.1			7	892			7.0	892	9.0	891	9.0	891
ST-106	903.4	7	896	5	898			7.0	896	12.2	892	12.2	892
ST-107	902.7	1	901					1.0	901	12.1	891	12.1	891
ST-108	906.6			4	902			4.0	902	12.5	895	12.5	895
ST-109	904.5			7	897			7.0	897			12.0	893
ST-110	913.2	1	912					1.0	912			9.0	905
ST-111	922.1			4	918			4.0	918			14.0	909
ST-112	936.8			1	935			1.0	935				
ST-113	941.8	2.3	939					2.3	939				
ST-114	913.1	4.5	908	2.5	910	5	908	5.0	908	5.2	908	5.2	908
ST-115	908.2			4.5	903			4.5	903			12.0	897
ST-116	913.0			0.5	912			0.5	912			9.5	904
ST-117	914.4			2.5	911			2.5	911	3.6	911	9.5	905
ST-118	914.9	14	900					14.0	900	6.0	909	6.0	909
ST-119	929.7			9	920			9.0	920			22.0	908
ST-120	940.8	0.5	940					0.5	940				
ST-121	944.6			4	940	12	932	12.0	932				
ST-122	959.5			2	957	7	952	7.0	952				
ST-123	913.4			6	907			6.0	907	25.0	889	25.0	889
ST-124	923.8	14	909	9	914			14.0	909			14.0	910
ST-125	932.7			9.5	923			9.5	923				
ST-126	957.0	12	944	7	949			12.0	944	12.0	945	12.0	945

Twin Cities Army Ammunition Plant (TCAAP) Redevelopment
Northeast of Highway 96 and US Highway 10
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Table 1

Boring Number	Surface Elevation	Topsoil / Organic Soils		Existing Fill		Soft Clays and Silts		Unsuitable Soil		Groundwater			
		Approximate Depth (ft)	Estimated Bottom Elevation	Approximate Depth (ft)	Estimated Bottom Elevation	Approximate Depth (ft)	Estimated Bottom Elevation	Approximate Depth (ft)	Estimated Bottom Elevation	Measured Depth (ft)	Measured Groundwater Elevation	Estimated Depth (ft)	Estimated Elevation
ST-127	952.8			6	946	12	940	12.0	940			18.0	935
ST-128	954.1			7	947	12	942	12.0	942				
ST-129	915.9			4.5	911			4.5	911			11.5	905
ST-130	915.1			7	908			7.0	908			14.0	902
ST-131	926.9			9.5	917			9.5	917			18.0	909
ST-132	936.0			3	933			3.0	933				
ST-133	945.0			12	932	18	926	18.0	926				
ST-134	949.0			7	942	12	937	12.0	937	24.0	926	24.0	926
ST-135	950.5			4.5	946			4.5	946				
ST-136	956.7	9	947	7	949			9.0	947				
ST-137	956.0	9	947	7	949			9.0	947				
ST-138	955.9	22	933	14.5	941	26	929	26.0	929	24.3	932	14.5	942
ST-139	951.8	12	939	9.5	942	14.5	937	14.5	937	9.6	943	9.5	943
ST-140	962.9	1	961					1.0	961				
ST-141	913.3			7	906			7.0	906	26.5	887	18.0	896
ST-142	933.7	19.5	914	15	918			19.5	914	23.7	910	15.0	919
ST-143	949.9			4	945			4.0	945				
ST-144	955.0			4.5	950			4.5	950				
ST-145	954.7			0.5	954			0.5	954				
ST-146	966.0			0.5	965	9	957	9.0	957				
ST-147	958.6	0.5	958					0.5	958				
ST-148	957.9	16	941	6	951			16.0	941	7.3	951	7.3	951
ST-149	990.9			22	968			22.0	968	19.0	972	19.0	972
ST-150	927.0	7	919	4.5	922			7.0	919				
ST-151	922.3			2	920			2.0	920	5.1	918	5.1	918
ST-152	942.0			12	929			12.0	929	12.3	930	12.3	930
ST-153	951.2			4.5	946	7	944	7.0	944			23.0	929
ST-154	956.7			16	940			16.0	940				
ST-155	959.9							0.0					
ST-156	959.8							0.0					
ST-157	995.9			14	981			14.0	981				
ST-158	955.5			21.5	934			21.5	934			22.0	934
ST-159	959.4			25	934			25.0	934	18.0	942	18.0	942
ST-160	920.2	8.5	911	7	913	12	908	12.0	908	11.8	909	11.8	909
ST-161	926.7			4.5	922			4.5	922			18.0	909
ST-162	956.7			1	955			1.0	955				
ST-163	949.2	17	932	14.5	934			17.0	932			17.0	933
ST-164	951.0			2	949			2.0	949			23.0	929
ST-165	953.1			2	951			2.0	951				
ST-166	955.0			2	952			2.0	952	21.2	934	21.2	934
ST-167	945.0			4.5	940	12	932	12.0	932	21.2	924	21.2	924

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Northeast of Highway 96 and US Highway 10
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		Approximate Depth (ft)	Estimated Bottom Elevation	Approximate Depth (ft)	Estimated Bottom Elevation	Approximate Depth (ft)	Estimated Bottom Elevation	Approximate Depth (ft)	Estimated Bottom Elevation	Measured Depth (ft)	Measured Groundwater Elevation	Estimated Depth (ft)	Estimated Elevation
ST-168	947.2			3	944			3.0	944				
ST-169	948.1	12.5	935	10.5	937			12.5	935			12.5	936
ST-170	949.7	12	937	10.5	939			12.0	937	12.6	938	12.6	938
ST-171	915.0	12	903	9.5	905			12.0	903	8.6	907	8.6	907
ST-172	925.1	17	908	7	918	24	901	24.0	901	6.0	920	6.0	920
ST-173	942.9	1	941					1.0	941				943
ST-174	944.9			4	940			4.0	940				945
ST-175	948.1			3	945			3.0	945	15.1	934	15.1	934
ST-176	951.0	12.5	938	10	941			12.5	938			16.0	936
ST-177	953.6	14.5	939	13	940			14.5	939				
ST-178	948.4	10	938	9.5	938			10.0	938			18.0	931
ST-179	929.3	0.5	928					0.5	928			14.0	916
ST-180	935.9	1	934					1.0	934			12.0	924
ST-181	935.0	1	933					1.0	933				
ST-182	941.8	0.5	941					0.5	941				
ST-183	933.7	0.5	933					0.5	933	18.0	916	18.0	916
ST-184	942.1			4.5	937			4.5	937			12.0	931
ST-185	901.0							0.0	900				
ST-186	925.2	9	916	7	918			9.0	916			14.0	912
ST-187	914.2	1	913					1.0	913			12.0	903
ST-188	934.5			4	930			4.0	930	19.0	916	19.0	916
ST-189	952.2	0.7	951					0.7	951				
ST-190	952.8			2	950			2.0	950				
ST-191	956.3			4	952			4.0	952				
ST-192	959.1							0.0	959				
ST-193	961.9			4	957			4.0	957				
ST-195	942.2			4.5	937	9	933	9.0	933				
ST-196	943.4			7	936	12	931	12.0	931	18.0	926	18.0	926
ST-197	944.3	9	935	7	937	18	926	18.0	926	9.0	936	9.0	936
ST-198	944.4	19	925	14	930	22	922	22.0	922			19.0	926
ST-199	944.2			7	937			7.0	937				
ST-200	949.2			14	935	22	927	22.0	927	20.0	930	20.0	930
ST-201	956.8			4.5	952			4.5	952				
ST-202	955.0			2.5	952			2.5	952				
ST-203	954.4			4.5	949			4.5	949				
ST-204	938.6			18.0	920			18.0	920				
ST-205	937.1			4.5	932			4.5	932			12.0	905
ST-206	916.0			4.0	912			4.0	912			9.0	900
ST-207	908.5			4.5	904			4.5	904			12.0	903
ST-208	914.5			4.5	909			4.5	909				
ST-209	892.8			7.0	885			7.0	885	9.1	884	9.1	884

Twin Cities Army Ammunition Plant (TCAAP) Redevelopment
Northeast of Highway 96 and US Highway 10
Arden Hills, Minnesota

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		Approximate Depth (ft)	Estimated Bottom Elevation	Approximate Depth (ft)	Estimated Bottom Elevation	Approximate Depth (ft)	Estimated Bottom Elevation	Approximate Depth (ft)	Estimated Bottom Elevation	Measured Depth (ft)	Measured Groundwater Elevation	Estimated Depth (ft)	Estimated Elevation
ST-210	898.8	6.0	892	4.5	894			6.0	892			12.0	887
ST-211	892.3			8.5	883			8.5	883	7.6	885	7.6	885
ST-212	895.1	13.0	882	9.5	885			13.0	882	7.3	888	7.3	888
ST-213	890.2		890	5.5	884			5.5	884	7.1	884	7.1	884
ST-214	884.9		884	9.5	875			9.5	875	12.7	873	12.7	873
ST-215	894.5	11.0	883	9.0	885			11.0	883	9.3	886	9.3	886
ST-216	895.0	13.5	881	11.5	883			13.5	881			13.5	882
ST-217	889.3	1.0	888		889			1.0	888	3.3	887	3.0	887
ST-218	885.7		885	7.0	878			7.0	878			12.0	874
ST-219	891.3	1.0	890		891			1.0	890	15.0	877	15.0	877
ST-220	897.0	2.0	894		896			2.0	894	17.8	880	17.8	880
ST-221	899.0	2.0	897		899			2.0	897	18.3	881	18.3	881
ST-222	898.6		898	9.5	889			9.5	889	18.9	880	18.9	880
ST-223	908.4		908	9.5	898			9.5	898	28.9	880	28.9	880
ST-224	938.6	6.0	932	4.0	934			6.0	932				



125' 0 250'
SCALE: 1" = 250'

⊕ DENOTES APPROXIMATE LOCATION OF STANDARD PENETRATION TEST BORING

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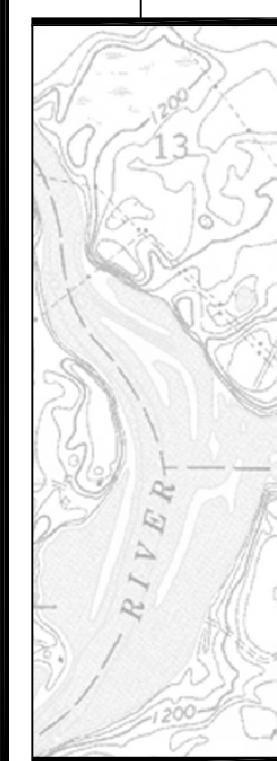
11001 Hampshire Avenue So.
Minneapolis, MN 55438
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FAX: (612) 955-2020

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Revisions

Date: Checked By:

Date	Checked By



Project No:
SP0605871

Drawing No:
SP0605871B

Scale: 1" = 250'

Drawn By: MRG

Date Drawn: 8/27/07

Checked By: JK

Last Modified: 10/3/07

SOIL BORING LOCATION SKETCH
GEOTECHNICAL EVALUATION
TCAP REDEVELOPMENT
NORTHEAST OF HIGHWAY 10 AND HIGHWAY 96
ARDEN HILLS, MINNESOTA

Sheet of Fig:



125' 0 250'
SCALE: 1" = 250'

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Date	Checked By



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Drawing No:
SP0605871B

Scale: 1" = 250'
Drawn By: MRG
Date Drawn: 8/27/07
Checked By: JK
Last Modified: 10/3/07

SOIL BORING LOCATION SKETCH
GEOTECHNICAL EVALUATION
TCAP REDEVELOPMENT
NORTHEAST OF HIGHWAY 10 AND HIGHWAY 96
ARDEN HILLS, MINNESOTA

Sheet of Fig:



SCALE: 1" = 250'

- 9 REMOTE APPROXIMATE LOCATION OF STANDARD PENETRATION TEST BORING
- XXI XX UNSUITABLE SOIL DEPTH / UNSUITABLE SOIL ELEVATION (FT.)
- UNSUITABLE SOIL ELEVATION CONTOUR (FT.)

DRAFT

Subject to Change



Professional Seal
SP0605871

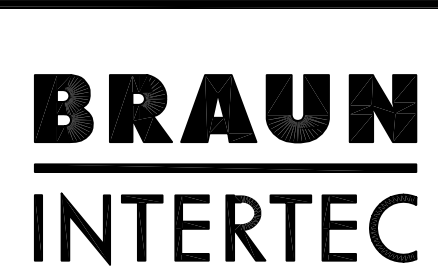
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 Scale: 1" = 250'
 Date: 10/5/2007
 Drawn By: JK
 Checked By: JK
 Last Modified: 10/3/07

SOIL BORING LOCATION AND UNSUITABLE SOIL DEPTH / ELEVATION / CONTOUR SKETCH
 GEOTECHNICAL EVALUATION
 TCAAP REDEVELOPMENT
 NORTHEAST OF HIGHWAY 10 AND HIGHWAY 96
 ARDEN HILLS, MINNESOTA

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SCALE: 1" = 20'

- 9 REMOTE APPROXIMATE LOCATION OF STANDARD PENETRATION TEST BORING
- XXI, XX GROUNDWATER DEPTH / GROUNDWATER ELEVATION (FT.)
- GROUNDWATER ELEVATION CONTOUR (FT.)

DRAFT

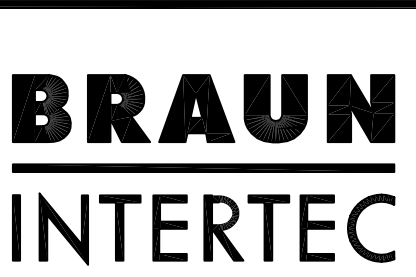
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SOIL BORING LOCATION AND GROUNDWATER DEPTH / ELEVATION / CONTOUR SKETCH
 GEOTECHNICAL EVALUATION
 TCAAP REDEVELOPMENT
 NORTHEAST OF HIGHWAY 10 AND HIGHWAY 96
 ARDEN HILLS, MINNESOTA

Sheet: 01
Fig: 01



SCALE: 1" = 250'

- 9 REMOTE APPROXIMATE LOCATION OF STANDARD PENETRATION TEST BORING
- XXI, XX GROUNDWATER DEPTH / GROUNDWATER ELEVATION (FT.)
- GROUNDWATER ELEVATION CONTOUR (FT.)

DRAFT

Subject to Change



SOIL BORING LOCATION AND GROUNDWATER DEPTH / ELEVATION / CONTOUR SKETCH
 GEOTECHNICAL EVALUATION
 TCAAP REDEVELOPMENT
 NORTHEAST OF HIGHWAY 10 AND HIGHWAY 96
 ARDEN HILLS, MINNESOTA

Drawing No: SP0605871B
 Scale: 1" = 250'
 Date Drawn: 8/27/07
 Checked By: JK
 Last Modified: 10/3/07



Revisions	Date	Checked By:

11011 Hampshire Avenue So.
 Minneapolis, MN 55438
 Fax: (952) 995-2020
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INTERTEC

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-1 LOCATION: N: 213690.577, E: 550665.094 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/19/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
908.1	0.0					
907.6	0.5	SM	SILTY SAND, trace of Roots, dark brown, wet. (Topsoil)			
		SP-SM	POORLY GRADED SAND with SILT, fine-grained, brown to light brown, moist, loose. (Lacustrine)	4		
				3		
				5		
				5		
896.1	12.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown, loose to medium dense. (Glaciofluvium)	6		
				13		
890.1	18.0	ML	SANDY SILT, Sand seams, brown, wet, medium dense. (Glaciofluvium)			
				19		
886.1	22.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, medium dense. (Glaciofluvium)			
882.1	26.0		END OF BORING.	15		
			Water not observed with 24 feet of hollow-stem auger in the ground.			
			Boring then grouted.			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-2 LOCATION: N: 213683.523, E: 550898.513 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/22/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPI BRAUN.GDT 10/2/07 14:44 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
901.3	0.0					
900.8	0.5	SM	SILTY SAND, trace of Roots, dark brown, moist. (Topsoil)			
		SM	SILTY SAND, fine-grained, reddish-brown to brown, moist, loose to medium dense. (Glaciofluvium)	8		
				16		
				13		
892.3	9.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown, moist, loose to medium dense. (Glaciofluvium)	11		
				9		
				7		
				14		
878.3	23.0	SM	SILTY SAND, fine-grained, gray, waterbearing.		▽	
875.3	26.0			21		
			END OF BORING. Water observed at 23 feet while drilling. Boring then grouted.			



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-3 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>902.4</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	SILTY SAND, trace roots, dark brown, moist, medium dense (SM)	TOPSOIL COARSE ALLUVIUM	11	M	SS	17					
2	SAND WITH SILT, fine grained, trace roots, brown, moist, medium dense (SP-SM)										
3	SAND WITH SILT, fine grained, brown, moist, loose (SP-SM)			9	M	SS	14				
4	SILTY SAND, fine grained, brown, light grayish brown, moist, medium dense, laminations of sand with silt (SM)										
5				13	M	SS	19				
6											
7											
8					16	M	SS	19			
9	SAND WITH SILT, fine grained, light grayish brown, a little brown, moist, medium dense, laminations of silty sand (SP-SM)										
10				15	M	SS	19				
11											
12	SAND WITH SILT, fine grained, light brownish gray, a little brown, moist, loose, laminations of silty sand (SP-SM)										
13				10	M	SS	17				
14	SILTY SAND, fine grained, light brownish gray, moist, loose (SM)										
15				7	M	SS	18				
16											
17											
18	SAND WITH SILT, fine grained, light brownish gray, a little brown, moist, dense, laminations of silty sand (SP-SM)										
19											
20											
21					34	M	SS	17			
22	SILT, gray, wet, dense (ML)										
23											
24											
25											
26		FINE ALLUVIUM	32	M/W	SS	16	25				
END OF BORING Northing=213682.3 Easting=551397.5											

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-24 1/2'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		6/20/07	2:15	26.5	24.5	24.6		None	
BORING COMPLETED: 6/20/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-11 (p. 1 of 4)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>901.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	SILTY SAND, surface roots, trace roots, dark brown, moist, loose (SM)	TOPSOIL COARSE ALLUVIUM	8	M	SS	14					
2	SAND WITH SILT, trace roots, light brown, moist, loose (SP-SM)										
3	SILTY SAND, fine grained, brown, a little light brown, moist, medium dense, laminations of sand with silt (SM)		15	M	SS	18					
4											
5											
6			20	M	SS	18					
7											
8	SAND WITH SILT, fine grained, light grayish brown to light brownish gray, moist, medium dense to loose (SP-SM)		15	M	SS	16					
9											
10											
11			11	M	SS	17					
12											
13											
14			6	M	SS	17					
15	SAND WITH SILT, fine grained, light brownish gray, a little brown, moist, medium dense, laminations of sandy silt (SP-SM)		18	M	SS	20					
16											
17											
18	SAND WITH SILT, fine grained, light gray, a little gray, moist, dense, laminations of silt (SP-SM)										
19											
20											
21			33	M	SS	17					
22											
23											
24	SAND WITH SILT, fine grained, light brownish gray, waterbearing, medium dense (SP-SM)										
25											
26			16	M/W	SS	19					
27											
28		TILL									

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-24½'	3.25" HSA								
24½'-99½'	RD w/DM	6/20/07	8:55	26.5	24.5	24.5		24.4	
BORING COMPLETED: 6/20/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-11 (p. 2 of 4)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS							
							WC	DEN	LL	PL	%-#200			
30	SANDY LEAN CLAY, a little gravel, gray, firm to stiff (CL) <i>(continued)</i>	TILL <i>(continued)</i>												
31			6	M	SS	24	17							
32														
33														
34														
35														
36					7	M	SS	24	17					
37														
38														
39														
40														
41					10	M	SS	24	20					
42														
43														
44														
45														
46			10	M	SS	24	17							
47														
48														
49														
50														
51			11	M	SS	24	19							
52														
53														
54														
55														
56			10	M	SS	24	20							
57														
58														
59														
60														
61			10	M	SS	22	18							
62														
63														



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-11 (p. 3 of 4)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
64	SANDY LEAN CLAY, a little gravel, gray, firm to stiff (CL) <i>(continued)</i>	TILL <i>(continued)</i>	5	M	SS	24	19					
65												
66												
67												
68	SILTY SAND, fine to medium grained, brownish gray, wet, loose (SM)	COARSE ALLUVIUM	5	M	SS	2	23					
69												
70												
71												
72												
73	LEAN CLAY WITH SAND, brown, hard (CL)	FINE ALLUVIUM	36	M	SS	24	17					
74												
75												
76												
77												
78	FAT CLAY, brown, hard to very stiff, laminations of silty sand (CH)		43	M	SS	24	33					
79												
80												
81												
82												
83												
84												
85												
86			27	M	SS	24	25					
87												
88	CLAYEY SAND, a little gravel, possible cobbles, brown, hard (SC)	TILL	62	M	SS	21	13					
89												
90												
91												
92												
93												
94												
95												
96			70	M	SS	21	11					
97												



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-11 (p. 4 of 4)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
98 -	CLAYEY SAND, a little gravel, possible cobbles, brown, hard (SC) <i>(continued)</i>	TILL <i>(continued)</i>										
99 -												
100 -												
101 -			98	M		SS	26	10				
	END OF BORING Northing=213491.7 Easting=551397.2											

INTERTEC

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-19 LOCATION: N: 213183.570, E: 550706.966 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/19/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:44 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
898.1	0.0							
897.1	1.0	SM	SILTY SAND, trace of Roots, dark brown, moist. (Topsoil)					
		SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown, moist. (Lacustrine)	9				
894.1	4.0	SM	SILTY SAND, fine-grained, brown, wet, medium dense. (Lacustrine)	11				
891.1	7.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown, moist, loose to medium dense. (Lacustrine)	11		6	6	
				9				
884.1	14.0	SM	SILTY SAND, light brown, moist, medium dense. (Glacial Outwash)	6				
				15				
880.1	18.0	SM	SILTY SAND, fine-grained, light brown, to gray, waterbearing, medium dense to very dense. (Glacial Outwash)		▽			
				35				
872.1	26.0			60				
			END OF BORING.					
			Water observed at 18 feet while drilling.					
			Boring then grouted.					

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-20 LOCATION: N: 213171.742, E: 551046.195 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/21/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:44 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
900.8	0.0							
900.3	0.5	SM	SILTY SAND, trace of Roots, dark brown, moist. (Topsoil)					
		SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown, rust at 2' sample depth, moist, loose to medium dense. (Lacustrine)	15				
				10				
				8				
891.8	9.0	ML	SANDY SILT, light brown, wet, loose. (Lacustrine)	6				
				9		17	62	
886.8	14.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown, moist, medium dense. (Lacustrine)	28		4	7	
882.8	18.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown, moist, dense. (Lacustrine)	34				
878.8	22.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, gray, waterbearing, loose. (Lacustrine)		▽			
874.8	26.0		END OF BORING. Water observed at 22 feet while drilling. Boring then grouted.	10				

INTERTEC

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-21 LOCATION: N: 213183.350, E: 553196.596 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/21/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:44 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
900.6	0.0					
900.1	0.5	SM	SILTY SAND, trace of Roots, brown, moist. (Topsoil)			
		SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown, moist, very loose to medium dense. (Lacustrine)	7		
				13		
				8		
				6		
				4		
886.6	14.0	SM	SILTY SAND, fine-grained, light brown, moist, loose. (Lacustrine)	9		
				10		
876.6	24.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, brownish-gray, waterbearing, medium dense. (Lacustrine)		▽	
874.6	26.0		END OF BORING.	11		
			Water observed at 24 feet while drilling.			
			Boring then grouted.			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota				BORING: ST-25				
DRILLER: K. Keck		METHOD: 3 1/4" HSA, Autohmr		DATE: 6/19/07		SCALE: 1" = 4'		
Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
895.9	0.0							
894.9	1.0	SM	SILTY SAND, trace of Roots, dark brown, moist. (Topsoil)					
		SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown, moist, loose.	8				
				6				
				8				
				7				
883.9	12.0	ML	SANDY SILT, gray with bands of orangish-brown, wet, loose. (Lacustrine)	6				
881.9	14.0	SM	SILTY SAND, fine-grained, Silt laminations, brown to dark brown, wet, medium dense. (Glaciofluvium)	18				
878.9	17.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown, waterbearing, medium dense. (Glaciofluvium)	6				
873.9	22.0	SC	CLAYEY SAND, trace of Gravel, gray, wet, medium dense. (Glacial Till)					
869.9	26.0		END OF BORING. Water observed at 17 feet while drilling. Boring then grouted.	6		13	42	LL = 23% PI = 12%

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:44 (See Descriptive Terminology sheet for explanation of abbreviations)

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-26
	LOCATION: N: 212751.010, E: 551024.200 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/21/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
896.2	0.0					
895.7	0.5	SM	SILTY SAND, trace of Roots, dark brown, moist. (Topsoil)			
		SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown to brown, rust at 15' sample depth, moist, loose to medium dense. (Lacustrine)	10 12 12 10 12 11		
877.2	19.0	ML	SANDY SILT, gray, waterbearing, medium dense. (Glaciofluvium)	19	▽	
870.2	26.0		END OF BORING. Water observed at 19 feet while drilling. Boring then grouted.	15		

(See Descriptive Terminology sheet for explanation of abbreviations)

BRAUN BASIC LOG OF BORING: SP0605871.GPJ BRAUN.GDT 10/2/07 14:44

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-27 LOCATION: N: 212677.165, E: 551403.522 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/21/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPI BRAUN.GDT 10/2/07 14:44 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
893.3	0.0							
892.3	1.0	SM	SILTY SAND, very fine- to fine-grained, dark brown, moist. (Topsoil)					
		SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown, moist, very loose to medium dense. (Lacustrine)	4				
				9				
				10				
				11		4	5	
881.3	12.0				▽			
		SP-SM	POORLY GRADED SAND with SILT, fine-grained, brown to light brown, waterbearing, loose. (Lacustrine)	8				
				6		22	11	
876.3	17.0							
		ML	SILT with SAND, grayish-brown to gray, wet, loose to medium dense. (Glaciofluvium)	9				
				24		24	82	
866.3	27.0							
		SP-SM	POORLY GRADED SAND with SILT, fine-grained, gray, waterbearing, medium dense. (Glaciofluvium)	28				
861.3	32.0							

INTERTEC

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-27 (cont.) LOCATION: N: 212677.165, E: 551403.522 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/21/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:44 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
861.3	32.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, rather stiff. (Glacial Till)	9				
856.3	37.0	SC	CLAYEY SAND, trace of Gravel, gray, wet, rather stiff. (Glacial Till)	9		15	46	
850.3	43.0	CL	SANDY LEAN CLAY, trace of Gravel, wet, rather stiff. (Glacial Till)	9				
				10		18	51	
				9				
				9				

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-27 (cont.)
	LOCATION: N: 212677.165, E: 551403.522 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/21/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING: SP0605871.GPJ BRAUN.GDT: 10/2/07 14:44 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
829.3	64.0		SANDY LEAN CLAY, trace of Gravel, wet, rather stiff. (Glacial Till) <i>(continued)</i>	9				
				9				
				12				
816.3	77.0	CL	SANDY LEAN CLAY, trace of Gravel, reddish-brown, wet, very stiff to hard. (Glacial Till)	21				
				34		14	62	
				41				
801.3	92.0	CL	LEAN CLAY, reddish-brown to grayish-brown with laminations of brown, very stiff to hard. (Glaciofluvium)	22				

INTERTEC

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-27 (cont.) LOCATION: N: 212677.165, E: 551403.522 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/21/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
797.3	96.0		LEAN CLAY, reddish-brown to grayish-brown with laminations of brown, very stiff to hard. (Glaciofluvium) (continued)					
792.3	101.0		END OF BORING. Water observed at 12 feet while drilling. Boring then grouted.	30				

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:44 (See Descriptive Terminology sheet for explanation of abbreviations)

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-31
	LOCATION: N: 212415.495, E: 551003.074 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/21/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:45 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
892.9	0.0	SM	SILTY SAND, trace of Roots, dark brown, moist. (Topsoil)			
	0.5	SP-SM	POORLY GRADED SAND with SILT, fine-grained, brown, moist, medium dense. (Lacustrine)			
888.9	4.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, orange-brown to grayish-brown with rust at 7' sample depth, loose to medium dense. (Lacustrine)			
				11		
				9		
				15		
				12	▽	
880.9	12.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, grayish-brown, waterbearing, medium dense. (Glaciofluvium)			
				22		
				23		
				16		
869.9	23.0	ML	SANDY SILT, gray, wet, medium dense. (Glaciofluvium)			
866.9	26.0		END OF BORING.			
			Water observed at 11 feet while drilling.			
			Boring then grouted.			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-32 LOCATION: N: 212330.333, E: 551226.541 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/20/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:45 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
893.5	0.0							
892.5	1.0	SM	SILTY SAND, trace of Roots, dark brown, moist. (Topsoil)					
		SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown, moist, very loose to loose. (Lacustrine)	2				
				7		5	5	
886.5	7.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, orange-brown, moist to wet, loose to medium dense. (Lacustrine)	11				
				9				
881.5	12.0	SM	SILTY SAND, fine-grained, grayish-brown, waterbearing, very loose to medium dense. (Lacustrine)	6	▽			
				2		23	29	
				12				
870.5	23.0	CL-ML	SILTY CLAY, gray, wet, loose. (Glaciofluvium)					
867.5	26.0			9		23	96	LL = 27 PI = 6
			END OF BORING.					
			Water observed at 13 feet while drilling.					
			Boring then grouted.					

INTERTEC

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-33
	LOCATION: N: 212183.655, E: 550725.056 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/19/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:45 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
895.7	0.0							
894.7	1.0	SM	SILTY SAND, trace of Roots, dark brown, moist. (Topsoil)					
		SP-SM	POORLY GRADED SAND with SILT, fine-grained, brown to light brown, moist, very loose to loose. (Lacustrine)	4				
				6				
				7				
886.7	9.0	SM	SILTY SAND, fine-grained, brown to grayish-brown, moist, medium dense. (Glaciofluvium)	13				
				11				
881.7	14.0	SM	SILTY SAND, fine-grained, grayish-brown, waterbearing, medium dense. (Glaciofluvium)	14	▽			
877.7	18.0	ML	SANDY SILT, gray, wet, medium dense. (Glaciofluvium)	14		22	55	
873.7	22.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, gray, waterbearing, loose. (Glaciofluvium)	7				
869.7	26.0		END OF BORING.					
			Water observed at 14 feet while drilling.					
			Boring then grouted.					

INTERTEC

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-34 LOCATION: N: 212181.205, E: 550971629 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/21/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:45 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
894.3	0.0							
893.3	1.0	SM	SILTY SAND, dark brown, moist. (Topsoil)					
		SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown to brown, moist, loose to medium dense. (Lacustrine)	5				
				12				
885.3	9.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, orange-brown, moist to wet, medium dense. (Lacustrine)	11		11	6	
		SP-SM	POORLY GRADED SAND with SILT, fine-grained, grayish-brown, waterbearing, very loose. (Lacustrine)	11	▽			
880.3	14.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, grayish-brown, waterbearing, very loose. (Lacustrine)	4				
876.3	18.0	SM	SILTY SAND, fine-grained, grayish-brown, waterbearing, medium dense. (Lacustrine)	13				
868.3	26.0		END OF BORING.	13				
			Water observed at 14 feet while drilling.					
			Boring then grouted.					

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-35
	LOCATION: N: 212022.662, E: 551147.722 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/20/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
896.0	0.0					
895.5	0.5	SM	SILTY SAND, trace of Roots, dark brown, moist. (Topsoil)			
		SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown to brown, moist, very loose to loose. (Lacustrine)	3		
				7		
				10		
				10		
				8		
				8	▽	
879.0	17.0	SM	SILTY SAND, fine-grained, grayish-brown to gray, waterbearing, medium dense. (Glaciofluvium)	13		
				11		
870.0	26.0		END OF BORING.			
			Water observed at 16 feet while drilling.			
			Boring then grouted.			

BRAUN BASIC LOG OF BORING SP0605871.GPI BRAUN.GDT 10/2/07 14:45 (See Descriptive Terminology sheet for explanation of abbreviations)

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-40
	LOCATION: N: 211680.392, E: 550589.817 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/22/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:45 (See Descriptive Terminology sheet for explanation of abbreviations)

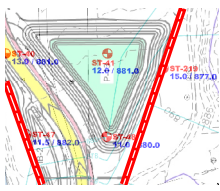
Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
893.7	0.0					
893.2	0.5	SM	SILTY SAND, trace of Roots, dark brown, moist. (Topsoil)			
		SM	SILTY SAND, fine-grained, dark brown, moist, very loose. (Lacustrine)	2		
889.7	4.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown to grayish-brown with rust at 10' sample depth, moist, very loose to medium dense. (Lacustrine)	3		
				8		Silty Sand 0' to 4' Poorly Graded Sand w Silt 4' to 26'
				9		
				11	▽	
879.7	14.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, grayish-brown to gray, waterbearing, loose to medium dense. (Lacustrine)	6		
				16		
				24		
867.7	26.0		END OF BORING.			
			Water observed at 13 feet while drilling.			
			Boring then grouted.			

Braun Project SP-06-05871		BORING: ST-41	
Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota		LOCATION: N: 211684.376. E: 550897.834 See attached sketch.	

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/21/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:45 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
892.5	0.0					
891.8	0.7	SM	SILTY SAND, trace of Roots, dark brown, moist. (Topsoil)			
		SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown to brown, moist, loose to medium dense. (Lacustrine)	15		
				8		Poorly Graded Sand w Silt 0.7' to 12' Silty Sand 12' to 22'
				12		
				12		
880.5	12.0	SM	SILTY SAND, fine-grained, brownish-gray, waterbearing, loose to medium dense. (Lacustrine)	10	▽	
				17		
				16		
870.5	22.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, gray, waterbearing, loose. (Lacustrine)			
866.5	26.0		END OF BORING.	7		
			Water observed at 12 feet while drilling. Boring then grouted.			





SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-42 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>885.3</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%#200
1	FILL, mixture of sandy silt and silty sand, with gravel, surface roots, trace roots, brown and dark brown	FILL	29	M	SS	14					
2											
3											
4											
5	FILL, mixture of sand with silt, silty sand and clayey sand, a little gravel, brown, a little brownish gray and gray		23	M	SS	8					
6											
7											
8											
9											
10	SAND WITH SILT, fine grained, brownish gray to gray and black, medium dense, lenses and laminations of organic silt (SP-SM)	COARSE ALLUVIUM OR TOPSOIL	17	M	SS	19					
11											
12	SAND, medium to fine grained, brown and gray, waterbearing, loose, lenses of sandy silt (SP)	COARSE ALLUVIUM	5	W	SS	4					
13											
14											
15	SANDY LEAN CLAY, a little gravel, gray, firm to stiff (CL)	TILL	7	M	SS	8	22				
16											
17											
18											
19											
20											
21	8	M	SS	15	16						
22											
23											
24											
25	13	M	SS	18	16						
26											
END OF BORING Northing=211590.1 Easting=551980.4											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-9½'	3.25" HSA								
9½'-24½'	RD w/DM	6/21/07	11:45	11.5	9.5	10.3		10.1	
BORING COMPLETED: 6/21/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-43 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>890.2</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mixture of clayey sand and silty sand, trace roots, brown and dark brown		11	M	SS	16	13				
2	SAND WITH SILT, trace roots, brown mottled to grayish brown, moist, dense, lenses and laminations of organic silt (SP-SM) (possible fill)	COARSE ALLUVIUM OR FILL	31	M	SS	14					
3											
4	SAND WITH SILT, light brown and gray mottled, waterbearing, medium dense (SP-SM)	COARSE ALLUVIUM	22	W	SS	16	20				
5											
6											
7	SILTY SAND, a little gravel, gray, medium dense (SM)	TILL	25	M	SS	14	11				
8											
9	SANDY LEAN CLAY, a little gravel, gray, firm to stiff (CL)		8	M	SS	15	18				
10											
11											
12											
13											
14											
15											
16											
17											
18	8	M	SS	22	15						
19											
20	6	M	SS	21	17						
21											
22	9	M	SS	20	18						
23											
24	8	M	SS	22	15						
25											
26	6	M	SS	21	23						
27											
END OF BORING Northing=211683.6 Easting=552398.6											

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-9½'	3.25" HSA								
9½'-24½'	RD w/DM	6/25/07	8:15	6.5	4.5	5.5		None	
		6/25/07	8:20	9.0	7.0	7.6		6.8	
BORING COMPLETED: 6/25/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-44 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: 888.6 MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mixture of sand with silt and clayey sand, a little gravel, trace roots, surface roots, pieces of plastic and concrete, brown and dark brown	FILL	5	M	SS	16	9				
2	SANDY LEAN CLAY, a little gravel, gray and brown mottled, firm, laminations of silty sand (CL)	TILL	8	M	SS	7	16				
3											
4											
5											
6											
7											
8											
9											
10											
11											
12											
13											
14											
15											
16											
17											
18											
19											
20											
21			9	M	SS	21	14				
22											
23											
24											
25											
26			11	M	SS	20	15				
END OF BORING Northing=211684.3 Easting=552896.6											

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-24½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		6/22/07	11:45	26.5	24.5	26.5		None	
BORING COMPLETED: 6/22/07									
DR: SG LG: SB Rig: 91C									

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-45 RI-3008-18
	LOCATION: N: 211681.830, E: 553399.700 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/18/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
892.4	0.0					
		FILL	FILL: Silty Sand, fine- to medium-grained, trace of Gravel, moist.			
888.4	4.0	FILL	FILL: Clayey Sand, trace of Roots, brown to dark gray.	26		
885.4	7.0	FILL	FILL: Silty Sand, fine-grained, waterbearing, medium dense.	12		Fuel Odor
883.9	8.5	FILL	FILL: Silty Sand, fine-grained, waterbearing, medium dense.	12		
			END OF BORING. (Per Tetra Tech).			
			Water observed at 6 1/2 feet while drilling.			
			Boring then grouted.			

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:45 (See Descriptive Terminology sheet for explanation of abbreviations)

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-46 LOCATION: N: 211683.156, E: 553897.273 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/11/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
895.4	0.0					
894.4	1.0	FILL	FILL: Silty Sand, trace of Roots, dark brown, moist.			
		FILL	FILL: Silty Sand, fine- to medium-grained, trace of Gravel with Wood fragments at 8' sample depth, reddish brown to gray moist to wet.	12		
				14		
				5		
				2	▽	
883.4	12.0	PT	PEAT, dark gray, wet. (Swamp Deposit)	3		
				3		
				5		
873.4	22.0	SP-SM	POORLY GRADED SAND with SILT, fine- to medium-grained, waterbearing, medium dense. (Glacial Outwash)			
869.4	26.0		END OF BORING.	11		
			Water observed at 11 feet while drilling. Boring then grouted.			

BRAUN BASIC LOG OF BORING SP0605871.GPI BRAUN.GDT 10/2/07 14:45 (See Descriptive Terminology sheet for explanation of abbreviations)



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-47 (p. 1 of 1)**

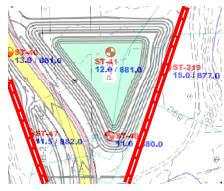
PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>893.3</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	SAND WITH SILT, surface roots, trace roots, fine grained, brown, a little dark brown, moist, very loose (SP-SM) (possible fill)	COARSE ALLUVIUM OR FILL	4	M	SS	13					
2	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, very loose (SP-SM) (possible fill)			7	M	SS	15				
3	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, loose, lenses and laminations of silty sand (SP-SM) (possible fill)	COARSE ALLUVIUM	8	M	SS	17					
4	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, loose, lenses and laminations of silty sand (SP-SM) (possible fill)			10	M	SS	16				
5	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, loose, lenses and laminations of silty sand (SP-SM) (possible fill)		9	M	SS	17					
6	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, loose, lenses and laminations of silty sand (SP-SM) (possible fill)		9	W	SS	16					
7	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, loose, lenses and laminations of silty sand (SP-SM) (possible fill)		11	M	SS	15					
8	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, loose, lenses and laminations of silty sand (SP-SM) (possible fill)		41	M	SS	18					
9	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, loose, lenses and laminations of silty sand (SP-SM) (possible fill)										
10	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, loose, lenses and laminations of silty sand (SP-SM) (possible fill)										
11	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, loose, lenses and laminations of silty sand (SP-SM) (possible fill)										
12	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, loose, lenses and laminations of silty sand (SP-SM) (possible fill)										
13	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, loose, lenses and laminations of silty sand (SP-SM) (possible fill)										
14	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, loose, lenses and laminations of silty sand (SP-SM) (possible fill)										
15	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, loose, lenses and laminations of silty sand (SP-SM) (possible fill)										
16	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, loose, lenses and laminations of silty sand (SP-SM) (possible fill)										
17	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, loose, lenses and laminations of silty sand (SP-SM) (possible fill)										
18	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, loose, lenses and laminations of silty sand (SP-SM) (possible fill)										
19	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, loose, lenses and laminations of silty sand (SP-SM) (possible fill)										
20	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, loose, lenses and laminations of silty sand (SP-SM) (possible fill)										
21	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, loose, lenses and laminations of silty sand (SP-SM) (possible fill)										
22	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, loose, lenses and laminations of silty sand (SP-SM) (possible fill)										
23	SAND WITH SILT, trace roots, fine grained, light brown and brown mottled, moist, loose, lenses and laminations of silty sand (SP-SM) (possible fill)										
24	SAND WITH SILT, a little gravel, fine grained, gray, waterbearing, loose (SP-SM)										
25	CLAYEY SAND, a little gravel, gray, stiff (SC)	TILL	10	M	SS	18	17				
26	END OF BORING Northing=211436.0 Easting=550652.0										

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-14½'	3.25" HSA								
14½'-24½'	RD w/DM	6/19/07	10:30	14.0	12.0	12.1		None	
		6/19/07	10:35	16.5	14.5	14.5		14.4	
BORING COMPLETED: 6/19/07									
DR: SG LG: SB Rig: 91C									

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-48
	LOCATION: N: 211431.925, E: 550895.208 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/21/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
890.9	0.0							
889.4	1.5	FILL	FILL: Silty Sand, trace of Roots and concrete debris, dark brown, moist.					
		SP-SM	POORLY GRADED SAND with SILT, fine-grained, orange-brown to light brown, moist, loose. (Lacustrine)	5				
				10				
879.9	11.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, brown to gray, waterbearing, very loose to loose. (Lacustrine)	6	▽			
		SM	SILTY SAND, fine-grained, grayish-brown to gray, waterbearing, medium dense. (Lacustrine)	4		22	11	
872.9	18.0			8				
864.9	26.0			14				
			END OF BORING.					
			Water observed at 11 feet while drilling.					
			Boring then grouted.					

Fill (silty sand) 0' to 1.5'
 Poorly Graded Sand w Silt 1.5' to 18'
 Silty Sand 18' to 26'

BRAUN BASIC LOG OF BORING SP0605871.GPI BRAUN.GDT 10/2/07 14:45 (See Descriptive Terminology sheet for explanation of abbreviations)

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota				BORING: ST-50				
DRILLER: K. Keck		METHOD: 3 1/4" HSA, Autohmr		DATE: 6/22/07		SCALE: 1" = 4'		
Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
882.8	0.0							
881.8	1.0	FILL	FILL: Silty Sand, very fine- to fine-grained, trace of Roots, dark brown, moist.					
		FILL	FILL: Clayey Sand, asphalt and concrete debris, trace of Roots, dark gray, moist.	15		8	16	OC = 2%
878.8	4.0	FILL	FILL: Poorly Graded Sand with Silt, very fine- to fine-grained, brown, moist.	11				
875.8	7.0	SM	SILTY SAND, slightly Organic, dark gray, wet, very loose. (Swamp Deposit)	3				
				3	▽	39	34	OC = 4 LL = 34 PI = 1
870.8	12.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, rather soft to rather stiff. (Glacial Till)	4				
				10				
				5				
				7				
856.8	26.0		END OF BORING. Water observed at 11 feet while drilling. Boring then grouted.					

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:45 (See Descriptive Terminology sheet for explanation of abbreviations)

INTERTEC

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-51 LOCATION: N: 211184.241, E: 550897.404 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/21/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:45 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
881.2	0.0							
880.4	0.8	FILL	FILL: Silty Sand, very fine- to fine-grained, trace of Roots, dark brown, moist.					
		FILL	FILL: Silty Sand, very fine- to fine-grained, mixed dark brown to brown, moist.	8				
				2		13	15	OC = 2%
874.2	7.0	SM	SILTY SAND, fine-grained, trace of fibers, dark gray, moist, very loose. (Swamp Deposit)	3		28	17	OC = 3
				4	▽			
869.2	12.0	SM	SILTY SAND, fine-grained, dark gray, waterbearing, loose. (Lacustrine)	5				
				5				
864.2	17.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, medium. (Glacial Till)	6				
				6				
855.2	26.0		END OF BORING. Water observed at 9 feet while drilling. Boring then grouted.					



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-52 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: 885.9 MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS								
							WC	DEN	LL	PL	%-#200				
1	4.5" Bituminous Pavement	FILL			SU										
2	FILL, mixture of clayey sand and silty sand, with gravel, trace roots, brown and light brownish gray			12	M	SS	12	10							
3	FILL, mixture of sand, silty sand and clayey sand, a little gravel, pieces of wood at about 5', brown, light brownish gray and black			18	M	SS	19								
4															
5															
6				41	M	SS	19								
7															
8				14	W	SS	17								
9															
10				16		SS	17								
11															
12	SAND WITH SILT, fine grained, gray, a little black, waterbearing, very loose, laminations of silty sand (SP-SM)		COARSE ALLUVIUM												
13		TILL	4	W	SS	14		19							
14	CLAYEY SAND, a little gravel, dark gray, soft (SC)														
15															
16	CLAYEY SAND, gray, a little brown, soft, laminations of silty sand (SC)			4	M	SS	19	20							
17															
18															
19	SANDY LEAN CLAY, a little gravel, gray, soft to firm (CL)														
20			4	M	SS	19	20								
21															
22															
23															
24															
25															
26			7	M	SS	17	19								
END OF BORING Northing=211278.6 Easting=551665.8															

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-14½'	3.25" HSA								
14½'-24½'	RD w/DM	6/21/07	10:45	14.0	12.0	12.4		10.9	
BORING COMPLETED: 6/21/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-53 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>885.5</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mostly sand with silt, with gravel, surface roots, trace roots, dark brown and brown	FILL	13	M	SS	12					
2											
3											
4	FILL, mixture of silty sand and sand with silt, trace roots, brown, light brown, a little black		9	M	SS	13					
5											
6											
7											
8	SILTY SAND, fine grained, brown, moist to about 9.5', then wet, medium dense, lenses and laminations of sand with silt (SM)	COARSE ALLUVIUM	21	W/M	SS	17					
9											
10											
11	CLAYEY SAND, a little gravel, brownish gray, firm (SC)	TILL	6	M	SS	24	19				
12											
13	SANDY LEAN CLAY, a little gravel, brownish gray, stiff (CL)		13	M	SS	23	19				
14											
15											
16											
17			10	M	SS	21	16				
18											
19											
20											
21			12	M	SS	24	17				
22											
23											
24											
25	END OF BORING Northing=211190.3 Easting=551897.8										

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-9½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
9½'-24½'	RD w/DM	6/21/07	1:00	11.5	9.5	10.6		10.3	
BORING COMPLETED: 6/21/07									
DR: SG LG: SB RIG: 91C									



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SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-54 (p. 1 of 1)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>888.9</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mixture of silty sand and clayey sand, a little gravel, trace roots, dark brown and brown, pieces of brick	FILL	18	M	SS	16					
2											
3			56	W/M	SS	1	19				
4											
5	CLAYEY SAND, brown and gray mottled, very soft, laminations of silty sand and sand with silt (SC)	WEATHERED TILL	4	W/M	SS	17					
6		TILL					22				
7	SANDY LEAN CLAY, a little gravel, gray, soft to stiff (CL)		5	M	SS	19	18				
8											
9											
10			15	M	SS	19	16				
11											
12											
13			8	M	SS	24	17				
14											
15			9	M	SS	23	14				
16											
17											
18											
19											
20											
21			10	M	SS	24	19				
22											
23											
24											
25											
26			10	M	SS	24	16				
END OF BORING Northing=211160.7 Easting=552421.7											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
0-24½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
		6/25/07	9:30	9.0	7.0	8.5			None
BORING COMPLETED: 6/25/07									
DR: SG	LG: SB	Rig: 91C							



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-55 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>892.4</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	FILL, mixture of clayey sand and silty sand, a little gravel, trace roots, pieces of bituminous, brown, gray and black	FILL	12	M	SS	13	10					
2												
3	SANDY LEAN CLAY, a little gravel, gray and brown mottled, firm, laminations of silty sand and silt (CL)	WEATHERED TILL	7	M	SS	14	18					
4												
5												
6	SANDY LEAN CLAY, a little gravel, light brownish gray, a little brown, stiff to very stiff, laminations of silty sand (CL)	TILL	9	M	SS	22	17					
7												
8												
9												
10	SANDY LEAN CLAY, a little gravel, dark gray, a little brown, stiff to firm, laminations of silty sand (CL)		14	M	SS	18	16					
11												
12												
13												
14												
15												
16	10	M	SS	21	17							
17												
18												
19	8	M	SS	19	15							
20												
21												
22	13	M	SS	24	15							
23												
24												
25	END OF BORING											
26	Northin=211171.6 Easting=552889.1											

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
0-24½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
		6/22/07	11:00	26.5	24.5	25.0			None
BORING COMPLETED: 6/22/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-57 (p. 1 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>893.6</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS								
							WC	DEN	LL	PL	%-#200				
1	4.25" Bituminous Pavement	FILL													
2	FILL, mixture of sand with silt and silty sand, light brown and brown				M	SU									
3				67	M	SS	18								
4															
5															
6				44	M	SS	18								
7	FILL, mixture of sand with silt and silty sand, a little gravel, brown, dark brownish gray and black														
8				64	M	SS	23								
9															
10				78	M	SS	17								
11															
12															
13				5	M	SS	14								
14															
15				12	M	SS	17								
16															
17															
18			38	W/M	SS	19									
19															
20															
21			14	M	SS	17									
22															
23	SAND WITH SILT, trace roots, fine grained, brownish gray, a little black, waterbearing, medium dense, laminations of organic silt (SP-SM)	TOPSOIL OR COARSE ALLUVIUM	19	W/M	SS	19									
24		COARSE ALLUVIUM													
25	SAND, fine grained, brownish gray, waterbearing, medium dense, lense of fine to medium grained sand (SP)		14	W	SS	22									
26															
27	SANDY LEAN CLAY, a little gravel, brownish gray, stiff (CL)	TILL													
28															

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
0-24½'	3.25" HSA								
24½'-29½'	RD w/DM	6/19/07	12:15	24.0	22.0	22.4			None
		6/19/07	12:20	26.5	24.5	24.1		22.5	
BORING COMPLETED: 6/19/07									
DR: SG LG: SB Rig: 91C									



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SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-57 (p. 2 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
30 - 31 -	SANDY LEAN CLAY, a little gravel, brownish gray, stiff (CL) (continued)	TILL (continued)	9	M	SS	24	15					
	END OF BORING Northing=211018.6 Easting=550730.8											



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-59 (p. 1 of 1)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>884.4</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mostly silty sand, a little gravel, pieces of bituminous, surface roots, trace roots, brown and dark brown	FILL	10	M	SS	14					
2	FILL, mixture of sand with silt and silty sand, a little gravel, trace roots, brown and dark brown		26	M	SS	16					
3			28	M	SS	19					
4											
5											
6											
7											
8											
9	FILL, mixture of organic clay and silty sand, trace roots, black and brown										
10									17		
11	LEAN CLAY WITH ORGANICS, trace roots, black, a little gray, stiff, laminations of sand (CL)	TOPSOIL									
12		COARSE ALLUVIUM							24		
13	SAND, fine grained, light brownish gray, waterbearing, medium dense (SP)		11	W	SS	17					
14	SAND, a little gravel, fine grained, brownish gray, a little black, waterbearing, very loose, laminations of organic silt (SP)		4	W	SS	16					
15											
16											
17											
18	SAND WITH SILT, fine grained, light brownish gray, waterbearing, dense (SP-SM)										
19			37	W	SS	18					
20											
21											
22											
23	SILTY SAND, fine grained, light brownish gray, wet, dense (SM)										
24											
25											
26			31	W	SS	18					
END OF BORING Northing=210936.8 Easting=550917.0											

Fill (silty sand) 0' to 2'
 Fill (sand w silt) 2' to 9'
 Fill (organic clay) 9' to 11'
 topsoil 11' to 12'
 Sand 12' to 18'
 Sand w silt 18' to 23'

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-14½'	3.25" HSA								
14½'-20½'	RD w/DM	6/19/07	1:40	14.0	12.0	12.2		12.1	
BORING COMPLETED: 6/19/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-60 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: 883.9 MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%#200
1	FILL, mixture of silty sand, sandy silt and organic silt, a little gravel, trace roots, brown, dark brown and black	FILL	10	M	SS	14					
2											
3	FILL, mixture of sand with silt, silty sand and clayey sand, a little gravel, organic clay, trace roots, brown, a little light brown and black	FILL	16	M	SS	14					
4											
5											
6			4	W/M	SS	14					
7											
8											
9			3	M	SS	14					
10											
11			5	W	SS	17					
12											
13			13	W	SS	12					
14	SAPRIC PEAT, black, laminations of waterbearing sand (PT)	SWAMP DEPOSIT									
15											
16			WH	W	SS	19	48				
17	SAND, fine grained, gray to brownish gray, waterbearing, medium dense (SP)										
18											
19			12	W	SS	16					
20											
21			22	W	SS	19					
22											
23											
24											
25											
26			28	W	SS	21					
END OF BORING Northing=210929.3 Easting=551151.1											

Fill (silty sand) 0' to 14'
Sapric Peat 14' to 16.5'
Sand 16.5' to 26'

DEPTH: DRILLING METHOD

WATER LEVEL MEASUREMENTS

DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL
6/19/07	2:45	9.0	7.0	7.0		None
6/19/07	2:50	11.5	9.5	9.0		8.0

NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG

BORING COMPLETED: 6/19/07

DR: SG LG: SB Rig: 91C



SUBSURFACE BORING LOG

AET JOB NO: 22-00081 LOG OF BORING NO. ST-61 (p. 1 of 1)
 PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>885.9</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS									
							WC	DEN	LL	PL	%-#200					
1	5" Bituminous Pavement	FILL			SU											
2	FILL, mixture of silty sand and sand with silt, a little gravel, trace roots, brown and gray			20	M	SS	14									
3				32	M	SS	17									
4																
5																
6				39	M	SS	19									
7	CLAYEY SAND, black, a little gray, hard, laminations of sand with silt (SC)	TOPSOIL						13								
8	SILTY SAND, fine grained, light brownish gray, moist, dense (SM)	COARSE ALLUVIUM	38	M	SS	20										
9	SILTY SAND, fine grained, gray, wet, loose (SM)			8	W	SS	18									
10																
11																
12	SAND WITH SILT, trace roots, fine grained, gray, a little black, waterbearing, medium dense, laminations of clayey sand (SP-SM)			12	W	SS	14									
13																
14																
15																
16			11	W	SS	10										
17																
18			10	W	SS	15										
19																
20	SAND WITH SILT, fine grained, gray, waterbearing, medium dense (SP-SM)		21	W	SS	17										
21																
22																
23																
24																
25																
26			22	W	SS	15										
END OF BORING Northing=210988.6 Easting=551565.3																

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-9½'	3.25" HSA								
9½'-24½'	RD w/DM	6/21/07	9:30	11.5	9.5	9.7		9.4	
BORING COMPLETED: 6/21/07									
DR: SG LG: SB Rig: 91C									

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota				BORING: ST-62				
DRILLER: K. Keck		METHOD: 3 1/4" HSA, Autohmr		DATE: 6/25/07		SCALE: 1" = 4'		
Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
884.3	0.0							
883.8	0.5	FILL	FILL: Silty Sand, trace of Roots, dark brown, moist.					
		FILL	FILL: Silty Sand, very fine- to fine-grained, trace of Gravel, mixed with Poorly Graded Sand, light brown to brown, moist.	18				
				22				
				14				
				7				
872.3	12.0	SM	SILTY SAND, fine-grained, trace of Roots, slightly Organic, dark gray, waterbearing, loose. (Swamp Deposit)	6	▽	25	17	OC = 2%
870.3	14.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, gray, waterbearing, loose. (Lacustrine)	5				
866.3	18.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, medium to rather stiff. (Glacial Till)	7				
858.3	26.0		END OF BORING. Water observed at 12 feet while drilling. Boring then grouted.	10				

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:45
 (See Descriptive Terminology sheet for explanation of abbreviations)

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-63
	LOCATION: N: 210687.940, E: 550637.026 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/25/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
886.1	0.0	FILL	FILL: Poorly Graded Sand with Silt, fine-grained, trace of Gravel, mostly brown mixed with dark brown, moist.			
				23		
				17		
				23		
877.1	9.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, brown to grayish-brown, loose. (Lacustrine)			
				7		
				6		
872.1	14.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, gray, waterbearing, very loose. (Lacustrine)		▽	
				3		
868.1	18.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, rather soft to rather stiff. (Glacial Till)			
				5		
860.1	26.0		END OF BORING.	10		
			Water observed at 14 feet while drilling.			
			Boring then grouted.			

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:45 (See Descriptive Terminology sheet for explanation of abbreviations)



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-64 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>883.5</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS							
							WC	DEN	LL	PL	%-#200			
1	2.25" Bituminous Pavement	FILL			SU									
2	FILL, mixture of gravelly sand with silt and clayey sand, trace roots, brown		7	M	SS	12								
3	FILL, mixture of sand with silt, silty sand and clayey sand, a little gravel, brown, a little dark brown, gray and black		12	M	SS	16	11							
4														
5														
6				32	M	SS	19	7						
7														
8	FILL, mixture of silty sand and organic clay, trace roots, brownish gray and black		25	M	SS	20								
9	SAPRIC PEAT, black (PT)	SWAMP DEPOSIT												
10														
11	SILTY SAND, trace roots, fine grained, gray and black, wet very loose, laminations of organic silt (SM)	COARSE ALLUVIUM	3	M	SS	14								
12														
13			3	W	SS	12								
14														
15	SANDY LEAN CLAY, a little gravel, gray, firm to stiff, laminations of sand (CL)	TILL	6	M	SS	12	14							
16														
17														
18														
19														
20														
21			6	M	SS	22	16							
22														
23														
24														
25														
26			12	M	SS	22	17							
END OF BORING Northing=210683.3 Easting=550895.7														

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-9½'	3.25" HSA								
9½'-24½'	RD w/DM	6/20/07	3:35	11.5	9.5	10.9		10.2	
BORING COMPLETED:	6/20/07								
DR: SG	LG: SB	Rig: 91C							



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-65 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>886.2</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS								
							WC	DEN	LL	PL	%-#200				
1	4.5" Bituminous Pavement	FILL			SU										
2	FILL, mixture of silty sand, sand with silt and clayey sand, with gravel, trace roots, dark brown, light brown and gray		14	M	SS	13									
3	SAND WITH SILT, fine grained, light brown and brown mottled, moist, medium dense, laminations of silty sand (SP-SM)	COARSE ALLUVIUM	22	M	SS	19									
4															
5	SAND WITH SILT, fine grained, light brown, moist to waterbearing, medium dense (SP-SM)		23	W	SS	17									
6															
7	SAND WITH SILT, fine grained, brownish gray, a little black, w														
8	then moist, medium sand (SP-SM)		15	M	SS	17									
9															
10															
11	SAND WITH SILT, trace roots, fine grained, gray and black, moist, medium dense, lenses of organic silt (SP-SM)		14	M	SS	20									
12															
13	SAND, fine grained, gray and light grayish brown mottled, waterbearing, medium dense (SP)		12	W	SS	19									
14															
15	SANDY SILT, gray and brownish gray, wet, loose, lenses and laminations of lean clay (ML)	FINE ALLUVIUM	8	M	SS	17									
16															
17															
18	CLAYEY SAND, a little gravel, gray, stiff, laminations of wet silty sand (SC)	TILL													
19															
20															
21			9	M	SS	20	14								
22															
23	SANDY LEAN CLAY, a little gravel, gray, stiff (CL)														
24															
25															
26			13	M	SS	19	19								
END OF BORING Northing=210684.0 Easting=551396.3															

Fill (silty sand) 0.3' to 2.5'
Sand w Silt 2.5' to 12'
Sand 12' to 14.5'
Sandy Silt 14.5' to 18'
Clayey Sand 18' to 23'

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-12'	3.25" HSA								
12'-24½'	RD w/DM	6/21/07	8:20	14.0	12.0	12.1		10.0	
BORING COMPLETED: 6/21/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-66 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: 888.9 MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS									
							WC	DEN	LL	PL	%#200					
1	FILL, mostly silty sand, a little gravel, trace roots, dark brown	FILL														
2	FILL, mixture of sand with silt and silty sand, a little gravel, trace roots, light brown and brown	COARSE ALLUVIUM	20	M	SS	19										
3	SILTY SAND, fine grained, brown, moist, medium dense (SM)		20	M	SS	18										
4	SAND WITH SILT, fine grained, light grayish brown to brown mottled, moist, medium dense, laminations of silty sand (SP-SM)		15	M	SS	15										
5			18	M	SS	18										
6			12	W	SS	17										
7	SAND WITH SILT, fine grained, light brownish gray and brown mottled, moist to about 9.5', then waterbearing, medium dense to loose (SP-SM)															
8																
9																
10																
11																
12																
13																
14																
15	SILTY SAND, a little gravel, fine to medium grained, gray, wet, very loose (SM)															
16																
17																
18																
19	CLAYEY SAND, a little gravel, gray, soft (SC)	TILL														
20																
21																
22																
23																
24	SANDY LEAN CLAY, a little gravel, gray, firm (CL)															
25																
26																
END OF BORING Northing=210683.5 Easting=551850.2																

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-9½'	3.25" HSA								
9½'-24½'	RD w/DM	6/21/07	2:05	11.5	9.5	10.2		10.0	
BORING COMPLETED: 6/21/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-67 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>888.7</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	FILL, mostly organic silt, surface roots, trace roots, dark brown and black	FILL	8	M	SS	15						
2	FILL, mixture of silty sand and clayey sand, a little gravel, trace roots, dark brown, brown and light brown											
3	SAND WITH SILT, a little gravel, light brown to light brown mottled, waterbearing, loose to medium dense (SP-SM)	COARSE ALLUVIUM	17	M	SS	14						
4												
5												
6												
7												
8	<div style="background-color: yellow; padding: 2px;"> Fill (silty sand) 0.7' to 3' Sand w Silt 3' to 12' Clayey Sand 12' to 14.5' Sandy Lean Clay 14.5' to 26' </div>		21	W	SS	17						
9			9	W	SS	15						
10												
11												
12	CLAYEY SAND, a little gravel, gray, stiff (CL)	TILL										
13			9	M	SS	10	17					
14												
15	SANDY LEAN CLAY, a little gravel, gray, firm to stiff											
16			6	M	SS	17	15					
17												
18												
19												
20												
21			11	M	SS	16	17					
22												
23												
24												
25												
26			10	M	SS	18	18					
END OF BORING Northing=210683.4 Easting=552397.5												

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-9½'	3.25" HSA								
9½'-24½'	RD w/DM	6/22/07	8:40	9.0	7.0	6.8		6.0	
BORING COMPLETED: 6/22/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-68 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>895.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	FILL, mixture of silty sand and sand with silt, a little gravel, surface roots, trace roots, light brown, brown and dark brown	FILL	13	M	SS	12						
2												
3			16	M	SS	6						
4												
5					12	M	SS	14				
6												
7	ORGANIC CLAY, trace roots, black, soft (OL/OH)	TOPSOIL OR SWAMP DEPOSIT	3	M	SS	10	39					
9	SAND WITH SILT, a little gravel, brown and gray, waterbearing, medium dense (SP-SM)	COARSE ALLUVIUM	28	W	SS	17						
10												
12	LEAN CLAY, gray and black, stiff, lenses of sandy lean clay, laminations of fat clay (CL)	TILL	14	M	SS	24	16					
13												
14	SANDY LEAN CLAY, a little gravel, gray, firm to stiff, laminations of lean clay (CL)		5	M	SS	6						
15												
16												
17												
18												
19												
20												
21			11	M	SS	19	17					
22												
23												
24												
25												
26			12	M	SS	21	18					
END OF BORING Northing=210684.3 Easting=552897.4												

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-4½'	3.25" HSA								
4½'-24½'	RD w/DM	6/22/07	1:00	6.5	4.5	6.0		5.2	
BORING COMPLETED: 6/22/07									
DR: SG LG: SB Rig: 91C									

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-69 RI-3002-05 LOCATION: N: 210744.173, E: 553428.679 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/19/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
894.8	0.0					
893.8	1.0	FILL	FILL: Poorly Graded Sand with Silt, trace of Gravel, dark brown, moist.			
		SM	SILTY SAND, fine-grained, gray, moist, loose. (Lacustrine)	8		
890.8	4.0	SP-SM	POORLY GRADED SAND with SILT, trace of Gravel, brown, waterbearing, very loose to loose. (Lacustrine)	10	▽	
				7		
				3		
				10		
880.8	14.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, medium to rather stiff. (Glacial Till)	10		
				8		
868.8	26.0		END OF BORING.	10		
			Water observed at 4 feet while drilling.			
			Boring then grouted.			

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:45
 (See Descriptive Terminology sheet for explanation of abbreviations)

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-70
	LOCATION: N: 210240.974. E: 550584.304 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/25/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
885.0	0.0					
883.5	1.5	SM	SILTY SAND, trace of Roots, dark brown, moist. (Topsoil)			
		SP-SM	POORLY GRADED SAND with SILT, fine-grained, orange-brown, moist, loose. (Lacustrine)	9		
				9		
878.0	7.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown to gray with rust, waterbearing, very loose to loose. (Lacustrine)	4	▽	
				4		
				8		
871.0	14.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, medium to rather stiff. (Glacial Till)	9		
				6		
				9		
859.0	26.0		END OF BORING.			
			Water observed at 7 feet while drilling.			
			Boring then grouted.			

BRAUN BASIC LOG OF BORING. SP0605871.GPJ BRAUN.CDT 10/2/07 14:45
 (See Descriptive Terminology sheet for explanation of abbreviations)

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-71 RI-1015-01
	LOCATION: N: 210151.469. E: 550929.912 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/25/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:45 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
887.3	0.0					
887.1	0.3	PAV	3" of Bituminous			
885.8	1.5	FILL	FILL: Poorly Graded Sand with Silt, fine-grained, trace of Gravel, brown, moist.			
		SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown to orange-brown, moist, loose to medium dense. (Lacustrine)	16		
				7		
880.3	7.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, brownish-gray to gray, waterbearing, very loose to loose. (Lacustrine)	4	▽	
				10		
875.3	12.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, rather stiff to very stiff. (Glacial Till)	25		
				14		
				11		
861.3	26.0		END OF BORING.	11		
			Water observed at 7 feet while drilling.			
			Boring then grouted.			



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-72 (p. 1 of 1)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>887.4</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%#200
1	FILL, mixture of silty sand and sand, light brown, brown and dark brown <div style="background-color: yellow; padding: 2px; display: inline-block;">Fill (silty sand) 0' to 7' Sand w Silt 7' to 10' Clayey Sand 10' to 18'</div>	FILL	6	M	SS	14					
2											
3			3	W	SS	12					
4											
5											
6											
7											
8	SAND WITH SILT, fine to medium grained, a little gravel, brown, waterbearing, loose (SP-SM)	COARSE ALLUVIUM	6	W	SS	14					
9											
10	SAND WITH SILT AND GRAVEL, medium to fine grained, brown, waterbearing, loose (SP-SM)	TILL	5	M	SS	18	16				
11	CLAYEY SAND, a little gravel, gray, firm (CL)										
12											
13											
14											
15											
16											
17											
18	SANDY LEAN CLAY, a little gravel, gray, stiff (CL)										
19											
20											
21											
22											
23											
24											
25											
26											
END OF BORING Northing=210217.1 Easting=551467.5											

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-24½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		6/26/07	11:55	6.5	4.5	4.9		4.8	
		6/26/07	12:20	26.5	24.5	26.5		None	
BORING COMPLETED: 6/26/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-73 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>891.3</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mixture of silty sand, sand with silt and sandy silt, a little gravel, surface roots, trace roots, dark brown, black and gray	FILL	8	M	SS	12					
2											
3			6	M	SS	8					
4	LEAN CLAY WITH ORGANICS, trace roots, black, firm (CL)	TOPSOIL									
5			6	M	SS	14	23				
6	SAND WITH SILT, fine grained, gray and brown mottled, waterbearing, medium dense (SP-SM)	COARSE ALLUVIUM									
7			13	W	SS	16					
8											
9	SAND WITH SILT, a little gravel, fine to medium grained, brown, a little gray mottled, waterbearing, medium dense (SP-SM)										
10			22	W	SS	17					
11	SAND WITH SILT, fine grained, light brownish gray, waterbearing, medium dense (SP-SM)										
12			25	W	SS	16					
13											
14	SANDY LEAN CLAY, a little gravel, gray, firm (CL)	TILL									
15			6	M	SS	15	24				
16	CLAYEY SAND, a little gravel, gray, firm (SC)										
17											
18			5	M	SS	24	19				
19	SANDY LEAN CLAY, a little gravel, gray, firm (CL)										
20			7	M	SS	17	18				
21											
22											
23											
24											
25											
26											
END OF BORING Northing=210182.9 Easting=551898.2											

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-9½'	3.25" HSA								
9½'-24½'	RD w/DM	6/21/07	3:00	11.5	9.5	10.0		9.0	
BORING COMPLETED: 6/21/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-74 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>891.8</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mixture of sand with silt and silty sand, a little gravel, surface roots, trace roots, brown	FILL	15	M	SS	15					
2	FILL, mixture of clayey sand and silty sand, a little gravel, trace roots, brown										
3			14	M	SS	18	15				
4											
5	SAND WITH SILT, a little gravel, fine to medium grained, light brown, waterbearing, loose to medium dense, laminations of silty sand (SP-SM)	COARSE ALLUVIUM	6	M	SS	13					
6											
7											
8			13	W	SS	15					
9											
10											
11	SAND WITH SILT, a little gravel, fine to medium grained, gray, medium dense (SP-SM)	TILL	19	W	SS	19					
12	CLAYEY SAND, a little gravel, gray, firm to very stiff (SC)										
13			7	M	SS	18	17				
14											
15											
16			10	M	SS	17	13				
17											
18	SANDY LEAN CLAY, a little gravel, gray, stiff (CL)										
19											
20											
21			10	M	SS	14	17				
22											
23											
24											
25											
26			13	M	SS	24	14				
END OF BORING Northing=210212.8 Easting=552416.8											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-24½'	3.25" HSA	6/7/07	3:05	6.5	4.5	4.8		None	
		6/27/07	3:10	9.0	7.0	7.0		6.4	
BORING COMPLETED:	6/27/07	6/27/07	3:35	26.5	24.5	26.5		None	
DR: SG	LG: SB	Rig: 91C							



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-75 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: 898.8 MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%#200	
1	SAND WITH SILT, fine to medium grained, a little gravel, surface roots, trace roots, brown (SP-SM) (possible fill)	COARSE ALLUVIUM OR FILL	8	M	SS	10						
2												
3												
4	CLAYEY SAND, a little gravel, brown, light brown and gray mottled, firm, lenses and laminations of sand with silt (SC)	MIXED ALLUVIUM	15	M	SS	10						
5												
6	SAND WITH SILT, a little gravel, trace roots, brown, waterbearing, loose (SP-SM)	COARSE ALLUVIUM	7	W/M	SS	14	14					
7												
8	SANDY SILT, a little gravel, trace roots, gray, wet, medium dense (ML)	FINE ALLUVIUM	12	W	SS	NR						
9												
10	CLAYEY SAND, a little gravel, brownish gray, stiff (SC)	TILL	10	W	SS	13	14					
11												
12	SANDY LEAN CLAY, a little gravel, dark gray, firm to stiff (CL)		7	W	SS	21	19					
13												
14												
15												
16					10	W	SS	18	18			
17												
18												
19												
20												
21			12	M	SS	18	16					
22												
23												
24												
25												
26			13	M	SS	17	11					

END OF BORING
Northing=210184.0
Easting=552897.6

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-4½'	3.25" HSA								
4½'-24½'	RD w/DM	6/25/07	10:30	6.5	4.5	4.9		4.3	
BORING COMPLETED: 6/25/07									
DR: SG LG: SB Rig: 91C									

Braun Project SP-06-05871
Geotechnical Evaluation
TCAAP Redevelopment
NE of Highway 10 and Highway 96
Arden Hills, Minnesota

BORING: **RI-4001-02 ST-76**
LOCATION: N: 210133.678, E: 553362.349 See attached sketch.

DRILLER: K. Keck METHOD: 3 1/4" HSA, Autohmr DATE: 7/23/07 SCALE: 1" = 4'

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:41 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
907.0	0.0					
906.0	1.0	FILL	FILL: Silty Sand, fine-grained, with Organics and Gravel, dark brown, moist.			
		FILL	FILL: Silty Sand, fine- to coarse-grained, with Gravel, brown, moist.			
			Petroleum odor at 5 1/2 feet.	19		
				32		
900.0	7.0	CL	SANDY LEAN CLAY, trace of Gravel, brown, moist, medium to rather stiff. (Glacial Till)	8		
				9		
				9		
893.0	14.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, moist, medium. (Glacial Till)	7		
				7		
881.0	26.0		END OF BORING. Water not observed during drilling. Water not observed with 24 1/2 feet of hollow-stem auger in the ground. Boring then grouted.	7		

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-77 LOCATION: N: 209775.961, E: 550699.340 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/26/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPI BRAUN.GDT 10/2/07 14:45 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
892.4	0.0					
891.4	1.0	FILL	FILL: Silty Sand, trace of Roots, dark brown, moist.			
		FILL	FILL: Silty Sand, fine-grained, trace of Gravel, dark brown to brown, moist.			
				9		
				8		
885.4	7.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown to grayish-brown, moist, loose to medium dense. (Lacustrine)	12		
				7	▽	
881.4	11.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, brownish-gray to gray, wet, very loose to loose. (Lacustrine)	4		
				5		
				10		
870.4	22.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, rather stiff. (Glacial Till)			
				9		
866.4	26.0		END OF BORING.			
			Water observed at 11 feet while drilling.			
			Boring then grouted.			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-78 LOCATION: N: 209684.547, E: 550898.325 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/26/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:45 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
886.1	0.0							
884.6	1.5	FILL	FILL: Silty Sand, trace of Roots, dark brown, moist.					
		FILL	FILL: Silty Sand, fine-grained, dark brown mixed with light brown and gray, moist.	3				
				2				
879.1	7.0	PT	PEAT, fibrous, dark gray, wet, very soft. (Swamp Deposit)	WH				
				WH		421	61	
872.1	14.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, gray, waterbearing, very loose to loose. (Lacustrine)	1	▽			
				4				
860.1	26.0			8				
			END OF BORING. Water observed at 14 feet while drilling. Boring then grouted.					



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-79 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: 888.9 MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS								
							WC	DEN	LL	PL	%-#200				
1	SILTY SAND, a little gravel, surface roots, trace roots, dark brown, moist, loose (SM)	TOPSOIL													
2	SILTY SAND WITH GRAVEL, trace roots, brown, moist, loose (SM)	COARSE ALLUVIUM	10	M	SS	16									
3	SAND, fine grained, light brown, moist, loose, laminations of silty sand (SP)		9	M	SS	16									
4															
5															
6															
7															
8	SAND, fine grained, light brown, waterbearing, medium dense to loose (SP)		11	W	SS	17									
9															
10															
11			7	W	SS	17									
12	SAND WITH SILT, fine grained, gray, waterbearing, very loose to very dense (SP-SM)														
13			2	W	SS	13									
14															
15															
16			55	W	SS	14									
17															
18	SILTY SAND, a little gravel, gray, waterbearing, very loose (SM)	TILL													
19															
20	SANDY LEAN CLAY, a little gravel, gray, soft to very stiff, laminations of silty sand at 24.5' (CL)		4	W	SS	16	15								
21															
22															
23															
24															
25															
26			17	M	SS	21	16								
END OF BORING Northing=209686.5 Easting=551325.7															

Silty Sand 0' to 2'
Sand 2' to 11.5'
Sand w Silt 11.5' to 18'

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-24½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		6/26/07	1:05	9.0	7.0	7.6		7.0	
BORING COMPLETED: 6/26/07									
DR: SG LG: SB Rig: 91C									

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-80 LOCATION: N: 209689.484, E: 551891.038 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/28/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	Tests or Notes
891.1	0.0	FILL	FILL: Silty Sand, very fine- to fine-grained, trace of Gravel, brown, moist.				
889.1	2.0	CL	SANDY LEAN CLAY, trace of Gravel, light brown to brown and gray mixed with rust, wet. (Glacial Till)	5			
884.1	7.0	CL	SANDY LEAN CLAY, trace of Gravel, grayish-brown, wet, rather stiff to medium. (Glacial Till)	9		15	
879.1	12.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, rather soft to medium. (Glacial Till)	7		14	
				5			
				7		16	
				7			
865.1	26.0		END OF BORING.	8			
			Water not observed with 24 1/2 feet of hollow-stem auger in the ground.				
			Boring then grouted.				

BRAUN BASIC LOG OF BORING. SP0605871.GPJ BRAUN.GDT 10/2/07 14:45 (See Descriptive Terminology sheet for explanation of abbreviations)



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-81 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>895.1</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS							
							WC	DEN	LL	PL	%-#200			
1	SANDY SILT, a little gravel, surface roots, trace roots, dark brown, moist, loose (ML)	TOPSOIL												
2	SAND WITH SILT, fine grained, a little gravel, trace roots, brown, moist, loose (SP-SM)	COARSE ALLUVIUM	10	M	SS	16								
3			6	M	SS	6								
4	SILTY SAND, trace roots, brown, waterbearing, loose, laminations of lean clay (SM)	TILL												
5	SANDY LEAN CLAY, a little gravel, brown and gray mottled, firm (CL)		7	M	SS	15	16							
6														
7	SANDY LEAN CLAY, a little gravel, brownish gray, a little brown, stiff, laminations of silty sand (CL)		11	M	SS	19	15							
8														
9	CLAYEY SAND, a little gravel, gray, firm (SC)		8	M	SS	22	15							
10														
11	SANDY LEAN CLAY, a little gravel, brownish gray, a little brown, stiff, laminations of silty sand (CL)		13	M	SS	21	16							
12														
13	SANDY LEAN CLAY, a little gravel, gray, firm to stiff (CL)		8	M	SS	17	15							
14														
15														
16														
17														
18														
19														
20														
21			11	M	SS	20	17							
22														
23														
24														
25														
26			12	M	SS	19	14							
END OF BORING Northing=209624.8 Easting=552198.7														

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-24½'	3.25" HSA	6/26/07	2:55	26.5	24.5	26.0		None	
BORING COMPLETED: 6/26/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-82 (p. 1 of 1)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>898.6</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mostly sandy silt, surface roots, trace roots, dark brown	FILL	25	M	SS	14					
2	FILL, mixture of silty sand and clayey sand, a little gravel, trace roots, light brown, brown and dark brown										
3			15	M	SS	13					
4											
5											
6				20	M	SS	16				
7	LEAN CLAY WITH ORGANICS, trace roots, gray and black, moist, very stiff, lenses and laminations of silty sand (CL)	TOPSOIL									
8			19	M/W	SS	17	15				
9	SILTY SAND, a little gravel, brownish gray, waterbearing, medium dense (SM)	TILL									
10	CLAYEY SAND, a little gravel, trace roots, gray and brown mottled, soft (SC)		3	M	SS	20	19				
11											
12			4	M	SS	21	17				
13											
14											
15	SANDY LEAN CLAY, gray, firm to stiff (CL)		6	M	SS	21	16				
16											
17											
18											
19											
20											
21			11	M	SS	22	14				
22											
23											
24											
25											
26			15	M	SS	22	15				
<p>END OF BORING Northing=209687.1 Easting=552381.8</p>											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-24½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		6/28/07	8:15	11.5	9.5	11.2		None	
		6/28/07	8:30	26.5	24.5	26.5		None	
BORING COMPLETED:	6/28/07								
DR: SG	LG: SB	Rig: 91C							



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-83 (p. 1 of 1)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>903.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mixture of sand with silt and silty sand, gravelly, trace roots, brown to dark brown Hit gas line between 1' to 2'	FILL	25	M	SS	12					
2											
END OF BORING Northing=209676.4 Easting=552900.3											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-2'	3.25" HSA								
BORING COMPLETED: 6/25/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-83A (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: 903.0 MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS									
							WC	DEN	LL	PL	%-#200					
1	2.5" Bituminous Pavement	FILL			SU											
2	FILL, mixture of clayey sand and silty sand with gravel, possible cobbles, brown			27	M	SS	6									
3																
4																
5	SANDY LEAN CLAY, a little gravel, light brown and gray mottled, stiff, laminations of silty sand (CL)	TILL														
6			9	M	SS	16	17									
7	SANDY LEAN CLAY, a little gravel, brown, very stiff (CL)															
8			18	M	SS	18	16									
9																
10	SANDY LEAN CLAY, a little gravel, dark gray, stiff to very stiff (CL)															
11																
12																
13																
14																
15																
16																
17																
18																
19																
20																
21																
22																
23																
24																
25																
26																
END OF BORING Northing=209676.4 Easting=552900.3																

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
DEPTH	DRILLING METHOD	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-24½'	3.25" HSA	6/28/07	10:00	26.5	24.5	26.0		None	
BORING COMPLETED:	6/28/07								
DR: SG	LG: SB	Rig: 91C							



SUBSURFACE BORING LOG

AET JOB NO: 22-00081 LOG OF BORING NO. ST-84 (p. 1 of 1)
 PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>911.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mostly silty sand, a little gravel, surface roots, trace roots, brown	FILL	13	M	SS	14					
2	FILL, mixture of sand with silt and sandy lean clay, a little gravel, trace roots, brown, gray, light gray and light brown										
3			7	M	SS	13	16				
4											
5	SANDY LEAN CLAY, a little gravel, light brown and gray mottled, with gray, firm to stiff (CL)	TILL	7	M	SS	17	19				
6											
7											
8				10	M	SS	20	17			
9											
10	SANDY LEAN CLAY, a little gravel, brown mottled, dark brown, stiff, laminations of silt (CL)			14	M	SS	24	15			
11											
12	SANDY LEAN CLAY, a little gravel, dark gray, stiff to very stiff (CL)			14	M	SS	19	15			
13											
14				15	M	SS	16	15			
15											
16											
17											
18											
19											
20											
21			19	M	SS	16	14				
22											
23											
24											
25											
26			21	M	SS	18	16				
END OF BORING Northing=209598.8 Easting=553354.1											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-24½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		7/18/07	3:35	26.5	24.5	26.5		None	
BORING COMPLETED:	7/18/07								
DR: SG	LG: SB/BRig: 91C								



SUBSURFACE BORING LOG

AET JOB NO: **22-00081** LOG OF BORING NO. **ST-85 (p. 1 of 1)**
 PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>892.1</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mixture of silty sand and sand with silt, a little gravel, surface roots, trace roots, brown	FILL	13	M	SS	15					
2	CLAYEY SAND, a little gravel, trace roots, dark brown, very stiff (SC)	TILL	23	M	SS	17	8				
3											
4	SILTY SAND, trace roots, fine to medium grained, moist, very loose, laminations of sand with silt (SM)		4	M	SS	18					
5											
6											
7	CLAYEY SAND, a little gravel, trace roots, gray and brown mottled, firm (SC)		5	M	SS	18	19				
8											
9											
10											
11	CLAYEY SAND, a little gravel, trace roots, dark gray, firm to stiff (SC)		5	M	SS	23	17				
12											
13											
14											
15											
16	SANDY LEAN CLAY, a little gravel, dark gray, stiff (CL)		10	M	SS	23	14				
17											
18											
19			9	M	SS	24	19				
20											
21											
22											
23			14	M	SS	22	15				
24											
25											
26											
END OF BORING Northing=209496.7 Easting=551939.7											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-24½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		6/27/07	9:35	9.0	7.0	8.2		None	
		6/27/07	10:05	26.5	24.5	26.5		None	
BORING COMPLETED: 6/27/07									
DR: SG LG: SB Rlg: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-86 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>897.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%#200
1	SAND WITH SILT, fine grained, a little gravel, trace roots, brown, moist, medium dense (SP-SM)	COARSE ALLUVIUM	22	M		15					
2											
3			22	M		19					
4											
5	SILTY SAND, a little gravel, brown and gray, moist, medium dense (SM)	COARSE ALLUVIUM	33	M		16					
6											
7											
8	SANDY LEAN CLAY, a little gravel, gray, firm to stiff, lenses of lean clay with sand at 8' (CL)	WEATHERED TILL	6	M		17					
9											
10											
11	SANDY LEAN CLAY, a little gravel, brown and gray mottled, stiff to firm, laminations of silty sand (CL)	TILL	9	M		20					17
12											
13			8	M		22	15				
14											
15			9	M		24	17				
16											
17											
18	SANDY LEAN CLAY, a little gravel, dark gray, a little brown, stiff, laminations of silty sand (CL)	TILL	13	M		21	14				
19											
20											
21											
22											
23	SANDY LEAN CLAY, a little gravel, dark gray, stiff (CL)	TILL	15	M		20	15				
24											
25											
26											
END OF BORING Northing=209336.2 Easting=552138.3											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-24½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		6/26/07	3:40	9.0	7.0	9.0		8.1	
BORING COMPLETED: 6/26/07									
DR: SG	LG: SB	Rig: 91C							



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-87 (p. 1 of 1)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>898.3</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	FILL, mixture of silty sand, clayey sand and sand with silt, a little gravel, surface roots, trace roots, dark brown and black	FILL	19	M	SS	14	7					
2	SANDY LEAN CLAY, a little gravel, trace roots, brown and gray mottled, firm (CL)	TILL	8	M	SS	14	17					
3												
4												
5												
6												
7												
8												
9												
10												
11												
12	SANDY LEAN CLAY, a little gravel, light brown and gray mottled, stiff (CL)		11	M	SS	19	16					
13	SANDY LEAN CLAY, a little gravel, brownish gray, a little brown, stiff, laminations of silty sand (CL)		14	M	SS	21	15					
14	SANDY LEAN CLAY, a little gravel, gray, stiff (CL)		11	M	SS	22	14					
15			14	M	SS	20	15					
16												
17												
18												
19												
20												
21			15	M	SS	21	14					
22												
23												
24												
25												
26			14	M	SS	19	15					
END OF BORING Northing=209443.6 Easting=552400.6												

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
0-24½' 3.25" HSA		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
		6/26/07	9:20	26.5	24.5	26.0			None
BORING COMPLETED: 6/26/07									
DR: SG LG: SB Rig: 91C									

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-88 LOCATION: N: 209329.011, E: 550811.898 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/28/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
889.7	0.0	FILL	FILL: Silty Sand, very fine- to fine-grained, trace of Gravel, trace of Roots at 5' sample depth, mixed dark brown to brown, moist.	16		
883.7	6.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, brown, moist, loose. (Lacustrine)	6	▽	
881.7	8.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, brownish-gray to gray, waterbearing, loose to medium dense. (Lacustrine)	15		
871.7	18.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, rather soft to rather stiff. (Glacial Till)	10		
863.7	26.0		END OF BORING. Water observed at 8 feet while drilling. Boring then grouted.	4		

BRAUN BASIC LOG OF BORING SP0605871.GPJ: BRAUN.GDT 10/2/07 14:45
 (See Descriptive Terminology sheet for explanation of abbreviations)

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-89
	LOCATION: N: 209181.170, E: 550895.450 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/27/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
890.4	0.0					
890.2	0.2	PAV FILL	2" of Bituminous FILL: Silty Sand, fine-grained, trace of Gravel, mixed dark brown to brown, moist.			
883.4	7.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, brown, waterbearing, medium dense. (Lacustrine)	21 10 14 10 11 11	▽	
872.4	18.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, rather soft. (Glacial Till)	4		
864.4	26.0		END OF BORING. Water observed at 7 feet while drilling Boring then grouted..	5		

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:46
 (See Descriptive Terminology sheet for explanation of abbreviations)

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-90
	LOCATION: N: 209183.404, E: 551399.923 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/26/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0603871.GPJ BRAUN.GDT 10/2/07 14:46
 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
891.0	0.0					
889.0	2.0	SM	SILTY SAND, very fine- to fine-grained, trace of Roots, dark brown, moist. (Topsoil)			
885.0	6.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown, moist, loose to medium dense. (Lacustrine)	9		
			Silty Sand 0' to 2' Poorly Graded Sand w Silt 2' to 17' Sandy Lean Clay 17' to 26'			
880.0	11.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, grayish-brown with rust, waterbearing, medium dense. (Lacustrine)	11	▽	
				15		
				14		
874.0	17.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, gray, waterbearing, loose to medium dense. (Lacustrine)	11		
				10		
865.0	26.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, medium. (Glacial Till)	6		
				7		
			END OF BORING.			
			Water observed at 6 feet while drilling.			
			Boring then grouted.			



AMERICAN
ENGINEERING
TESTING, INC.

SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-91 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>893.1</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mostly sandy silt, a little gravel, surface roots, trace roots, dark brown	FILL	13	M	SS	16					
2	FILL, mixture of silty sand and sand with silt, a little gravel, pieces of brick, brown and black										
3			26	M	SS	21					
4											
5											
6				28	M	SS	23				
7											
8				14	M	SS	18				
9	ORGANIC CLAY, trace roots, trace shells, black, stiff to very soft (OL/OH)	SWAMP DEPOSIT					89				
10	BOGLIME, trace roots, gray, a little black, moist, very soft to firm, lense of sapric peat (OL)						118				
11			2	M	SS	23	149				
12											
13			7	W	SS	20					
14	SAND WITH SILT, a little gravel, gray, waterbearing, loose (SP-SM)	COARSE ALLUVIUM TILL									
15	CLAYEY SAND, a little gravel, gray, very soft to stiff (SC/CL)		2	M	SS	18	17				
16											
17											
18											
19											
20											
21			6	M	SS	17	15				
22											
23											
24											
25											
26			9	M	SS	18	14				
END OF BORING Northing=209184.1 Easting=551898.2											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-24½'	3.25" HSA	6/27/07	11:05	14.0	12.0	12.5		12.0	
BORING COMPLETED: 6/27/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-92 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>898.4</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS						
							WC	DEN	LL	PL	%#200		
1	1.5" Bituminous Pavement	FILL	26	M	SU	3	7						
2	FILL, mixture of silty sand and clayey sand, a little gravel, pieces of bituminous, brown				SS								
3	ORGANIC CLAY, a little gravel, black, a little gray, stiff, laminations of sandy lean clay (OL/OH)	SWAMP DEPOSIT	10	M	SS	17	13						
4	ORGANIC CLAY, trace roots, black, soft (OL/OH)												
5													
6		TILL	4	M	SS	17	46						
7	CLAYEY SAND, a little gravel, gray, soft, laminations of sand with silt (SC)												
8													
9	CLAYEY SAND, a little gravel, trace roots, brown and gray mottled, soft to firm (SC)												
10													
11		TILL	3	M	SS	18	18						
12													
13													
14													
15	SANDY LEAN CLAY, a little gravel, brownish gray, a little black, stiff, laminations of silty sand (CL)	TILL	10	M	SS	23	16						
16													
17													
18	SANDY LEAN CLAY, a little gravel, gray, stiff (CL)	TILL	10	M	SS	24	13						
19													
20													
21													
22		TILL	15	M	SS	21	14						
23													
24													
25													
26													
END OF BORING Northing=209185.3 Easting=552151.9													

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-24 1/2'	3.25" HSA	6/27/07	12:25	26.5	24.5	26.0		None	
BORING COMPLETED: 6/27/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-93 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>901.2</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	FILL, mixture of sand with silt and silty sand, surface roots, trace roots, dark brown	FILL	14	M	SS	16						
2	FILL, mixture of clayey sand and silty sand, a little gravel, trace roots, brown and dark brown											
3			8	M	SS	14						
4	CLAYEY SAND, a little gravel, brown, firm to stiff (SC)	TILL					13					
5							17					
6					8	M	SS	20				
7												
8					9	M	SS	23	16			
9												
10	SANDY LEAN CLAY, a little gravel, light brown to brown, firm to stiff (CL)		7	M	SS	22	16					
11												
12												
13				11	M	SS	24	16				
14												
15												
16			13	M	SS	24	18					
17												
18	SANDY LEAN CLAY, a little gravel, gray, stiff (CL)											
19												
20												
21				11	M	SS	22	16				
22												
23												
24												
25												
26			11	M	SS	23	16					
END OF BORING Northing=209104.1 Easting=552534.6												

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
0-24½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
		6/27/07	2:10	26.5	24.5	26.5			None
BORING COMPLETED: 6/27/07									
DR: SG LG: SB Rig: 91C									

INTERTEC

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-94
	LOCATION: N: 209180.273, E: 552902.330 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/11/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:46 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	Tests or Notes
922.9	0.0	SM	SILTY SAND, trace of Roots, dark brown, moist. (Topsoil)				
918.9	4.0	CL	SANDY LEAN CLAY, trace of Gravel, light brown, wet, rather soft to medium. (Glacial Till)	8			
				5		16	
				5			
				7			
				8			
				7			
904.9	18.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, medium to rather stiff. (Glacial Till)				
				11			
				7			
893.9	29.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, moist, medium. (Glacial Till)				
				7			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-94 (cont.)
	LOCATION: N: 209180.273, E: 552902.330 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/11/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	Tests or Notes
890.9	32.0		SANDY LEAN CLAY, trace of Gravel, gray, moist, medium. (Glacial Till) (continued)				
				7			
881.9	41.0		END OF BORING. Water not observed with 39 1/2 feet of hollow-stem auger in the ground. Boring then grouted.	7			

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:46 (See Descriptive Terminology sheet for explanation of abbreviations)

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-95 LOCATION: N: 209184.739, E: 553397.793 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/11/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:46 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
924.0	0.0					
922.0	2.0	FILL	FILL: Silty Sand, trace of Roots, trace of Roots, dark brown, moist.			
		FILL	FILL: Sandy Lean Clay, mixed light brown, brown and dark brown, moist.	6		
917.0	7.0	SC	CLAYEY SAND, trace of Roots, dark gray, moist. (Buried Topsoil)	9		
915.0	9.0	CL	SANDY LEAN CLAY, trace of Gravel, greenish gray to light brown, wet. (Glacial Till)	10		
				5		
				4	▽	
				5		
907.0	17.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, rather stiff to stiff. (Glacial Till)			
				13		
898.0	26.0			10		
			END OF BORING.			
			Water observed at 13 feet while drilling.			
			Boring then grouted.			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-96
	LOCATION: N: 208934.186, E: 550891.188 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/27/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
892.3	0.0	FILL	FILL: Sandy Lean Clay, dark brown to light brown, wet.			
888.3	4.0	PT	PEAT, dark gray, wet, rather soft to soft. (Swamp Deposit)	10		
883.3	9.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, gray, waterbearing, loose to medium dense. (Lacustrine)	5 3 7 8 12	▽	
869.3	23.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, medium. (Glacial Till)	8		
866.3	26.0		END OF BORING. Water observed at 9 feet while drilling. Boring then grouted.	8		

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:46 (See Descriptive Terminology sheet for explanation of abbreviations)



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-97 (p. 1 of 1)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>891.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	FILL, mixture of sand with silt, silty sand and clayey sand, a little gravel, surface roots, trace roots, dark brown and black	FILL	14	M	SS	17	12					
2												
3	SAND WITH SILT, fine grained, a little gravel, brown, moist, medium dense (SP-SM)	COARSE ALLUVIUM	26	M	SS	10	9					
4												
5												
6												
7												
8												
9	SILTY SAND, a little gravel, trace roots, fine to medium grained, gray, waterbearing, very loose to medium dense (SM)		2	W	SS	14						
10												
11												
12												
13												
14												
15	SANDY SILT, dark gray, waterbearing, medium dense, laminations of lean clay at 20' (ML)	FINE ALLUVIUM	2	W	SS	10						
16												
17												
18												
19												
20												
21	CLAYEY SAND, a little gravel, dark gray, firm (SC)	TILL	16	W	SS	15	24					
22												
23												
24												
25												
26												
END OF BORING Northing=208912.1 Easting=551143.1												

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-9½'	3.25" HSA								
9½'-24½'	3.25" HSA	7/25/07	8:05	9.0	7.0	7.0		6.1	
BORING COMPLETED:	7/11/07								
DR: SG	LG: SB	Rig: 91C							

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota				BORING: ST-98		
DRILLER: K. Keck		METHOD: 3 1/4" HSA, Autohmr		DATE: 6/26/07		SCALE: 1" = 4'
Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
892.8	0.0	FILL	FILL: Sandy Lean Clay, dark brown, moist.			
				11		
				5		
885.8	7.0	PT	PEAT, dark gray, wet, medium. (Swamp Deposit)		▽	
883.8	9.0	SP-SM	POORLY GRADED SAND with SILT, fine- to medium-grained, with Gravel, light brown to gray with rust at 12' sample depth, waterbearing, medium dense to dense. (Glaciofluvium)	*		* 50 blows for 5 inches
				26		
				18		
876.8	16.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, medium. (Glacial Till)			
				7		
				7		
866.8	26.0		END OF BORING. Water observed at 8 feet while drilling. Boring then grouted.			

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:46 (See Descriptive Terminology sheet for explanation of abbreviations)

INTERTEC

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-99 LOCATION: N: 208681.670, E: 550941.756 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/27/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:46 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
892.8	0.0	FILL	FILL: Clayey Sand, trace of Gravel, mixed dark gray to brown, moist to wet.	13				
				5				
				4				
				5		17	43	LL = 30% PI = 15
880.8	12.0	PT	PEAT, dark gray, wet, rather soft. (Swamp Deposit)	4	▽			
				4		322		OC = 69
874.8	18.0	SM	SILTY SAND, fine-grained, gray to brownish-gray, waterbearing, very loose to loose. (Lacustrine)	4				
866.8	26.0		END OF BORING.	7				
			Water observed at 12 feet while drilling.					
			Boring then grouted.					

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-100 LOCATION: N: 208686.177, E: 551398.832 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/26/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING. SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
895.7	0.0	FILL	FILL: Clayey Sand, fine- to medium-grained, trace of Gravel, dark brown to brown, moist.			
			<div style="background-color: yellow; padding: 2px; display: inline-block;"> Fill (Clayey Sand) 0' to 6' Peat 6' to 9' Poorly Graded Sand 9' to 22' </div>			
889.7	6.0	PT	PEAT, Fibrous, dark gray, wet, rather stiff. (Swamp Deposit)	9		
886.7	9.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, gray, waterbearing, loose to medium dense. (Glacial Till)	6	▽	
				11		
				12		
				14		
873.7	22.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, medium. (Glacial Till)	6		
869.7	26.0		END OF BORING. Water observed at 9 feet while drilling. Boring then grouted.	6		



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-101 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>897.9</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	FILL, mixture of sand with silt, silty sand and clayey sand, a little gravel, surface roots, trace roots, brown	FILL	28	M	SS	12						
2												
3			5	M	SS	13	10					
4												
5	CLAYEY SAND, a little gravel, trace roots, dark gray, a little black, very soft, laminations of organic clay (SC)	MIXED ALLUVIUM OR TOPSOIL	1	W/M	SS	4						
6												
7	SANDY LEAN CLAY, a little gravel, trace roots, dark gray, very soft to firm, a lense of sand with silt at 9.5' to 9.9' (CL)	TILL	1	M	SS	2	15					
8												
9												
10			5	M	SS	10	18					
11	CLAYEY SAND, a little gravel, trace roots, gray and brown, stiff (SC)											
12												
13			10	M	SS	19	14					
14	SANDY LEAN CLAY, a little gravel, dark gray, stiff (CL)											
15												
16			10	M	SS	19	14					
17												
18												
19												
20												
21												
22												
23												
24												
25												
26												
	END OF BORING Northing=208686.2 Easting=551895.0											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	
0-24 1/2'	3.25" HSA							
		6/29/07	9:40	9.0	7.0	7.1		None
		6/29/07	9:45	11.5	9.5	10.5		10.4
BORING COMPLETED: 6/29/07		6/29/07	10:00	26.5	24.5	26.5		None
DR: SG LG: SB Rig: 91C								



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-102 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>902.2</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	FILL, mostly sandy silt, a little gravel, surface roots, trace roots, dark brown, possible cobbles	FILL	10	M	SS	3						
2	FILL, moslty clayey sand, a little gravel, trace roots, brown, dark brown and a little gray		5	M	SS	6	12					
3												
4												
5												
6			16	M	SS	7	16					
7	CLAYEY SAND, a little gravel, dark gray, firm to stiff (SC)	TILL										
8			10	M	SS	19	13					
9												
10												
11												
12												
13												
14												
15												
16												
17												
18	SANDY LEAN CLAY, a little gravel, dark gray, stiff (CL)											
19												
20												
21												
22												
23												
24												
25												
26												
END OF BORING Northing=208692.0 Easting=552395.5												

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
0-24½'	3.25" HSA	6/29/07	9:00	26.5	24.5	26.0			None
BORING COMPLETED: 6/29/07									
DR: SG LG: SB Rig: 91C									

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-103 RI-1007-07
	LOCATION: N: 208804.674, E: 552906.273 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/19/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING: SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	Tests or Notes
924.9	0.0						
923.9	1.0	FILL	FILL: Silt, dark brown, moist.				
		FILL	FILL: Lean Clay, brown to dark brown, dry.				
920.9	4.0	CL	SANDY LEAN CLAY, gray with iron staining, moist to wet, rather soft to rather stiff. (Glacial Till)	18			
				8			
				5			
				4		18	
				7			
				10			
906.9	18.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, moist, medium to stiff. (Glacial Till)				
				8			
				8			
				15			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BOREHOLE: ST-103 RI-1007-07 (cont.)
	LOCATION: N: 208804.674, E: 552906.273 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/19/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	Tests or Notes
892.9	32.0		SANDY LEAN CLAY, trace of Gravel, gray, moist, medium to stiff. (Glacial Till) (continued)				
				11			
884.9	40.0		END OF BORING. Water not observed with 38 1/2 feet of hollow-stem auger in the ground. Boring then grouted.	13			

BRAUN BASIC LOG OF BORING. SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: RI-4003-01 ST-104 LOCATION: N: 208682.011, E: 552906.273 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/23/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:41 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
934.9	0.0					
934.4	0.5	FILL	FILL: Silty Sand, fine-grained, dark brown, moist.			
		FILL	FILL: Silty Sand, fine- to coarse-grained, trace of Gravel, with Clay layers, brown, moist.			
930.9	4.0	FILL	FILL: Silty Sand, fine-grained, mixed with Clay, black, moist.	11		
927.9	7.0	OL	ORGANIC CLAY, black wet, very soft to rather soft. (Swamp Deposit)	8		
				2		
				5		
922.9	12.0	CL	SANDY LEAN CLAY, bluish-gray, wet, soft to rather soft. (Lacustrine)	5		
				3		
916.9	18.0	CL	SANDY LEAN CLAY, trace of Gravel, brown, moist, medium. (Glacial Till)	7		
911.9	23.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, moist, medium. (Glacial Till)	6		
908.9	26.0		END OF BORING.			
			Water not observed during drilling.			
			Water not observed with 24 1/2 feet of hollow-stem auger in the ground.			
			Boring then grouted.			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-105 LOCATION: N: 208435.237, E: 551147.610 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/27/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
899.1	0.0					
		FILL	FILL: Clayey Sand, fine- to medium-grained, trace of Gravel, grayish-brown, moist.	13		
895.1	4.0	FILL	FILL: Silty Sand, fine-grained, dark brown, moist.	13		
892.1	7.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown, moist, loose. (Lacustrine)	6		
890.1	9.0	SP-SM	POORLY GRADED SAND with SILT, fine-grained, light brown to grayish-brown, waterbearing, loose to medium dense. (Lacustrine)	8	▽	
				6		
				7		
				13		
877.1	22.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, rather stiff. (Glacial Till)			
873.1	26.0			9		
			END OF BORING. Water observed at 9 feet while drilling. Boring then grouted.			



SUBSURFACE BORING LOG

AET JOB NO: 22-00081 LOG OF BORING NO. ST-106 (p. 1 of 1)
 PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>903.4</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mixture of clayey sand, silty sand and organic clay, a little gravel, surface roots, trace roots, dark brown, brown and black	FILL	11	M	SS	14	9				
2											
3			26	M	SS	19	15				
4								8			
5	LEAN CLAY WITH ORGANICS, trace roots, black, stiff, lense of silty sand (CL)	TOPSOIL	14	M	SS	19	22				
6											
7	SILTY SAND, a little gravel, fine to medium grained, light brown, medium dense (SM)	COARSE ALLUVIUM	15	M	SS	16					
8											
9											
10	SAND WITH SILT, a little gravel, fine to medium grained, light brown, water bearing, medium dense (SP-SM)		16	M	SS	17					
11											
12											
13			23	W	SS	16	13				
14	CLAYEY SAND, a little gravel, brown, very stiff to stiff (SC)	TILL									
15	CLAYEY SAND, a little gravel, gray, soft to stiff (SC)		4	M	SS	17	17				
16											
17											
18											
19											
20											
21			7	M	SS	20	14				
22											
23											
24											
25											
26			11	M	SS	23	14				
END OF BORING Northing=208433.1 Easting=551647.6											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
0-24½'	3.25" HSA	7/11/07	9:20	14.0	12.0	12.4			12.2
		7/11/07	9:30	26.5	24.5	26.4			None
BORING COMPLETED: 7/11/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-107 (p. 1 of 1)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>902.7</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	SAND WITH SILT, fine grained, trace roots, light brown, moist, medium dense (SP-SM)	TOPSOIL	11	M	SS	15	4					
2		COARSE ALLUVIUM										
3				14	M	SS	20					
4												
5												
6												
7												
8					15	M	SS	21				
9												
10												
11					11	W/M	SS	19				
12	SILTY SAND, a little gravel, fine to medium grained, brownish gray, waterbearing, medium dense to very loose, lense of silt with sand at 11' (SM)											
13			9	W	SS	15						
14												
15												
16				2	W	SS	8					
17												
18	CLAYEY SAND, a little gravel, gray, stiff to firm (SC)	TILL										
19												
20												
21					14	M	SS	19	12			
22												
23												
24												
25												
26					8	M	SS	23	15			
END OF BORING Northing=208260.5 Easting=551137.6												

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	
0-24½'	3.25" HSA	7/11/07	10:30	14.0	12.0	12.3		12.1
		7/11/07	10:40	26.5	24.5	24.9		None
BORING COMPLETED: 7/11/07								
DR: SG LG: SB Rig: 91C								

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: RI-1011-02 ST-108 LOCATION: N: 208186.939, E: 551402.993 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/24/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:41 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
906.6	0.0					
906.4	0.2	PAV FILL	2" of Bituminous FILL: Silty Sand, fine- to medium-grained, dark brown, moist.			
902.6	4.0	SP	POORLY GRADED SAND, fine-grained, brown with iron staining, moist, loose to medium dense. (Glacial Outwash)	26		
897.6	9.0	SP	POORLY GRADED SAND, fine-grained, brown, wet to waterbearing, medium dense. (Glacial Outwash)	5		
				11		
				15		
				12	▽	
				22		
888.6	18.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, rather soft. (Glacial Till)			
				5		
880.6	26.0			5		
			END OF BORING. Water observed at 12 1/2 feet while drilling. Boring then grouted.			



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-109 (p. 1 of 1)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>904.5</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	FILL, mostly silty sand, a little gravel, surface roots, trace roots, dark brown FILL, mixture of sand with silt, silty sand and clayey sand, a little gravel, trace roots, dark brown and light brown	FILL	11	M	SS	12						
2						15						
3			7	M	SS	11						
4												
5							16					
6												
7	SANDY LEAN CLAY, a little gravel, gray, a little brown, stiff to firm, laminations of silty sand (CL) Fill (sand w silt) 0.5' to 7' Sandy lean clay 7' to 26.5'	TILL	12	M	SS	19	16					
8			8	M	SS	14	17					
9			9	M	SS	24	16					
10	SANDY LEAN CLAY, a little gravel, dark gray, firm to stiff (CL)											
11												
12			7	M	SS	17	17					
13												
14												
15												
16												
17												
18												
19												
20												
21			8	M	SS	22	16					
22												
23												
24												
25												
26			10	M	SS	20	17					
END OF BORING Northing=208177.9 Easting=551886.3												

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-24½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		7/18/07	9:40	26.5	24.5	24.9		None	
BORING COMPLETED: 7/18/07									
DR: SG LG: SB/BRig: 91C									

INTERTEC

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: RI-1007-05 ST-110 LOCATION: N: 208182.321, E: 552397.605 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/24/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:41 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
913.2	0.0					
912.2	1.0	FILL	FILL: Silty Sand, fine-grained, trace of Gravel, dark brown, moist.			
		CL	SANDY LEAN CLAY, trace of Gravel, brown with iron staining, wet, rather soft to medium. (Glacial Till)	6		
				5		
				8		
904.2	9.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, moist, medium to rather stiff. (Glacial Till)	9		
				8		
				8		
				8		
				12		
887.2	26.0		END OF BORING. Water not observed during drilling. Water not observed with 24 1/2 feet of hollow-stem auger in the ground. Boring then grouted.			

INTERTEC

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: RI-1007-06 ST-111 LOCATION: N: 208184.910, E: 552896.448 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/24/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:41 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
922.1	0.0					
921.1	1.0	FILL	FILL: Silty Sand, fine-grained, with Roots, trace of Gravel, dark brown, moist.			
		FILL	FILL: Organic Clay, with Silty Sand layer, black to dark brown, moist.			
918.1	4.0			11		
		SP	POORLY GRADED SAND, fine-grained, brown, wet, loose. (Glacial Outwash)	8		
915.1	7.0					
		CL	SANDY LEAN CLAY, trace of Gravel, brown, wet, rather soft. (Glacial Till)	4		
913.1	9.0					
		CL	SANDY LEAN CLAY, trace of Gravel, brown and gray, moist, medium to rather stiff. (Glacial Till)	8		
				9		
908.1	14.0					
		CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, medium. (Glacial Till)	7		
				7		
				6		
893.6	28.5			6		
			END OF BORING.			
			Water not observed with 27 feet of hollow-stem auger in the ground.			
			Boring then grouted.			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-112 LOCATION: N: 208188.459, E: 553395.319 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/29/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
936.8	0.0							
936.7	0.1	PAV	2" Bituminous Pavement.					
935.8	1.0	FILL SM	FILL: Poorly Graded Sand with Silt, trace of Gravel, brown, moist. POORLY GRADED SAND with SILT, fine- to medium-grained, trace of Gravel, reddish brown, moist, loose to medium dense. (Glacial Outwash)	7				
				24		4	9	
				28				
927.8	9.0	SP	POORLY GRADED SAND, fine- to medium-grained, reddish brown, moist, medium dense. (Glacial Till)	27				
924.8	12.0	SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish brown, moist, medium dense to dense. (Glacial Till)	32				
918.8	18.0	SP	POORLY GRADED SAND, fine- to medium-grained, trace of Gravel, light brown, dense to very dense.	39				
910.8	26.0			50		2		
			END OF BORING. Water not observed with 24 1/2 feet of hollow-stem auger in the ground. Boring then grouted.					

INTERTEC

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-113 LOCATION: N: 208182.505, E: 553828.508 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/29/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
941.8	0.0	SC	CLAYEY SAND, trace of roots, dark brown, moist. (Topsoil)			
939.5	2.3	SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish brown, moist, medium dense. (Glacial Till)	17		
929.8	12.0	SP	POORLY GRADED SAND, fine- to medium-grained, trace of Gravel, light brown, moist, medium dense to dense. (Glacial Outwash)	16		
915.8	26.0		END OF BORING. Water not observed with 24 1/2 feet of hollow-stem auger in the ground. Boring then grouted.	33		



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-114 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>913.1</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	FILL, mixture of clayey sand and sandy silt, a little gravel, surface roots, trace roots, black and brown	FILL	8	M	SS	6	8					
2												
3	ORGANIC CLAY, trace roots, black, a little gray, firm, laminations of silty sand (OL/OH)	SWAMP DEPOSIT	7	M	SS	15	38					
4							24					
5	LEAN CLAY, gray, very stiff (CL)	FINE ALLUVIUM		▼			44					
6	SANDY SILT, a little gravel, trace roots, gray, a little dark gray, wet, medium dense, laminations of lean clay (ML)		19	M	SS	12	28					
7												
8	SILTY SAND, a little gravel, fine to medium grained, gray, waterbearing, very loose (SM)	TILL	3	W	SS	16	19					
9												
10	SANDY LEAN CLAY, a little gravel, gray, soft to very stiff (CL)		3	M	SS	15	18					
11												
12												
13			9	M	SS	16	16					
14												
15			11	M	SS	16	17					
16												
17												
18												
19												
20			14	M	SS	24	16					
21												
22												
23												
24												
25												
26			17	M	SS	24	16					
END OF BORING Northing=207847.6 Easting=552482.9												

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
0-24½'	3.25" HSA	7/18/07	11:40	9.0	7.0	7.0			5.2
		7/18/07	12:05	26.5	24.5	26.5			None
BORING COMPLETED: 7/18/07									
DR: SG LG: SB/BRig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-115 (p. 1 of 1)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>908.2</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	FILL, mostly silty sand, surface roots, trace roots, dark brown	FILL	15	M	SS	21						
2	FILL, mixture of clayey sand and sand with silt, trace roots, brown and black		20	M	SS	20						
3												
4												
5	SAND WITH SILT, a little gravel, medium to fine grained, brown, moist to wet, medium dense (SP-SM)	COARSE ALLUVIUM	12	M	SS	18						
6												
7												
8												
9												
10												
11			11	W/M	SS	17	15					
12	CLAYEY SAND, a little gravel, dark brown, stiff (SC)	TILL										
13	CLAYEY SAND, a little gravel, dark gray, soft to stiff (SC)		4	M	SS	19	17					
14												
15												
16				7	M	SS	21	16				
17												
18												
19												
20												
21			8	M	SS	23	13					
22												
23												
24												
25												
26			10	M	SS	24	16					
END OF BORING Northing=207758.9 Easting=551498.2												

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-24½'	3.25" HSA	7/11/07	11:20	11.5	9.5	10.1		None	
		7/11/07	11:40	26.5	24.5	24.5		None	
BORING COMPLETED: 7/11/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-116 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>913.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	SILTY SAND, a little gravel, surface roots, trace roots, brown and dark brown, moist, medium dense (SM)	TOPSOIL	13	M	SS	17					
2		COARSE ALLUVIUM									
3	SILTY SAND, trace roots, fine to medium grained, brown, medium dense, lense of clayey sand at 3.5' (SM)	TILL	20	M	SS	18	14				
4											
5	CLAYEY SAND, a little gravel, light brownish gray and gray mottled, very stiff (SC)	TILL	8	M	SS	19	18				
6											
7	CLAYEY SAND, a little gravel, gray and brown mottled, firm to stiff (SC)	TILL	11	M	SS	23	17				
8											
9	SANDY LEAN CLAY, a little gravel, gray, firm to stiff (CL)	TILL	8	M	SS	21	17				
10											
11	SANDY LEAN CLAY, a little gravel, gray, firm to stiff (CL)	TILL	6	M	SS	24	19				
12											
13	SANDY LEAN CLAY, a little gravel, gray, firm to stiff (CL)	TILL	7	M	SS	24	19				
14											
15	SANDY LEAN CLAY, a little gravel, gray, firm to stiff (CL)	TILL	7	M	SS	24	17				
16											
17	SANDY LEAN CLAY, a little gravel, gray, firm to stiff (CL)	TILL	9	M	SS	24	17				
18											
19	SANDY LEAN CLAY, a little gravel, gray, firm to stiff (CL)	TILL	7	M	SS	24	17				
20											
21	SANDY LEAN CLAY, a little gravel, gray, firm to stiff (CL)	TILL	7	M	SS	24	17				
22											
23	SANDY LEAN CLAY, a little gravel, gray, firm to stiff (CL)	TILL	9	M	SS	24	17				
24											
25	SANDY LEAN CLAY, a little gravel, gray, firm to stiff (CL)	TILL	9	M	SS	24	17				
26											
END OF BORING Northing=207685.1 Easting=551917.3											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
0-24½'	3.25" HSA	7/11/07	1:00	26.5	24.5	24.5			None
BORING COMPLETED: 7/11/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-117 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>914.4</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS											
							WC	DEN	LL	PL	%-#200							
1	4" Bituminous Pavement	FILL			SU													
1-2	FILL, mixture of clayey sand, sandy silt and sand with silt, a little gravel, pieces of bituminous, brown, gray and black		18	M	SS	12	8											
2-3	SAND WITH SILT, fine grained, gray, medium dense (SP-SM)	COARSE ALLUVIUM	19	▼	SS	15												
3-4	SAND WITH SILT, fine to medium grained, brownish gray, medium dense (SP-SM)																	
4-5			13	W	SS	16												
5-6																		
6-7																		
7-8			15	W	SS	17												
8-9																		
9-10	SANDY LEAN CLAY, a little gravel, dark gray, firm to stiff (CL)	TILL	8	M	SS	14	17											
10-11																		
11-12																		
12-13			8	M	SS	16	16											
13-14																		
14-15																		
15-16			9	M	SS	22	16											
16-17																		
17-18																		
18-19																		
19-20																		
20-21			10	M	SS	24	17											
21-22																		
22-23																		
23-24																		
24-25																		
25-26			12	M	SS	24	17											
END OF BORING Northing=207683.1 Easting=552397.2																		

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	
0-24½'	3.25" HSA	7/18/07	10:30	6.5	4.5	6.5		3.6
		7/18/07	10:55	26.5	24.5	26.5		None
BORING COMPLETED: 7/18/07								
DR: SG LG: SB/BRig: 91C								

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-118 LOCATION: N: 207677.923, E: 552643.604 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/10/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
914.9	0.0	OL	ORGANIC CLAY, trace of Roots, dark gray, wet. (Topsoil)			
910.9	4.0	SM	SILTY SAND, fine-grained, gray, wet, very loose. (Lacustrine)	7		
905.9	9.0	PT	PEAT, dark gray, wet. (Swamp Deposit)	3	▽	
900.9	14.0	SM	SILTY SAND, fine-grained, gray, waterbearing, very loose. (Lacustrine)	3		
892.9	22.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, medium. (Glacial Till)	2		
888.9	26.0		END OF BORING. Water observed at 6 feet while drilling. Boring then grouted.	3		
				7		

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-119 LOCATION: N: 207678.910, E: 552897.340 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/9/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
929.7	0.0					
929.2	0.5	FILL	FILL: Silty Sand, trace of Roots, dark brown, moist.			
		FILL	FILL: Silty Sand, fine- to medium-grained, trace of Gravel, brown, moist to wet.			
				20		
				13		
				12		
920.7	9.0	CL	SANDY LEAN CLAY, light brown to brown with gray and rust, wet, rather soft to rather stiff. (Glacial Till)			
				5		
				8		
				9		
				19		
907.7	22.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, medium. (Glacial Till)			
				8		*NR Suspected Cobble or Boulder
903.7	26.0		END OF BORING.			
			Water not observed with 24 1/2 feet of hollow-stem auger in the ground.			
			Boring then grouted.			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-120 LOCATION: N: 207684.634, E: 553359.506 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/29/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
940.8	0.0							
940.3	0.5	SM CL	SILTY SAND, trace of roots, dark brown, moist. SANDY LEAN CLAY, trace of Gravel, yellowish brown, moist, rather stiff to medium. (Glacial Till)					
				7				
				5				
				9				
931.8	9.0	CL	LEAN CLAY with Sand, reddish brown, moist, very stiff. (Glaciofluvium)					
				21		16	73	LL = 26% PI = 10%
				21				
				18				
922.8	18.0	SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish brown, moist, medium dense to dense. (Glacial Till)					
				48				
				27				
911.8	29.0	SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish-brown, moist, medium dense to very dense. (Glacial Till)					
				23				

INTERTEC

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-120 (cont.) LOCATION: N: 207684.634, E: 553359.506 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/29/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
908.8	32.0		SILTY SAND, fine- to medium-grained, trace of Gravel, reddish-brown, moist, medium dense to very dense. (Glacial Till) <i>(continued)</i>					
904.8	36.0	SP	No sample recovery at 35 1/2 feet.	*				* 70 blows for 12 inches
898.8	42.0	SP	POORLY GRADED SAND, fine- to medium-grained, brown, moist, dense. (Glacial Till)	29				
894.8	46.0	SP	POORLY GRADED SAND, fine- to coarse-grained, trace of Gravel, brown, moist, very dense. (Glacial Outwash)	53				
			END OF BORING. Water not observed with 44 1/2 feet of hollow-stem auger in the ground. Boring then grouted.					

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota		BORING: ST-121 LOCATION: N: 207682.711, E: 553899.659 See attached sketch.	
DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/2/07	SCALE: 1" = 4'

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
944.6	0.0	FILL	FILL: Silty Sand, trace of Gravel, trace of roots in upper foot, mixed light brown to brown, moist.			
940.6	4.0	CL	SANDY LEAN CLAY, trace of Gravel, grayish brown to brown, with rust, wet, rather soft to rather stiff. (Glacial Till)	24		
935.6	9.0	CL	SANDY LEAN CLAY, trace of Gravel, light brown to brown, rather soft to rather stiff. (Glacial Till)	13		
930.6	14.0	CL-ML	SILTY CLAY, reddish brown, wet. (Glaciofluvium)	4		
				4		
				11		
				21		
				16		
918.6	26.0			10		
			END OF BORING.			
			Water not observed with 24 1/2 feet of hollow-stem auger in the ground.			
			Boring then grouted.			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-122 LOCATION: N: 207684.411, E: 554279.817 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/28/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
959.5	0.0					
		FILL	FILL: Silty Sand, with Gravel, brown, moist.			
957.5	2.0					
		CL	SANDY LEAN CLAY, trace of Gravel, brown, moist, rather soft to medium. (Glacial Till)	5		
				5		
				7		
				7		
945.5	14.0					
		SP	POORLY GRADED SAND, fine- to medium-grained, reddish-brown, moist, medium dense. (Glacial Outwash)	32		
941.5	18.0					
		SC	CLAYEY SAND, trace of Gravel, reddish-brown, moist, very stiff. (Glacial Till)	25		
937.5	22.0					
		SM	SILTY SAND, fine- to medium-grained, reddish-brown, moist, very dense. (Glaciofluvium)	70		
933.5	26.0					
			END OF BORING.			
			Water not observed with 24 1/2 feet of hollow-stem auger in the ground.			
			Boring then grouted.			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: RI-1007-03 ST-123 LOCATION: N: 207468.088, E: 552331.548 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/24/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:41 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
913.4	0.0					
912.4	1.0	FILL	FILL: Silty Sand, fine- to medium-grained, dark brown, moist.			
		FILL	FILL: Sandy Lean Clay, trace of Gravel, brown, moist.			
			Fill (silty sand) 0' to 1' Fill (sandy lean clay) 1' to 6' Sandy Lean Clay 6' to 26'	12		
907.4	6.0	CL	No sample recovery at 5 1/2 feet. SANDY LEAN CLAY, trace of Gravel, gray, moist to wet, rather soft to medium. (Glacial Till)	12		
				7		
				6		
				5		
				6		
				6		
887.4	26.0		END OF BORING. Water observed at 25 feet while drilling. Boring then grouted.	7	▽	

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: RI-4009-01 ST-124
	LOCATION: N: 207429.899, E: 552647.857 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/24/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
923.8	0.0					
922.8	1.0	FILL	FILL: Silty Sand, fine- to medium-grained, trace of Gravel, dark brown, moist.			
		FILL	FILL: Sandy Lean Clay, brown, moist to wet.			
				7		
				12		
				6		
914.8	9.0	OL	ORGANIC CLAY, black, wet. (Swamp Deposit)			
				5		
911.8	12.0	CL	SANDY LEAN CLAY, trace of Gravel, grayish-brown, wet, soft. (Lacustrine)			
				3		
909.8	14.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, medium. (Glacial Till)			
				7		
				6		
897.8	26.0		END OF BORING.			
			Water not observed during drilling.			
			Water not observed with 24 1/2 feet of hollow-stem auger in the ground.			
			Boring then grouted.			
				6		



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-125 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>932.7</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS									
							WC	DEN	LL	PL	%-#200					
1	FILL, mixture of silty sand and silty clay, surface roots, trace roots, possible cobbles, dark brown	FILL	27	M	SS	9	7									
2	FILL, mostly clayey sand with gravel, possible cobbles, trace roots, brown		31	M	SS	11	7									
3																
4																
5																
6																
7																
8																
9																
10	CLAYEY SAND, a little gravel, trace roots, brown, firm (SC)	TILL	7	M	SS	6	10									
11																
12																
13			7	M	SS	11	10									
14	SAND WITH GRAVEL, trace roots, well graded, light brownish gray, moist, medium dense (SW)	COARSE ALLUVIUM TILL	13	M	SS	16	14									
15																
16	CLAYEY SAND, a little gravel, light brownish gray, stiff (SC)															
17																
18	CLAYEY SAND, a little gravel, possible cobbles, brown, very stiff (SC)		20	M	SS	21	10									
19																
20																
21																
22																
23	SAND WITH SILT, a little gravel, fine to medium grained, brown, moist, very dense (SP-SM)	COARSE ALLUVIUM	66	M	SS	NR										
24																
25																
26	END OF BORING Northing=207429.3 Easting=552897.4															

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-24½'	3.25" HSA	7/6/07	10:30	26.5	24.5	26.5		None	
BORING COMPLETED: 7/6/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081 LOG OF BORING NO. ST-125A (p. 1 of 2)
 PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>932.7</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	No samples taken to 29.5', Refer to Log of Boring ST-125										
2											
3											
4											
5											
6											
7											
8											
9											
10											
11											
12											
13											
14											
15											
16											
17											
18											
19											
20											
21											
22											
23											
24											
25											
26											
27											
28											
29											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
0-39½'	3.25" HSA	7/25/07	1:20	41.5	39.5	41.5			None
BORING COMPLETED: 7/25/07									
DR: SG LG: BR Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-125A (p. 2 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
30	SAND, a little gravel, fine to medium grained, light brown, moist, medium dense to dense (SP)	COARSE ALLUVIUM	31	M		SS	20					
31												
32												
33												
34												
35												
36	25	M		SS	19							
37												
38												
39												
40												
41												
<p>END OF BORING Northing=207429.3 Easting=552897.4</p>												

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-126 LOCATION: N: 207401.256, E: 554161.240 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/28/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
957.0	0.0	FILL	FILL: Silty Sand, fine- to medium-grained, trace of Gravel, brown to light brown, moist.	8		
950.0	7.0	OL	ORGANIC CLAY, dark gray, wet, soft to rather soft. (Swamp Deposit)	2		
945.0	12.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, rather soft to medium. (Glacial Till)	4		
931.0	26.0		END OF BORING. Water observed at 12 feet while drilling. Boring then grouted.	9		



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-126A (p. 1 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>957.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	No samples taken to 29.5', Refer to Log of Boring ST-126										
2											
3											
4											
5											
6											
7											
8											
9											
10											
11											
12											
13											
14											
15											
16											
17											
18											
19											
20											
21											
22											
23											
24											
25											
26											
27											
28											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
0-39½'	3.25" HSA	7/23/07	11:15	41.5	39.5	41.5			None
BORING COMPLETED: 7/23/07									
DR: SG LG: BR Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-126A (p. 2 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
30	SILTY SAND, fine to medium grained, brown, moist, very dense (SM)	COARSE ALLUVIUM TILL	79	M	SS	22	7					
31	LEAN CLAY WITH SAND, brown, hard, lense of sand with silt and gravel at 31 feet (CL)											
32												
33	SILTY SAND WITH GRAVEL, fine to medium grained, brown, moist, very dense (SM)		148	M	SS	15						
34												
35												
36												
37												
38	SILTY SAND, a little gravel, fine to medium grained, brown, a little light brown, moist, very dense, lense of sand with silt and gravel at 41 feet (SM)		63	M	SS	24						
39												
40												
41												
END OF BORING Northing=207401.3 Easting=554161.2												

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-127 RI-4006-21 LOCATION: N: 207512.939, E: 554545.415 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/2/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING: SP0605871.GPJ BRAUN.GDT 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
952.8	0.0							
951.8	1.0	FILL	FILL: Silty Sand, trace of Roots, dark brown, moist.					
		FILL	FILL: Clayey Sand, trace of Gravel, dark brown to brown and reddish brown, moist.	8				
946.8	6.0	SC	CLAYEY SAND, Organic, dark gray, wet, soft to medium. (Swamp Deposit)	4				
				4				
				3		35	42	OC = 6
				6				
938.8	14.0	CL	LEAN CLAY, olive gray, wet, rather soft. (Swamp Deposit)	3				
934.8	18.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, rather stiff. (Glacial Till)	12				
926.8	26.0		END OF BORING. Water not observed with 24 1/2 feet of hollow-stem auger in the ground. Boring then grouted.	11		15		

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-128 RI-4006-20 LOCATION: N: 207347.078, E: 554700.746 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/2/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	Tests or Notes
954.1	0.0						
953.1	1.0	FILL	FILL: Silty Sand, trace of roots, dark brown, moist.				
		FILL	FILL: Sandy Lean Clay, trace of Gravel, reddish brown, moist.	10			
				7			
947.1	7.0	CL	SANDY LEAN CLAY, slightly Organic, dark gray to olive gray, soft. (Swamp Deposit)	3			
				2		21	OC = 3%
942.1	12.0	CL	SANDY LEAN CLAY, grayish brown with rust, wet, medium. (Glacial Till)	7			
				7			
936.1	18.0	SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish brown, moist, medium dense to dense. (Glacial Till)	26			
928.1	26.0			33*			*Suspect cobble or boulder
			END OF BORING. Water not observed with 24 1/2 feet of hollow-stem auger in the ground. Boring then grouted.				



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-129 (p. 1 of 1)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>915.9</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	FILL, mixture of clayey sand, silty sand and sand with silt, a little gravel, surface roots, trace roots, dark brown and brown	FILL	18	M	SS	15	6					
2												
3			12	M	SS	15						
4												
5	CLAYEY SAND, a little gravel, dark brown and brown mottled, firm (SC)	TILL	5	M	SS	19	12					
6	CLAYEY SAND, a little gravel, brown and gray mottled, firm, laminations of silt and sandy lean clay (SC)											
7			8	M	SS	19	16					
8												
9												
10	CLAYEY SAND, a little gravel, brown and brownish gray mottled, stiff (SC)			12	M	SS	18	15				
11												
12	CLAYEY SAND, a little gravel, dark gray, firm to stiff (SC)			5	M	SS	22	16				
13												
14				8	M	SS	24	18				
15												
16												
17												
18												
19												
20			9	M	SS	24	18					
21												
22												
23												
24												
25												
26			11	M	SS	24	18					
END OF BORING Northing=207214.0 Easting=551928.3												

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
0-24½'	3.25" HSA	7/11/07	2:05	26.5	24.5	24.5			None
BORING COMPLETED: 7/11/07									
DR: SG LG: SB Rig: 91C									

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-130 LOCATION: N: 207127.116, E: 552428.985 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/13/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
915.1	0.0							
914.8	0.3	FILL	FILL: Silty Sand, fine-grained, dark brown, moist.					
		FILL	FILL: Silt, black, moist.					
				8		9	4	
				5				
908.1	7.0	CL	SANDY LEAN CLAY, brown, wet. (Glacial Till)	5				
906.1	9.0	CL	SANDY LEAN CLAY, trace of Gravel, brown and gray, wet, rather soft to medium. (Glacial Till)	8				
				5				
901.1	14.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, medium. (Glacial Till)	6				
				7				
				7				
889.1	26.0		END OF BORING. Water not observed during drilling. Water not observed with 24 1/2 feet of hollow-stem auger in the ground. Boring then grouted.					



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-131 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>926.9</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mostly sandy silt, surface roots, trace roots, grayish brown	FILL	17	M	SS	17	7				
2	FILL, mixture of clayey sand and sandy lean clay, a little gravel, possible cobbles, trace roots, grayish brown and brown						6				
3			11	M	SS	5	11				
4											
5			6	M	SS	19	17				
6							18				
7	FILL, mixture of sand with silt and silty sand, trace roots, brown and black		8	M	SS	12					
8	SILTY SAND, trace roots, fine grained, dark brown, a little brown, moist, loose (SM)	COARSE ALLUVIUM OR FILL									
9	(possible fill)										
10	SANDY LEAN CLAY, a little gravel, trace roots, gray, a little brown and black, firm (CL)	WEATHERED TILL	6	M	SS	20					
11							21				
12	SANDY LEAN CLAY, a little gravel, light brownish gray, a little brown, stiff to very stiff, laminations of silty sand (CL)	TILL	12	M	SS	15					
13							17				
14			18	M	SS	19					
15							16				
16											
17											
18	SANDY LEAN CLAY, a little gravel, brownish gray to dark gray, very stiff to stiff (CL)										
19			23	M	SS	24					
20							16				
21											
22											
23											
24											
25			14	M	SS	24					
26							16				
END OF BORING Northing=207188.3 Easting=552633.8											

Fill (sandy silt) 0' to 0.5'
 Fill (clayey sand) 0.5' to 7'
 Fill (sand w silt) 7' to 8'
 Silty Sand 8' to 9.5'
 Sandy Lean Clay 9.5' to 26'

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
0-24½'	3.25" HSA	7/6/07	11:45	26.5	24.5	26.5			None
BORING COMPLETED:	7/6/07								
DR: SG	LG: SB	Rig: 91C							



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-132 (p. 1 of 1)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>936.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS								
							WC	DEN	LL	PL	%-#200				
1	4.25" Bituminous Pavement	FILL			SU										
2	FILL, mixture of clayey sand and sandy lean clay, a little gravel, dark brown, light brownish gray and black			9	M	SS	5	9							
3	SILTY SAND, a little gravel, possible cobbles, brown, medium dense to dense (SM)	FILL (clayey sand) 0.3' to 3' Silty Sand 3' to 7' Clayey Sand 7' to 18'			SS	20	11								
4															
5															
6			36	M	SS	18									
7	CLAYEY SAND, a little gravel, possible cobbles, brown, very stiff (SC)	COARSE ALLUVIUM			SS	19	9								
8															
9															
10															
11															
12															
13															
14															
15															
16															
17															
18	SAND WITH SILT AND GRAVEL, fine to medium grained, light brown, moist, dense (SP-SM)	COARSE ALLUVIUM													
19															
20															
21															
22															
23	GRAVELLY SAND WITH SILT, possible cobbles, medium to fine grained, light brown, moist, very dense (SP-SM)	COARSE ALLUVIUM													
24															
25															
26															
	END OF BORING Northing=207171.7 Easting=552904.3														

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
0-24½'	3.25" HSA	7/6/07	9:00	26.5	24.5	25.2			None
BORING COMPLETED: 7/6/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-132A (p. 1 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>936.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	No samples taken in upper 29.5', Refer to Log of Boring ST-132										
2											
3											
4											
5											
6											
7											
8											
9											
10											
11											
12											
13											
14											
15											
16											
17											
18											
19											
20											
21											
22											
23											
24											
25											
26											
27											
28											
29											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
0-39½'	3.25" HSA	7/25/07	2:47	41.5	39.5	41.0			None
BORING COMPLETED: 7/25/07									
DR: SG LG: BR Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-132A (p. 2 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
30	SAND, a little gravel, fine to medium grained, light brown, moist, dense to medium dense (SP)	COARSE ALLUVIUM	49	M		SS	19					
31												
32												
33												
34												
35			39	M		SS	23					
36												
37												
38												
39												
40												
41												
END OF BORING Northing=207171.7 Easting=552904.3												

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-133 RI-4007-20 LOCATION: N: 207181.741, E: 553397.254 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/20/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	Tests or Notes
945.0	0.0						
944.0	1.0	FILL	FILL: Silt, with Roots, dark brown, moist.				
		FILL	FILL: Sand Lean Clay, trace of Gravel, brown, moist.				
941.0	4.0	FILL	FILL: Sandy Lean Clay, reddish-brown, gray and dark brown, moist.	9			
				4			
				3			
				3			
933.0	12.0	FILL	FILL: Silty Sand, fine- to medium-grained, dark brown, wet.	3			
931.0	14.0	SM	SILTY SAND, fine- to medium-grained, reddish-brown, wet, very loose. (Glacial Till)	4			
927.0	18.0	CL	SANDY LEAN CLAY, trace of Gravel, reddish-brown, wet, medium. (Glacial Till)	7			
922.0	23.0	SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish-brown, wet, stiff. (Glacial Till)	16			
917.0	28.0	SP-SM	POORLY GRADED SAND with SILT, fine- to coarse-grained, trace of Gravel, brown, wet, stiff. (Glacial Till)	15			
913.0	32.0						

BRAUN BASIC LOG OF BORING SP-06-05871.GPJ BRAUN.GDT 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)

INTERTEC

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-133 RI-4007-20 (cont.) LOCATION: N: 207181.741, E: 553397.254 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/20/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	Tests or Notes
913.0	32.0	SP	POORLY GRADED SAND, fine- to coarse-grained, trace of Gravel, brown, moist, loose. (Glacial Outwash)				
				7		4	
				10			
899.0	46.0			10		3	
			END OF BORING. Water not observed with 44 1/2 feet of hollow-stem auger in the ground. Boring then grouted.				

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-134 RI-4007-22
	LOCATION: N: 207146.617, E: 553719.021 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/20/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:43
 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
949.0	0.0					
948.0	1.0	FILL	FILL: Silt, with Root Fibers, dark brown, moist.			
		FILL	FILL: Silty Sand, brown to dark brown, moist.			
			With Roots and pieces of topsoil at 5 1/2 feet.	7		
				4		
942.0	7.0	CL	SANDY LEAN CLAY, trace of Gravel, reddish-brown, wet, rather soft to rather stiff. (Glacial Till)	4		
				4		
				10		
935.0	14.0	SM	SILTY SAND, fine- to coarse-grained, trace of Gravel, reddish-brown, wet, medium dense. (Glacial Till)	15		
931.0	18.0	SP	POORLY GRADED SAND, fine- to coarse-grained, trace of Gravel, reddish-brown, wet to waterbearing, very loose to medium dense. (Glacial Outwash)	7		
					▽	
				3		
				10		

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-134 RI-4007-22 (cont.)
	LOCATION: N: 207146.617, E: 553719.021 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/20/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
917.0	32.0		POORLY GRADED SAND, fine- to coarse-grained, trace of Gravel, reddish-brown, wet to waterbearing, very loose to medium dense. (Glacial Outwash) (continued)			
				4		
				24		
906.0	43.0	SP	POORLY GRADED SAND, fine-grained, brown, waterbearing, loose. (Glacial Till)			
903.0	46.0		END OF BORING. Water down 24 feet with 44 1/2 feet of hollow-stem auger in the ground. Boring then grouted.	7		

BRAUN BASIC LOG OF BORING SP0605871.GPI BRAUN.GDT 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-135 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>950.5</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mixture of sand with silt and silty sand, a little gravel, pieces of bituminous, surface roots, trace roots, dark brown	FILL	36	M	SS	15					
2	FILL, mixture of silty sand and clayey sand, a little gravel, trace roots, cinders and clinkers, brown		18	M	SS	13	11				
3											
4											
5	SANDY LEAN CLAY, a little gravel, trace roots, brown, very stiff to hard, laminations of silt (CL)	TILL	20	M	SS	20	14				
6											
7											
8			19	M	SS	23	15				
9											
10			31	M	SS	18	19				
11	CLAYEY SAND WITH GRAVEL, brown, hard, lense of silty sand (SC)						8				
12	SAND WITH GRAVEL, medium to fine grained, brown, moist, very dense (SP)	COARSE ALLUVIUM	56	M	SS	14					
13											
14	GRAVEL WITH SAND, brown, moist, dense (GP)		44	M	SS	4					
15											
16											
17											
18	SAND WITH GRAVEL, medium to fine grained, brown, moist, very dense (SP)		58	M	SS	8					
19											
20											
21											
22											
23											
24											
25			63	M	SS	12					
26											
END OF BORING Northing=207176.8 Easting=553896.4											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	
0-24½'	3.25" HSA	6/28/07	1:15	26.5	24.5	26.4		None
BORING COMPLETED: 6/28/07								
DR: SG LG: SB Rig: 91C								



SUBSURFACE BORING LOG

AET JOB NO: 22-00081 LOG OF BORING NO. ST-135A (p. 1 of 2)
 PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>950.5</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	No samples taken in upper 29.5', Refer to Log of Boring ST-135											
2												
3												
4												
5												
6												
7												
8												
9												
10												
11												
12												
13												
14												
15												
16												
17												
18												
19												
20												
21												
22												
23												
24												
25												
26												
27												
28												
29												

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
0-44½'	3.25" HSA	7/23/07	9:12	46.5	44.5	46.5			None
BORING COMPLETED: 7/23/07									
DR: SG LG: BR Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-135A (p. 2 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
30	SAND WITH SILT AND GRAVEL, medium to fine grained, brown, moist, dense (SP-SM)	COARSE ALLUVIUM	39	M	SS	20					
31	SAND WITH SILT, fine grained, light brown, moist, dense (SP-SM)										
32											
33											
34			40	M	SS	18					
35											
36											
37			29	M	SS	22					
38	SAND WITH SILT, a little gravel, medium to fine grained, brown, a little dark brown, moist, medium dense, lenses and laminations of silty sand (SP-SM)										
39											
40											
41			35	M	SS	20					
42											
43											
44											
45											
46											
END OF BORING Northing=207176.8 Easting=553896.4											

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-136 LOCATION: N: 207181.552, E: 554398.416 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 6/28/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
956.7	0.0	FILL	FILL: Clayey Sand, trace of Gravel, brown, moist.					
				9				
				10				
949.7	7.0	OL	ORGANIC SILT, trace of fibers, dark green, wet, rather soft. (Swamp Deposit)					
947.7	9.0	CL	SANDY LEAN CLAY, trace of Gravel, brown and gray with rust, wet, medium. (Glacial Till)					
				7				
				7				
942.7	14.0	SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish brown, moist, medium dense to dense. (Glacial Till)					
				29				
				28		6	25	LL = 13 PI = 1
930.7	26.0			35				
			END OF BORING. Water not observed with 24 1/2 feet of hollow-stem auger in the ground. Boring then grouted.					

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-137 LOCATION: N: 207060.401, E: 554647.884 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/2/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0603871.GPJ BRAUN.GDT 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
956.0	0.0					
955.0	1.0	FILL	FILL: Silty Sand, with Gravel, brown, moist.			
		FILL	FILL: Clayey Sand, trace of Gravel, reddish-brown, moist.			
				7		
				5		
949.0	7.0	OL	ORGANIC CLAY, trace of Roots, dark brown, moist. (Buried Topsoil)			
947.0	9.0			4		
		SM	SILTY SAND, trace of Gravel, reddish-brown, moist, medium dense to dense. (Glacial Till)			
				7		
				18		
				44		
939.0	17.0	SP	POORLY GRADED SAND, fine- to medium-grained, trace of Gravel, light brown, moist, medium dense. (Glacial Outwash)			
				11		
930.0	26.0			18		
			END OF BORING.			
			Water not observed with 24 1/2 feet of hollow-stem auger in the ground.			
			Boring then grouted.			



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-138 (p. 1 of 2)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>955.9</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	% #200	
1	FILL, mixture of sand with silt and silty sand, surface roots, trace roots, dark brown	FILL	17	M	SS	14	8					
2	FILL, mixture of clayey sand and sand with silt, a little gravel, trace roots, brown, light brown and gray		13	M	SS	7	10					
3												
4												
5				15	M	SS	12	13				
6												
7												
8				9	W/M	SS	15	14				
9												
10												
11												
12												
13												
14												
15	ORGANIC CLAY, trace roots, black, very soft to soft (OL/OH)	SWAMP DEPOSIT	4	M	SS	20	41					
16												
17												
18												
19												
20												
21												
22	LEAN CLAY WITH SAND, trace roots, pieces of wood, dark gray, firm (CL)	FINE ALLUVIUM	8	W/M	SS	4	38					
23												
24												
25	CLAYEY SAND, a little gravel, dark gray, very soft (SC)	TILL	3	W/M	SS	24	23					
26	CLAYEY SAND, a little gravel, gray, soft, lenses and laminations of silty sand (SC)											
27												
28												

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-29½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		6/29/07	11:45	26.5	24.5	25.0		24.3	
		6/29/07	11:50	31.5	29.5	31.5		None	
BORING COMPLETED: 6/29/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081 LOG OF BORING NO. ST-138 (p. 2 of 2)
 PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
30	SANDY LEAN CLAY, a little gravel, brown, stiff (CL) (continued)				SI		17				
31	SAND WITH SILT, a little gravel, fine to medium grained, brown, moist, medium dense (SP-SM)		14	M	SS	24					
	END OF BORING Northing=207165.1 Easting=554854.2	COARSE ALLUVIUM									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-139 (p. 1 of 1)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>951.8</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	FILL, mixture of clayey sand, sandy lean clay, silty sand and sand with silt, a little gravel, trace roots, brown and dark brown	FILL	26	M	SS	12	9					
2												
3			21	M	SS	19	8					
4							10					
5												
6												
7												
8					10	M	SS	15	20			
9									18			
10	ORGANIC CLAY, trace roots, pieces of wood, dark gray, firm, lenses and laminations of silty sand (OL/OH)	SWAMP DEPOSIT	5	W	SS	12	20					
11							38					
12	SANDY SILT, pieces of wood, dark gray, wet, very loose, laminations of silty sand (ML)	FINE ALLUVIUM	4	W/M	SS	14	19					
13							18					
14	SILTY SAND, fine grained, gray, waterbearing, very loose (SM)	COARSE ALLUVIUM										
15	SANDY LEAN CLAY, a little gravel, gray and brown mottled, stiff, laminations of silty sand (CL)	TILL	10	M	SS	19	19					
16												
17												
18	SANDY LEAN CLAY, a little gravel, brown, firm, laminations of silty sand (CL)		8	M	SS	21	19					
19												
20												
21												
22												
23												
24	CLAYEY SAND WITH GRAVEL, brown, hard (SC)		54	M	SS	18	11					
25												
26	SAND WITH SILT, a little gravel, fine to medium grained, brown, moist, very dense (SP-SM)	COARSE ALLUVIUM										

END OF BORING
Northing=206844.7
Easting=553486.0

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	
0-24 1/2'	3.25" HSA	6/28/07	10:44	11.5	9.5	9.8		9.6
		6/28/07	11:05	26.5	24.5	26.5		None
BORING COMPLETED: 6/28/07								
DR: SG LG: SB Rig: 91C								



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-139A (p. 1 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>951.8</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	No samples taken in upper 29.5', Refer to Log of Boring ST-139										
2											
3											
4											
5											
6											
7											
8											
9											
10											
11											
12											
13											
14											
15											
16											
17											
18											
19											
20											
21											
22											
23											
24											
25											
26											
27											
28											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
0-44½'	3.25" HSA	7/23/07	1:16	46.5	44.5	46.5			None
BORING COMPLETED: 7/23/07									
DR: SG LG: BR Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-139A (p. 2 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
30	SILTY SAND WITH GRAVEL, medium to fine grained, brown, moist, dense (SM) (possible cobbles)	COARSE ALLUVIUM	40	M		6					
31											
32											
33											
34											
35											
36											
37											
38	SAND WITH SILT, a little gravel, fine to medium grained, light brown, moist, dense (SP-SM)		33	M		21					
39											
40											
41											
42											
43	SAND, fine grained, light brown, dense (SP)		33	M		20					
44											
45											
46											
END OF BORING Northing=206844.7 Easting=553486.0											

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: RI-4006-15 ST-140 LOCATION: N: 206836.519, E: 554906.041 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/2/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ, BRAUN.GDT 10/2/07 14:41 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
962.9	0.0	CL	SANDY LEAN CLAY, trace of Gravel, yellowish-brown to brown, moist, medium dense. (Glacial Till)	8		
951.9	11.0	SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish-brown, moist. (Glacial Till)	42		
939.9	23.0	SP	POORLY GRADED SAND, fine- to medium-grained, trace of Gravel, brown, moist, very dense. (Glacial Outwash)	54		
936.9	26.0		END OF BORING. Water not observed with 24 1/2 feet of hollow-stem auger in the ground. Boring then grouted.			*70 blows for 6" (set) suspected Cobble or Boulder *50 blows for 1" (set) suspected Cobble or boulder



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-140A (p. 1 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>962.9</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	No samples taken in upper 19.5', Refer to Log of Boring ST-140										
2											
3											
4											
5											
6											
7											
8											
9											
10											
11											
12											
13											
14											
15											
16											
17											
18											
19											
20	CLAYEY SAND, a little gravel, brown, hard (SC)		TILL	37	M	SS	20	9			
21	SAND WITH SILT, a little gravel, fine to medium grained, light brown, moist, dense (SP-SM)		COARSE ALLUVIUM	44	M	SS	16				
22											
23											
24											
25											
26											
27											
27											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
0-44½'	3.25" HSA	7/25/07	11:10	41.5	39.5	41.4			None
BORING COMPLETED: 7/25/07									
DR: SG LG: BR Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-140A (p. 2 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
29	GRAVELLY SAND WITH SILT, medium to fine grained, light brown, moist, very dense (SP-SM)		116	M	SS	14					
30											
31											
32			79	M	SS	17					
33											
34											
35	GRAVEL WITH SILTY SAND, light brown, moist, very dense (GP)		99	M	SS	16					
36											
37											
38	END OF BORING Northing=206836.5 Easting=554906.0										
39											
40											
41											



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-141 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>913.3</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mixture of silty sand and sandy silt, a little gravel, surface roots, trace roots, brown	FILL	17	M	SS	17					
2	FILL, mixture of silty sand, clayey sand and sandy lean clay, a little gravel, trace roots, organic clay, brown, dark brown, gray and black		19	M	SS	22	10				
3											
4											
5			11	M	SS	17	9				
6											
7	CLAYEY SAND, a little gravel, trace roots, brown and dark gray, stiff (SC)	TILL	9	M	SS	17	19				
8	SANDY LEAN CLAY, a little gravel, dark gray and gray mottled, a little brown, firm, laminations of silty sand and clayey sand (CL)		5	M	SS	20	22				
9											
10											
11											
12	SANDY LEAN CLAY, a little gravel, light brown and gray mottled, stiff, laminations of brown silty sand (CL)		10	M	SS	12	19				
13											
14			14	M	SS	24	18				
15											
16											
17											
18	CLAYEY SAND, a little gravel, dark gray, stiff to very stiff (SC)		14	M	SS	24	15				
19											
20											
21											
22											
23											
24											
25			18	M	SS	16	15				
26											
END OF BORING Northing=206683.1 Easting=552396.6											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	
0-24½'	3.25" HSA	7/18/07	1:50	26.5	24.5	26.5		26.5
BORING COMPLETED: 7/18/07								
DR: SG LG: BR Rig: 91C								



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-142 (p. 1 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>933.7</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mostly sandy silt, surface roots, trace roots, dark brownish gray	FILL	17	M	SS	10	9				
2	FILL, mostly clayey sand, a little gravel, possible cobbles, trace roots, brown and grayish brown						8				
3			14	M	SS	4	10				
4											
5			8	M	SS	7	13				
6											
7											
8			2	W/M	SS	2	18				
9											
10	FILL, mixture of silty sand, clayey sand and sand with silt, a little gravel, dark grayish brown, gray and black		18	M	SS	19	20				
11											
12											
13			10	M	SS	14					
14											
15							20				
16	SANDY SILT, trace roots, dark brownish gray, a little gray, moist, loose, lenses and laminations of silty sand (ML)	FINE ALLUVIUM	6	M	SS	18	34				
17	SILT WITH ORGANICS, black, wet, loose (ML)		5	W	SS	23	27				
18											
19											
20	CLAYEY SAND, gray, a little brown, stiff, laminations of wet silty sand (SC)	MIXED ALLUVIUM	9	M/W	SS	19	20				
21											
22											
23											
24	LEAN CLAY, trace roots, gray, a little black, stiff, laminations of fat clay (CL)	FINE ALLUVIUM									
25			11	M	SS	17	32				
26	SILTY SAND, fine grained, gray, wet, medium dense (SM)	COARSE ALLUVIUM									
27	SILTY SAND, a little gravel, fine to medium grained, gray, wet, medium dense (SM)		14	W	SS	20					
28											

Fill (sandy silt) 0' to 0.5'
Fill (clayey sand) 0.5' to 9.5'
Fill (silty sand) 9.5' to 15'



DEPTH: 0-29½'	DRILLING METHOD: 3.25" HSA	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	
		7/5/07	9:35	26.0	24.5	24.8		23.7
		7/5/07	2:00	31.5	29.5	29.9		24.5
BORING COMPLETED: 7/5/07								
DR: SG LG: SB Rig: 91C								



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-142 (p. 2 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
30	SANDY LEAN CLAY, a little gravel, gray, firm (CL)	TILL	5	M	SS	14	19					
31	<p>END OF BORING</p> <p>Northing=206683.8 Easting=552897.3</p>											

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-143 RI-4007-02
	LOCATION: N: 206653.285, E: 553400.699 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/18/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
949.9	0.0					
949.3	0.6	FILL	FILL: Silty Sand, trace of Roots, dark brown, moist.			
		FILL	FILL: Clayey Sand, trace of Gravel, mixed light brown and dark brown, moist.			
945.9	4.0			29		
		SP-SM	POORLY GRADED SAND with SILT, fine- to medium-grained, trace of Gravel, light brown, moist.	18		
940.9	9.0			5		
		CL	SANDY LEAN CLAY, trace of Roots, dark brown, moist. (Buried Topsoil)	4		
937.9	12.0			3		
		CL	SANDY LEAN CLAY, trace of Gravel, grayish-brown, wet, rather soft to medium. (Glacial Till)	6		
930.9	19.0			5		
		CL	SANDY LEAN CLAY, trace of Gravel, reddish-brown, wet, rather soft to rather stiff. (Glacial Till)	10		
921.9	28.0			21		
		SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish-brown, moist, medium dense to dense. (Glacial Till)			

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:43
 (See Descriptive Terminology sheet for explanation of abbreviations)

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-143 RI-4007-02 (cont.)
	LOCATION: N: 206653.285, E: 553400.699 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/18/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
917.9	32.0		SILTY SAND, fine- to medium-grained, trace of Gravel, reddish-brown, moist, medium dense to dense. (Glacial Till) <i>(continued)</i>			
				18		
				35		
903.9	46.0		END OF BORING. Water not observed with 44 1/2 feet of hollow-stem auger in the ground. Boring then grouted.	12		

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)



SUBSURFACE BORING LOG

AET JOB NO: 22-00081 LOG OF BORING NO. ST-144 (p. 1 of 1)
 PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>955.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS							
							WC	DEN	LL	PL	%-#200			
1	Concrete	FILL			SU									
1-2	FILL, mostly sand with silt, a little gravel, brown		10	M	SS	9								
2-3	FILL, mixture of clayey sand and silty sand, a little gravel, brown		21	M	SS	20								
4-5	SILTY SAND, a little gravel, fine grained, brown, medium dense (SM)	TILL					13							
5-6			12	M	SS	20								
6-8			15	M	SS	20	13							
8-9	SILTY SAND, a little gravel, fine to medium grained, brown, medium dense (SM)	COARSE ALLUVIUM												
9-10			12	M	SS	8								
10-11	SAND WITH SILT, a little gravel, medium to fine grained, brown, moist, medium dense (SP-SM)		15	M	SS	12								
11-12														
12-13														
13-14														
14-15	SAND WITH SILT WITH GRAVEL, medium to fine grained, brown, moist, medium dense to dense (SP-SM)													
15-16														
16-17														
17-18														
18-19														
19-20														
20-21														
21-22														
22-23														
23-24														
24-25														
25-26														
26														
END OF BORING Northing=206618.1 Easting=553935.0														

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	
0-24½'	3.25" HSA	7/9/07	8:55	26.5	24.5	26.2		None
BORING COMPLETED: 7/9/07								
DR: SG LG: SB Rig: 91C								



SUBSURFACE BORING LOG

AET JOB NO: 22-00081 LOG OF BORING NO. ST-144A (p. 1 of 2)
 PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>955.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	No samples taken in upper 24.5', Refer to Log of Boring ST-144										
2											
3											
4											
5											
6											
7											
8											
9											
10											
11											
12											
13											
14											
15											
16											
17											
18											
19											
20											
21											
22											
23											
24											

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
0-44½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
		7/24/07	12:02	46.5	44.5	46.0			None
BORING COMPLETED: 7/24/07									
DR: SG LG: BR Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-144A (p. 2 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
25	SAND WITH SILT, a little gravel, fine to medium grained, light brown, moist, dense to very dense (SP-SM)	COARSE ALLUVIUM	35	M		20						
26												
27												
28												
29	SAND WITH SILT, fine to medium grained, light brown, moist, dense to very dense (SP-SM)		51	M		24						
30												
31												
32												
33	SAND WITH SILT, fine to medium grained, light brown, moist, dense to very dense (SP-SM)		39	M		21						
34												
35												
36												
37	SAND WITH SILT, a little gravel, fine to medium grained, light brown, moist, very dense (SP-SM)		53	M		22						
38												
39												
40												
41	SAND WITH SILT, a little gravel, fine to medium grained, light brown, moist, very dense (SP-SM)		57	M		22						
42												
43												
44												
45	END OF BORING Northing=206618.1 Easting=553935.0											
46												



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-145 (p. 1 of 1)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>954.7</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mixture of clayey sand and silty sand, a little gravel, surface roots, trace roots, light brown	FILL TILL	24	M	SS	16	7				
2							9				
3	CLAYEY SAND, a little gravel, trace roots, brown, laminations of silty sand, very stiff (SC)		25	M	SS	13	12				
4							7				
5							14				
6							14				
7											
8							670.9				
9											
10							8				
11			16	M	SS	12					
12	SAND, a little gravel, medium to fine grained, brown, moist, medium dense to dense (SP)	COARSE ALLUVIUM	18	M	SS	14					
13											
14											
15											
16			33	M	SS	14					
17											
18	GRAVELLY SAND WITH SILT, fine to coarse grained, brown, moist, very dense (SP-SM)		60	M	SS	12					
19											
20											
21											
22											
23	SAND WITH SILT, fine to medium grained, brown, moist, dense (SP-SM)		44	M	SS	15					
24											
25											
26											
END OF BORING Northing=206685.1 Easting=554399.4											

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
0-24½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
		6/29/07	1:40	26.5	24.5	26.5			None
BORING COMPLETED: 6/28/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-145A (p. 1 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>954.7</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	No samples taken in upper 29.5', Refer to Log of Boring ST-145											
2												
3												
4												
5												
6												
7												
8												
9												
10												
11												
12												
13												
14												
15												
16												
17												
18												
19												
20												
21												
22												
23												
24												
25												
26												
27												
28												
29												

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-39½'	3.25" HSA	7/24/07	9:55	41.5	39.5	40.4		None	
BORING COMPLETED: 7/24/07									
DR: SG LG: BR Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-145A (p. 2 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
30	SAND WITH SILT, fine grained, light brown, a little black, moist, medium dense, laminations of silt at 31 feet (SP-SM)	COARSE ALLUVIUM	25	M	SS	17					
31											
32											
33	SANDY SILT, light brown, moist, dense (ML)	FINE ALLUVIUM	50	M	SS	19					
34											
35											
36											
37											
38	SAND WITH SILT, a little gravel, medium to fine grained, brown, moist, very dense (SP-SM)	COARSE ALLUVIUM	57	M	SS	22					
39											
40											
41											
<p>END OF BORING Northing=206685.1 Easting=554399.4</p>											

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-146 RI-4006-07 LOCATION: N: 206671.599, E: 554897.200 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/2/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
966.0	0.0							
965.5	0.5	SM	SILTY SAND, trace of Roots, dark brown, moist. (Topsoil)					
		CL	SANDY LEAN CLAY, trace of Gravel, brown, moist. (Glacial Till)	10				
962.0	4.0	CL	SANDY LEAN CLAY, trace of Gravel, yellowish-brown, wet. (Glacial Till)	7				
957.0	9.0	CL	SANDY LEAN CLAY, trace of Gravel, reddish-brown, moist, hard. (Glacial Till)	33		22	54	LL = 29 PI = 17
955.0	11.0	SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish-brown, moist, medium dense to dense. (Glacial Till)	82				NR Suspected Cobble or Boulder
				46		6	25	
				32				
940.0	26.0		END OF BORING.	32				
			Water not observed with 24 1/2 feet of hollow-stem auger in the ground.					
			Boring then grouted.					



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-146A (p. 1 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>966.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	No samples taken in upper 27', Refer to Log of Boring ST-146											
2												
3												
4												
5												
6												
7												
8												
9												
10												
11												
12												
13												
14												
15												
16												
17												
18												
19												
20												
21												
22												
23												
24												
25												
26												

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
0-44½'	3.25" HSA	7/25/07	8:46	46.5	44.5	46.0			None
BORING COMPLETED: 7/25/07									
DR: SG LG: BR Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-146A (p. 2 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
28	GRAVEL WITH SILTY SAND, light brown, moist, dense to very dense (GP)	COARSE ALLUVIUM	79	M		19						
29												
30												
31												
32												
33												
34												
35												
36			57	M	SS	14						
37												
38	SAND, a little gravel, fine to medium grained, light brown, moist, dense (SP)											
39												
40												
41						40	M	SS	17			
42												
43	GRAVELLY SAND WITH SILT, medium to fine grained, light brown, moist, very dense (SP-SM)											
44												
45												
46			56	M	SS	18						
END OF BORING Northing=206671.6 Easting=554897.2												

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: RI-4006-16 ST-147
	LOCATION: N: 206638.252. E:555108.351 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/2/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
958.6	0.0					
958.1	0.5	SM	SILTY SAND, trace of Roots, dark brown, moist. (Topsoil)			
		SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish-brown, moist, medium dense. (Glacial Till)	20		
				22		
				28		
949.6	9.0	SP	POORLY GRADED SAND, fine- to medium-grained, trace of Gravel, light brown, moist, medium dense to very dense. (Glacial Outwash)	22		
				52		Suspected Cobble or Boulder
				34		
				38		
932.6	26.0		END OF BORING.	53		
			Water not observed with 24 1/2 feet of hollow-stem auger in the ground.			
			Boring then grouted.			



SUBSURFACE BORING LOG

AET JOB NO: 22-00081 LOG OF BORING NO. ST-147A (p. 1 of 2)
 PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>958.6</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	No samples taken in upper 29.5', Refer to Log of Boring ST-147										
2											
3											
4											
5											
6											
7											
8											
9											
10											
11											
12											
13											
14											
15											
16											
17											
18											
19											
20											
21											
22											
23											
24											
25											
26											
27											
28											
29											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
0-44½'	3.25" HSA	7/24/07	2:20	45.5	44.5	45.4			None
BORING COMPLETED: 7/24/07									
DR: SG LG: BR Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-147A (p. 2 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
30	GRAVEL WITH SAND, brown, very dense (GP)	COARSE ALLUVIUM	119	M	SS	16					
31											
32											
33											
34											
35	GRAVELLY SAND WITH SILT, medium to fine grained, brown, moist, very dense (SP-SM)		84/0.8	M	SS	14					
36											
37											
38											
39											
40			100/0.5	M	SS	8					
41											
42											
43											
44											
45			62/0.5	M	SS	8					
END OF BORING Northing=206638.3 Easting=555108.4											



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-148 (p. 1 of 2)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>957.9</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS							
							WC	DEN	LL	PL	%-#200			
1	6" Bituminous Pavement	FILL			SU									
2	FILL, mostly gravelly silty sand, brown and light brown		26	M	SS	9								
3	FILL, mixture of sandy lean clay, clayey sand and silty sand, a little gravel, pieces of bituminous, brown, gray, dark gray and brown, a little black		17	M	SS	10	10							
4			20	M	SS	20								
5			1	W	SS	14								
6														
7														
8														
9														
10	ORGANIC CLAY, trace roots, black and dark gray, soft (OL/OH)	SWAMP DEPOSIT			SS									
11	HEMIC PEAT, dark brown, a little gray, laminations of lean clay (PT)		2	M	SS	21	26	182						
12	ORGANIC CLAY, trace roots, black, soft to very soft (OL/OH)		2	M	SS	24	102							
13			1	M	SS	23	31							
14														
15														
16	SILTY CLAY, gray and black, very soft, laminations of silty sand (CL-ML)	FINE ALLUVIUM TILL			SS									
17	CLAYEY SAND, a little gravel, gray and brownish gray, very soft to very stiff (SC)		2	M	SS	16	18							
18			1	M	SS	7	23							
19														
20														
21														
22														
23														
24														
25														
26														
27														
28														
29														
30	CLAYEY SAND, a little gravel, dark brown, very stiff (SC)				SS			10						
31					SS	20		15						

DEPTH: 0-39½'	DRILLING METHOD: 3.25" HSA	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	
		7/17/07	12:05	9.0	7.0	7.4		7.3
		7/17/07	12:45	36.5	34.5	35.7		33.9
BORING COMPLETED: 7/17/07		7/17/07	12:50	41.5	39.5	40.3		39.9
DR: SG	LG: SB/BRig: 91C							











SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-148 (p. 2 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
32	SANDY LEAN CLAY, a little gravel, dark brown, a little brown, very stiff, lense of silty sand at 31.5' (CL)											
33												
34	CLAYEY SAND, a little gravel, trace roots, brown, hard to firm (SC)											
35												
36			36	W	SS	16	10					
37												
38												
39												
40			8	W	SS	19	14					
41												
<p>END OF BORING Northing=206425.9 Easting=554458.6</p>												

INTERTEC

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota					BORING: ST-149		
DRILLER: K. Keck			METHOD: 3 1/4" HSA, Autohmr		DATE: 7/11/07		SCALE: 1" = 4'
Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	Tests or Notes
990.9	0.0						
989.9	1.0	FILL	FILL: Silty Sand, trace of Roots, moist.				
		FILL	FILL: SANDY LEAN CLAY, trace of GRAVEL, mixed light brown to dark brown, moist.	11			
				7			
				5			
				8			
978.9	12.0	FILL	FILL: Silty Sand, fine- to medium-grained, trace of gravel, reddish-brown to grayish-brown, moist to wet.	7			
				4			
					▽		
				6			
968.9	22.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet. (Glacial Till)	4			
				4			
				4		23	
958.9	32.0						

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-149 (cont.)
	LOCATION: N: 206391.747, E: 555391.606 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/11/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING: SP0605871.GPJ BRAUN.GDT 10/2/07 14:43
 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	Tests or Notes
958.9	32.0	CL	SANDY LEAN CLAY, trace of Gravel, reddish-brown, wet.				
				*			*50 blows for 6"
952.9	38.0	SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish-brown, moist, medium dense to hard.	30			
				59		7	
939.9	51.0		END OF BORING. Water observed at 19 feet while drilling. Boring then grouted. Given moist conditions at depth groundwater appears to be trapped in clay layer.	29			



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-150 (p. 1 of 1)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>927.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mostly sandy silt, surface roots, trace roots, dark brownish gray	FILL	34	M	SS	12	7				
2	FILL, mixture of clayey sand and silty sand, a little gravel, trace roots, grayish brown and brown		21	M	SS	19					
5	CLAYEY SAND, trace roots, dark brown, firm (SC) (possible fill)	TOPSOIL OR FILL	7	M	SS	3	16				
8	CLAYEY SAND, a little gravel, brown and light gray mottled, stiff (SC/CL)	TILL	10	M	SS	16	17				
10	CLAYEY SAND, a little gravel, light brownish gray, a little brown, stiff, laminations of silty sand (SC/CL)		11	M	SS	21	18				
16	SANDY LEAN CLAY, a little gravel, brown and dark brown, very stiff (CL)		16	M	SS	24	14				
24	SANDY LEAN CLAY, a little gravel, brown, very stiff, lenses and laminations of sand (CL)		23	M	SS	21	14				
END OF BORING Northing=206182.3 Easting=552896.3											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-24½'	3.25" HSA	7/6/07	2:10	26.5	24.5	26.5		None	
BORING COMPLETED: 7/6/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-151 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>922.3</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mixture of clayey sand and silty sand, surface roots, trace roots, black and brown	FILL	24	M	SS	14	33				
2	CLAYEY SAND, a little gravel, trace roots, brown, very stiff (SC)	TILL	18	M	SS	24	12				
3											
4	SAND WITH SILT, fine to medium grained, a little gravel, brown, wet, medium dense (SP-SM)	COARSE ALLUVIUM	13	W	SS	11					
5											
6	SANDY LEAN CLAY, a little gravel, gray, a little reddish brown, firm to stiff, laminations of silty sand at 7.5' (CL)	TILL	5	M	SS	18	18				
7											
8											
9											
10											
11											
12	LEAN CLAY, brownish gray, a little gray, hard to very stiff, laminations of silt (CL)	FINE ALLUVIUM	7	M	SS	24	15				
13											
14											
15											
16	END OF BORING Northing=206254.2 Easting=553051.5		10	M	SS	21	15				
17											
18											
19											
20			33	M	SS	21	17				
21											
22											
23											
24											
25											
26			25	M	SS	24	25				
27											

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
DEPTH	DRILLING METHOD	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
0-24½'	3.25" HSA	7/9/07	9:35	6.5	4.5	5.2			5.1
		7/9/07	10:00	26.5	24.5	26.4			None
BORING COMPLETED: 7/9/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081 LOG OF BORING NO. ST-152 (p. 1 of 1)
 PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>942.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS							
							WC	DEN	LL	PL	%-#200			
1	5" Bituminous Pavement	FILL			SU									
	FILL, mostly sand with silt and gravel, brown		21	M	SS	5								
2	FILL, mixture of sandy lean clay and clayey sand, with gravel, brown and gray		16	M	SS	11	15							
3														
4														
5			9	M	SS	5	16							
6			7	M	SS	12	24							
7														
8														
9														
10	SILTY SAND, fine grained, dark brown, wet, very loose (SM) (possible fill)	COARSE ALLUVIUM OR FILL	3	W	SS	14								
11														
12	SAND WITH SILT, fine grained, brownish gray, waterbearing, medium dense (SP-SM)	COARSE ALLUVIUM	11	W	SS	13								
13														
14	SILTY SAND, fine grained, gray, wet, medium dense (SM)		15	W	SS	14								
15														
16	CLAYEY SAND, gray, stiff (SC)	TILL					22							
17	CLAYEY SAND, a little gravel, grayish brown, stiff (SC)		9	M	SS	21	17							
18														
19	CLAYEY SAND, a little gravel, brown and gray mottled, stiff (SC)		12	M	SS	21	17							
20														
21														
22														
23	SANDY LEAN CLAY, a little gravel, gray, stiff (CL)		14	M	SS	8	15							
24														
25														
26														
END OF BORING Northing=206184.0 Easting=553397.6														

DEPTH	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-24 1/2'	3.25" HSA	7/5/07	11:50	14.0	12.0	12.5		12.3	
		7/5/07	12:10	26.5	24.5	26.4		None	
BORING COMPLETED: 7/5/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

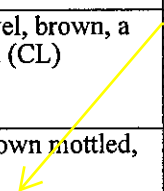
AET JOB NO: 22-00081

LOG OF BORING NO. ST-153 (p. 1 of 1)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>951.2</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mostly gravelly silty sand, possible cobbles, surface roots, trace roots, brown	FILL	22	M	SS	12					
2											
3	FILL, mixture of clayey sand and sandy lean clay, a little gravel, brown and dark brown		25	M	SS	10	13				
4											
5	SANDY LEAN CLAY, a little gravel, brown, a little gray, firm, laminations of sand (CL)		5	M	SS	17	20				
6											
7	CLAYEY SAND, a little gravel, brown mottled, stiff (SC)		12	M	SS	18	17				
8											
9											
10	CLAYEY SAND, a little gravel, light brownish gray, a little brown, mottled, stiff, lenses and laminations of silty sand (SC)		12	M	SS	17	18				
11											
12	SANDY LEAN CLAY, a little gravel, light brownish gray and brown mottled, stiff to very stiff (CL)		14	M	SS	23	15				
13											
14											
15											
16											
17											
18											
19											
20											
21											
22											
23											
24	CLAYEY SAND, a little gravel, possible cobbles, dark grayish brown to brown, hard (SC)		30	M	SS	22	13				
25			100/0.9	M	SS	12	7				
END OF BORING Northing=206179.7 Easting=553901.3											

Fill 0' to 4.5'
Sandy Lean Clay
4.5' to 7'



DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
0-24½'	3.25" HSA	7/5/07	11:05	25.9	24.5	25.3			None
BORING COMPLETED:	7/5/07								
DR:	SG LG: SB Rig: 91C								



AMERICAN
ENGINEERING
TESTING, INC.

SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-153A (p. 1 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>951.2</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	No samples taken in upper 29.5', Refer to Log of Boring ST-153											
2												
3												
4												
5												
6												
7												
8												
9												
10												
11												
12												
13												
14												
15												
16												
17												
18												
19												
20												
21												
22												
23												
24												
25												
26												
27												
28												
29												

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-44½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		7/23/07	3:07	46.5	44.5	46.4		None	
BORING COMPLETED: 7/23/07									
DR: SG LG: BR Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-153A (p. 2 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
30	SILTY SAND, a little gravel, fine to medium grained, brown, moist, very dense (SM)	TILL	73	M	SS	22					
31											
32											
33	CLAYEY SAND WITH GRAVEL, brown, moist, hard to very stiff, lense of silty sand, a little gravel at 46 feet (SC)		56	M	SS	20	11				
34											
35											
36											
37											
38											
39											
40											
41											
42											
43											
44											
45											
46											
END OF BORING Northing=206179.7 Easting=553901.3											

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-154 RI-4008-15 LOCATION: N: 206184.030, E: 554408.064 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/17/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING, SP0605871.GPJ, BRAUN.GDT, 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
956.7	0.0					
955.7	1.0	FILL	FILL: Silty Sand, brown, moist.			
		FILL	FILL: Sandy Lean Clay, with Sand layer, brown, dark brown and reddish-brown, moist.			
			With topsoil lense at 8 feet.			
947.7	9.0	FILL	FILL: Sandy Lean Clay, trace of Gravel, brown, moist.			
945.7	11.0	FILL	FILL: Sandy Lean Clay, with topsoil chunks, brown to dark brown, wet.			
940.7	16.0	SC	With Organic Clay layer at 15 1/2 feet. CLAYEY SAND, trace of Gravel, brown and gray with iron staining, wet, medium. (Lacustrine)			
928.7	28.0	CL	SANDY LEAN CLAY, trace of Gravel, brown with iron staining, moist, stiff. (Glacial Till)			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BOREHOLE: ST-154 RI-4008-15 (cont.) LOCATION: N: 206184.030, E: 554408.064 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/17/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
924.7	32.0		SANDY LEAN CLAY, trace of Gravel, brown with iron staining, moist, stiff. (Glacial Till) <i>(continued)</i>			
				15		
919.7	37.0	SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish-brown, moist, dense. (Glacial Till)			
				38		
915.7	41.0		END OF BORING. Water not observed during drilling. Water not observed with 39 1/2 feet of hollow-stem auger in the ground. Boring then grouted.			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota				BORING: ST-157		
DRILLER: K. Keck		METHOD: 3 1/4" HSA, Autohmr		DATE: 7/10/07		SCALE: 1" = 4'
Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
995.9	0.0					
994.9	1.0	FILL	FILL: Silty Sand, trace of Roots, dark brown, moist.			
		FILL	FILL: Sandy Lean Clay, mixed, light brown to grayish brown, moist.	12		
				7		
				12		
986.9	9.0	FILL	FILL: Silty Sand, fine-grained, trace of Roots at 12' Sample Depth, brown to dark brown, moist.	9		
				6		
981.9	14.0	CL	SANDY LEAN CLAY, trace of Gravel, light brown to brown, moist, medium to very stiff. (Glacial Till)	7		
				18		
				13		
				14		
963.9	32.0					

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:43
 (See Descriptive Terminology sheet for explanation of abbreviations)

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-157 (cont.)
	LOCATION: N: 206185.157, E: 555395.351 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/10/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
963.9	32.0	SM	SILTY SAND, fine-grained, reddish-brown, moist, dense to very dense. (Glaciofluvium)	38		
				38		
				*		*50 blows to for 5" (set) suspected cobble or boulder
948.9	47.0	SP	POORLY GRADED SAND, fine- to medium-grained, trace of Gravel, light brown, moist, very dense. (Glacial Outwash)	74		
				88		
				*		*50 blows for 4" (set)

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-157 (cont.)
	LOCATION: N: 206185.157, E: 555395.351 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/10/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
931.9	64.0		POORLY GRADED SAND, fine- to medium-grained, trace of Gravel, light brown, moist, very dense. (Glacial Outwash) (continued)	52		
				68		
				*		*50 blows for 6" (set)
914.9	81.0		END OF BORING. Water not observed with 79 1/2 feet of hollow-stem auger in the ground. Boring then grouted.	58		

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:43
 (See Descriptive Terminology sheet for explanation of abbreviations)



SUBSURFACE BORING LOG

AET JOB NO: 22-00081 LOG OF BORING NO. ST-158 (p. 1 of 1)
 PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>955.5</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS						
							WC	DEN	LL	PL	% #200		
1	FILL, mixture of sandy lean clay, clayey sand and silty sand, surface roots, trave roots, brown, light brown and gray	FILL	26	M		SS	14	7					
2													
3						35	M	SS	15	8			
4													
5						7	M	SS	18	14			
6													
7													
8						12	M	SS	17	22			
9													
10						15	M	SS	19	15			
11													
12													
13						9	M	SS	15	19			
14	FILL, mixture of clayey sand and sandy lean clay, a little gravel, trace roots, brown and gray, a little black, lense of organic clay, laminations of sandy silt												
15					8	M	SS	14	20				
16													
17													
18					10	M	SS	24	17				
19													
20													
21			9	M	SS	20	19	24					
22	LEAN CLAY, trace roots, gray and black, stiff, lense of organic clay, lense of silty sand (CL)												
23	CLAYEY SAND, a little gravel, gray, soft (SC)				4	M	SS	24	19				
24													
25	SANDY LEAN CLAY, a little gravel, gray, a little brown, stiff, laminations of lean clay (CL)												
26			11	M	SS	24	21						
END OF BORING Northing=206042.2 Easting=554402.2													

← Fill (Sandy lean clay) 0' to 13'

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-24 1/2'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		7/17/07	11:05	26.5	24.5	26.5		None	
BORING COMPLETED: 7/17/07									
DR: SG LG: SB/BRig: 91C									

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-159 LOCATION: N: 206047.880. E: 554563.240 See attached sketch.
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DRILLER: M. Rowland	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/12/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
959.4	0.0	FILL	Poorly Graded Sand with Silt, fine- to medium-grained, trace of Gravel, brown, moist then waterbearing at 20' sample depth. (Lacustrine)			
		FILL	Poorly Graded Sand with Silt, fine- to medium-grained, trace of Gravel, brown, moist then waterbearing at 20' sample depth. (Lacustrine)			
			END OF BORING. Water observed at 18 feet while drilling. Boring then grouted.		▽	

Fill (Poorly graded sand with silt) 0' to 26'



BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-160 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>920.2</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mixture of silty sand and clayey sand, a little gravel, trace roots, surface roots, black and brown	FILL	24	M	SS	13	15				
2	FILL, mixture of sand with silt, clayey sand, and silty sand, a little gravel, trace roots, brown and gray, possible cobbles										
3			21	M	SS	23	10				
4											
5											
6				11	M	SS	12	16			
7	LEAN CLAY WITH ORGANICS, trace roots, black, firm (CL)	TOPSOIL OR SWAMP DEPOSITS	8	M	SS	20	20				
9	SILTY SAND, trace roots, fine grained, dark gray, loose (SM)	COARSE ALLUVIUM									
10	SILTY SAND, trace roots, fine grained, gray, waterbearing, very loose with lenses and laminations of lean clay with sand (SM)		4	W/M	SS	14					
11											
12	SAND WITH SILT, fine grained, brown, waterbearing, medium dense (SP-SM)		11	W	SS	15					
13											
14											
15	CLAYEY SAND, a little gravel, gray, soft to stiff, laminations of silty sand at 26' (SC)	TILL	4	M	SS	17	16				
16											
17											
18											
19											
20											
21			11	M	SS	4	19				
22											
23											
24											
25											
26			10	M	SS	22	16				
END OF BORING Northing=205865.8 Easting=553115.1											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-24½'	3.25" HSA	7/9/07	10:45	14.0	12.0	12.3		11.8	
		7/9/07	11:00	26.5	24.5	26.5		None	
BORING COMPLETED: 7/9/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081 LOG OF BORING NO. ST-161 (p. 1 of 1)
 PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>926.9</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mostly gravelly sand with silt, light grayish brown	FILL	15	M	SS	4					
2	FILL, mixture of sandy lean clay, clayey sand and silty sand, a little gravel, light brownish gray and brown	FILL	15	M	SS	20	12				
3											
4	LEAN CLAY WITH SAND, gray, a little black, firm, laminations of organic clay and silty sand (CL)	FINE ALLUVIUM	7	M	SS	5	13				
5											
6	CLAYEY SAND, a little gravel, gray, stiff (SC)	TILL	10	M	SS	14	18				
7											
8	CLAYEY SAND, a little gravel, light brownish gray and brown mottled, stiff to firm, laminations of silty sand (SC)	TILL	9	M	SS	20	18				
9											
10	CLAYEY SAND, a little gravel, dark gray, stiff (SC)	TILL	8	M	SS	21	18				
11											
12	CLAYEY SAND, a little gravel, dark gray, stiff (SC)	TILL	12	M	SS	21	18				
13											
14	CLAYEY SAND, a little gravel, dark gray, stiff (SC)	TILL	13	M	SS	20	13				
15											
16	CLAYEY SAND, a little gravel, dark gray, stiff (SC)	TILL	14	M	SS	21	16				
17											
18	CLAYEY SAND, a little gravel, dark gray, stiff (SC)	TILL	14	M	SS	21	16				
19											
20	CLAYEY SAND, a little gravel, dark gray, stiff (SC)	TILL	14	M	SS	21	16				
21											
22	CLAYEY SAND, a little gravel, dark gray, stiff (SC)	TILL	14	M	SS	21	16				
23											
24	CLAYEY SAND, a little gravel, dark gray, stiff (SC)	TILL	14	M	SS	21	16				
25											
26	CLAYEY SAND, a little gravel, dark gray, stiff (SC)	TILL	14	M	SS	21	16				
27											
END OF BORING Northing=205683.9 Easting=553397.5											

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
0-24 1/2'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
		7/9/07	3:05	26.5	24.5	26.5			None
BORING COMPLETED: 7/9/07									
DR: SG LG: SB Rig: 91C									

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-162
	LOCATION: N: 205685.324, E: 553896.610 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/3/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
956.7	0.0					
955.7	1.0	SM	SILTY SAND, trace of Roots, dark brown, moist. (Topsoil)			
		CL	SANDY LEAN CLAY, trace of Gravel, yellowish-brown to brown with rust lenses scattered, moist, medium to stiff. (Glacial Till)	7		
				9		
				13		
				14		
				15		
				16		
				13		
				11		
927.7	29.0	CL	SANDY LEAN CLAY, trace of Gravel, reddish-brown, moist, rather stiff. (Glacial Till)	12		

BRAUN BASIC LOG OF BORING. SP0605871.GPJ BRAUN.GDT 10/2/07 14:43
 (See Descriptive Terminology sheet for explanation of abbreviations)

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-162 (cont.) LOCATION: N: 205685.324, E: 553896.610 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/3/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:43 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
924.7	32.0					
923.7	33.0	SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish-brown, moist, dense. (Glacial Till)	35		
918.7	38.0	SP	POORLY GRADED SAND, fine- to coarse-grained, brown, moist, dense. (Glacial Outwash)	40		
910.7	46.0		END OF BORING. Water not observed with 44 1/2 feet of hollow-stem auger in the ground. Boring then grouted.	40		



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-163 (p. 1 of 1)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>949.2</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS								
							WC	DEN	LL	PL	%-#200				
1	7.5" Bituminous Pavement	FILL													
2	FILL, mixture of sand with silt, silty sand and clayey sand with gravel, brown and brownish gray		15	M		12									
3	FILL, mixture of clayey sand and sandy lean clay, a little gravel, gray and brown		19	M		12									
4							17								
5								13							
6				15	M		8								
7								16							
8				3	M		8								
9								18							
10								20							
11				10	M		15								
12								17							
13			5	M		7									
14							19								
15	SAPRIC PEAT, black (PT)	SWAMP DEPOSIT													
16	ORGANIC CLAY, trace roots, black, firm (OL/OH)		7	M		20									
17	LEAN CLAY WITH SAND, dark brownish gray, firm, laminations of sand (CL)	FINE ALLUVIUM													
18			5	M		21									
19	CLAYEY SAND, a little gravel, gray, soft, laminations of fine grained sand (SC)	TILL													
20			3	W/M		14									
21								18							
22															
23															
24															
25															
26															
	END OF BORING Northing=205683.4 Easting=554148.1														

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-24 1/2'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		7/2/07	1:55	26.5	24.5	26.4		None	
BORING COMPLETED: 7/2/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081 LOG OF BORING NO. ST-164 (p. 1 of 1)
 PROJECT: TCAAP Redevelopment; Arden Hills, MN

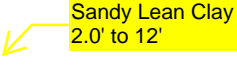
DEPTH IN FEET	SURFACE ELEVATION: <u>951.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS								
							WC	DEN	LL	PL	%-#200				
1	4.25" Bituminous Pavement	FILL			SU										
1	FILL, mixture of silty sand and clayey sand with gravel, brown and gray		19	M	SS	13	8								
2	SANDY LEAN CLAY, a little gravel, brownish gray and brown, stiff (CL)	TILL													
3			15	M	SS	1	16								
4		Sandy Lean Clay 2.0' to 8.5'													
5			15	M	SS	12	14 16								
6															
7															
8			9	M	SS	19	21								
9	SILTY SAND, trace roots, fine grained, dark brown and gray, moist, loose, lenses and laminations of sand with silt (SM)	COARSE ALLUVIUM													
10			4	W/M	SS	12									
11	SAND WITH SILT, fine grained, brown and gray, very loose, moist to wet, laminations of lean clay with sand (SP-SM)														
12	CLAYEY SAND, a little gravel, gray, firm (SC)	TILL													
13			6	M	SS	22	19								
14															
15	CLAYEY SAND, a little gravel, gray and brown mottled, firm (SC)														
16			5	M	SS	18	17								
17															
18	CLAYEY SAND, a little gravel, brown, stiff (SC)														
19															
20															
21			11	M	SS	24	15								
22															
23	CLAYEY SAND, a little gravel, dark gray, very stiff (SC)														
24															
25															
26			18	M	SS	24	15								
END OF BORING Northing=205683.9 Easting=554395.9															

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-24½'	3.25" HSA	7/5/07	10:00	26.5	24.5	26.5		None	
BORING COMPLETED: 7/5/07									
DR: SG LG: SB Rig: 91C									

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-165 RI-4008-36 LOCATION: N: 205684.892, E: 554894.762 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/16/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:44 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
953.1	0.0					
952.8	0.3	PAV	3 1/2" of Bituminous			
		FILL	FILL: Silty Sand, fine-grained, brown, moist.			
951.1	2.0	CL	SANDY LEAN CLAY, trace of Gravel, brown, moist, medium to very stiff. (Glacial Till)	7		
				8		
				13		
				19		
941.1	12.0	ML	SILT, reddish-brown, moist, medium dense. (Glaciofluvium)	24		
940.1	13.0	SP	POORLY GRADED SAND, fine-grained, reddish-brown, moist, medium dense. (Glacial Outwash)	23		
935.1	18.0	ML	SILT, reddish-brown, moist, medium dense. (Glaciofluvium)	22		
930.1	23.0	SM	SILTY SAND, fine-grained, with Cobbles, reddish-brown, moist, dense. (Glacial Till)	39		
927.1	26.0		END OF BORING. Water not observed with 24 1/2 feet of hollow-stem auger in the ground. Boring then grouted.			



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-166 (p. 1 of 1)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>955.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS								
							WC	DEN	LL	PL	%-#200				
1	4.75" Bituminous Pavement	FILL			SU										
2	FILL, mostly gravelly sand with silt, brown (possible base)		19	M	SS	3									
3	SANDY LEAN CLAY, a little gravel, light grayish brown, a little light brown, stiff to very stiff, laminations of sandy silt (CL)	TILL OR WEATHERED TILL	11	M	SS	7	14								
4															
5			16	M	SS	20	15								
6															
7	SANDY LEAN CLAY, a little gravel, light grayish brown, a little brown and light brown, very stiff, laminations of silty sand and sandy silt (CL)	TILL	20	M	SS	18	15	15							
8															
9			23	M	SS	21	15								
10															
11			20	M	SS	17	15								
12															
13			19	M	SS	21	15								
14															
15															
16															
17															
18	CLAYEY SAND, a little gravel, possible cobbles, brown, a little light brownish gray, hard (SC)														
19															
20			88	M	SS	14	13								
21															
22															
23	SAND, a little gravel, medium to fine grained, brown, waterbearing, dense (SP)	COARSE ALLUVIUM													
24															
25															
26	SILTY SAND, fine grained, brown, wet, dense (SM)		41	W	SS	24									
END OF BORING Northing=205683.0 Easting=555397.1															

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	
0-24½'	3.25" HSA	7/5/07	8:35	26.5	24.5	24.1		21.2
BORING COMPLETED: 7/5/07								
DR: SG LG: SB Rig: 91C								



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-167 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>945.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS									
							WC	DEN	LL	PL	%-#200					
1	5.75" Bituminous Pavement	FILL			SU											
2	FILL, mostly gravelly sand with silt, brown (possible base)			20	M	SS	8									
3	FILL, mixture of clayey sand, sandy lean clay and silty sand, a little gravel, brown and grayish brown	WEATHERED TILL			SS	3	9									
4																
5	SANDY LEAN CLAY, a little gravel, grayish brown, firm (CL)			8	M	SS	12	15								
6																
7	SANDY LEAN CLAY, a little gravel, gray, firm, lenses and laminations of organic clay and sand (CL)	TILL			SS	9	20									
8				5	M	SS	9									
9	CLAYEY SAND, gray, a little brown, soft (SC)															
10																
11	CLAYEY SAND, a little gravel, gray, a little brown, soft, laminations of silty sand (SC)			3	M	SS	15	24	24							
12																
13	SANDY LEAN CLAY, a little gravel, brown, soft (CL)				SS	20	22									
14				4	M	SS	20									
15	SANDY LEAN CLAY WITH GRAVEL, gray, stiff (CL)			9	M	SS	17	19								
16																
17																
18	CLAYEY SAND, a little gravel, gray, stiff (SC)															
19																
20																
21			9	M	SS	19	16									
22																
23																
24																
25																
26			13	M	SS	22	15									
END OF BORING Northing=205551.6 Easting=553796.8																

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	
0-24 1/2'	3.25" HSA	7/2/07	8:40	21.5	19.5	21.5		21.2
		7/2/07	8:45	26.5	24.5	26.5		None
BORING COMPLETED: 7/2/07								
DR: SG LG: SB Rig: 91C								



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-168 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>947.2</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS								
							WC	DEN	LL	PL	%-#200				
1	10.5" Bituminous Pavement	FILL			SU										
2	FILL, mostly gravelly sand with silt, brown				M	SS	8								
3	FILL, mostly sandy lean clay, a little sapric peat, trace roots, gray and brown, a little black	WEATHERED TILL			SS	14	19								
4	SANDY LEAN CLAY, trace roots, light brownish gray and light gray, firm, laminations of silt (CL)			5	M	SS	14	16							
5	SANDY LEAN CLAY, a little gravel, trace roots, light brownish gray, a little light gray, stiff to very stiff, laminations of silt (CL)	FILL			SS	17	14								
6	SANDY LEAN CLAY, a little gravel, trace roots, light brownish gray, a little light gray, stiff to very stiff, laminations of silt (CL)			15	M	SS	17	15							
7						SS	19	16							
8				20	M	SS	19	16							
9						SS	14	14							
10				21	M	SS	14	14							
11						SS	18	16							
12	SANDY LEAN CLAY, a little gravel, light brownish gray, a little brown, very stiff, laminations of silty sand (CL)			18	M	SS	18	16							
13						SS	12	14							
14	CLAYEY SAND, a little gravel, grayish brown, a little brown, very stiff (SC)			29	M	SS	12	14							
15					SS	17	15								
16	CLAYEY SAND, a little gravel, brown, very stiff (SC)		21	M	SS	17	15								
17					SS	19	16								
18	SANDY LEAN CLAY, a little gravel, brown, very stiff (CL)		24	M	SS	19	16								
19					SS	6									
20			16	M	SS	6									
21					SS	6									
22					SS	6									
23	GRAVELLY SAND, medium grained, brown, moist, medium dense (SP)	COARSE ALLUVIUM													
24															
25															
26															
END OF BORING Northing=205519.6 Easting=554022.3															

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	
0-24½'	3.25" HSA	7/2/07	11:20	26.5	24.5	26.5		None
BORING COMPLETED: 7/2/07								
DR: SG LG: SB Rig: 91C								



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-169 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>948.1</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS							
							WC	DEN	LL	PL	%-#200			
1	10.75" Bituminous Pavement	FILL												
2	FILL, mixture of sand with silt with gravel and clayey sand with gravel, brown		28	M		11	7							
3	FILL, mixture of sand with silt and crushed limestone with gravel, brown and light brown		26	W		6								
4														
5	FILL, mixture of sandy lean clay, clayey sand and silty sand, a little gravel, gray and brown		16	M		12	13							
6														
7														
8						12	17							
9														
10						19								
11	ORGANIC CLAY, a little gravel, trace roots, gray, very stiff to soft (OL/OH)	SWAMP DEPOSIT					39							
12							24							
13	SANDY LEAN CLAY, a little gravel, gray, soft (CL/SC)	TILL	3	M		20	22							
14	SANDY LEAN CLAY, a little gravel, gray, a little brown, stiff, laminations of silty sand (CL)													
15				15	M		18	17						
16														
17														
18														
19														
20			14	M		19	16							
21														
22														
23														
24														
25														
26			14	M		22	20							
END OF BORING Northing=205580.7 Easting=554087.9														

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	
0-24½'	3.25" HSA	7/2/07	12:20	26.5	24.5	26.5		None
BORING COMPLETED:	7/2/07							
DR: SG	LG: SB	Rig: 91C						



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-170 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>949.7</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	6.25" Bituminous Pavement	FILL										
2	FILL, mixture of sand with silt and gravel and clayey sand, brown and brownish gray		14	M		4						
3	FILL, mixture of clayey sand and sandy lean clay, a little gravel, light brownish gray, gray and a little brown		9	M		6	15					
4							15					
5						17						
6						17						
7												
8						17	14					
9												
10							18					
11	LEAN CLAY WITH ORGANICS, trace roots, black, firm (CL)	TOPSOIL				12	23					
12	SILTY SAND, fine grained, gray and brown mottled, wet, medium dense (SM)	COARSE ALLUVIUM				15						
13												
14												
15	SANDY LEAN CLAY, a little gravel, gray, a little brown, firm, laminations of silty sand (CL)	TILL				19	20					
16												
17												
18	CLAYEY SAND WITH GRAVEL, gray, a little brown, stiff, laminations of silty sand (SC)											
19												
20												
21						21	17					
22												
23	CLAYEY SAND, a little gravel, gray, stiff (SC)											
24												
25												
26						5	17					
END OF BORING Northing=205558.5 Easting=554273.5												

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-24½'	3.25" HSA	7/2/07	2:40	14.0	12.0	13.2		12.6	
		7/2/07	2:55	26.5	24.5	26.5		None	
BORING COMPLETED: 7/2/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-171 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>915.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS								
							WC	DEN	LL	PL	%-#200				
1	3.25" Bituminous Pavement	FILL			SU										
	FILL, mostly sand with silt and gravel, brown		21	M	SS	14									
2	FILL, mixture of sandy lean clay and clayey sand, a little gravel, light grayish brown to light brownish gray														
3			11	M	SS	8	14								
4			4	M	SS	16	16								
5	SAND, a little gravel, medium to fine grained, brown, waterbearing, loose (SP)	TOPSOIL OR COARSE ALLUVIUM													
8			5			SS	17	20							
9	SAND, a little gravel, medium to fine grained, brown, waterbearing, loose (SP)	TOPSOIL OR COARSE ALLUVIUM													
10	SILTY SAND, fine grained, gray and black, wet, very loose, lenses and laminations of organic silt and sand (SM)		3	W	SS	19	18								
12	CLAYEY SAND, a little gravel, trace roots, gray, very soft, lenses of waterbearing sand with silt (SC)	TILL													
13			1	M	SS	22	18								
15	SANDY LEAN CLAY, a little gravel, gray, a 2" thick fine grained sand lenses at 15.6'	TILL													
16			5	M	SS	24	19								
17		TILL													
18															
19															
20															
21			11	M	SS	17	17								
22		TILL													
23															
24															
25															
26			12	M	SS	23	19								
END OF BORING Northing=205644.7 Easting=553192.7															

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-24½'	3.25" HSA	7/6/07	12:50	11.5	9.5	9.5		8.6	
		7/6/07	1:05	26.5	24.5	26.4		None	
BORING COMPLETED: 7/6/07									
DR: SG LG: SB Rig: 91C									

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-172 LOCATION: N: 205439.106, E: 553392.375 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/9/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:44 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
925.1	0.0							
924.6	0.5	FILL	FILL: Silty Sand, trace of Roots, dark brown, moist.					
		FILL	FILL: Sandy Lean Clay, trace of Gravel, mixed light brown to brown, moist.					
				14				
				9	▽			
918.1	7.0	SC	CLAYEY SAND, Organic, dark gray, wet, soft to rather soft. (Swamp Deposit)	4				
				3				
				2		29	48	LL = 36 PI = 23
911.1	14.0	PT	PEAT, dark gray, soft. (Swamp Deposit)	3				
908.1	17.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, very soft to medium. (Glacial Till)					
				WH				
899.1	26.0			6				
			END OF BORING. Water observed at 6 feet while drilling. Boring then grouted.					

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-173 LOCATION: N: 205433.823, E: 553574.478 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/9/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:44 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
942.9	0.0					
941.9	1.0	SM	SILTY SAND, trace of Roots, dark brown, wet. (Topsoil)			
		CL	SANDY LEAN CLAY, trace of Gravel, light brown, moist, very stiff to hard. (Glacial Till)	19		
				46		*NR Suspected Cobble or Boulder
935.9	7.0	CL	SANDY LEAN CLAY, trace of Gravel, light brown to brown with gray and rust, wet, rather soft to rather stiff. (Glacial Till)	9		
				5		
				7		
				9		
				10		
920.9	22.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, very stiff. (Glacial Till)			
				17		
916.9	26.0		END OF BORING.			
			Water not observed with 24 1/2 feet of hollow-stem auger in the ground.			
			Boring then grouted.			



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-174 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>944.9</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mixture of sandy silt and sandy lean clay, a little gravel, possible cobbles, surface roots, trace roots, brown and light brown	FILL	15	M	SS	12	12				
2											
3			17	M	SS	3	13				
4	SANDY LEAN CLAY, a little gravel, possible cobbles, trace roots, gray and brown mottled, a little light gray, very stiff, laminations of sandy silt (CL)	WEATHERED TILL	21	M	SS	19	14				
5											
6	SANDY LEAN CLAY, a little gravel, apparent cobbles, trace roots, light grayish brown, a little brown, very stiff, laminations of sandy silt (CL) (possible fill)	TILL	27	M	SS	16	16				
7											
8			21	M	SS	NR					
9	CLAYEY SAND, a little gravel, grayish brown to gray, very stiff to stiff (SC)		21	M	SS	20	15				
10											
11			21	M	SS	20	15				
12											
13	CLAYEY SAND, a little gravel, apparent cobbles, brown and gray mottled, hard, laminations of sand with silt (SC)		19	M	SS	18	16				
14											
15			15	M	SS	18	16				
16											
17	CLAYEY SAND, a little gravel, apparent cobbles, brown and gray mottled, hard, laminations of sand with silt (SC)		32	M	SS	20	13				
18											
19											
20											
21	CLAYEY SAND, a little gravel, apparent cobbles, brown and gray mottled, hard, laminations of sand with silt (SC)										
22											
23											
24											
25	CLAYEY SAND, a little gravel, apparent cobbles, brown and gray mottled, hard, laminations of sand with silt (SC)										
26											
END OF BORING Northing=205436.5 Easting=553898.8											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-24½'	3.25" HSA	7/2/07	10:10	26.5	24.5	26.4		None	
BORING COMPLETED: 7/2/07									
DR: SG LG: SB Rig: 91C									

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-174
	LOCATION: N: 205436.535, E: 553898.789 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/25/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
944.9	0.0		Power Auger to 29 feet. No samples obtained.			
915.9	29.0	SM	SILTY SAND, fine-grained, trace of Gravel, reddish-brown, moist, medium dense to dense. (Glacial Till)	16		

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:44 (See Descriptive Terminology sheet for explanation of abbreviations)

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-174 (cont.)
	LOCATION: N: 205436.535, E: 553898.789 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/25/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
912.9	32.0		SILTY SAND, fine-grained, trace of Gravel, reddish-brown, moist, medium dense to dense. (Glacial Till) <i>(continued)</i>			
				31		
903.9	41.0		END OF BORING. Water not observed during drilling. Water not observed with 39 1/2 feet of hollow-stem auger in the ground. Boring then grouted.	27		

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:44 (See Descriptive Terminology sheet for explanation of abbreviations)



SUBSURFACE BORING LOG

AET JOB NO: 22-00081 LOG OF BORING NO. ST-175 (p. 1 of 1)
 PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>948.1</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	FILL, mixture of clayey sand and silty sand, a little gravel, surface roots, trace roots, pieces of brick, brown and dark brown	FILL	18	M	SS	15	6					
2												
3	SANDY LEAN CLAY, a little gravel, trace roots, brown, very stiff (CL)	TILL	18	M	SS	19	11	11				
4												
5	CLAYEY SAND, a little gravel, trace roots, brown, stiff (SC)		12	M	SS	17	12					
6												
7	SANDY LEAN CLAY, a little gravel, trace roots, gray, stiff to hard (CL)		30	M	SS	20	15	20				
8												
9	CLAYEY SAND, a little gravel, brown, hard (SC)		10	M	SS	7	16					
10												
11	CLAYEY SAND, a little gravel, brownish gray, stiff to firm, laminations of silty sand (SC)		8	M	SS	7	13					
12												
13	LEAN CLAY WITH SAND, gray and brown mottled, firm, laminations of silty sand (CL)		7	W	SS	16	31					
14												
15	SANDY LEAN CLAY, a little gravel, gray, soft, laminations of brown silty sand (CL)		8	M	SS	18	17					
16												
17	SANDY LEAN CLAY, a little gravel, gray, soft (CL)		7	M	SS	16	18					
18												
19	SANDY LEAN CLAY, a little gravel, gray, soft (CL)		7	M	SS	16	18					
20												
21	SANDY LEAN CLAY, a little gravel, gray, soft (CL)		7	M	SS	16	18					
22												
23	SANDY LEAN CLAY, a little gravel, gray, soft (CL)		7	M	SS	16	18					
24												
25	SANDY LEAN CLAY, a little gravel, gray, soft (CL)		7	M	SS	16	18					
26												
END OF BORING Northing=205434.5 Easting=554148.7												

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
0-24 1/2'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
		7/3/07	8:45	16.5	14.5	15.4			15.1
		7/3/07	8:50	21.5	19.5	20.3			None
BORING COMPLETED: 7/3/07		7/3/07	9:00	26.5	24.5	26.5			None
DR: SG	LG: SB	Rig: 91C							



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-176 (p. 1 of 1)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>951.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mostly sand with silt, a little gravel, surface roots, trace roots, brown	FILL	17	M	SS	8					
2	FILL, mixture of sand with silt and clayey sand, a little gravel, trace roots, brown, dark brown and gray		23	M	SS	14					
3											
4											
5								16			
6											
7											
8				13	M	SS	10	13			
9											
10	LEAN CLAY WITH ORGANICS, trace roots, black and dark brown, stiff, laminations of silty sand (CL)	TOPSOIL	9	M	SS	17	13				
11							29				
12	SANDY LEAN CLAY, a little gravel, trace roots, gray, a little black, stiff to soft (CL)	WEATHERED TILL	9	M	SS	16	20				
13											
14											
15			3	M	SS	20	23				
16	SANDY LEAN CLAY, a little gravel, gray, soft to stiff (CL)	TILL					22				
17											
18											
19											
20											
21			15	M	SS	22	18				
22											
23	CLAYEY SAND WITH GRAVEL, gray (SC)										
24							16				
	END OF BORING , Obstructed to SS at 24.8' Northing=205436.4 Easting=554397.8		50/0.3	M	SS	4					

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
0-24½'	3.25" HSA	6/29/07	3:10	24.8	24.5	24.8			None
BORING COMPLETED: 6/29/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-177 (p. 1 of 1)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>953.6</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mixture of sand with silt and silty sand, a little gravel, surface roots, trace roots, piece of metal at 3", brown FILL, mixture of sand with silt and clayey sand, a little gravel, trace roots, brown and gray	FILL	28	M	SS	12					
2											
3			5								
4											
5											
6											
7											
8											
9											
10											
11											
12											
13			CLAYEY SAND, trace roots, black and dark brown, a little gray, firm, laminations of silty sand (SC) CLAYEY SAND, a little gravel, trace roots, gray and brown, stiff, laminations of sandy lean clay (SC)	WEATHERED TILL OR TOPSOIL TILL	6	M	SS	16	19		
14											
15	CLAYEY SAND, a little gravel, brown and gray mottled, a little black, stiff, laminations of silty sand (SC)	WEATHERED TILL OR TOPSOIL TILL	10	M	SS	9	18				
16											
17											
18											
19											
20											
21											
22											
23											
24											
25											
26											
END OF BORING Northing=205436.7 Easting=555146.5											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
0-24½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
		7/3/07	10:15	26.5	24.5	26.5			None
BORING COMPLETED: 7/3/07									
DR: SG LG: SB Rig: 91C									

INTERTEC

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-179 LOCATION: N: 205297.766, E: 553501.960 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/9/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:44 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
929.3	0.0					
928.8	0.5	SM	SILTY SAND, trace of Roots, dark brown, wet. (Topsoil)			
		SM	SILTY SAND, moist, stiff.			
925.3	4.0	CL	SANDY LEAN CLAY, trace of Gravel, light brown to grayish-brown with rust and dark brown, wet medium to rather stiff. (Glacial Till)	13		
				8		
				8		
				8		
915.3	14.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, medium to rather stiff. (Glacial Till)	9		
				8		
				7		
903.3	26.0		END OF BORING.	10		
			Water not observed with 24 1/2 feet of hollow-stem auger in the ground.			
			Boring then grouted.			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-180 LOCATION: N: 205254.843, E: 553896.986 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/3/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:44 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
935.9	0.0					
934.9	1.0	SM	SILTY SAND, trace of Roots, dark brown, moist. (Topsoil)			
		CL	SANDY LEAN CLAY, trace of Gravel, yellowish-brown to grayish-brown with rust at 5' sample depth, rather stiff tot very stiff. (Glacial Till)	18		
				12		
				14		
				16		
923.9	12.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, rather stiff to stiff. (Glacial Till)	13		
				9		
				10		
909.9	26.0		END OF BORING.	10		
			Water not observed with 24 1/2 feet of hollow-stem auger in the ground.			
			Boring then grouted.			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-181 LOCATION: N: 205262.130, E: 554099.221 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/9/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:44 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
935.0	0.0	SM	SILTY SAND, trace of Roots, dark brown, moist. (Topsoil)			
934.0	1.0	CL	SANDY LEAN CLAY, light brown, moist, rather stiff to very stiff. (Glacial Till)	11		
				15		
				19		
				19		
923.0	12.0	CL	SANDY LEAN CLAY, trace of Gravel, light brown to brown, wet, stiff to very stiff. (Glacial Till)	13		
				16		
				24		
913.0	22.0	CL	SANDY LEAN CLAY, trace of Gravel, reddish-brown to gray, stiff. (Glacial Till)			
909.0	26.0		END OF BORING.	14		
			Water not observed with 24 1/2 feet of hollow-stem auger in the ground.			
			Boring then grouted.			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-182
	LOCATION: N: 205241.842, E: 554395.289 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/3/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPI BRAUN.GPT 10/2/07 14:44 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
941.8	0.0					
941.3	0.5	SM CL	SILTY SAND, trace of Roots, dark brown, moist. (Topsoil)			
			SANDY LEAN CLAY, trace of Gravel, yellowish-brown to brown, rust lenses scattered, moist, very stiff to hard. (Glacial Till)	22		
				23		
				29		
				22		
				26		
				26		
920.8	21.0	SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish-brown, moist, medium dense. (Glacial Till)	34		
915.8	26.0			27		
			END OF BORING.			
			Water not observed with 24 1/2 feet of hollow-stem auger in the ground.			
			Boring then grouted.			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: ST-183
	LOCATION: N: 205240.257, E: 554898.811 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/9/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:44
 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
933.7	0.0					
933.2	0.5	SM CL	SILTY SAND, trace of Roots, dark brown, moist. (Topsoil) SANDY LEAN CLAY, light to brown seams of rust, wet rather soft to rather stiff. (Glacial Till)			
				9		
				8		
				6		
				4		
				6		
				5		
915.7	18.0	SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish-brown, waterbearing, medium dense. (Glaciofluvium)		▽	
				12		
907.7	26.0			16		
			END OF BORING. Water observed at 18 feet while drilling. Boring then grouted.			

Sandy Lean Clay
0.5' to 18'



SUBSURFACE BORING LOG

AET JOB NO: 22-00081 LOG OF BORING NO. ST-184 (p. 1 of 1)
 PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>942.1</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	FILL, mixture of silty sand, sand with silt and clayey sand, a little gravel, surface roots, trace roots, dark brown, brown and black	FILL	15	M	SS	14						
2												
3			10	M	SS	1	9					
4												
5	SILTY SAND, fine grained, brown and dark brown to brownish gray, wet, medium dense (SM)	COARSE ALLUVIUM	12	W	SS	16						
6												
7												
8			15	W	SS	15						
9												
10	SANDY LEAN CLAY, a little gravel, brownish gray, stiff, laminations of waterbearing sand (CL)	TILL	9	M/W	SS	18	20					
11												
12	CLAYEY SAND, a little gravel, gray, firm (SC)	TILL	7	M	SS	22	19					
13												
14												
15												
16			6	M	SS	23	18					
17												
18	SANDY LEAN CLAY, a little gravel, gray, stiff (CL)	TILL	13	M	SS	21	15					
19												
20												
21												
22												
23	CLAYEY SAND, a little gravel, gray, very stiff (SC)	TILL	16	M	SS	18	14					
24												
25												
26												
END OF BORING Northing=205236.9 Easting=555364.5												

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
0-24½'	3.25" HSA	7/3/07	12:50	6.5	4.5	5.8			None
		7/3/07	1:15	26.5	24.5	26.5			None
BORING COMPLETED: 7/3/07									
DR: SG LG: SB Rig: 91C									

INTERTEC

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: RI-1001-06 ST-185
	LOCATION: N: 208603.970, E: 552291.002 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/24/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
901.0	0.0		Redrill of Geo Probe Hole. Power Auger to 16 feet.			
885.0	16.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, medium. (Glacial Till)			
				7		
875.0	26.0		END OF BORING. Water not observed during drilling. Water not observed with 24 1/2 feet of hollow-stem auger in the ground. Boring then grouted.			
				7		

BRAUN BASIC LOG OF BORING SP0605871.GPJ: BRAUN.GDT 10/2/07 14:41 (See Descriptive Terminology sheet for explanation of abbreviations)

Braun Project SP-06-05871
Geotechnical Evaluation
TCAAP Redevelopment
NE of Highway 10 and Highway 96
Arden Hills, Minnesota

BORING: RI-1007-04 ST-186

LOCATION: N: 207579.028, E: 552738.508 See attached sketch.

DRILLER: K. Keck

METHOD: 3 1/4" HSA, Autohmr

DATE: 7/24/07

SCALE: 1" = 4'

BRAUN BASIC LOG OF BORING SP0603871.GPJ BRAUN.GDT 10/2/07 14:41 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
925.2	0.0					
924.2	1.0	FILL	FILL: Silty Sand, fine-grained, dark brown, moist.			
		FILL	FILL: Silty Sand, fine- to medium-grained, mixed with Lean Clay, brown, moist.			
922.2	3.0	FILL	FILL: Lean Clay, mixed with Silty Sand, brown, moist.	18		
				10		
918.2	7.0	SM	SILTY SAND, with Organic fines, black, moist. (Buried Topsoil)	8		
916.2	9.0	SM	SILTY SAND, fine- to coarse-grained, trace of Gravel, brown, wet, rather stiff. (Glacial Till)	9		
914.2	11.0	CL	SANDY LEAN CLAY, trace of Gravel, brown, wet, medium. (Glacial Till)	8		
911.2	14.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, rather stiff to stiff. (Glacial Till)	9		
				13		
899.2	26.0			9		
			END OF BORING. Water not observed during drilling. Water not observed with 24 1/2 feet of hollow-stem auger in the ground. Boring then grouted.			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: RI-1007-08 ST-187 LOCATION: N: 208970.244, E: 552766.455 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/19/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:41 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
914.2	0.0	PT	PEAT, mixed with Sand, black, moist. (Swamp Deposit)			
913.2	1.0	CL	SANDY LEAN CLAY, trace of Gravel, brown with iron staining, moist, medium. (Glacial Till)	8		
				6		
				7		
				7		
902.2	12.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, moist to wet, medium to rather stiff. (Glacial Till)	11		
				10		
				7		
888.2	26.0		END OF BORING. Water not observed with 24 1/2 feet of hollow-stem auger in the ground. Boring then grouted.	7		

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: RI-4003-03 ST-188 LOCATION: N: 209408.636, E: 552935.915 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/23/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:41 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
918.0	0.0					
917.7	0.3	PAV	3" of Bituminous			
		FILL	FILL: Silty Sand, fine- to medium-grained, trace of Gravel, dark brown, moist.			
914.0	4.0			28		
		CL	SANDY LEAN CLAY, trace of Gravel, brown, moist, medium to rather stiff. (Glacial Till)	7		
				9		
907.0	11.0			12		
		CL	SANDY LEAN CLAY, trace of Gravel, reddish-brown, moist, stiff. (Glacial Till)	16		
904.0	14.0					
		SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish-brown, moist, medium dense. (Glacial Till)	22		
899.0	19.0				▽	
		CL	LEAN CLAY, reddish-brown, wet to moist, medium to rather stiff. (Glacial Till)	9		
				8		
889.0	29.0					* Water observed at 19 feet while drilling.
		SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish-brown, moist, dense. (Glacial Till)			Water not observed with 29 1/2 feet of hollow-stem auger in the ground.
887.0	31.0			36		Boring then grouted.
			END OF BORING. *			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: RI-4008-35 ST-189
	LOCATION: N: 205755.343, E: 554523.702 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/17/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
952.2	0.0					
951.5	0.7	SM CL	SILTY SAND, fine-grained, with Organic fines, dark brown, moist. (Topsoil)			
			SANDY LEAN CLAY, brown, moist, medium to rather stiff. (Glacial Till)	12		
				8		
			With Gravel layer at 8 feet.	19		
943.2	9.0	CL	SANDY LEAN CLAY, trace of Gravel, brown with iron staining, moist, very stiff (Glacial Till)	18		
940.2	12.0	CL	SANDY LEAN CLAY, trace of Gravel, brown, moist, stiff. (Glacial Till)	14		
				15		
934.2	18.0	SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish-brown, moist, dense. (Glacial Till)	35		
926.2	26.0		END OF BORING. Water not observed during drilling. Water not observed with 24 1/2 feet of hollow-stem auger in the ground. Boring then grouted.	40		

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: RI-4008-36 ST-190 LOCATION: N: 205738.823, E: 554735.210 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/17/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	Tests or Notes
952.8	0.0						
952.5	0.3	PAV	4" of Bituminous				
950.8	2.0	FILL	FILL: Poorly Graded Sand with Silt, fine- to medium-grained, trace of Gravel, brown, moist.				
948.8	4.0	SC	CLAYEY SAND, gray with iron staining, moist, stiff. (Glacial Till)	16			
		CL	SANDY LEAN CLAY, trace of Gravel, brown and gray with iron staining, wet, rather soft to medium. (Glacial Till)	6		15	
			Clayey Sand 2.0' to 4' Sandy Lean Clay 4' to 12'	6			
				5			
940.8	12.0	CL	SANDY LEAN CLAY, trace of Gravel, brown and gray with iron staining, wet, soft. (Glacial Till)	2			
938.8	14.0	CL	SANDY LEAN CLAY, trace of Gravel, brown, moist, medium to very stiff. (Glacial Till)	8			
				19			
929.8	23.0	SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish-brown, moist, medium dense. (Glacial Till)				
926.8	26.0		END OF BORING.	26			
			Water not observed during drilling.				
			Water not observed with 24 1/2 feet of hollow-stem auger in the ground.				
			Boring then grouted.				

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: RI-4008-40 ST-191
	LOCATION: N: 205745.555, E: 555106.431 See attached sketch.

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/16/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
956.3	0.0					
955.3	1.0	FILL	FILL: Silty Sand, fine- to medium-grained, with Organic fines, dark brown, moist.			
		FILL	FILL: Silty Sand, fine- to medium-grained, trace of Gravel and Roots, brown, moist.			
952.3	4.0	CL	SANDY LEAN CLAY, trace of Gravel, brown, moist, loose to medium dense. (Glacial Till)	33		
				9		
				17		
				14		
				13		
942.3	14.0	SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish-brown, moist, medium dense to dense. (Glacial Till)	23		
				34		
930.3	26.0			25		
			END OF BORING.			
			Water not observed during drilling.			
			Water not observed with 24 1/2 feet of hollow-stem auger in the ground.			
			Boring then grouted.			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: RI-4008-42 ST-192 LOCATION: N: 205926.630, E: 555134.452 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/16/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
959.1	0.0		Soil samples taken with Geoprobe to 6 foot depth.			
953.1	6.0	SM	SILTY SAND, fine-grained, trace of Gravel, reddish-brown, moist, medium dense. (Glacial Till)	16		
948.1	11.0	SP	POORLY GRADED SAND, fine- to medium-grained, brown, moist, loose to medium dense. (Glacial Outwash)	29		
				24		
				8		
941.1	18.0	SM	SILTY SAND, fine-grained, trace of Gravel, reddish-brown, moist, medium dense. (Glacial Till)			
938.1	21.0	SP	POORLY GRADED SAND, fine- to coarse-grained, trace of Gravel, brown, moist, medium dense. (Glacial Outwash)	13		
933.1	26.0		END OF BORING.	26		
			Water not observed during drilling.			
			Water not observed with 24 1/2 feet of hollow-stem auger in the ground.			
			Boring then grouted.			

Braun Project SP-06-05871		BORING: RI-4008-43 ST-193	
Geotechnical Evaluation		LOCATION: N: 206148.097, E: 555162.879 See attached sketch.	
TCAAP Redevelopment			
NE of Highway 10 and Highway 96			
Arden Hills, Minnesota			

DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/16/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
961.9	0.0					
		FILL	FILL: Silty Sand, fine- to medium-grained, with Organic fines, trace of Gravel, dark brown, moist.			
957.9	4.0			40		
		SM	SILTY SAND, fine- to medium-grained, trace of Gravel, reddish-brown, moist, medium dense to dense. (Glacial Till)	17		
				44		
			Cobble at 10 1/2 feet.	38		
			Cobbles at 13 feet.	37		
945.9	16.0			33		
		SP	POORLY GRADED SAND, fine- to medium-grained, trace of Gravel, brown, moist, medium dense to dense. (Glacial Outwash)			
943.9	18.0					
		SP	POORLY GRADED SAND, fine- to coarse-grained, trace of Gravel, brown, moist, medium dense to dense. (Glacial Outwash)			
				34		
				16		* Water not observed during drilling.
						Water not observed with 29 1/2 feet of hollow-stem auger in the ground.
930.9	31.0			30		Boring then grouted.
			END OF BORING. *			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: RI-4008-44 ST-194 LOCATION: See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/16/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
0.0					
0.5	FILL	FILL: Silty Sand, fine- to medium-grained, with Organics, dark brown, moist.			
	FILL	FILL: Sandy Lean Clay, brown, moist.			
4.0	FILL	FILL: Silty Sand, fine- to medium-grained, trace of Gravel, reddish-brown, moist.	25		
9.0		With trace of topsoil at 8 feet.	56		
	SM	SILTY SAND, fine- to medium-grained, with Sand layers, trace of Gravel, reddish-brown, moist, very dense. (Glacial Till)	68		
			*		* 50 blows for 6 inches
16.0	SP	POORLY GRADED SAND, fine- to coarse-grained, trace of Gravel, brown, moist, dense. (Glacial Outwash)	83		
			32		
			35		
28.0		END OF BORING. *	33		* Water not observed during drilling.
					Water not observed with 24 1/2 feet of hollow-stem auger in the ground.
					Boring then grouted.

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: RI-4009-06 ST-195 LOCATION: N: 206589.315, E: 553291.605 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/18/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	Tests or Notes
942.2	0.0						
941.9	0.3	PAV FILL	3" of Bituminous FILL: Poorly Graded Sand with Silt, fine-grained, brown, moist.	8			
937.7	4.5	CL	SANDY LEAN CLAY, brown, wet, rather soft. (Lacustrine)	4		15	
933.2	9.0	CL	SANDY LEAN CLAY, trace of Gravel, brown with iron staining, moist, rather stiff. (Glacial Till)	10			
930.2	12.0	CL	SANDY LEAN CLAY, trace of Gravel, reddish-brown, moist, stiff to very stiff. (Glacial Till)	16			
924.2	18.0	CL	SANDY LEAN CLAY, trace of Gravel, grayish-brown, moist, very stiff. (Glacial Till)	26			
921.2	21.0		END OF BORING. Water not observed during drilling. Water not observed with 19 1/2 feet of hollow-stem auger in the ground. Boring then grouted.	17			

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: RI-4009-07 ST-196 LOCATION: N: 206407.349, E: 553441.134 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/18/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
943.4	0.0					
943.1	0.3	PAV	3" of Bituminous			
		FILL	FILL: Poorly Graded Sand with Silt, fine-grained, brown, moist.			
939.4	4.0	FILL	FILL: Sandy Lean Clay, with topsoil chunks, olive, wet.			
936.4	7.0	CL	SANDY LEAN CLAY, brown with iron staining, wet, rather soft. (Lacustrine)	4		
931.4	12.0	CL	SANDY LEAN CLAY, trace of Gravel, brown and gray with iron staining, moist, rather soft to stiff. (Glacial Till)	4		
925.4	18.0	CL	SANDY LEAN CLAY, trace of Gravel, brown, wet, stiff. (Glacial Till)	5		
922.4	21.0		END OF BORING. Water observed at 18 feet while drilling. Boring then grouted.	8		
				13	▽	

Fill (poorly graded sand) 0.3' to 4'
 Fill (sandy lean clay) 4' to 7'
 Sandy Lean Clay 7' to 21'



Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: RI-4009-08 ST-197 LOCATION: N: 206255.301, E: 553565.997 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/18/07	SCALE: 1" = 4'
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Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
944.3	0.0	PAV	3" of Bituminous			
	0.3	FILL	FILL: Silty Sand, fine- to medium-grained, trace of Gravel, reddish-brown, moist.			
			Fill (silty sand) 0.3' to 4' Fill (sandy lean clay) 4' to 7' Organic Clay 7' to 9'			
940.3	4.0	FILL	FILL: Sandy Lean Clay, trace of Gravel, brown, gray and olive, moist.	11		
				7		
937.3	7.0	OL	ORGANIC CLAY, black, wet. (Swamp Deposit)	2		
935.3	9.0	SM	SILTY SAND, fine-grained, gray, waterbearing, very loose. (Lacustrine)	2	▽	
933.3	11.0	CL	SANDY LEAN CLAY, gray with iron staining, wet, soft. (Lacustrine)	3		
930.3	14.0	CL	SANDY LEAN CLAY, trace of Gravel, brown, wet, soft to medium. (Glacial Till)	3		
923.3	21.0		END OF BORING. Water observed at 9 feet while drilling. Water down 9 feet with 19 1/2 feet of hollow-stem auger in the ground. Boring then grouted.	6		

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Braun Project SP-06-05871
Geotechnical Evaluation
TCAAP Redevelopment
NE of Highway 10 and Highway 96
Arden Hills, Minnesota

BORING: **RI-4009-09 ST-198**

LOCATION: N: 206068.255, E: 553728.185 See attached sketch.

DRILLER: K. Keck

METHOD: 3 1/4" HSA, Autohmr

DATE: 7/17/07

SCALE: 1" = 4'

BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
944.4	0.0	PAV	4" of Bituminous					
		FILL	FILL: Silty Sand, fine- to medium-grained, trace of Gravel, brown, moist.					
			No sample recovery at 5 1/2 feet.	19				
				6				
936.4	8.0	FILL	FILL: Clayey Sand, gray, brown and olive, moist.	1				
				7		17	47	
				5		15		
930.4	14.0	CL	SANDY LEAN CLAY, with Organic fines, black, wet. (Swamp Deposit)	3				
925.4	19.0	CL	SANDY LEAN CLAY, gray, wet, soft. (Lacustrine)	3				
920.4	24.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, very stiff.					
918.4	26.0		(Glacial Till)	19				
			END OF BORING.					
			Water not observed during drilling.					
			Water not observed with 24 1/2 feet of hollow-stem auger in the ground.					
			Boring then grouted.					

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: RI-4009-10 ST-199 LOCATION: N: 205827.000, E: 553879.890 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/17/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	MC %	P200 %	Tests or Notes
944.2	0.0							
943.7	0.5	FILL	FILL: Silt, brown, moist.					
		FILL	FILL: Sandy Lean Clay, brown, dry.					
940.2	4.0	FILL	FILL: Silty Sand, fine- to medium-grained, brown, moist.	20				
937.2	7.0	CL	SANDY LEAN CLAY, trace of Gravel, brown, wet, medium to stiff. (Glacial Till)	4		7	20	
				8				
				7				
				7				
				11				
923.2	21.0		No sample recovery at 20 1/2 feet. END OF BORING. Water not observed during drilling. Water not observed with 19 1/2 feet of hollow-stem auger in the ground. Boring then grouted.	16				

Braun Project SP-06-05871 Geotechnical Evaluation TCAAP Redevelopment NE of Highway 10 and Highway 96 Arden Hills, Minnesota	BORING: RI-4009-11 ST-200 LOCATION: N: 205748.933, E: 554156.383 See attached sketch.
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DRILLER: K. Keck	METHOD: 3 1/4" HSA, Autohmr	DATE: 7/17/07	SCALE: 1" = 4'
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BRAUN BASIC LOG OF BORING SP0605871.GPJ BRAUN.GDT 10/2/07 14:42 (See Descriptive Terminology sheet for explanation of abbreviations)

Elev. feet	Depth feet	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	BPF	WL	Tests or Notes
949.2	0.0					
948.0	1.2	FILL	FILL: Silty Sand, fine- to medium-grained, trace of Gravel, dark brown, moist.			
		FILL	FILL: Poorly Graded Sand with Silt, fine- to medium-grained, trace of Gravel, brown, moist.	21		
945.2	4.0	FILL	FILL: Sandy Lean Clay, trace of Gravel and wood, brown to dark brown, wet.	8		
				7		
935.2	14.0	SC	CLAYEY SAND, black, wet. (Swamp Deposit)	5		
933.2	16.0	SC	CLAYEY SAND, gray, wet, soft. (Lacustrine)	4		
				2	▽	
927.2	22.0	CL	SANDY LEAN CLAY, trace of Gravel, gray, wet, rather soft. (Glacial Till)			
923.2	26.0		END OF BORING.	2		
			Water down 20 feet with 24 1/2 feet of hollow-stem auger in the ground.			
			Boring then grouted.			



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-201/AB- 1 (p. 1 of 2)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>956.8</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1-2	FILL, mixture of silty sand and clayey sand, a little gravel, surface roots, trace roots, brown and dark brown	FILL	12	M	SS	7					
3-4	FILL, mixture of sand with silt and clayey sand, a little gravel, brown, a little gray and light brown		14	M	SS	17	7				
5-6	SANDY LEAN CLAY, a little gravel, brown, a little light brown and dark brown, stiff, laminations of silt and silty sand (CL)	WEATHERED TILL	14	M	SS	23	15				
7-8	SANDY LEAN CLAY, a little gravel, brown, a little light brown and dark brown, stiff, laminations of silt and silty sand (CL)	TILL	29	M	SS	16	8				
9-10	CLAYEY SAND, a little gravel, brown, a little light brown, very stiff, laminations of silt (SC)		24	M	SS	18	7				
11-13			21	M	SS	22	7				
14-15	SAND WITH SILT AND GRAVEL, medium to fine grained, brown, moist, dense (SP-SM)	COARSE ALLUVIUM	40	M	SS	7					
16-18			26	M	SS	22					
19-20	SAND WITH SILT, a little gravel, fine to medium grained, light brown, moist, medium dense to dense (SP-SM)		47	M	SS	19					
21-22											
23-24											
25-26											
27-28											

Fill 0' to 4.5'
Sandy Lean Clay
4.5' to 7'

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-34½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		7/26/07	8:52	31.5	34.5	36.5		None	
BORING COMPLETED: 7/26/07									
DR: SG	LG: BR	Rig: 91C							



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-201/AB- 1 (p. 2 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	% #200	
30	SAND WITH SILT, a little gravel, fine to medium grained, light brown, moist, medium dense to dense (SP-SM) (continued)		40	M	SS	24						
31												
32												
33												
34												
35			45	M	SS	20						
36												
<p>END OF BORING Northing=206215.3 Easting=554201.0</p>												



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-202/AB-2 (p. 1 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>955.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mostly silty sand, a little gravel, trace roots, brown	FILL	57	M	SS	7					
2											
3	SANDY SILT, a little gravel, brown, moist, medium dense (ML)	TILL	24	M	SS	17	11				
4											
5	CLAYEY SAND, a little gravel, brown, very stiff (SC)										
6	SILTY SAND, a little gravel, possible cobbles, fine to medium grained, brown, moist, medium dense (SM)		19	M	SS	15					
7		COARSE ALLUVIUM									
8	SAND, a little gravel, brown, moist, medium dense (SP) (possible fill)		20	M	SS	16					
9	SAND WITH GRAVEL, medium grained, brown, moist, medium dense (SP)										
10	SAND WITH GRAVEL, possible cobbles, medium to fine grained, brown, moist, dense (SP)		37	M	SS	14					
11											
12	GRAVEL WITH SAND, brown, moist, medium dense (GP)		27	M	SS	8					
13											
14	SAND, fine to medium grained, light brown, moist, dense, laminations of silty sand (SP)		40	M	SS	14					
15											
16											
17											
18	SAND, a little gravel, medium grained, light brown, moist, dense (SP)		43	M	SS	17					
19											
20											
21											
22											
23	SAND, a little gravel, fine to medium grained, light brown, moist, very dense (SP)		51	M	SS	18					
24											
25											
26											
27											
28											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-39½'	3.25" HSA	7/26/07	10:55	41.5	39.5	41.0		None	
BORING COMPLETED: 7/26/07									
DR: SG LG: BR Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-202/AB- 2 (p. 2 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
30	SAND, a little gravel, medium to fine grained, light brown, moist, very dense (SP) <i>(continued)</i>		55	M		SS	19					
31												
32												
33	SAND, fine grained, light brown, moist, dense, laminations of silt (SP)		48	M		SS	18					
34												
35												
36	SAND WITH GRAVEL, medium grained, brown, moist, very dense (SP)		54	M		SS	19					
37												
38												
39	END OF BORING Northing=206823.0 Easting=554162.7											
40												
41												



SUBSURFACE BORING LOG

AET JOB NO: **22-00081** LOG OF BORING NO. **ST-203/AB-3 (p. 1 of 2)**
 PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>954.4</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mostly silty sand, surface roots, trace roots, dark brown	FILL	8	M	SS	14	10				
2	FILL, mostly clayey sand, a little gravel, trace roots, light brown, dark brown and brown		13	M	SS	24	12				
3											
4											
5	CLAYEY SAND, a little gravel, brown, very stiff (SC)	TILL	22	M	SS	17	11				
6											
7											
8			27	M	SS	19	7				
9											
10											
11			28	M	SS	19	6				
12											
13	SAND WITH SILT AND GRAVEL, medium to fine grained, brown, moist, medium dense to very dense (SP-SM)	COARSE ALLUVIUM	47	M	SS	10	7				
14											
15											
16			26	M	SS	14					
17											
18											
19											
20											
21			33	M	SS	16					
22											
23											
24											
25											
26			74	M	SS	12					
27											
28	SAND, medium to fine grained, brown, moist, medium dense to dense (SP)										

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
0-49½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
		7/27/07	2:47	51.5	49.5	51.5			None
BORING COMPLETED: 7/27/07									
DR: SG LG: BR Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081 LOG OF BORING NO. ST-203/AB-3 (p. 2 of 2)
 PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
30	SAND, medium to fine grained, brown, moist, medium dense to dense (SP) <i>(continued)</i>		24	M	SS	19					
31											
32											
33											
34											
35											
36			39	M	SS	16					
37											
38	SAND WITH GRAVEL, fine to medium grained, light brown, moist, medium dense (SP)		26	M	SS	17					
39											
40											
41											
42											
43											
44											
45											
46			23	M	SS	20					
47											
48											
49											
50	SILTY CLAY, brown, very stiff (CL-ML)	FINE ALLUVIUM COARSE ALLUVIUM	23	M	SS	16	19				
51	SAND WITH SILT, fine to medium grained, brown, moist, medium dense (SP-SM)										
END OF BORING Northing=206974.8 Easting=553863.2											



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-204/AB- 4 (p. 1 of 2)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>938.6</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	FILL, mostly silty sand, surface roots, trace roots, brown FILL, mixture of clayey sand, sandy lean clay and silty sand, a little gravel, trace roots, brown, light brown, dark brown and gray	FILL	31	M	SS	10	7					
2			42	M	SS	18	9					
3												
4												
5												
6					13	M	SS	17	19			
7												
8					2	M	SS	NR				
9												
10												
11					3	M	SS	9	14			
12												
13					4	W	SS	12				
14												
15												
16					2	W	SS	17	17			
17												
18	CLAYEY SAND, a little sandy silt, a little gravel, brown, stiff (SC)	TILL	10	M	SS	17	19					
19												
20	SANDY LEAN CLAY, a little gravel, light brown, a little brown, stiff, laminations of silt (CL)	TILL	11	M	SS	20	27					
21												
22	SAND WITH SILT AND GRAVEL, medium to fine grained, brown, a little dark brown, moist, very dense, lense of clayey sand at 24' very dense to medium dense (SP-SM)	COARSE ALLUVIUM	52	M	SS	14						
23												
24												
25												
26					22	M	SS	7				
27												
28												

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
0-34½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
		7/30/07	10:00	14.0	12.0	12.2			None
		7/30/07	10:30	36.5	34.5	36.5			None
BORING COMPLETED: 7/30/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-204/AB- 4 (p. 2 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
30	SAND WITH SILT AND GRAVEL, medium to fine grained, brown, a little dark brown, moist, very dense, lense of clayey sand at 24' very dense to medium dense (SP-SM) (continued)	COARSE ALLUVIUM (continued)	18	M	SS	13					
31											
32											
33											
34											
35											
36			21	M	SS	12					
<p>END OF BORING Northing=207058.0 Easting=553053.9</p>											



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-205/AB-5 (p. 1 of 2)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>937.1</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	FILL, mixture of clayey sand and silty sand, a little gravel, surface roots, trace roots, brown and light brown	FILL	27	M	SS	12	6					
2												
3			39	M	SS	15	6					
4												
5	SILTY CLAY, brown, very stiff to hard (CL-ML)	FINE ALLUVIUM	22	M	SS	20	13					
6												
7												
8			17	M	SS	24	23					
9												
10												
11												
12												
13			20	M	SS	24	17					
14												
15												
16	SAND WITH SILT AND GRAVEL, medium to fine grained, brown, moist, dense (SP-SM)	COARSE ALLUVIUM	33	M	SS	14	12					
17												
18												
19												
20												
21			47	M	SS	10						
22												
23	GRAVELLY SAND WITH SILT, medium to fine grained, brown, moist, dense to very dense (SP-SM)											
24												
25												
26					44	M	SS	10				
27												
28												

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-44½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		7/30/07	8:50	46.5	44.5	46.4		None	
BORING COMPLETED: 7/30/07									
DR: SG	LG: SB	Rig: 91C							



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-205/AB-5 (p. 2 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
30	GRAVELLY SAND WITH SILT, medium to fine grained, brown, moist, dense to very dense (SP-SM) <i>(continued)</i>		60	M	SS	14						
31												
32												
33												
34	SAND WITH SILT, fine to medium grained, brown, moist, very dense (SP-SM)		50+	M	SS	8						
35												
36												
37												
38	SAND WITH SILT, fine to medium grained, brown, moist, very dense (SP-SM)		57	M	SS	16						
39												
40												
41												
42	SAND WITH SILT, fine to medium grained, brown, moist, very dense (SP-SM)		63	M	SS	17						
43												
44												
45												
46	END OF BORING Northing=207537.6 Easting=553124.1											



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-206/AB-6 (p. 1 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>916.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mostly silty sand, surface roots, trace roots, dark brown	FILL	32	M	SS	14	5				
2	FILL, mixture of clayey sand and silty sand, a little gravel, trace roots, dark brown										
3			26	M	SS	13	12				
4	SANDY LEAN CLAY, a little gravel, gray and brown mottled, a little black, stiff, laminations of silt (CL)	WEATHERED TILL	12	M	SS	17	18				
5											
6											
7	SANDY LEAN CLAY, a little gravel, dark brown and brown mottled, stiff (CL)	TILL	15	M	SS	20	18				
8											
9											
10	SANDY LEAN CLAY, a little gravel, dark brown, very stiff (CL)		16	M	SS	22	16				
11											
12	SANDY LEAN CLAY, a little gravel, dark gray, stiff to very stiff (CL)		15	M	SS	19	14				
13											
14											
15											
16			12	M	SS	20	15				
17											
18											
19											
20											
21			14	M	SS	22	16				
22											
23											
24											
25											
26											
27											
28											

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
0-29½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
		7/30/07	12:00	31.5	29.5	31.2			None
BORING COMPLETED: 7/30/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-206/AB-6 (p. 2 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
30 -	SANDY LEAN CLAY, a little gravel, dark gray, stiff to very stiff (CL) <i>(continued)</i>		18	M	CL SS	23	13					
31 -												
<p>END OF BORING Northing=207925.7 Easting=552804.2</p>												



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TESTING, INC.

SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-207/AB-7 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>908.5</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mixture of clayey sand and silty sand, a little gravel, surface roots, trace roots, dark brown and brown	FILL	33	M	SS	12	4				
2											
3			22	M	SS	9	6				
4											
5	SANDY LEAN CLAY, a little gravel, dark brown, a little brown, stiff, laminations of silt (CL)	TILL	15	M	SS	17	16				
6											
7			14	M	SS	18	17				
8											
9	SANDY LEAN CLAY, a little gravel, dark gray, stiff (CL)		10	M	SS	20	17				
10											
11			11	M	SS	20	15				
12											
13											
14											
15											
16											
17											
18											
19											
20											
21											
22											
23											
24											
25											
26											
END OF BORING Northing=208187.8 Easting=552129.6											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
0-24½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
		7/30/07	1:30	26.5	24.5	24.9			None
BORING COMPLETED: 7/30/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO. **ST-208/AB- 8 (p. 1 of 2)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>914.5</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mixture of silty sand and clayey sand, a little gravel, surface roots, trace roots, brown, dark brown and black	FILL	27	M	SS	12	4				
2											
3											
4											
5	SANDY LEAN CLAY, a little gravel, light brown and gray, firm (CL)	WEATHERED TILL	5	M	SS	17	19				
6											
7											
8			7	M	SS	19	16				
9											
10	SANDY LEAN CLAY, a little gravel, brown, stiff (CL)	TILL	10	M	SS	22	15				
11											
12	SANDY LEAN CLAY, a little gravel, dark gray, stiff (CL)	TILL	13	M	SS	22	15				
13											
14											
15											
16						9	M	SS	20	15	
17											
18											
19											
20											
21			12	M	SS	23	15				
22											
23											
24											
25											
26			11	M	SS	24	16				
27											
28											

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-34½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		7/30/07	2:35	36.5	34.5	36.3		None	
BORING COMPLETED: 7/30/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO. ST-208/AB-8 (p. 2 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
30	SANDY LEAN CLAY, a little gravel, dark gray, stiff (CL) (continued)		14	M		20	16					
31												
32												
33												
34												
35												
36												
	END OF BORING Northing=208345.2 Easting=552536.3											



SUBSURFACE BORING LOG

AET JOB NO: **22-00081** LOG OF BORING NO. **ST-209/AB-9 (p. 1 of 1)**
 PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>892.8</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mostly silty sand with gravel, trace roots, brown FILL, mixture of silty sand and clayey sand, a little gravel, trace roots, brown, light brown, dark brown and black	FILL	36	M		13	4				
2			10	M		16	11				
3			18	M		16	6				
4	SAND WITH SILT, a little gravel, fine to medium grained, brownish gray, waterbearing, medium dense (SP-SM)	COARSE ALLUVIUM	16	W/M		14					
5			7	W		14					
6			9	M		20	16				
7	SANDY LEAN CLAY, a little gravel, dark gray, firm to stiff (CL)	TILL	6	M		16	16				
8			9	M		20	16				
9			9	M		23	16				
10			9	M		23	16				
11	END OF BORING Northing=210352.9 Easting=552356.5										

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
0-19½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
		7/31/07	7:46	9.0	7.0	7.9			None
		7/31/07	7:43	11.0	9.5	9.3			9.1
BORING COMPLETED:	7/31/07	7/31/07	7:55	21.5	19.5	20.0			None
DR: SG	LG: SB	Rig: 91C							



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO **ST-210/AB-10 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>898.8</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mixture of silty sand and clayey sand, a little gravel, trace roots, pieces of brick, brown and dark brown	FILL	37	M		15					
2											
3			11	M		12					
4											
5	SILT WITH ORGANICS, trace roots, black, loose (ML)	TOPSOIL	5	W/M		19	19				
6	SILTY SAND, fine grained, trace roots, dark gray, waterbearing, loose to medium dense (SM)	COARSE ALLUVIUM	11	W		2					
7	SANDY LEAN CLAY, a little gravel, brown and gray mottled, a little dark brown, firm, laminations of silt (CL)	WEATHERED TILL	6	W		16	14				
8											
9											
10	SANDY LEAN CLAY, a little gravel, dark gray, stiff to very stiff (CL)	TILL	9	M		16	14				
11											
12			9	M		18	16				
13											
14											
15											
16											
17											
18											
19											
20											
21			20	M		19	15				
END OF BORING Northing=210578.9 Easting=553291.9											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-19½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		7/31/07	8:35	6.5	4.5	4.9		None	
		7/31/07	8:50	21.5	19.5	21.5			
BORING COMPLETED: 7/31/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO **ST-211/AB-11 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>892.3</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mixture of silty sand, clayey sand and sandy silt, surface roots, trace roots, brown, dark brown and gray	FILL	20	M		15					
2											
3			14	M		14					
4											
5											
6			12	M		18	19				
7											
8						17	W	18	20		
9	SILTY CLAY, brown, very stiff (CL-ML)	FINE ALLUVIUM TILL									
10	CLAYEY SAND, a little gravel, dark gray, stiff to firm (SC)		10	M		14	13				
11											
12											
13											
14			7	M	21	15					
15	SANDY LEAN CLAY, a little gravel, dark gray, stiff to firm (CL)										
16			9	M	23	17					
17											
18											
19											
20											
21			8	M	22	16					
END OF BORING Northing=210737.1 Easting=552615.0											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
0-19½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
		7/31/07	9:30	9.0	7.0	11.8			7.6
		7/31/07	9:40	21.5	19.5	21.5			None
BORING COMPLETED: 7/31/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO **ST-212/AB-12 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>895.1</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS							
							WC	DEN	LL	PL	%-#200			
1	3.5" Bituminous pavement	FILL			SU									
	FILL, mostly silty sand with gravel, brown		26	M	SS	14								
2	FILL, mixture of clayey sand and silty sand, a little gravel, organic clay, brown, dark brown, gray and black						5							
3			25	M	SS	15	10							
4														
5														
6				23	W/M	SS	16	11						
7														
8			33	W	SS	NR								
9														
10	SILT WITH ORGANICS, trace roots, black, moist, very loose (ML)	SWAMP DEPOSIT OR TOPSOIL						25						
11	LEAN CLAY WITH ORGANICS, trace roots, black, very soft to soft (CL)		2	M	SS	17	23							
12						26								
13	SILTY SAND, trace roots, fine grained, dark gray, waterbearing, loose (SM)	COARSE ALLUVIUM						114						
14			5	W/M	SS	16								
15														
16			9	W	SS	7								
17														
18	CLAYEY SAND, a little gravel, dark gray, firm (SC)	TILL												
19														
20														
21			8	M	SS	16	17							
END OF BORING Northing=211046.0 Easting=553106.0														

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-24½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		7/31/07	10:50	9.0	7.0	8.1		7.3	
		7/31/07	11:10	21.5	19.5	19.5		18.9	
BORING COMPLETED: 7/31/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO ST-213/AB-13 (p. 1 of 1)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>890.2</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	FILL, mixture of silty sand, sandy silt and clayey sand, a little gravel, surface roots, trace roots, brown, dark brown and black	FILL	26	M	SS	16	4				
2											
3											
4											
5											
6	SAND WITH SILT, fine to medium grained, brownish gray, a little gray, waterbearing, loose, laminations of sandy silt (SP-SM)	COARSE ALLUVIUM	10	M	SS	16	18				
7											
8	SILTY SAND, a little gravel, gray, a little dark gray, waterbearing, medium dense, lenses and laminations of lean clay and sandy lean clay (SM)	TILL	12	W	SS	14	24				
9											
10	SANDY LEAN CLAY, a little gravel, dark gray, loose, lense of medium to fine grained silty sand at 10', laminations of sand with silt (CL)	TILL	8	M/W	SS	15	19				
11											
12											
13											
14											
15											
16											
17											
18											
19											
20											
21											
END OF BORING Northing=211044.1 Easting=552565.3											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
0-19½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
		8/2/07	9:45	9.0	7.0	7.2			7.1
		8/2/07	10:00	21.5	19.5	19.5			None
BORING COMPLETED: 8/2/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO **ST-214/AB-14 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>884.9</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1-9	FILL, mixture of sandy silt, clayey sand, silty sand and sand with silt, a little gravel, surface roots, trace roots, brown, dark brown, gray and black	FILL	51	M	SS	10	6				
10-11	SAND WITH SILT, fine grained, gray, a little dark gray, waterbearing, medium dense, laminations of silty sand (SP-SM)	COARSE ALLUVIUM	WH	W	SS	17					
12-14	SILTY SAND, trace roots, fine grained, dark gray, a little black, waterbearing, loose, lense of organic clay at 15.5' (SM)		5	W	SS	14					
15-16	SAND WITH SILT, fine grained, gray, waterbearing, medium dense (SP-SM)		12	W	SS	17					
17-18	SAND WITH SILT AND GRAVEL, medium to fine grained, dark gray, waterbearing, medium dense (SP-SM)										
19-21	SANDY LEAN CLAY, a little gravel, gray, firm (CL)	TILL	6	M	SS	15	18				
END OF BORING Northing=211075.3 Easting=551748.2											

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
		DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
0-19½'	3.25" HSA	8/2/07	8:15	11.5	9.5	9.5		None	
		8/2/07	8:20	14.0	12.0	12.0		None	
BORING COMPLETED: 8/2/07		8/2/07	8:25	16.0	14.5	14.5		12.7	
DR: SG LG: SB Rig: 91C		8/2/07	8:30	21.5	19.5	19.5		18.7	



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO **ST-215/AB-15 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>894.5</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	FILL, surface roots with silty sand, dark brown	FILL	5	M	SS	1	11					
2	FILL, mostly sand with silt, a little gravel, trace roots, light brown											
3												
4												
5	FILL, mixture of silty sand, clayey sand and sandy lean clay, a little gravel, trace roots, dark brown, brown, light brown and gray	TOPSOIL	9	M	SS	12	15					
6												
7												
8												
9												
10	LEAN CLAY, trace roots, organics, black, a little dark gray, stiff, laminations of silty sand (CL)	COARSE ALLUVIUM	9	M	SS	15	21					
11	SILTY SAND, a little gravel, fine to medium grained, gray, a little dark gray, waterbearing, loose, laminations of lean clay (SM)											
12												
13												
14		TILL	17	W	SS	12						
15												
16												
17		TILL	16	W	SS	5						
18	SANDY LEAN CLAY, a little gravel, gray, firm (CL)											
19												
20												
21			5	M	SS	16	17					
END OF BORING Northing=211443.2 Easting=553634.2												

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-19½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		8/1/07	12:15	11.5	9.5	9.9		9.3	
		8/1/07	12:34	21.5	19.5	20.0		18.3	
BORING COMPLETED: 8/1/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: 22-00081 LOG OF BORING NO. ST-216/AB-16 (p. 1 of 1)
 PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	SURFACE ELEVATION: <u>895.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS							
							WC	DEN	LL	PL	%-#200			
1	3" Bituminous pavement	FILL												
1	FILL, mostly silty sand with gravel, brown		32	M	SS	14	8							
2	FILL, mixture of clayey sand, sandy lean clay and silty sand, a little gravel, pieces of concrete, brown, dark brown, gray and black		36	M	SS	17	8							
3			46	M	SS	15	6							
4			26	M	SS	17								
5			11	M	SS	16	15							
6			15	W	SS	18	74							
7	ORGANIC CLAY, trace roots, black, stiff (OL/OH)		SWAMP DEPOSIT											
8	ORGANIC CLAY, trace roots, pieces of wood, black, a little gray, stiff, laminations of silty sand (OL/OH)			15	W	SS	18	74						
9	SAND WITH SILT, fine grained, gray, waterbearing, medium dense to very loose (SP-SM)		COARSE ALLUVIUM											
10				7	W	SS	19							
11		3		M	SS	17	16							
12	CLAYEY SAND, a little gravel, dark gray, soft (SC)	TILL												

END OF BORING
 Northing=211426.9
 Easting=553105.1

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-19½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		8/1/07	11:00	11.5	9.5	11.8		None	
		8/1/07	11:10	14.0	12.0	12.0		None	
BORING COMPLETED: 8/1/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO **ST-217/AB-17 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>899.3</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS							
							WC	DEN	LL	PL	%-#200			
1	CLAYEY SAND, surface roots, trace roots, dark brown, moist, medium dense (SC)	TOPSOIL												
2	SAND WITH SILT, fine grained, light brown, a little brown, moist, medium dense, laminations of silt (SP-SM)	COARSE ALLUVIUM	14	M	SS	15								
3			17	M	SS	16								
4	SAND WITH SILT, fine grained, gray, a little brown, waterbearing, medium dense, laminations of silt (SP-SM)													
5			28	M	SS	15								
6	CLAYEY SAND, a little gravel, dark gray, firm (SC)	TILL												
7			8	M	SS	14	13							
8	CLAYEY SAND, a little gravel, brown, stiff (SC)													
9			10	M	SS	13	14							
10	CLAYEY SAND, a little gravel, dark gray, firm to stiff (SC)													
11			8	M	SS	17	15							
12														
13			10	M	SS	16	16							
14														
15			11	M	SS	24	15							
16														
17														
18														
19														
20														
21														
END OF BORING Northing=211423.6 Easting=552556.4														

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-19½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		8/2/07	9:00	9.0	7.0	7.0		3.3	
		8/2/07	9:10	21.5	19.5	20.9		None	
BORING COMPLETED: 8/2/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO **ST-218/AB-18 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>885.7</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	FILL, mixture of sand with silt and silty sand, a little gravel, trace roots, brown, light brown and dark brown	FILL	22	M	SS	4						
2												
3												
4												
5												
6												
7	SILTY SAND, fine to medium grained, brownish gray, waterbearing, loose (SM)	COARSE ALLUVIUM	10	W	SS	17						
8												
9	SANDY LEAN CLAY, a little gravel, brown, a little light brown, firm, laminations of silty sand (CL)	TILL	7	M	SS	18	17					
10												
11												
12												
13	CLAYEY SAND, a little gravel, dark gray, stiff (SC)		9	M	SS	19	13					
14												
15												
16												
17												
18												
19												
20												
21			9	M	SS	24	16					
END OF BORING Northing=211416.5 Easting=552009.6												

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
0-19½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
		8/1/07	2:05	9.0	7.0	7.4			None
		8/1/07	2:20	21.5	19.5	19.8			None
BORING COMPLETED: 8/1/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO **ST-219/AB-19 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>891.3</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS							
							WC	DEN	LL	PL	%-#200			
1	SILTY SAND, surface roots, trace roots, light brown and brown, moist, dense (SM) SAND WITH SILT, fine grained, brown, moist, loose to medium dense (SP-SM)	TOPSOIL												
2		COARSE ALLUVIUM	37	M	SS	15								
3			18	M	SS	14								
4														
5														
6				9	M	SS	16							
7														
8				18	M	SS	18							
9														
10				16	M	SS	18							
11														
12	SILTY SAND, fine grained, grayish brown, a little brown, waterbearing, medium dense, laminations of silt (SM)													
13			14	W	SS	16								
14														
15				23	W	SS	14							
16														
17														
18	SAND WITH SILT, fine grained, gray, waterbearing, dense (SP-SM)													
19														
20														
21				31	W	SS	19							
22														
23														
24														
25														
26				36	W	SS	21							
END OF BORING Northing=211642.1 Easting=551067.3														

DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-24½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		7/30/07	12:15	14.0	12.0	12.5		None	
		7/30/07	12:25	16.5	14.5	15.0		15.0	
BORING COMPLETED:	7/30/07	7/30/07	1:00	26.5	24.5	24.5		23.1	
DR:	SG LG: SB Rig: 91C								



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO **ST-220/AB-20 (p. 1 of 2)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>897.0</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	SILTY SAND WITH GRAVEL, surface roots, trace roots, light brown, brown and dark brown, moist, medium dense (SM)	TOPSOIL	18	M	SS	14					
2		COARSE ALLUVIUM	20	M	SS	14					
3	SAND WITH SILT, fine grained, light brown, moist, medium dense (SP-SM)		19	M	SS	14					
4			17	M	SS	20					
5			14	M	SS	19					
6	SILTY SAND, fine grained, brown, waterbearing, medium dense to loose (SM)		10	M	SS	18					
7		15	W	SS	17						
8		7	W	SS	16						
9		12	W	SS	17	30					
10	SILT, gray, wet, medium dense (ML)	FINE ALLUVIUM									
11		COARSE ALLUVIUM									

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS						NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG	
0-29½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL		WATER LEVEL
		8/1/07	9:05	16.5	14.5	14.6			None
		8/1/07	9:10	21.5	19.5	19.5			17.8
BORING COMPLETED:	8/1/07	8/1/07	9:25	31.5	29.5	29.5			26.3
DR: SG	LG: SB	Rig: 91C							



SUBSURFACE BORING LOG

AET JOB NO: 22-00081

LOG OF BORING NO ST-220/AB-20 (p. 2 of 2)

PROJECT: TCAAP Redevelopment; Arden Hills, MN

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
30 -	SILTY SAND, fine grained, gray, waterbearing, very loose (SM) <i>(continued)</i>	COARSE ALLUVIUM <i>(continued)</i>	4	W	SI	20					
31 -					SS						
<p>END OF BORING Northing=211992.3 Easting=550710.8</p>											



SUBSURFACE BORING LOG

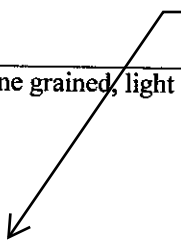
AET JOB NO: **22-00081**

LOG OF BORING NO **ST-221/AB-21 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: 899.0 MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%-#200
1	SILTY SAND, a little gravel, surface roots, trace roots, brown, dark brown and black, moist, medium dense (SM)	TOPSOIL	19	M	SS	16					
2	SILTY SAND, fine grained, brown, moist, loose to medium dense (SM)	COARSE ALLUVIUM	9	M	SS	15					
3											
4											
5	SAND WITH SILT, fine grained, light brown, moist, loose (SP-SM)		13	M	SS	18					
6											
7											
8											
9	SILT WITH SAND, light brown, a little brown, wet, medium dense, laminations of lean clay (ML)	FINE ALLUVIUM	10	M	SS	17					
10											
11											
12											
13											
14											
15	SILTY SAND, fine grained, brown, waterbearing, loose (SM)	COARSE ALLUVIUM	17	W	SS	17	26				
16											
17	CLAYEY SAND, a little gravel, dark gray, stiff (SC)	TILL	9	M	SS	8	15				
18											
19											
20	END OF BORING Northing=212465.7 Easting=550556.0										

Silty Sand 0' to 7'
Sand w Silt 7' to 17'
Silty Sand 17' to 23'



DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-24½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		8/1/07	7:55	16.5	14.5	14.5		None	
		8/1/07	8:00	21.5	19.5	19.2		18.3	
BORING COMPLETED: 8/1/07		8/1/07	8:10	26.5	24.5	24.5		23.9	
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO **ST-222/AB-22 (p. 1 of 1)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>898.6</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS				
							WC	DEN	LL	PL	%#200
1	FILL, mostly silty sand, a little gravel, trace roots, dark brown <div style="border: 1px solid green; padding: 2px; display: inline-block;"> Fill (silty sand) 0' to 7' Fill (clayey sand) 7' to 9.5' Sand w Silt 9.5' to 14.5' Silty Sand 14.5' to 21' </div>	FILL	21	M	SS	15					
2			7	M	SS	14					
3			4	M	SS	16					
4			9	M	SS	14	11				
5			14	M	SS	16					
6			27	M	SS	19					
7			30	W/M	SS	24					
8	FILL, mixture of clayey sand and silty sand, trace roots, brown and light brown		9	M	SS	14	11				
9											
10	SAND WITH SILT, fine grained, trace roots, light brown and brown, moist, medium dense (SP-SM)	COARSE ALLUVIUM	14	M	SS	16					
11											
12	SAND WITH SILT, fine grained, light brown, moist, medium dense (SP-SM)			27	M	SS	19				
13											
14	SILTY SAND, fine grained, brown, waterbearing, medium dense to loose (SM)			30	W/M	SS	24				
15											
16											
17											
18											
19											
20											
21			6	W	SS	16					
END OF BORING Northing=212964.6 Easting=550545.4											

DEPTH: DRILLING METHOD		WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-19½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		7/31/07	3:22	21.5	19.5	20.0		18.9	
BORING COMPLETED: 7/31/07									
DR: SG LG: SB Rig: 91C									



SUBSURFACE BORING LOG

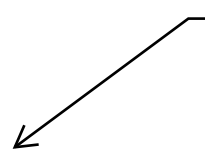
AET JOB NO: **22-00081**

LOG OF BORING NO **ST-223/AB-23 (p. 1 of 2)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	SURFACE ELEVATION: <u>908.4</u> MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS					
							WC	DEN	LL	PL	%-#200	
1	FILL, mostly silty sand, surface roots, trace roots, pieces of wood at 5', brown, light brown and dark brown	FILL	27	M	SS	15						
2												
3												
4												
5												
6					4	M	SS	12				
7												
8					3	M	SS	13				
9												
10	SAND WITH SILT, fine grained, light brown, a little brown, moist, loose to medium dense, laminations of silt (SP-SM)	COARSE ALLUVIUM	5	M	SS	15						
11												
12												
13												
14					17	M	SS	17				
15												
16					9	M	SS	17				
17												
18												
19												
20												
21			27	M	SS	19						
22												
23	SAND, fine to medium grained, light brown, a little brown and gray, moist, medium dense, laminations of silt and silty sand (SP)											
24												
25												
26					17	M	SS	16				
27												
28												

Fill (silty sand) 0' to 9.5'
Sand w Silt 9.5' to 23'
Sand 23' to 33'



DEPTH:	DRILLING METHOD	WATER LEVEL MEASUREMENTS							NOTE: REFER TO THE ATTACHED SHEETS FOR AN EXPLANATION OF TERMINOLOGY ON THIS LOG
0-34½'	3.25" HSA	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING FLUID LEVEL	WATER LEVEL	
		7/31/07	2:15	31.5	29.5	29.5		28.9	
		7/31/07	2:25	36.5	34.5	34.5		33.9	
BORING COMPLETED: 7/31/07									
DR: SG LG: SB Rig: 91C									






SUBSURFACE BORING LOG

AET JOB NO: **22-00081**

LOG OF BORING NO **ST-223/AB-23 (p. 2 of 2)**

PROJECT: **TCAAP Redevelopment; Arden Hills, MN**

DEPTH IN FEET	MATERIAL DESCRIPTION	GEOLOGY	N	MC	SAMPLE TYPE	REC IN.	FIELD & LABORATORY TESTS											
							WC	DEN	LL	PL	%-#200							
30	SAND, fine to medium grained, gray, waterbearing, loose (SP) <i>(continued)</i>																	
31															9	W	SS	17
32																		
33	CLAYEY SAND, a little gravel, dark gray, firm (SC)	TILL																
34																		
35																		
36			6	M	SS	23	18											
END OF BORING Northing=213465.1 Easting=550540.6																		

Infiltration Basins (Minnesota Stormwater Manual)



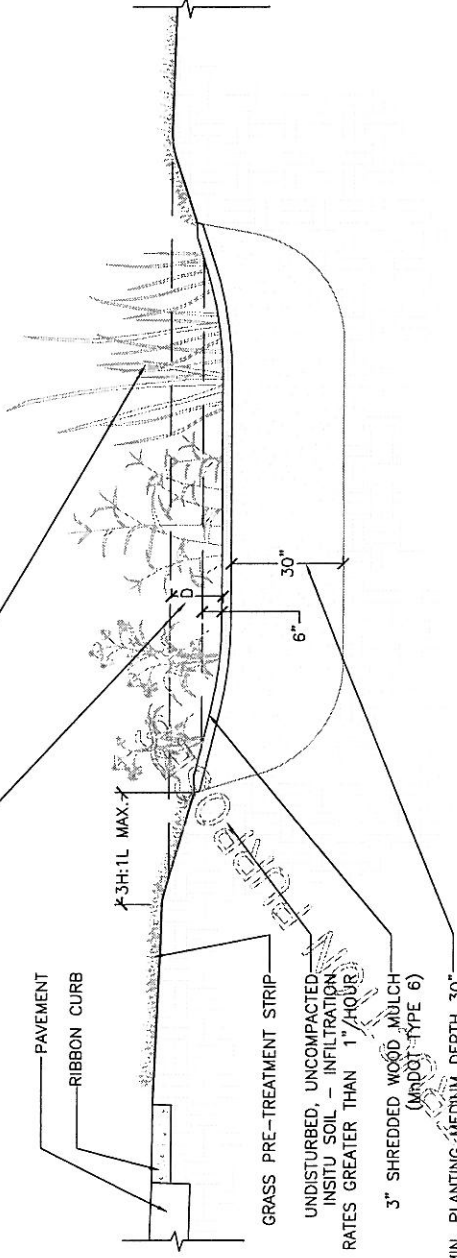
Minnesota Pollution Control Agency
 385 PASEO PARKWAY
 ST. PAUL, MN 55103-5000
 TEL: (612) 223-3000
 WEB: WWW.PCA.MN.GOV

Date	_____
Drawn By	_____
Checked By	_____
Project Name	_____
Client Name	_____
Location	_____
Scale	_____
Sheet No.	_____
Total Sheets	_____

- CONSTRUCTION SEQUENCING**
1. INSTALL SILT FENCE AND/OR OTHER APPROPRIATE TEMPORARY EROSION CONTROL DEVICES TO PREVENT SEDIMENT FROM LEAVING OR ENTERING THE PRACTICE DURING CONSTRUCTION.
 2. ALL DOWN-SLOPE GRADIENT PERMETER SEDIMENT CONTROL BMP'S MUST BE IN PLACE BEFORE ANY UP GRADIENT LAND DISTURBING ACTIVITY BEGINS.
 3. PERFORM CONTINUOUS INSPECTIONS OF EROSION CONTROL PRACTICES.
 4. INSTALL UTILITIES (WATER, SANITARY SEWER, ELECTRIC, PHONE, FIBER OPTIC, ETC) PRIOR TO SETTING FINAL GRADE OF BIORETENTION DEVICE.
 5. ROUGH GRADE THE SITE. IF BIORETENTION AREAS ARE BEING USED AS TEMPORARY SEDIMENT BASINS LEAVE A MINIMUM OF 3 FEET OF COVER OVER THE PRACTICE TO PROTECT THE UNDERLYING SOILS FROM CLOSING.
 6. PERFORM ALL OTHER SITE IMPROVEMENTS.
 7. SEED AND MULCH ALL AREAS AFTER DISTURBANCE.
 8. CONSTRUCT BIORETENTION DEVICE UPON STABILIZATION OF CONTRIBUTING DRAINAGE AREA.
 9. IMPLEMENT TEMPORARY AND PERMANENT EROSION CONTROL PRACTICES.
 10. PLANT AND MULCH BIORETENTION DEVICE.
 11. REMOVE TEMPORARY EROSION CONTROL DEVICES AFTER THE CONTRIBUTING DRAINAGE AREA IS ADEQUATELY VEGETATED.
- GENERAL NOTES**
1. IN THE EVENT THAT SEDIMENT IS INTRODUCED INTO THE BMP DURING OR IMMEDIATELY FOLLOWING EXCAVATION, THIS MATERIAL SHALL BE REMOVED FROM THE PRACTICE PRIOR TO CONTINUING CONSTRUCTION.
 2. GRADING OF BIORETENTION DEVICES SHALL BE ACCOMPLISHED USING LOW-COMPACTION EARTH-MOVING EQUIPMENT TO PREVENT COMPACTION OF UNDERLYING SOILS.
 3. ALL SUB MATERIALS BELOW THE SPECIFIED BIORETENTION DEPTH (ELEVATION) SHALL BE UNDISTURBED, UNLESS OTHERWISE NOTED.

PLANT MATERIAL TOLERANT OF INUNDATION AND DROUGHT. NATIVE PLANTS RECOMMENDED.

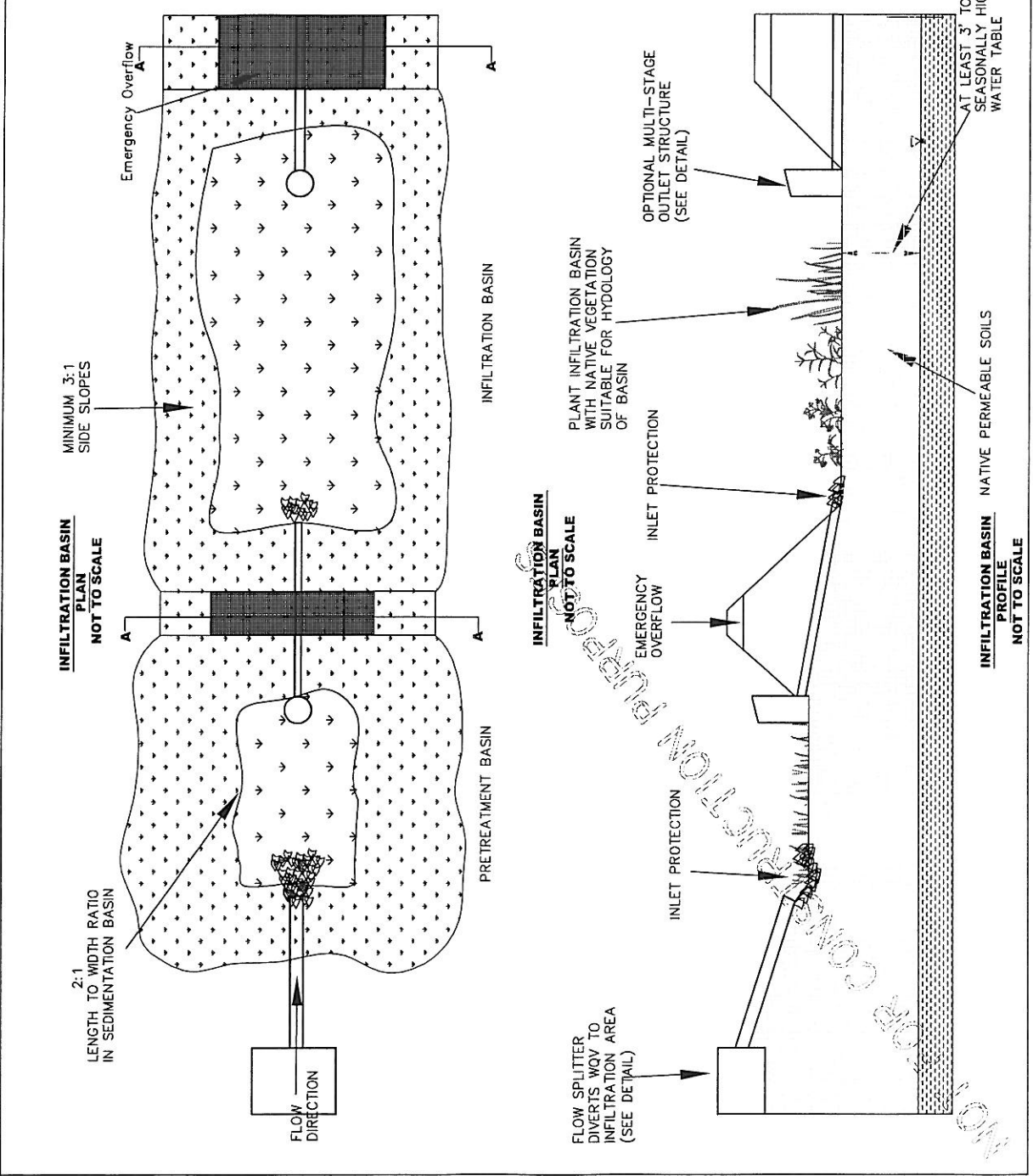
DEPTH REQUIRED TO DRAIN PRACTICE IN 48 HOURS OR LESS, NOT TO EXCEED 18".



INFILTRATION / RECHARGE FACILITY CROSS-SECTION
 NOT TO SCALE

NOT FOR CONSTRUCTION

- CONSTRUCTION SPECIFICATIONS:**
1. PERFORM CONTINUOUS INSPECTIONS OF EROSION CONTROL PRACTICES.
 2. INSTALL SILT FENCE ALONG THE PERIMETER OF THE SITE TO PREVENT SEDIMENT FROM LEAVING THE SITE DURING THE CONSTRUCTION PROCESS.
 3. ALL DOWNGRADIENT PERIMETER SEDIMENT CONTROL BMPs MUST BE IN PLACE BEFORE ANY UP GRADIENT LAND-DISTURBING ACTIVITY BEGINS.
 4. REMOVE TOPSOIL FROM THE SITE AND PLACE IN TEMPORARY STOCKPILE LOCATION. TEMPORARY SEED THE STOCKPILE.
 5. INSTALL UNDERGROUND UTILITY (WATER, SANITARY SEWER, ELECTRIC) AND DETERMINE THE LOCATION AND FUNCTION OF STORMWATER BMPs INTO CONSIDERATION.
 7. SEED AND MULCH DISTURBED AREAS ON SITE.
 8. CONSTRUCT THE ROADS TAKING THE LOCATION AND FUNCTION OF STORM WATER BMPs INTO CONSIDERATION.
 9. PERFORM ALL OTHER SITE IMPROVEMENTS TAKING THE LOCATION AND FUNCTION OF THE STORM WATER BMPs INTO CONSIDERATION.
 10. FINAL GRADE THE SITE.
 11. STABILIZE THE SITE BY IMPLEMENTING THE NATIVE SEEDING AND PLANTING PORTION OF THE LANDSCAPING PLAN.
 12. INSTALL THE EROSION CONTROL BLANKET.
 13. REMOVE THE SILT FENCE AFTER THE SITE IS STABILIZED PER PROJECT ENGINEER APPROVAL.
- GENERAL NOTES:**
1. THIS SHALL BE ALL TEMPORARY EROSION CONTROL MEASURES (IN ACCORDANCE WITH MOST GENERAL CONDITIONS 287) PRIOR TO THE START OF ANY CONSTRUCTION THAT MAY CAUSE ANY SEDIMENTATION OR SILTATION AT THE SITE.
 2. INSTALL STORM DRAIN INLET PROTECTION TO PREVENT CLOGGING OF THE STORM DRAIN. LONGS TO DOWNSTREAM STORMWATER FACILITIES OR WATERBODIES.
 3. IF THE STORMWATER BMP IS BEING DESIGNED TO SERVE AS A TEMPORARY SEDIMENTATION BASIN, THE CONSTRUCTION OF FINAL GRADE TO PROTECT THE CONTRIBUTING DRAINAGE AREA HAS BEEN COMPLETED AND THE SITE IS STABILIZED, EXCAVATE THE INFILTRATION BASIN TO FINAL GRADE AND COMPLETE CONSTRUCTION OF THE BMP.
 4. GRADING OF THE INFILTRATION BASIN SHALL BE ACCOMPLISHED USING CONSTRUCTION EQUIPMENT THAT IS CAPABLE OF PREVENTING COMPACTING OF THE UNDERLYING SOILS. SMALL TRACKED DOZERS AND SUBCATS WITH RUBBER TRACKS ARE RECOMMENDED.
 5. EXCAVATE THE INFILTRATION BASIN TO THE SPECIFIED DEPTH. ELEVATION. IT IS RECOMMENDED THAT ALL SUB MATERIAL BELOW THE SPECIFIED ELEVATION SHALL BE LEFT UNDISTURBED, UNLESS OTHERWISE DIRECTED BY THE ENGINEER.
 6. GRADE TO THE DEPTH (ELEVATION) SPECIFIED IN THE CONSTRUCTION DOCUMENTS UNLESS OTHERWISE DIRECTED BY THE ENGINEER.
 7. IN THE EVENT THAT SEDIMENT IS INTRODUCED INTO THE BMP DURING OR IMMEDIATELY FOLLOWING EXCAVATION, THIS MATERIAL WILL NEED TO BE REMOVED FROM THE BASIN PRIOR TO INITIATING THE NEXT STEP IN THE CONSTRUCTION PROCESS. SEDIMENT THAT HAS BEEN WASHED INTO THE BASIN DURING THE EXCAVATION PROCESS SHALL BE REMOVED FROM THE BASIN MATERIAL, SIGNIFICANTLY REDUCING THE INFILTRATION CAPACITY OF THE SOILS.
 8. SEEDING AND INSTALLATION OF EROSION CONTROL BLANKET SHALL BE COMPLETED WITHIN 48 HOURS OF FINAL GRADING.
 9. INFILTRATION AREAS SHALL BE STAYED OFF DURING CONSTRUCTION TO RESTRICT HEAVY EQUIPMENT TRAFFIC FROM COMPACTING NATIVE SOILS.



NO.	REVISION DESCRIPTION	DATE	BY

Date: _____
 Drawn By: _____
 Designated By: _____
 Checked By: _____
 Approved By: _____

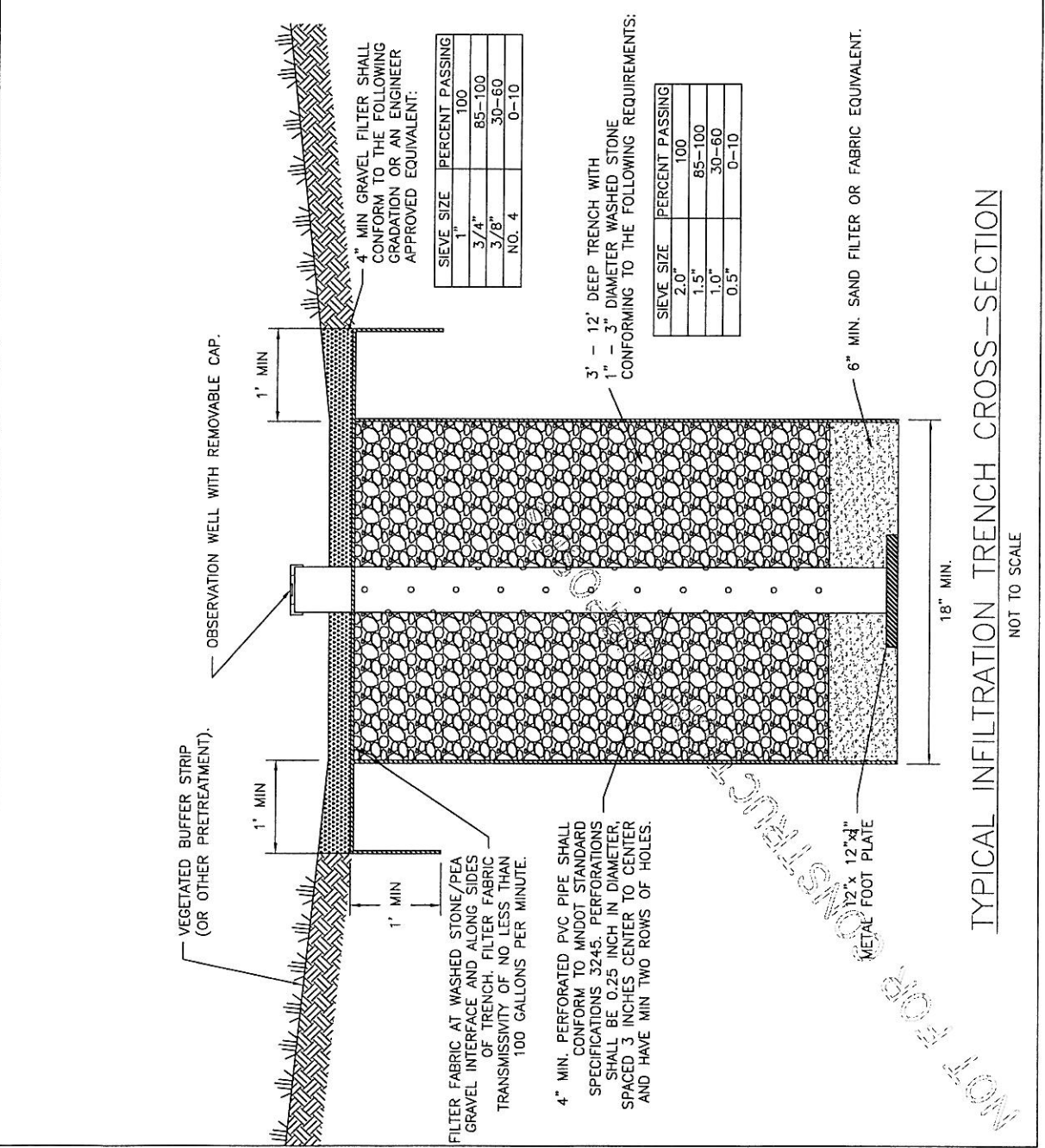


Minnesota Pollution Control Agency
 800 Lakeshore Road
 St. Paul, MN 55155-1202
 Phone: (612) 296-3322
 Fax: (612) 296-3322
 Website: www.pca.state.mn.us

2005 MINNESOTA
 STORMWATER
 MANUAL
 CROSS-SECTION
 INFILTRATION
 TRENCH
 TYPICAL
 Sheet No. _____ of _____
 Sheets

- CONSTRUCTION SEQUENCE:**
1. PERFORM CONTINUOUS INSPECTION OF EROSION CONTROL PRACTICES.
 2. INSTALL SILT FENCE ALONG THE PERIMETER OF THE SITE TO PREVENT SEDIMENT FROM LEAVING THE SITE DURING THE CONSTRUCTION PROCESS.
 3. ALL DOWNGRADIENT PERIMETER SEDIMENT CONTROL BMP'S MUST BE IN PLACE BEFORE ANY UP GRADING LAND-DISTURBING ACTIVITY BEGINS.
 4. REMOVE TOPSOIL FROM THE SITE AND PLACE IN TEMPORARY STOCKPILE LOCATION. TEMPORARY SEED THE STOCKPILE.
 5. INSTALL UNDERGROUND UTILITIES (WATER, SANITARY SEWER, ELECTRIC AND PHONES) TAKING THE LOCATION AND FUNCTION OF STORM WATER BMP'S INTO CONSIDERATION.
 6. ROUGH GRADE THE SITE. IF THE INFILTRATION TRENCH IS GOING TO BE USED FOR TEMPORARY SEDIMENT CONTROL, GRADE THE INFILTRATION TRENCH TO WITHIN THREE (3) FEET OF FINAL GRADE TO PREVENT CLOSING OF IN-SITU SOIL.
 7. SEED AND MULCH DISTURBED AREAS ON SITE.
 8. CONSTRUCT THE ROADS TAKING THE LOCATION AND FUNCTION OF STORM WATER BMP'S INTO CONSIDERATION.
 9. REFORM ALL OTHER SITE IMPROVEMENTS TAKING THE LOCATION AND FUNCTION OF THE STORM WATER BMP'S INTO CONSIDERATION.
 10. FINAL GRADE THE SITE.
 11. STABILIZE THE SITE BY IMPLEMENTING THE IN-THE SEEDING AND PLANTING PORTION OF THE LANDSCAPING PLAN.
 12. REMOVE THE SILT FENCE AFTER THE SITE IS STABILIZED PER PROJECT ENGINEER APPROVAL.

- GENERAL NOTES:**
1. INSTALL ALL TEMPORARY EROSION CONTROL MEASURES (IN ACCORDANCE WITH MPOOT GENERAL CONDITIONS 573) PRIOR TO SITE DISTURBANCE.
 2. INSTALL STORM DRAIN INLET PROTECTION TO PREVENT CLOGGING OF THE STORM DRAIN AND SEDIMENT LOADS TO DOWNSTREAM STORM WATER FACILITIES OR WATERBODIES.
 3. IF THE STORM WATER BMP IS BEING DESIGNED TO SERVE AS A TEMPORARY SEDIMENT CONTROL MEASURE, THE INFILTRATION TRENCH SHALL BE GRADED TO PREVENT CLOGGING OF IN-SITU SOIL. ONCE CONSTRUCTION IN THE CLOSING OF THE INFILTRATION TRENCH TO FINAL GRADE AND THE SITE IS STABILIZED, EXCAVATE THE INFILTRATION TRENCH TO FINAL GRADE AND COMPLETE CONSTRUCTION OF THE INFILTRATION TRENCH.
 4. GRADING OF THE INFILTRATION TRENCH SHALL BE ACCOMPLISHED USING LOW IMPACT EARTH-MOVING EQUIPMENT TO PREVENT COMPACTION OF THE UNDERLYING SOILS. VEHICLES SUCH AS BACK HOES, SMALL DOZERS AND BULLCATS ARE RECOMMENDED.
 5. EXCAVATE THE INFILTRATION TRENCH TO THE SPECIFIED DEPTH (ELEVATION). ALL SUB SOILS BELOW THE SPECIFIED DEPTH SHALL BE LEFT UNDISTURBED, UNLESS OTHERWISE DIRECTED BY THE ENGINEER.
 6. GRADE TO THE DEPTH (ELEVATION) SPECIFIED IN THE CONSTRUCTION DOCUMENTS UNLESS OTHERWISE DIRECTED BY THE ENGINEER.
 7. IN THE EVENT THAT SEDIMENT IS INTRODUCED INTO THE BMP DURING OR IMMEDIATELY FOLLOWING EXCAVATION, THE SEDIMENT WILL NEED TO BE REMOVED FROM THE INFILTRATION TRENCH PRIOR TO PLANTING THE NEXT STEP IN THE INFILTRATION TRENCH CONSTRUCTION PROCESS.
 8. MATERIAL EXCAVATED FROM THE INFILTRATION TRENCH SHALL BE DISPOSED OF ON-SITE AT LOCATION (TEMPORARY STOCKPILE AREAS) DESIGNATED BY ENGINEER.
 9. CLEAN, WASHED 1 TO 3/8 INCH GRAVEL SHALL BE PLACED IN THE BOTTOM OF THE INFILTRATION TRENCH TO THE DEPTH SPECIFIED IN THE CONSTRUCTION DOCUMENTS. GRAVEL SHOULD BE PLACED IN LIFTS AND LIGHTLY COMPACTED WITH PLATE COMPACTORS.



SIEVE SIZE	PERCENT PASSING
1"	100
3/4"	85-100
3/8"	30-60
NO. 4	0-10

SIEVE SIZE	PERCENT PASSING
2.0"	100
1.5"	85-100
1.0"	30-60
0.5"	0-10



Operation and maintenance of Infiltration basin

Effective long-term performance of infiltration practices requires an infiltration management plan (*HIGHLY RECOMMENDED*) and performance monitoring (*HIGHLY RECOMMENDED*).

Warning: Dedicated and routine maintenance schedule with clear guidelines and schedules are *REQUIRED*

The infiltration management plan (operation plan) should address the following items:

- periods of inundation;
- wet/dry cycling of soils; and
- operating instructions for drawdown valves gates and removable weirs.

The monitoring plan should address the following items:

- inspection and efficiency assessment;
- water quality monitoring; and
- monitoring of groundwater elevations, long-term infiltration capacity and plant tolerances.

Elements to be considered for the development of a maintenance plan are broken into the following categories:

- Design Phase Maintenance Considerations;
- Construction Phase Maintenance Considerations; and
- Post-Construction Maintenance Considerations.

In general terms, the most frequently cited maintenance concern for infiltration practices is clogging caused by organic matter and fine silts. Common operational problems include:

- clogging and sediment deposition;
- erosion of contributing land or in channels leading to the practice; and
- maintaining appropriate surface vegetation.

The table below provides a summary of common problems for infiltration trenches and basins.

Summary of infiltration practices cost components.

[Link to this table](#)

Implementation Stage	Primary Cost Components	Basic Cost Estimate	Other Considerations
Site Preparation	Tree & plant protection	Protection Cost (\$/acre) * Affected Area (acre)	Removal of existing structures, topsoil removal and stockpiling

Implementation Stage	Primary Cost Components	Basic Cost Estimate	Other Considerations
Site Formation	Infiltration area protection	Silt fence cost (\$/foot) * Perimeter of infiltration area	
	Clearing & grubbing	Clearing Cost (\$/acre) * Affected Area (acre)	
	Topsoil salvage	Salvage Cost (\$/acre) * Affected Area	
	Excavation / grading	X-ft Depth Excavation Cost (\$/acre) * Area (acre)	Soil & rock fill material, tunneling
	Hauling material offsite	Excavation Cost * (% of Material to be hauled away)	
Structural Components	Vault structure (for underground infiltration)	(\$/structure)	
	Media (for infiltration trenches)	Media cost (\$/cubic yard) * filter volume (cubic yard)	Pipes, catchbasins, manholes, valves, vaults
	Geotextile	Geotextile cost (\$/cy) * area of trench, including walls	
	inlet structure	(\$/structure)	
	Overflow structure	(\$/structure)	
	Observation well	(\$/structure)	
	Soil preparation	Topsoil or amendment cost (\$/acre) * Area (acre)	
Site Restoration	Seeding	Seeding Cost (\$/acre) * Seeded Area (acre)	Tree protection, soil amendments, seed bed preparation, trails
	Filter strip	Sod cost (\$/square foot) * filter strip area	
	Planting / transplanting	Planting Cost (\$/acre) * Planted Area (acre)	
Annual Operation, Maintenance, and Inspection	Sediment removal	Removal Cost (\$/acre) * Area (acre) * Frequency (1 time per 5 years)	
	Debris removal	Removal Cost (\$/acre) * Area (acre) * Frequency (2 time per year)	Vegetation maintenance, cleaning of structures
	Inspection	Inspection Cost (\$) * Inspection Frequency (6 times per year)	
	Mowing (for some vegetative filters)	Mowing Cost (\$) * Mowing Frequency (6 times per year)	

Contents

- 1 Design phase maintenance considerations
- 2 Construction phase maintenance considerations
 - 2.1 Avoid excessive compaction
 - 2.2 Stabilize vegetation before and after construction
 - 2.3 Correctly install filter fabrics
 - 2.4 Carefully finish final grading
 - 2.5 Keep infiltration practices “Off-line” until construction is complete
 - 2.6 Establish permanent vegetation
 - 2.7 Post-construction operation and maintenance
- 3 Related pages

Design phase maintenance considerations

Implicit in the design guidance in the previous sections is the fact that many design elements for infiltration systems can minimize the maintenance burden and maintain pollutant removal efficiency.

- Open lawn areas are *RECOMMENDED* locations for infiltration practices because of their accessibility.
- It is *HIGHLY RECOMMENDED* that every dry well, infiltration trench and subsurface infiltration system design include an observation well consisting of an anchored 6-inch diameter perforated PVC pipe fitted with a cap to facilitate periodic inspection and maintenance. It is also *HIGHLY RECOMMENDED* that infiltration basins include a draw down device that can be used for winter diversion and to conduct regular maintenance.
- It is *HIGHLY RECOMMENDED* that a mechanism such as a multi-stage outlet structure be incorporated into the design of the pre-treatment and infiltration practices to facilitate draining for maintenance purposes.
- It is *RECOMMENDED* that a minimum of 3 soil borings or pits be dug in the same location as the proposed infiltration practice.

Warning: It is *REQUIRED* that a way to visually verify proper system operation be installed with each infiltration practice.

Warning: Providing easy access (typically 8 feet wide) to infiltration practices for routine maintenance is *REQUIRED*

Construction phase maintenance considerations

Infiltration practices are particularly vulnerable during the construction phase for two reasons. First, if the construction sequence is not followed correctly, construction sediment can clog the practice. In addition, heavy construction can result in compaction of the soil, which can then reduce the soil’s infiltration rate. For this reason, a careful construction sequence needs to be followed. Critical construction elements for infiltration practices are shown in the following table.

Infiltration Trench and infiltration basin - Construction inspection checklists.

Link to this table

To access an Excel version of form (for field use), click here.

Project:

Location:

Site Status:

Date:
 Time:
 Inspector:

Construction Sequence	Satisfactory / Unsatisfactory	Comments
1. Pre-Construction		
Pre-construction meeting		
Runoff diverted		
Soil permeability verified		
Groundwater / bedrock verified		
Project benchmark established		
Facility location staked out		
Temporary erosion and sediment control established		
2. Excavation		
Size and location per plans		
Side slopes stable		
Depth adjusted to soil layer with specified soil type and permeability		
Sub-soil not adjacent to excavation area and stabilized with vegetation and/ or silt fence		
Stockpile location not adjacent to excavation area and stabilized with vegetation and/ or silt fence		
3. Filter Fabric Placement		
Fabric per specifications		
Fabric per specifications		
Placed per plan location		
4. Aggregate Material		
Size as specified		
Clean / washed material		
Placed properly		
5. Observation Well		
Pipe size per plans		
Under-drain installed per plans		
Inlet installed per plans		
Pre-treatment devices installed per plans		
6. Vegetation		
Complies with planting specifications		
Topsoil complies with composition and placement in specifications		
Permanent erosion control measures in place		
7. Final Inspection		
Dimensions per plans		
Check dams operational		
Inlet / outlet operational		
Effective stand of vegetation and stabilization		

Contributing watershed stabilized before flow is routed to the facility

Comments:

Actions to be taken:

Avoid excessive compaction

Warning: It is *REQUIRED* that in order to prevent soil compaction, the proposed infiltration area be staked off and marked during construction to prevent heavy equipment and traffic from traveling over it.

In addition, it is *HIGHLY RECOMMENDED* that the side walls of dry wells and infiltration trenches be roughened if they have been smeared by heavy equipment.

Stabilize vegetation before and after construction

Excessive sediment loadings can occur without the use of proper erosion and sediment control practices during the construction process.

Warning: It is *REQUIRED* that upland drainage areas be properly stabilized with a thick layer of vegetation, particularly immediately following construction, to reduce sediment loads.

Warning: If infiltration practices are in-place during construction activities, it is *REQUIRED* that sediment and runoff be kept away the infiltration area, such as with diversion berms and soil-stabilizing vegetation around the perimeter of the practice.

Correctly install filter fabrics

Large tree roots should be trimmed flush with the sides of dry wells and infiltration trenches to prevent puncturing or tearing of the filter fabric during subsequent installation procedures. When laying out the geotextile, the width should include sufficient material to compensate for perimeter irregularities in the dry well or trench and for a 6-inch minimum top overlap. The filter fabric itself should be tucked under the sand layer on the bottom of the dry well or infiltration trench, and stones or other anchoring objects should be placed on the fabric at the trench sides to keep the excavation open during windy periods. Voids may occur between the fabric and the excavated sides of the practice. Natural soils should be placed in any voids to ensure fabric conformity to the excavation sides.

Carefully finish final grading

Initial infiltration basin excavation should be carried to within 2 feet of the final elevation of the basin floor.

Warning: It is *REQUIRED* that infiltration systems not be excavated to final grade until the contributing drainage area has been constructed and fully stabilized.

The final phase excavation should remove all accumulated sediment and be done by light tracked equipment to avoid compaction of the basin floor and provide a well-aerated, highly porous surface texture.

Keep infiltration practices “Off-line” until construction is complete

Warning: It is *REQUIRED* that sediment and runoff be kept completely away from the infiltration area during construction. Thus, infiltration practices should never serve as sediment control devices during site construction.

It is *HIGHLY RECOMMENDED* that construction of sediment practices be suspended during snowmelt or rainfall, in order to prevent soil smearing, clumping, or compaction.

Establish permanent vegetation

- Establishing dense vegetation on the basin side slopes is *HIGHLY RECOMMENDED*, to reduce erosion and sloughing and
- Provide a natural means of maintaining relatively high infiltration rates. Vegetative cover at inflow points to the basin is also *HIGHLY RECOMMENDED* to provide erosion protection and reduce sediment accumulation. The use of native grasses is *RECOMMENDED* for seeding primarily due to their adaptability to local climates and soil conditions.
- Inspections during construction are needed to ensure that the infiltration practice is built in accordance with the approved design and standards and specifications. Detailed inspection checklists should be used that include sign-offs by qualified individuals at critical stages of construction to ensure that the contractor’s interpretation of the plan is acceptable to the designer.

Post-construction operation and maintenance

Warning: A maintenance plan clarifying maintenance responsibility is *REQUIRED*.

Effective long-term operation of infiltration practices necessitates a dedicated and routine maintenance schedule with clear guidelines and schedules. Some important post-construction maintenance considerations are provided below.

- A legally binding and enforceable maintenance agreement should be executed between the practice owner and the local review authority.
- Adequate access must be provided for all infiltration practices for inspection, maintenance, and landscaping upkeep, including appropriate equipment and vehicles.
- General infiltration trench maintenance activities and schedule are provided in the tables below.

Typical maintenance problems for infiltration trenches and basins.

Link to this table

Problem	Practices Applied To	Comments
Clogging, sediment deposition	Both	Key issue for infiltration practice. Requires vigilant inspection and maintenance.
Surface Vegetation	Both	

		Often important to maintain vigorous growth at the base of infiltration practices (basins). Important to restrict woody vegetation from the surface of infiltration trenches.
Erosion of contributing land or in channels leading to practice	Both	In these practices, it is important to monitor not only the practice itself, but also upland infiltration to minimize the sediment load.
Damage to filter fabric	Trench	Infrequent but important maintenance concern.
Scouring at Inlet	Both	Similar issues to Ponds. Need to promote non-erosive flows that are evenly distributed
Access Issues	Both	Similar issues to Ponds. Need access for inspection and maintenance.
Concrete Failure	Basins, if they include a riser structure	Similar issues to ponds and wetlands.
Problems with the Embankment	Basins	Similar issues to dry ponds.

Typical maintenance activities for infiltration trenches and infiltration basins.

Link to this table

Activity	Schedule
Replace pea gravel/topsoil and top surface filter fabric (when clogged).	As needed
Ensure that contributing area, practice and inlets are clear of debris.	
Ensure that the contributing area is stabilized.	
Remove sediment and oil/grease from pre-treatment devices, as well as overflow structures.	Monthly
Mow grass filter strips should be mowed as necessary. Remove grass clippings.	
Repair undercut and eroded areas at inflow and outflow structures	
Inspect pre-treatment devices and diversion structures for sediment build-up and structural damage. Remove trees that start to grow in the vicinity of the trench.	Semi-annual Inspection
Disc or otherwise aerate basin bottom. De-thatch basin bottom.	Annually
Scrape basin bottom and remove sediment. Restore original crosssection and infiltration rate. Seed or sod to restore ground cover.	Every 5 years
Perform total rehabilitation of the trench to maintain design storage capacity. Excavate trench walls to expose clean soil	Upon Failure

Related pages

- Overview for Infiltration basin
- Design criteria for Infiltration basin

- Construction specifications for Infiltration basin
- **Operation and maintenance of Infiltration basin**
- Cost-benefit considerations for Infiltration basin
- External resources for Infiltration basin
- References for Infiltration basin

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Site K Infiltration Analysis

Technical Memo



Responsive partner.
Exceptional outcomes.

To: Ms. Shanna Schmitt, Project Manager
Minnesota Pollution Control Agency
VIC Program
520 Lafayette Road
St. Paul, Minnesota 55155-4194

Ms. Amy Hاديaris, Technical Analyst
Minnesota Pollution Control Agency
VIC Program
520 Lafayette Road
St. Paul, Minnesota 55155-4194

From: Pamela Massaro, Wenck Associates, Inc.

Copy: Beth Engum, Ramsey County Public Works
Rick Kubler, Gray Plant Mooty
Rick Van Allen, Bay West, PG
Joe Otte, Wenck Associates, Inc.
Jon Horn, PE, Kimley Horn
Tom Lincoln, PE, Kimley Horn
Beth Kunkel, Kimley Horn
Jon Libby, Kimley Horn

Date: August 3, 2015

Subject: Site K Infiltration Analysis

Attachments: Bay West's Proposed Actions Letter dated February 4, 2015
Exhibits from the Comprehensive Stormwater Management Plan
Exhibit from the Fiscal Year 2013 Annual Performance Report

As follow up to Bay West's letter dated February 4, 2015 and the meeting held on March 18, 2015 at Ramsey County Public Works, this memorandum presents the additional information regarding the proposed development. The purpose of this memorandum is to document:

- The predicted infiltration contributing to the surficial aquifers in the vicinity of the Installation Restoration Program (IRP) Site K before the start of the redevelopment project (circa 2012) and at the end of the redevelopment project (circa, TBD); and
- The measures prescribed in the comprehensive stormwater management planning process to limit impacts on the remaining impacted surficial groundwater associated with Site K after the current demolition and remediation efforts.

Background and 2012 conditions:

The first stage of the Rice Creek Commons (formerly Twin Cities Army Ammunition Plant) redevelopment Project consists of demolition and remediation activities on the 427-acres of land (Site), described in the lease/purchase agreements. On April 9, 2013, the MPCA issued a No Association Determination (NAD) to Ramsey County for demolition, remediation and redevelopment activities. Wenck submitted a Site plan and NAD request on behalf of Ramsey County on September 16, 2014. This NAD request was subsequently followed by a supplemental NAD request with more detailed Site stormwater construction plans in a letter prepared by Bay West dated February 4, 2015. Ramsey County now seeks a NAD for additional redevelopment activities at the Site, as described in Bay West's February 4, 2015 letter.

Demolition and remediation are nearing completion and have involved removal and off-site disposal of soils (above groundwater) from areas investigated and determined to have contaminants of concern above applicable MPCA Tier 1 Soil Reference Values (SRVs) and Soil Leaching Values (SLVs). Upon completion of the remaining cleanup activities, a Certificate of Completion is contemplated, as are modifications to the Uniform Environmental Covenant and Easement and Land Use Control Remedial Design, which, among other things, currently restrict the Site to industrial uses. For more information, you can visit the website (ricecreekcommons.com). There are many documents related to Site-wide environmental remediation available under the "Due Diligence" and "Documents for Developers" weblinks.

This memorandum assumes the predevelopment conditions for the site to be those existing in 2012, before commencement of demolition, cleanup and other redevelopment activities. In 2012, the Site included open space, buildings, roads, utilities, no engineered infiltration devices, and two operating groundwater remediation systems. The surficial geology on (**Exhibit 1**) shows the extent of the surficial aquifer (Unit 1), the extent to which the glacial till (aquitard) is exposed at the surface (Unit 2) and where the underlying (Unit 3) Hillside/Arsenal Sand aquifer is exposed at the surface. As part of the several decades of investigation and remediation at the Site, the geology is well documented.

The entire Site is underlain by bedrock and Unit 3 deep below the surface. Unit 2 also underlies the entire site, and varies in thickness, is exposed at the surface (in areas shown on **Exhibit 1**), and where it is present, prevents surface infiltration from recharging Unit 3. Unit 2 is not present east of the Site, where Unit 3 is exposed at the surface. **Exhibits 2** and **3** display the conceptual geologic layering below the surface at the Site in a cross-sectional view. The groundwater levels and flow directions in Unit 1 and Unit 3 have been extensively monitored and mapped. **Exhibit 4** shows the groundwater gradient for Unit 1 based on 1987 data. Where there are not Unit 1 groundwater gradients mapped it is assumed that the Unit 1 aquifer does not exist. As is evident on **Exhibit 4**, the Unit 1 aquifer is not continuous across the Site.

In the vicinity of former building 103 (IRP Site K) and former building 102 the groundwater flows in a north/northwesterly direction (respectively) toward Rice Creek. Just south and west of former building 102, Unit 1 groundwater also flows northwesterly toward Rice Creek. Subsequent annual groundwater monitoring, published in the Annual Performance Reports (APRs), prepared by the U.S. Army as required by the USEPA/MPCA, confirms the discontinuous extent and general groundwater flow directions as shown on **Exhibit 4**. **Exhibit 5** shows the 2012 groundwater gradient in the IRP Site K area, which is consistent in its direction toward Rice Creek as published in 1987.

Using LiDAR data and information available on soils, land use, and storm sewers (on Site in 2012), the predevelopment condition subwatersheds were delineated (**Exhibit 6**). The 2012 drainage break is the dividing line for surface hydrology and stormwater conveyance that sends drainage from the southern portion of the Site to Round Lake, while the remainder of the Site drains to Rice Creek. **Exhibit 6** also shows flow arrows representing stormwater flow directions. The 2012 drainage break roughly corresponds to the northern extent of the glacial till (aquitard) exposed at the surface (Unit 2). It is believed that the aging storm sewer infrastructure historically leaked stormwater, increasing infiltration where the bedding was above the groundwater table. The storm sewer bedding material may also have

intermittently acted as a french drain causing localized groundwater dewatering during periods of elevated near surface groundwater. Prior to demolition and remediation activities, it is estimated that subwatersheds 16, 20, 21, 23 and 24 (**Exhibit 6**) were influencing and feeding the near surface groundwater tributary to the former building 102 and 103 plumes (**Exhibit 7**).

Exhibit 7 displays the areas proposed to be excluded from stormwater reintroduction (Bay West's February 4, 2015 letter). One area is associated with former Building 502 (Site I) and the other is the area of former Buildings 102 and 103. Stormwater infiltration BMPs located directly above the plumes would not mobilize contamination, since the soils are being remediated to meet Tier 1 SRVs and applicable SLVs above the groundwater table. The remaining shallow plumes (below the vadose zone) in these areas could be affected by increased volumes of stormwater causing the plumes to be diluted with clean water or change the residence time of the plume reaching the receiving waterbody. We are taking the conservative approach in the comprehensive stormwater management planning process to prevent stormwater from the redevelopment to be infiltrated in these areas shown on **Exhibit 7**.

Since TCE is heavier than water and has a low solubility value, TCE is classified as a dense nonaqueous phase liquid, or DNAPL. This class of chemicals will tend to sink through the water column (both surface and ground) until they encounter a barrier that is sufficiently impermeable to stop them. The glacial till (Unit 2) below Unit 1 in the vicinity of Site K (former building 103) and former building 102, acts as a barrier between the surficial aquifer (Unit 1) and the underlying Hillside/Arsenal Sand aquifer. Furthermore this area is excluded from stormwater reintroduction. Both Unit 1 and Unit 2 are labeled in the attached **Exhibit 8** for Site K (former building 103 plume) from the APR final report.

Proposed Ultimate Development:

The proposed fully developed Site plan (**Exhibit 9**) is only conceptual in nature at this time, leaving final design for various parts of the Site to developers. Stormwater ponds are necessary to meet the Rice Creek Watershed District's requirements of Rule C.6 (Water Quality Treatment) and Rule C.7 (Peak Stormwater Runoff Control). Each subwatershed's runoff is routed individually and directed to a pond and stormwater control structure, or drains directly to the receiving waterbody (**Exhibit 10**). The stormwater pond bottoms will be constructed to minimize infiltration utilizing techniques such as specifying compaction over the entire pond bottom. There are many examples in the seven-county metro area of failed infiltration BMPs due to unintentional compaction. Even unintentional compaction resulting from construction equipment driven over an infiltration trench can limit the infiltration rate of a stormwater BMP and compromise success. It is therefore reasonable to assume that intentional compaction of the stormwater pond bottoms would limit infiltration of the redevelopment stormwater into the underlying Unit 1 surficial groundwater table.

Based on the Master Plan land use designations, the impervious acreage was estimated using SCS TR-55 guidance (e.g., commercial areas assumed to be 85% impervious, etc.). The Site was modeled using HydroCAD for both 2012 and proposed fully-developed conditions to predict hydrology and hydraulic routing of stormwater. Wenck reviewed available soil borings in locations of the proposed ponds to estimate infiltration rates of the stormwater ponds. There is an increase in total area flowing to Rice Creek through a

combination of onsite infiltration and wet detention ponds. **Table 1** compares the drainage areas and area-weighted average of runoff characteristics (in terms of curve number) of the subwatersheds identified as influencing and feeding the near surface groundwater tributary to IRP Site K for the 2012 Site conditions and the proposed fully developed Site conditions.

Table 1: Summary of subwatersheds

Subwatersheds	Scenario	Total Area (acres)	Areas of Impervious (Acres)	Composite CN
16, 20, 21, 22, 23, 24	2012 Conditions	120.7	52.1	84
15, 16,17, 26	Proposed Fully-Developed Conditions	113.3	46.9	78

Generally, the near surface groundwater flows tend to mimic the hydrologic subwatersheds. As was previously stated, prior to demolition and remediation activities subwatersheds 16, 20, 21, 23 and 24 (**Exhibit 6**) were influencing and feeding the near surface groundwater tributary to the former building 102 and 103 plumes (**Exhibit 7**).

When the Site has achieved the proposed full developed conditions in the Master Plan (**Exhibit 9**), it is estimated that subwatersheds 26, 15, 16 and 17 (**Exhibit 10**) would potentially influence and feed the near surface groundwater tributary to Site K (**Exhibit 7**). Stormwater from subwatersheds 8, 9, 10, 11, 12, 13 and 14 are routed individually and directed to ponds with compacted bottoms that limit infiltration. The stormwater ponds are a treatment system in series that reduce total suspended solids and total phosphorus are controlled by an outlet control structure. Stormwater from subwatershed 18 is planned to be centrally collected and infiltrated. At this location the near surface groundwater flows to the northwest toward Rice Creek, thus not influencing the near surface groundwater tributary to Site K.

Table 2 presents HydroCAD predicted surface infiltration to Unit 1 in the vicinity of Site K for four different precipitation events: the 1-inch, the two-year 24-hour, the 10-year 24-hour, and the 100-year 24-hour. The HydroCAD model predicts less infiltration in the area tributary to IRP Site K for proposed development conditions as compared to 2012 (predevelopment) conditions.

Table 2: Predicted infiltration in the vicinity of IRP Site K

Subwatersheds	Scenario	Infiltration Predicted (acre-ft)			
		1-inch	100-year 24-hour	10-year 24-hour	2-year 24-hour
			7.31-inch	4.22-inch	2.82-inch
16, 20, 21, 22, 23, 24	2012 Conditions	8.3	18.7	16.4	15.3
15, 16,17, 26	Proposed Fully Developed Conditions	5.5	15.1	12.6	10.5

Conclusions:

Based on modeling completed to support the comprehensive stormwater management planning and steps taken to ensure the regulatory requirements of federal, state and local water resource regulations are met, the fully developed conditions planned for Rice Creek Commons result in no increase in infiltration in the vicinity of Site K. The following points are important assumptions for this conclusion:

- The groundwater flow direction in the vicinity of Site K has not changed direction since mapped in 1987, as supported in subsequent annual publications of APRs.
- The ongoing groundwater treatment activity at IRP Site K continues to be effective based upon monitoring as documented in the draft 2014 APR (note that the stormwater utilities were removed in 2014).
- The redevelopment comprehensive stormwater management plan will not allow stormwater to be infiltrated through the use of engineering infiltration BMPs in the vicinity of Site K, as shown on **Exhibit 7**.
- Stormwater pond bottoms will be compacted to limit infiltration to Unit 1 groundwater.
- Redevelopment will include routing of stormwater to ponds with compacted bottoms that limits infiltration.
- Native soils (Type C and D soils with low permeability) naturally constrain infiltration on Site.
- Remediation efforts (including the removal and off-site disposal of vadose zone soils) will have cleaned up the soils above the groundwater table in Unit 1, so that redevelopment stormwater will pass through clean soils above the Unit 1 groundwater table.
- TCE at IRP Site K is a dense chemical that tends to sink through the Unit 1 groundwater and Unit 2 is acting as a barrier to further migration.



Bay West LLC
5 Empire Drive, St. Paul, MN 55103
651-291-0456 * FAX 651-291-0099

www.baywest.com • info@baywest.com

February 4, 2015

Ms. Shanna Schmitt, Project Manager
Minnesota Pollution Control Agency
VIC Program
520 Lafayette Road
St. Paul, Minnesota 55155-4194

Ms. Amy Hadiaris, Technical Analyst
Minnesota Pollution Control Agency
VIC Program
520 Lafayette Road
St. Paul, Minnesota 55155-4194

**Subject: Proposed Actions Letter and Request for No Association Determination
TCAAP Infrastructure Development
Arden Hills, Minnesota
MPCA Site ID: VP22892/PB4687
Bay West Project No. J130147**

Dear Ms. Schmitt and Ms. Hadiaris:

Bay West LLC (Bay West) has prepared this letter to describe the proposed actions to be taken by Ramsey County (the County) in regards to mass grading, road construction, utility installation and other infrastructure improvements at the former Twin Cities Army Ammunition Plant (TCAAP) located at 2020 Highway Avenue, Arden Hills, Minnesota, in Ramsey County (the Site). The County is seeking the protection of a No Association Determination (NAD) under the Minnesota Environmental Response and Liability Act (MERLA) with respect to the identified releases of hazardous substances at the property. This NAD request is intended as a supplement to the Proposed Actions Letter/Request for No Association Determination submitted on behalf of Ramsey County by Wenck Associates, Inc. (Wenck) dated September 16, 2014 (September 16 PAL/NAD Request).

As is noted in the September 16 PAL/NAD Request, the County seeks issuance of a No Association Determination for the proposed actions described below, as well as concurrence from the U.S. EPA and the Army with those proposed actions.

BACKGROUND

The Site consists of approximately 427 acres of the former TCAAP located in portions of Section 9 and 16, Township 30 North, Range 23 West. The County currently owns 397 of the 427 acres comprising the Site. The County has leased the remaining 30 acres of the Site from the United States (Leased Property). The Leased Property was known to contain areas of contamination in excess of the Minnesota Pollution Control Agency (MPCA) Tier II Soil Reference Values for industrial land use (Tier II SRVs). The County was to function as a response action contractor to complete the soil response action activities on the Leased Property on behalf of the United States Army (Army) to satisfy the Army's obligations under the



Proposed Actions Letter TCAAP Infrastructure Development

Federal Facilities Agreement. The County will take title to the Leased Property once the remediation is complete.

The County entered into an agreement with Carl Bolander & Sons, Inc. (Bolander) to complete hazardous material abatement, demolition of Site improvements, and remediation of releases of hazardous substances and petroleum to the soil at the Site. The scope of remediation activities under Bolander's agreement with the County required remediation of all soil located above the unconfined water table aquifer with identified contamination exceeding MPCA Tier 1 SRVs for residential land use.

Remediation work commenced at the Site in May 2013 and is scheduled to be complete by October 2015. Once the Site remediation is complete, the County will request that the MPCA issue a Commissioner's Certificate of Completion for soil, and request amendment of the Operable Unit 2 (OU2) Land Use Control Remedial Design (LUCRD) and associated environmental covenants to allow residential use throughout the Site.

IDENTIFIED RELEASE

The identified release at the Site includes all hazardous substances identified and described in the MPCA's file, including, without limitation, heavy metals (e.g., antimony, arsenic, barium, copper, chromium, manganese, mercury, lead, thallium and vanadium), polynuclear aromatic hydrocarbons (PAHs), polychlorinated biphenyls (PCBs) and volatile organic compounds (VOCs). The Identified Release refers to contamination of all affected media at the Site including soil, groundwater, soil gas, and any other affected media.

PROPOSED ACTIONS

We request that the MPCA issue a NAD to the County with respect to the Identified Release for the following proposed actions:

- Excavation, grading, movement, and stockpiling of soil located above the unconfined water table aquifer as these activities relate to construction site preparation, road construction, mass grading, utility installation and other infrastructure improvements. A figure illustrating the proposed spine road alignment and associated sewer and water utilities is attached as **Exhibit A**.
- Excavation, movement, stockpiling, and appropriate disposal of soil located below the unconfined water table aquifer when these activities occur in areas of known groundwater contamination and are completed in accordance with an MPCA-approved Response Action Plan (RAP). These areas are depicted on **Exhibit B** from the September 16 PAL/NAD request and **Exhibit C**, the grading plan for Rice Creek.
- Excavation, grading, movement, and stockpiling of soil located below the unconfined water table aquifer when these activities occur in areas with no known groundwater contamination.



Proposed Actions Letter TCAAP Infrastructure Development

- Construction of surface water management features, stormwater retention and infiltration basins, mitigated wetlands, and the new channel for Rice Creek in areas with no known contamination in the unconfined water table aquifer. These areas are depicted on **Exhibit B** from the September 16 PAL/NAD request and **Exhibit C**, the grading plan for Rice Creek.
- Construction of surface water management features, stormwater retention basins, mitigated wetlands, and the new channel for Rice Creek in areas with known contamination in the unconfined water table aquifer when these activities are completed in accordance with an MPCA-approved RAP. These areas are depicted on **Exhibit B** from the September 16 PAL/NAD request and **Exhibit C**, the grading plan for Rice Creek. Ramsey County will seek the concurrence of MPCA, Army, and USEPA prior to construction of stormwater management features in these areas to ensure they do not alter the groundwater flow regime in areas affected by historical solvent releases.
- Dewatering of excavation areas and discharge of water to an on-site infiltration basin when this work is completed in areas of the Site with no known contamination in the unconfined water table aquifer.
- Dewatering of excavation areas and discharge of water to the local POTW when this work is completed in areas of the Site with known or suspected contamination in the unconfined water table aquifer and is completed in compliance with a discharge permit.
- Sampling, excavation, segregation, stockpiling, and appropriate disposal of previously unidentified contaminated media in accordance with an MPCA-approved Construction Contingency Plan (CCP).
- All other actions of the County and its contractors and subcontractors associated with acquiring, maintaining, and preparing the Site for sale and redevelopment consistent with the Master Plan approved by the County and City of Arden Hills, including without limitation construction, installation and/or relocation of roads, utilities, sewers, water lines and other infrastructure.

If you have any questions regarding this request, please contact either me or Donovan Hannu using the contact information provided below.

Respectfully yours,

Rick Van Allen, PG
Senior Project Manager
651-291-3441

Donovan Hannu, PE
Project Manager
651-291-3424



**Proposed Actions Letter
TCAAP Infrastructure Development**

Attachments

- Exhibit A - TCAAP Preliminary Spine Road and Utility Plan
- Exhibit B – Figure 1 from the Wenck September 2014 PAL/NAD
- Exhibit C - Rice Creek channel excavation and meander

cc: Heather Worthington, Ramsey County
Beth Engum, Ramsey County
Rick Kubler, Gray Plant Mooty

Exhibit A

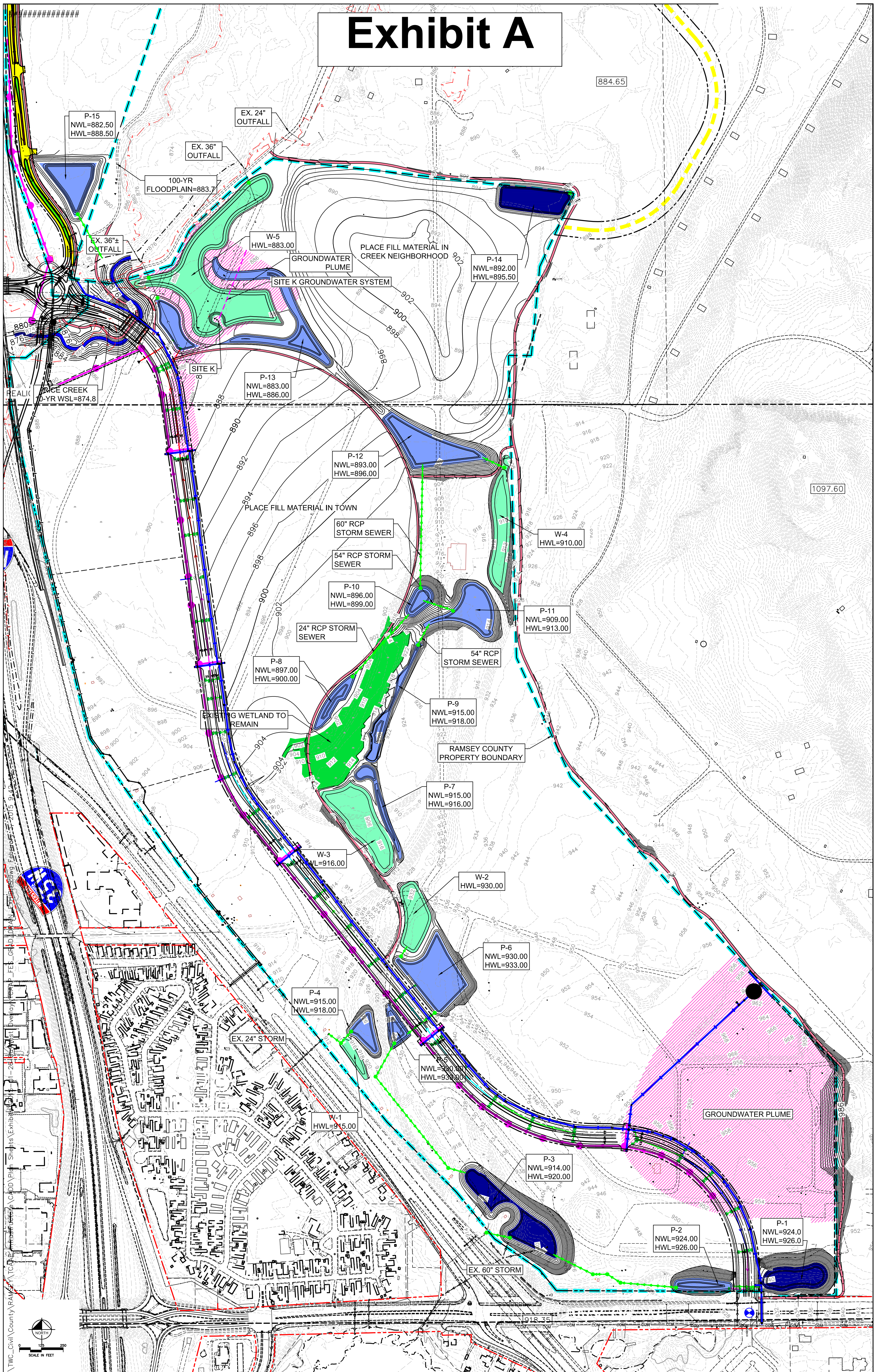





Exhibit B

Legend

-  Subject Property
-  Areas Excluded from Stormwater Reintroduction
-  Site K TCE, Building 102 (All VOC) Plumes



2012 Aerial Photograph (Source: ESRI)
 650 325 0 650 Feet
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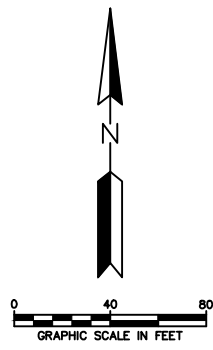
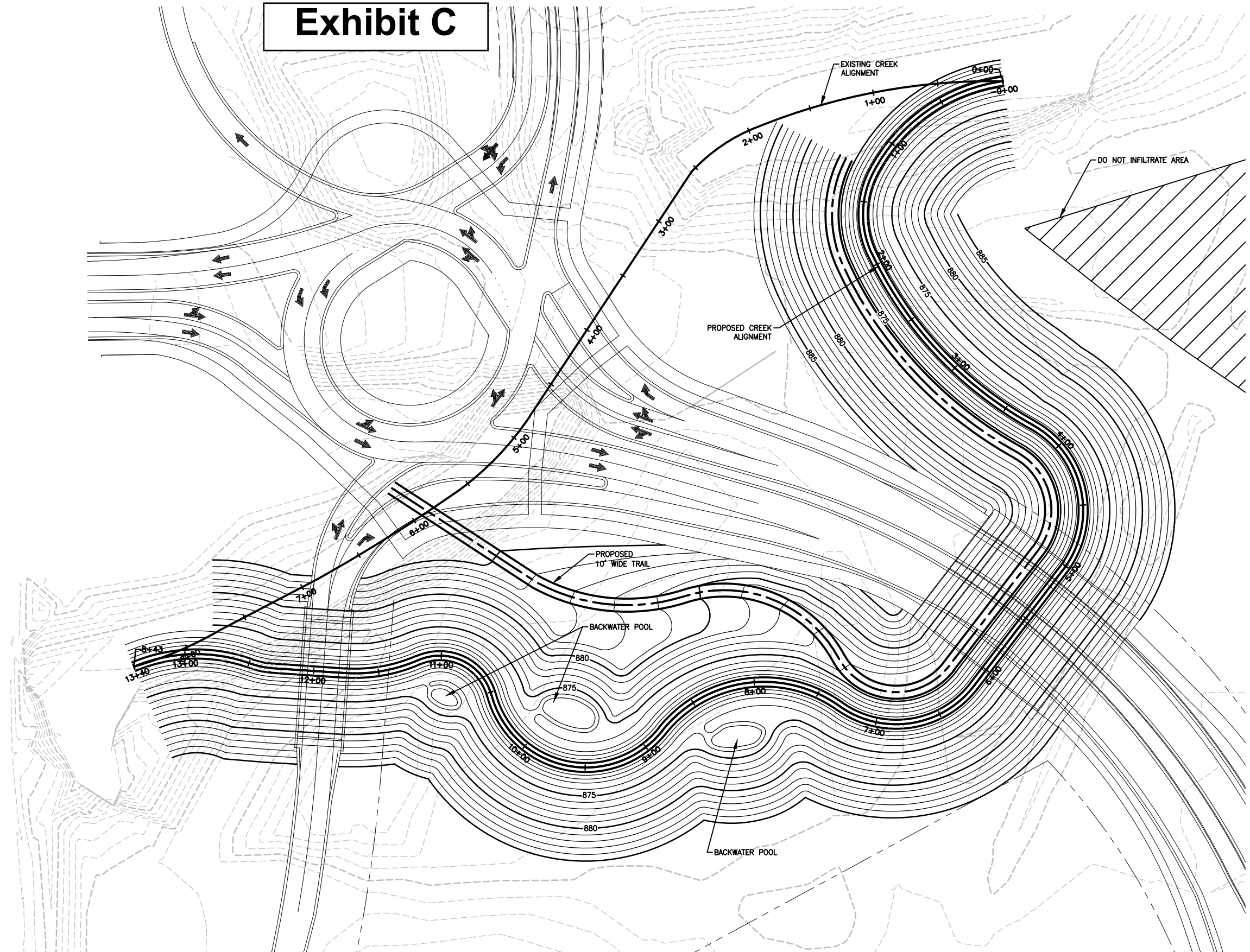
Source: Esri, DigitalGlobe, GeoEye, i-cubed, USDA, USGS, AEX, Getmapping, Aerogrid, IGN, IGP, swisstopo, and the GIS User Community

RAMSEY COUNTY
 TCAAP: 427-acre Redevelopment Infiltration


Wenck
 Engineers - Scientists
 Business Professionals
 www.wenck.com
 1800 Pioneer Creek Center
 Maple Plain, MN 55359-0429
 1-800-472-2232

AUG 2014
 Figure 1

Exhibit C



REV	REVISION DESCRIPTION	DWN	APP	REV DATE
1	DRAFT GRADING PLAN	MJS		12/12/14
0	DRAFT GRADING PLAN	MJS		12/11/14

SEAL	
------	--

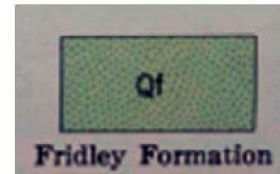
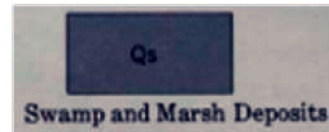
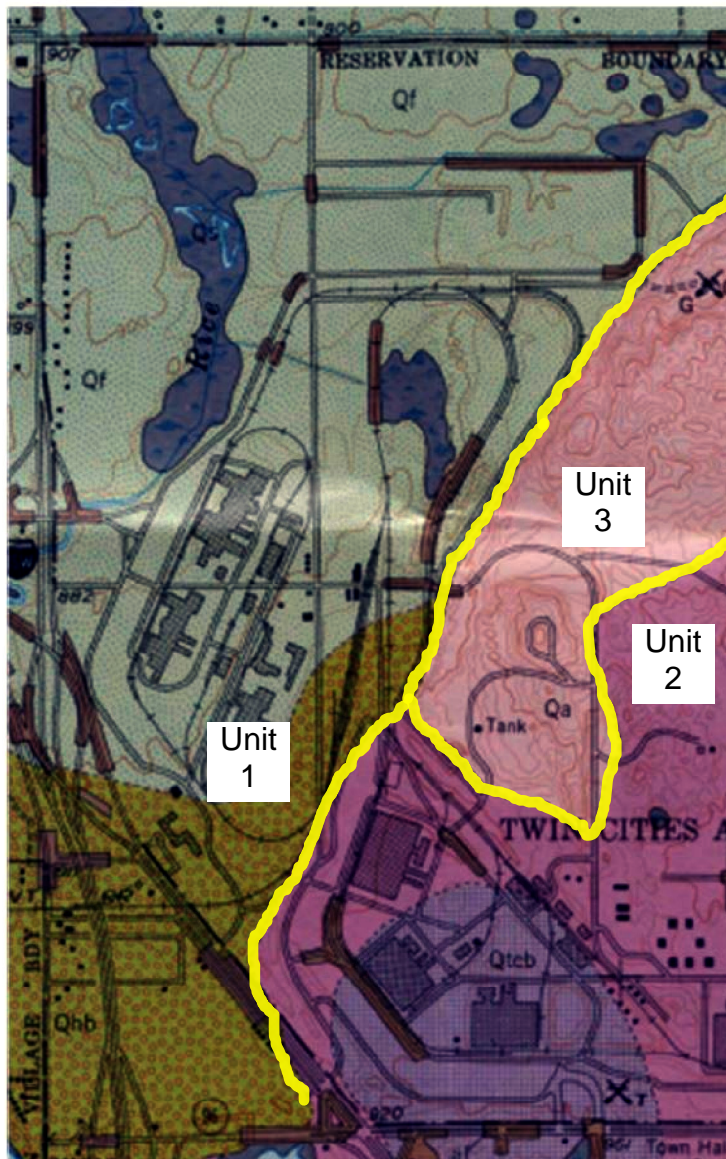
PRIME CONSULTANT

Wenck
Wenck Associates, Inc.
Consulting Engineers
Web Site: www.wenck.com

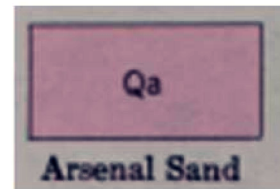
PROJECT TITLE	
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SHEET TITLE			
CREEK MEANDER GRADING			
DWN BY	CHK'D	APP'D	DWG DATE
			DEC 2014
PROJECT NO.		SCALE	
		AS SHOWN	
SHEET NO.		REV NO.	
		1	

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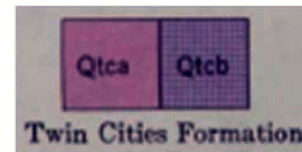
Unit 1: Water Bearing
(Low Yield)



Unit 3: Water Bearing
(High Yield)



Unit 1: Water Bearing
(Low Yield)



Unit 2 (Aquitard)

Source: Stone, John E., "Geologic Map Series 2. Surficial Geology of the New Brighton Quadrangle, MN GM-2 (1966.)"

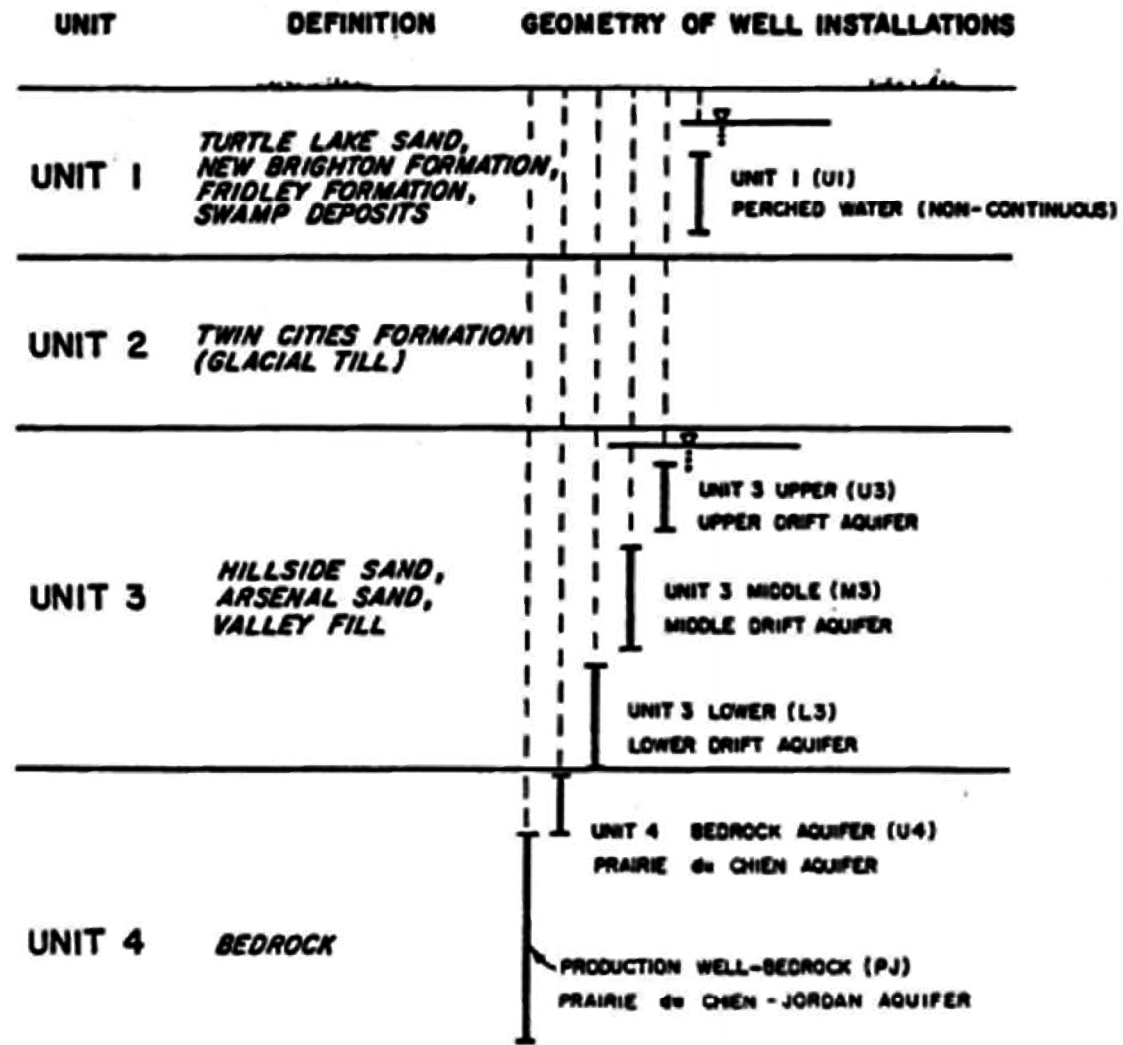


FIGURE 2.15 Illustration Showing Four Hydrologic Units at TCAAP
[Source: Ref. 5207]

Source: Wenck, "Five-year review report of the final remedy for the New Brighton/Arden Hills Superfund Site" Figure 2-15, August 2009

RAMSEY COUNTY

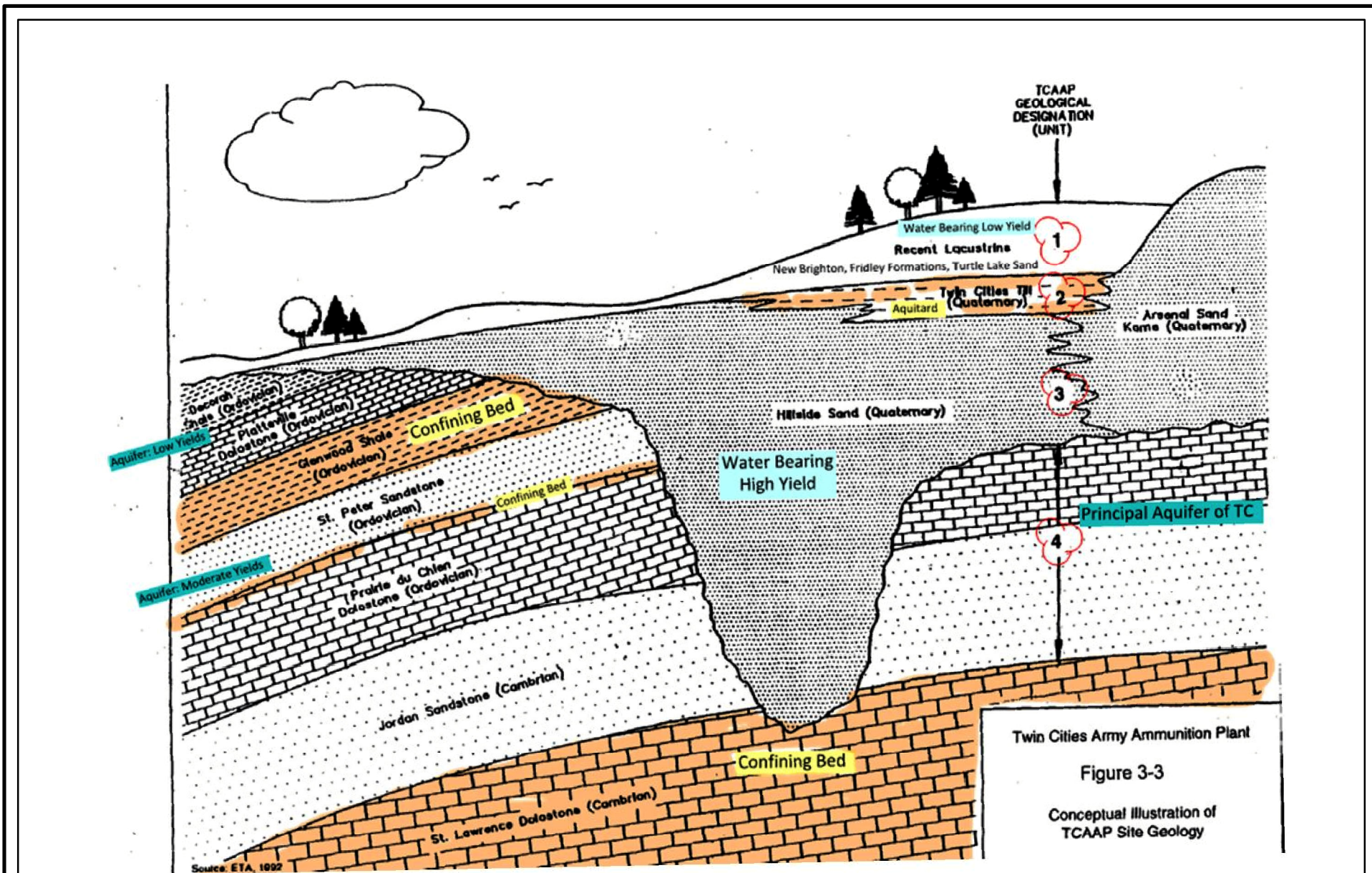
TCAAP Four Hydrologic Units



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JUNE 2015

EXHIBIT 2



Source: Wenck, "Five-year review report of the final remedy for the New Brighton/Arden Hills Superfund Site" Figure 3-3, August 2009

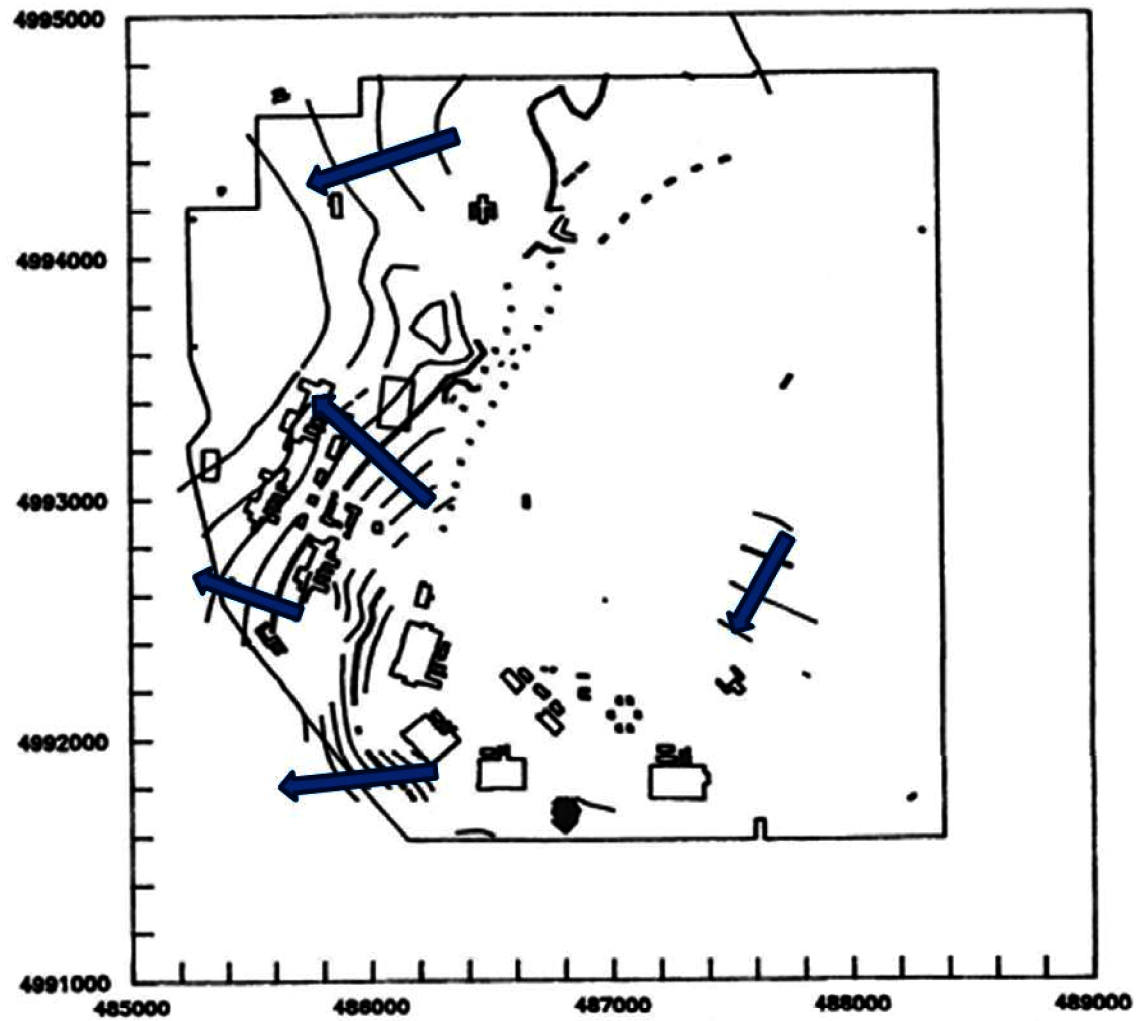
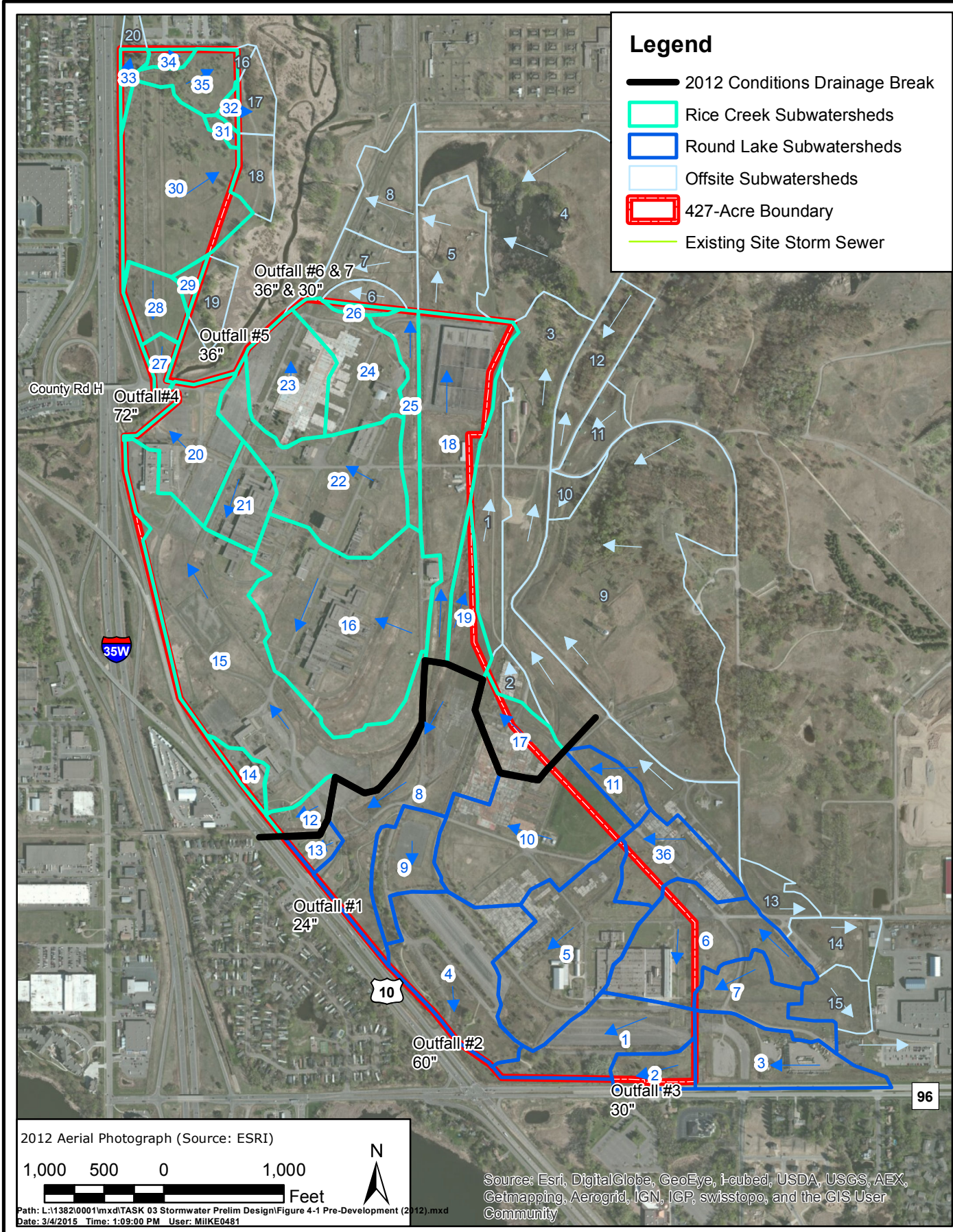


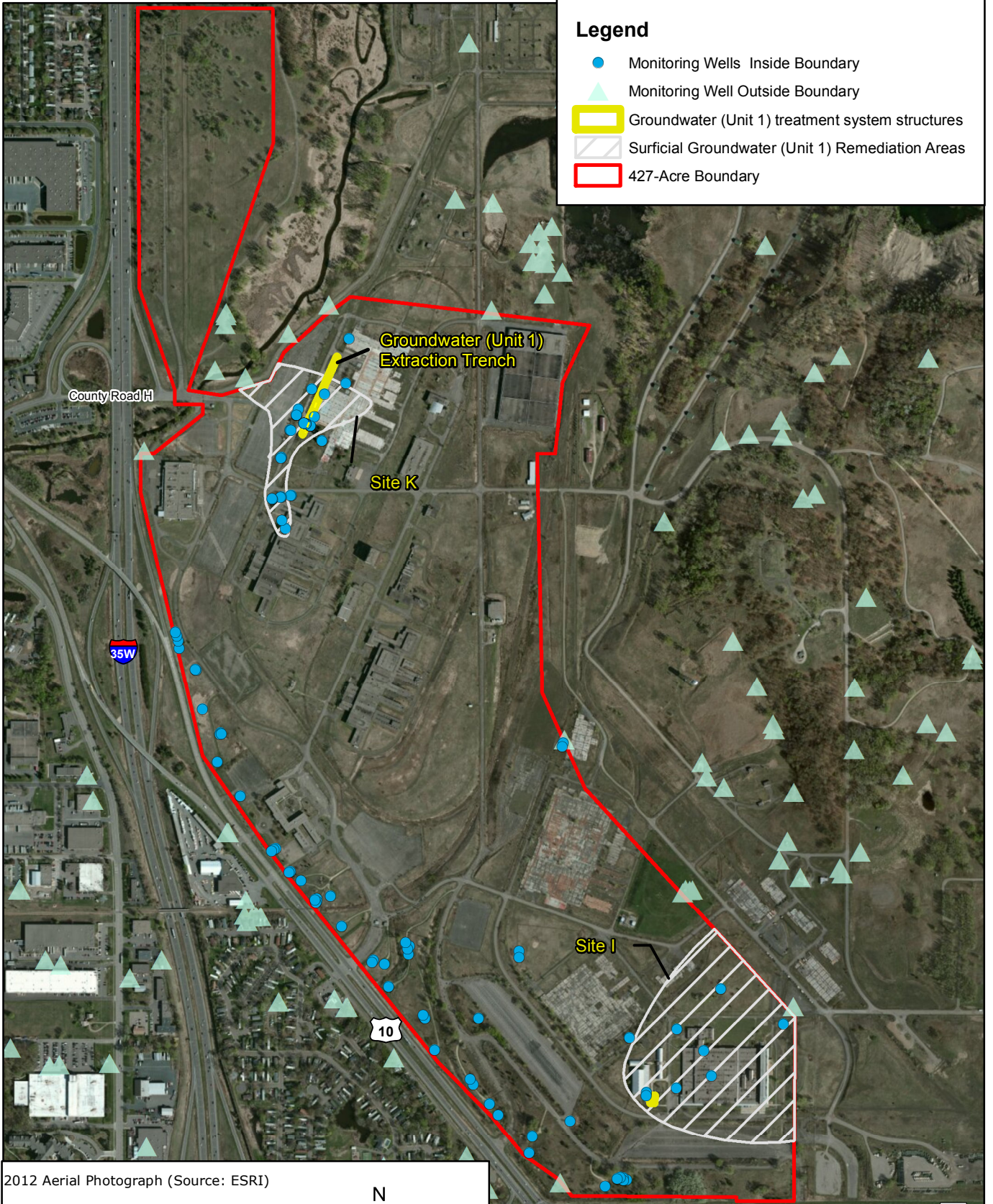
FIGURE 2.16 Water-Level Elevations for Hydrologic Unit 1 at TCAAP, February 17, 1987 (Source: USATHAMA database)

Source: Wenck, "Five-year review report of the final remedy for the New Brighton/Arden Hills Superfund Site" Figure 2-16, August 2009
 Path: L:\1382\0001\mxd\TASK 03 Stormwater Prelim Design\Fig4-3 Pre-Development (1987) Groundwater Gradient Map.mxd
 Date: 3/4/2015 Time: 1:13:44 PM User: MIKE0481



Legend

- Monitoring Wells Inside Boundary
- ▲ Monitoring Well Outside Boundary
- Groundwater (Unit 1) treatment system structures
- Surficial Groundwater (Unit 1) Remediation Areas
- 427-Acre Boundary



2012 Aerial Photograph (Source: ESRI)
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Source: Esri, DigitalGlobe, GeoEye, I-cubed, USDA, USGS, AEX, Getmapping, Aerogrid, IGN, IGP, swisstopo, and the GIS User Community 96

SOUTHEAST

NORTHWEST

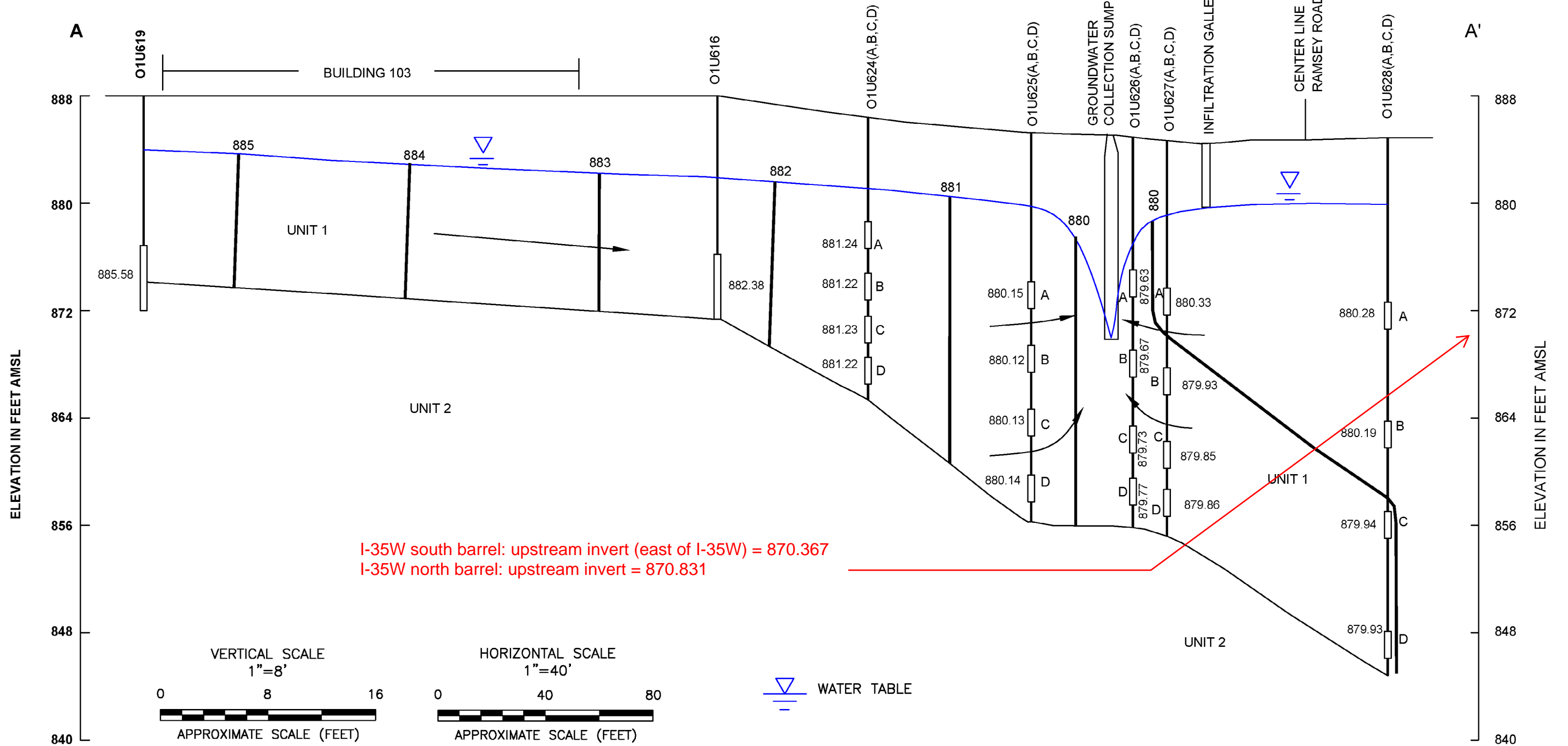


EXHIBIT 8



Geographic Information Systems

Stantec

2335 HIGHWAY 36 WEST, ST. PAUL, MINNESOTA 55113 PHONE 651.636.4600 www.stantec.com

PREPARED FOR:
ALLIANT TECHSYSTEMS
TWIN CITY ARMY AMMUNITION PLANT
ARDEN HILLS, MINNESOTA

SITE K
HYDROGEOLOGIC CROSS SECTION A-A'
06/01/12 (Q115)

FIGURE:

9-3

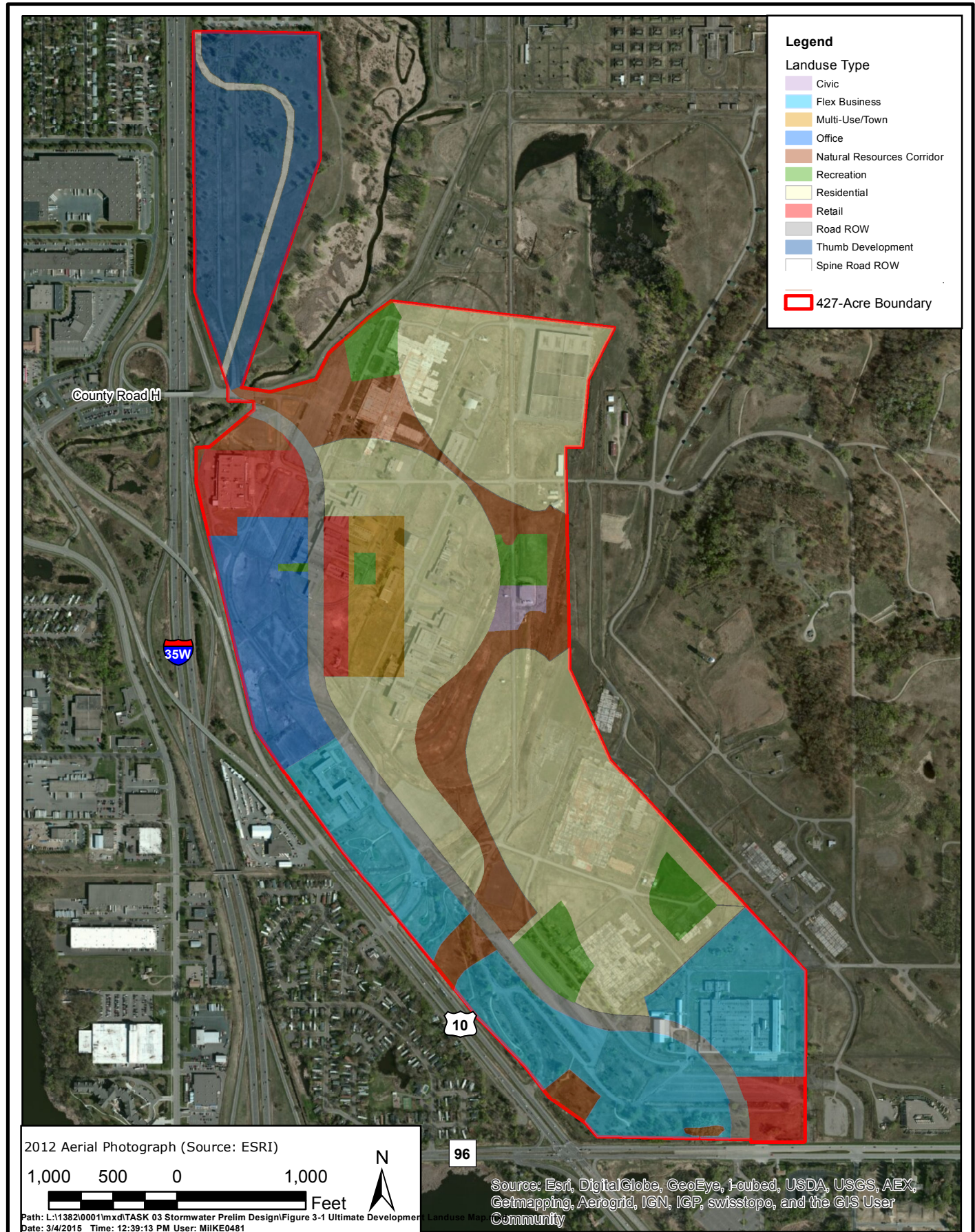
JOB NUMBER:
182602642

DRAWN BY:
TF

CHECKED BY:
AG

APPROVED BY:
AG

DATE:
11/21/13



Legend

Landuse Type

- Civic
- Flex Business
- Multi-Use/Town
- Office
- Natural Resources Corridor
- Recreation
- Residential
- Retail
- Road ROW
- Thumb Development
- Spine Road ROW

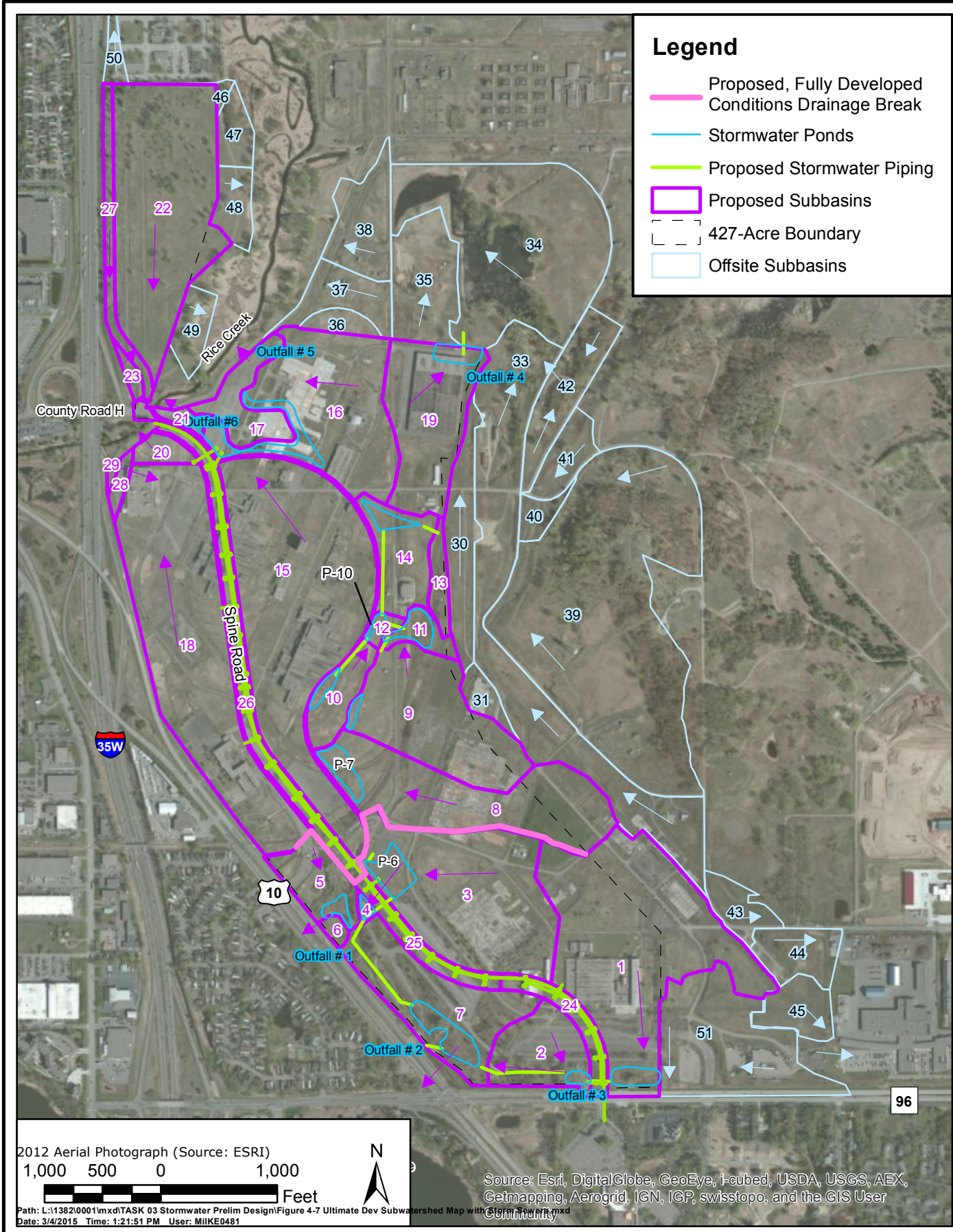
427-Acre Boundary

2012 Aerial Photograph (Source: ESRI)

1,000 500 0 1,000 Feet

Path: L:\1382\0001\mxd\TASK 03 Stormwater Prelim Design\Figure 3-1 Ultimate Development Landuse Map
 Date: 3/4/2015 Time: 12:39:13 PM User: MIKE0481

Source: Esri, DigitalGlobe, GeoEye, i-cubed, USDA, USGS, AEX, Getmapping, Aerogrid, IGN, IGP, swisstopo, and the GIS User Community





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Toll Free: 800-472-2232

Email: wenckmp@wenck.com

Web: wenck.com

MINNESOTA

Maple Plain
763-479-4200

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952-831-5408
Windom
507-831-2703

New Hope
800-368-8831
Woodbury
651-294-4580

COLORADO

Denver
602-370-7420

GEORGIA

Roswell
678-987-5840

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Mandan
701-751-3370
Williston
800-472-2232

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Sheridan
307-675-1148